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Supplementary Water Management Report ENGINEERS MANAGERS INFRASTRUCTURE PLANNERS DEVELOPMENT

& Response to Contentions

For a Proposed Residential Development

Prepared for: Warriewood Developers Pty Ltd

Project address: Lots 1 & 2 in DP 349085 (No. 45-49) and Lot 2 in DP 972209 (No.43)

Warriewood Road, Warriewood

Document No.: CC230177_WMP

Version No.: B

Dated: 6 March 2024





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VERSION HISTORY

Version	Date	Purpose	Prepared By	Approved By
A	15/12/2023	Supplementary Water Management Report	Nathan Broadbent	
В	6/03/2024	Revised Supplementary Water Management Report	Nathan Broadbent	

Review Panel				
Division/Office	Name			

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Annexures

Annexure A ACOR Consultants (CC) Pty Ltd Civil Engineering Plans, reference CC230177, sheets C1.00 to C4.03 (26 sheets), Revision C dated 6 March 2024.



1 Introduction

This report has been prepared as a supplementary report to the Engineering Report prepared by C & M Consulting Engineers, Report No. R2192, Revision C, dated 4 June 2021 and seeks to address the relevant design short falls outlined in the Statement of Facts and Contentions (SOFCAS) reference No. 2023/00096634. This report also incorporates engineering concepts discussed with Northern Beaches Council and their representatives during the preparation of this report.

During the preparation of this report, information was sourced from various documents. These included, but are not limited to the following:

- Addendum Flood Impact Assessment prepared by BMT.
- Civil Engineering Works prepared by C & M for the subject site, drawing No.'s 02192-100 to 02192 (14 sheets), revision 4, dated 26 July 2023.
- Detailed Site Investigation report prepares by Sydney Environmental Group, Report No. 2148-DSI-01-280723.v1f, dated 28 July 2023.
- Engineering Report prepared by C & M, reference R02192, Revision C, dated 4 June 2021.
- Northern Beaches Council Pittwater Development Control Plan (DCP) 21
- Northern Beaches Council Warriewood Valley Water Management Specification 2001
- Northern Beaches Council Water Management for Development Policy.
- Statement of Facts and Contentions (SOFCAS) reference No. 2023/00096634 filed 18 May 2023.

2 Site overview

The subject site is known as Lots 1 & 2 in DP 349085 (No. 45-49) and Lot 2 in DP 972209 (No.43) Warriewood Road, Warriewood.

The site is bound by Warriewood Road to the north, Narrabeen Creek to the south and residential development to the east and west.

Current structures on the site includes two dilapidated former dwellings and several ancillary minor structures. Prior to partial clearing of the site in 2020, the site contained approximately 20 rows of greenhouses, the purpose of which was not known at the time of preparation of this report.

The locality of the site in relation to Narrabeen Creek and surrounding development is depicted in Figure 1.





Figure 1. Site locality plan

The site comprises an area of approximately 2.157 ha. This includes a developable area of approximately 1.37 ha including road and drainage works.

2.1 Proposed development

The proposed development consists of two Residential Flat Buildings and a Torrens Title Subdivision creating 11 new residential allotments.

The primary details of the proposed development are depicted on Site Plan prepared by Archidrome, Drawing No.A03, Revision 12, dated 6 March 2024 and generally comprises the following:

- Two (2) Residential Flat Buildings (RFBs) with a basement.
- Creation of eleven (11) Torrens Title residential lots.
- Extension of the Lorikeet Grove corridor through the site including drainage works.
- Road upgrade works within Warriewood Road including drainage infrastructure and additional roadside parking.



- Construction of a stormwater culvert within the existing drainage easement positioned along the site's eastern boundary.
- Construction of two bioretention basins within the Narrabeen Creek wetland.
- Construction of a suspended walkway through the wetland.

We note that works assessed under as part of a future application to Council required under Section 138 of the Roads Act are not discussed in detail in this report.

2.2 Topography

The site falls generally towards Narrabeen Creek from northeast to southwest. Ground surface elevations on the site are generally within the range of RL 12.5 m AHD near northwest corner of the stie to RL 2.4 m AHD within the drainage channel along the eastern boundary.

2.3 Existing geotechnical conditions

A detailed description of the site can be found in the in Detailed Site Investigation report prepares by Sydney Environmental Group (SEG), Report No. 2148-DSI-01-280723.v1f, dated 28 July 2023.

The SEG report identifies 23 test pits were installed in various locations across the developable site using a combination of hand augering and mechanical excavation with a 5t excavator.

The bore logs indicate most of the subsurface conditions within the developable parts of the site consist of clayey fill underlain by natural clay to various depths of up to 1.5 metres bgs.

3 Water Cycle Assessment

A water balance was prepared for the site under existing and proposed conditions and considers three years independent years of daily rainfall data obtained for the Mona Vale rainfall gauge and average monthly evapotranspiration data for the area. Rainfall years were selected to allow assessment of existing and proposed conditions for a typical dry year, average year, and wet year. The selected years and associated rainfall depths are as follows:

- 2002 (dry year) with a rainfall total of 865 mm.
- 1984 (average year) with a rainfall total of 1068 mm.
- 1998 (wet year) with a rainfall total of 1650 mm.

A water balance was prepared for each year under existing and proposed conditions to determine following design constraints and parameters.

- Proposed surface runoff volume check against predevelopment conditions modelling (refer Section 3)
- Anticipated reuse demand based on varying rainfall totals for each year assessed.



3.1 Water balance – existing conditions

A water balance was conducted for the developable site area under existing conditions. The parameters adopted for the water balance assessment is outlined in Table 1.

Table 1 - Water balance for existing conditions

Existing case			
Areas	Value	Unit	
total area	13,470	m ²	
pervious area	12,123	m ²	
impervious area	1,347	m ²	
Reuse			
no reuse provided for existing conditions			
Losses			
pervious area loss (mm)	5	mm	
impervious area loss (mm)	1	mm	
Statistics			
number of runoff days	71	days	
number of runoff days	76	days	
number of runoff days	98	days	
Annual runoff total			
dry year	6.27	ML	
average year	8.41	ML	
wet year	15.23	ML	

The results of the site water balance for the existing case were used to assess the potential for a variation in post-development stormwater runoff from the site. We note the first principles approach used in this assessment does not account infiltration. Subsequently, the results from MUSIC will ultimately be relied upon and this water balance will inform re-use rates for MUSIC modelling and also provide a check for stormwater quantities.



3.2 Post-development water balance methodology

The post-development water balance was prepared to for each of the three years described above (wet, dry, and average rainfall). The flow of the water balance calculations is depicted below in Figure 2.

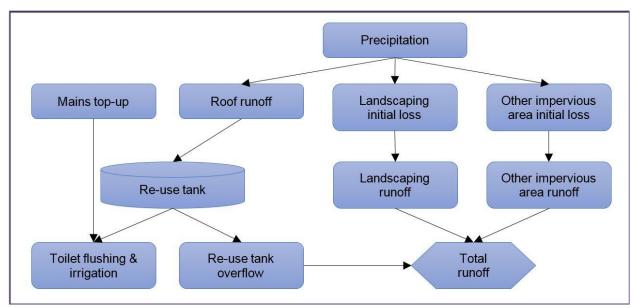


Figure 2. Water balance flow

3.2.1 Pervious area treatment

The pervious area initial loss for areas of proposed deep soil was increased to reflect a sandy loam topsoil as opposed to silty clay which is present under existing conditions.

3.2.2 Rainwater re-use calculations

Re-use was determined specifically for each year based on moisture deficit. The moisture deficit was calculated daily by subtracting the daily evapotranspiration from the daily rainfall. Only on days where the moisture deficit is equal to or greater than 10 mm was a re-use event triggered.

Based on the above methodology, the annual re-use demand varied for each year depending on the rainfall trend. i.e. for drier years, the demand for irrigation is higher than it is for wet years. This is captured in this methodology and the results are presented in Section 3.3.

3.3 Water balance – proposed conditions

A water balance was conducted for the developable site area under proposed conditions for each of the assessed years. The results are presented in Table 2 for the dry year, Table 3 for the average rainfall year and Table 4 for the wet year.



Table 2. Post-development water balance (dry year)

Post-development site (dry year)			
Areas			
total area	13,470	m ²	
impervious area draining to re-use	2,800	m ²	
pervious area	4,700	m ²	
impervious area graded to landscaping	0	m ²	
impervious area bypassing reuse & landscaping	5970	m ²	
Reuse			
total reuse tank volume	105	m^3	
initial fraction full of reuse tanks	100	%	
irrigation use per event (pervious areas only)	0.015	m	
deficit before irrigation	10	mm	
Losses			
pervious area loss (mm)	10	mm	
impervious area loss (mm)	1	mm	
area to re-use loss (mm)	0.5	mm	
Statistics			
number of tank spill days	6		
number of irrigation days	80		
annual irrigation volume	4.79	ML	
Annual runoff total			
number of runoff days	71	days	
total runoff volume (dry year)	6.40	ML	

Table 3. Post-development water balance (average rainfall year)

Post-development site (average year)			
Areas			
total area	13,470	m ²	
impervious area draining to re-use	2,800	m ²	
pervious area	4,700	m ²	
impervious area graded to landscaping	0	m ²	
impervious area bypassing reuse & landscaping	5970	m ²	
Reuse			
total reuse tank volume	105	m ³	
initial fraction full of reuse tanks	100	%	
irrigation use per event (pervious areas only)	0.015	m	
deficit before irrigation	10	mm	
Losses			



pervious area loss (mm)*	10	mm
impervious area loss (mm)	1	mm
area to re-use loss (mm)	0.5	mm
Statistics		
number of tank spill days	30	
number of irrigation days	75	
annual irrigation volume	4.49	ML
Annual runoff total		
number of runoff days	76	days
total runoff volume (average rainfall year)	9.26	ML

Table 4. Post-development water balance (wet year)

Post-development site (wet year)			
Areas			
total area	13,470	m ²	
impervious area draining to re-use	2,800	m²	
pervious area	4,700	m ²	
impervious area graded to landscaping	0	m ²	
impervious area bypassing reuse & landscaping	5970	m ²	
Reuse			
total reuse tank volume	105	m^3	
initial fraction full of reuse tanks	100	%	
irrigation use per event (pervious areas only)	0.015	m	
deficit before irrigation	10	mm	
Losses			
pervious area loss (mm)*	10	mm	
impervious area loss (mm)	1	mm	
area to re-use loss (mm)	0.5	mm	
Statistics			
number of tank spill days	44		
number of irrigation days	72		
annual irrigation volume	4.31	ML	
Annual runoff total			
number of runoff days	99	days	
total runoff volume (wet year)	16.38	ML	

Based on the foregoing, it has been determined that under proposed conditions, post development flows are +-10% of pre-development conditions. Additionally, the number of runoff days under proposed conditions are the same as pre-development conditions except for the wet year which reports one additional runoff generating event per year when compared to existing conditions.



4 Water Quality Assessment

4.1 Water quality monitoring plan

A water quality monitoring plan has been prepared by H2O Consulting Group. We understand the water monitoring plan will be used to set baseline water quality parameters for the site and will also define minimum testing requirements.

4.2 Water quality monitoring data

Water quality testing is being undertaken by H2O Consulting Group.

5 Water Quality Management

5.1 Erosion and sediment control during construction

During construction, an erosion and sediment control plan to be prepared prior to issue of construction certificate will be designed and implemented in accordance with Council requirements and the requirements of NSW DPE document 'Managing Urban Stormwater: Soils and Construction' 2008.

We note an erosion and sediment control plan is not required at Development Application stage in accordance with the Warriewood Water Management Specification checklist included under Annexure xx.

5.2 Proposed stormwater quality measures – post-development

Stormwater runoff quality from the development has been assessed under pre- and post-development conditions using MUSIC software which considers the treatment train depicted in Figure 1. The water quality treatment systems for the site consist of the following:

- Bio-retention basin No.1 comprising an area of 474 m² for the treatment of proposed Lots 6-11 and the RFB site.
- Bio-retention basin No.2 comprising an area of 300 m² for the treatment of proposed Lots 1-5 and Lorikeet Grove.
- Below ground Gross Pollutant Traps (GPTs) (one (1) for each bio-retention basin).
- 5kL rainwater re-use tanks on each torrents title lot and a 50kL rainwater re-use tank common to the RFB buildings. Total proposed rainwater re-use tank volume is 105 kL for the development.

5.3 MUSIC modelling

MUSIC modelling software is a commonly utilised and accepted software for modelling the effectiveness of stormwater quality treatment devices.

The following scenarios were assessed for the three years described in Section 3.

- Existing site conditions (pre-development)
- Proposed development (post-development) with proposed water quality treatment measures.

The MUSIC model schematic is presented in Figure 3, and a summary of inputs and results are presented thereafter.



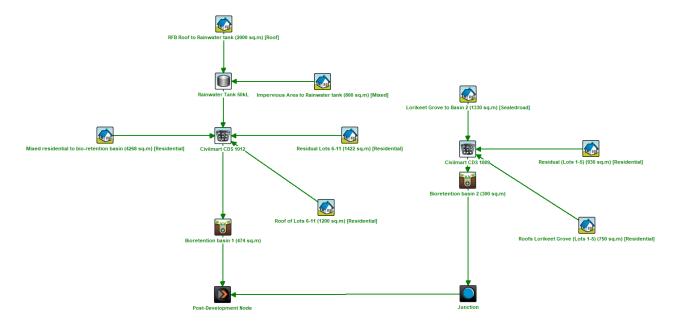


Figure 3. MUSIC model schematic

5.3.1 Catchment areas

The pre- and post-development catchments considered are presented in Table 5 below.

Table 5. MUSIC catchment summary

Catchment	Area (ha)	Impervious Fraction (%)
Existing site	1.347	10
RFB roof to rainwater tank	0.2	100
RFB (residential) to bio-retention basin 1	0.426	47
Impervious RFB area to rainwater tank (refer to BASIX)	0.08	100
Roof of lots 6-11	0.12	100
Residual area for lots 6-11	0.142	30
Basin 1 area	0.047	0
Lorikeet Grove to basin 2	0.133	85
Roof of lots 1-5	0.072	10
Residual area for lots 1-5	0.09	100
Basin 2	0.03	0



5.3.2 Source of rainfall and evapotranspiration data

Rainfall data was sourced from the Bureau of Meteorology via the website.

The data was sourced as daily rainfall data and average monthly evapotranspiration for the Mona Vale golf course (gauge 066141) as described in Section 3.

5.3.3 Pollutant loads

The Event Mean Concentrations for pollutants loading was defined in the MUSIC model in accordance with the parameters required by Council WMS. These EMCs are presented in Table 6 below.

Table 6. Pollutant loads

Land use	Total Suspended Solids (TSS) (mg/L)	Total Phosphorus (TP) (mg/L)	Total Nitrogen (TN) (mg/L)
Rural residential (existing site conditions)	35	0.1	1
Urban (proposed site)	100	0.3	1.5

5.3.4 Pollutant capture targets

The proposed treatment system was tested against two pollutant targets. Firstly, against the general requirements of Northern Beaches Council's Water Management for Development Policy (WMDP). Secondly, against the requirements of Council WMS. The targets outlined in the WMDP are presented in Table 7 and the WMS requires post-development pollutants to be less than or equal to pre-development conditions.

Table 7. Pollutants capture targets.

Pollutant	Target
Total suspended solids (TSS)	80% capture
Total phosphorus (TP)	65% capture
Total nitrogen (TN)	45% capture
Gross pollutants (GP)	90% capture

5.3.5 Bio-retention basin features

The features of the bio-retention basins are presented below in Table 8.

Table 8. Bio-retention basin properties.

Basin	Basin Area (m²)	Extended detention depth (m)	Saturated hydraulic conductivity (mm/hr)	Underdrain (yes/no)	Basin lined (yes/no)
Basin 1	474	0.3	100	yes	yes
Basin 2	300	0.2	100	yes	yes



5.3.6 Gross pollutants traps

The pollutants traps proposed for the site include the following:

- Basin 1 CDS unit 1012
- Basin 2 CDS unit 1009

The CDS units are based on preliminary sizing from Civil Mart to cater for flows up to the 1 year ARI flows. An approved equal may be appropriately designed at construction certificate stage if the above is not desirable.

5.3.7 Music modelling results

Based on the foregoing, the outputs from the pre-development MUSIC model were compared to post-development outputs. The results are presented in Table 9, Table 10, and Table 11 for each year assessed.

Table 9. MUSC results (dry year).

	Parameter	Existing case	Post-dev pollutant load (without treatment)	Post-dev (with treatment)	Post-dev reduction (%)	Pre-dev vs Pos-dev reduction (%)
	Flow					
	(ML/yr)	3.91	7.42	4.2	43.5%	-7.4%
Dry	TSS (kg/yr)	203	1110	11.8	98.9%	94.2%
(2002)	TP (kg/yr)	0.411	2.24	0.273	87.8%	33.6%
	TN (kg/yr)	4.82	14	2.67	80.9%	44.6%
	GP (kg/yr)	50.3	212	0	100.0%	100.0%

Table 10. MUSIC results (average year).

	Parameter	Existing case	Post-dev pollutant load (without treatment)	Post-dev (with treatment)	Post-dev reduction (%)	Pre-dev vs Pos-dev reduction (%)
	Flow	F 7	0.07	F 00	44.00/	2.460/
	(ML/yr)	5.7	9.97	5.88	41.0%	-3.16%
Ave	TSS (kg/yr)	218	1450	15.7	98.9%	92.80%
(1984)	TP (kg/yr)	0.679	3.38	0.372	89.0%	45.21%
	TN (kg/yr)	5.04	18	3.72	79.4%	26.19%
	GP (kg/yr)	77.7	281	0	100.0%	100.00%

Table 11. MUSIC results (wet year).

	Parameter	Existing case	Post-dev pollutant load (without treatment)	Post-dev (with treatment)	Post-dev reduction (%)	Pre-dev vs Pos-dev reduction (%)
	Flow					
	(ML/yr)	13.7	17	12.9	23.8%	5.84%
Wet	TSS (kg/yr)	576	2120	33.2	98.4%	94.24%
(1998)	TP (kg/yr)	1.61	5.41	0.8	85.2%	50.31%
	TN (kg/yr)	14.4	32.3	7.87	75.6%	45.35%
	GP (kg/yr)	98.9	323	0	100.0%	100.00%



Based on the foregoing, the proposed water quality management system meets the water quality targets prescribed by Council Water Management for Development Policy and the Warriewood Water Management Specification. Additionally, we note that the annual flow has generally been maintained from pre-development to post-development within acceptable tolerances (+-10% of pre-development flows). Subsequently, no additional flow control such as pumping is required to maintain existing flow behaviour on the site.

6 Stormwater Quantity Management

Stormwater runoff rate and quantity is managed by the provision od detention storage in the bio-retention basins. The storage volume for quantity management has been provided above the extended detention depths report in Section 5.3.5.

A DRAINS model was prepared to assess pre- and post-development stormwater runoff from the site.

The catchment considered in the DRAINS model for each basin is depicted in Civil Engineering Plans prepared by ACOR Consultants Pty Ltd, Reference CC230177, Sheet C1.01, Revision C, dated 6 March 2024 (copy enclosed under Annexure A).

The DRAINS model configuration represents the stormwater management on-site detention basins depicted on in Civil Engineering Plans prepared by ACOR Consultants Pty Ltd, Reference CC230177, Sheets C3.05 & C3.06, Revision C, dated 6 March 2024 (copy enclosed under Annexure A).

A copy of the DRAINS model files has been forwarded to Council for assessment.

6.1 DRAINS catchment parameters.

A summary of the DRAINS catchment parameters for both pre- and post-development conditions are presented below in Table 12.

Table 12. DRAINS parameters.

Parameters	Existing conditions basin 1	Existing conditions basin 2	Proposed conditions basin 1	Proposed conditions basin 2
Catchment area (ha)	0.931	0.286	0.931	0.286
Impervious fraction (%)	0	0	70	90
Time of concentration	Pervious - 8 mins	Pervious - 8 mins	Pervious - 8 mins Impervious - 5 mins	Pervious - 8 mins Impervious - 5 mins



6.2 On-site detention results

The pre- and post-development results for basin 1 and basin 2 catchments are presented in Table 13 and Table 14 respectively.

Table 13. On-site detention results basin 1.

Basin 1 catchment					
	Pre-	Post-	Detention		
	development	development	volume		
Storm event	flow (m ³ /s)	flow (m ³ /s)	utilised (m ³)		
1yr ARI +CC	0.219	0.152	125		
5yr ARI +CC	0.418	0.227	235		
10yr ARI +CC	0.481	0.248	260		
20yr ARI +CC	0.565	0.275	330		
100yr ARI +CC	0.687	0.567	400		

Table 14. On-site detention results basin 2.

Basin 2 catchment					
Storm event	Pre- development flow (m³/s)	Post- development flow (m³/s)	Detention volume utilised (m³)		
1yr ARI +CC	0.067	0.024	85		
5yr ARI +CC	0.128	0.061	115		
10yr ARI +CC	0.148	0.085	118		
20yr ARI +CC	0.173	0.093	142		
100yr ARI +CC	0.211	0.127	170		

Based on the foregoing, the proposed detention basins attenuate flows for all storms up to and including the 100 Year ARI storm event with consideration of climate change (30% increase in rainfall).

6.3 Distribution of outflows from the detention basins

Both proposed detention basins discharge into a specifically designed energy dissipation and level spreader system. The detail of the system is depicted on Civil Engineering Plans prepared by ACOR Consultants Pty Ltd, Reference CC230177, Sheets C3.05 & C3.06, Revision C, dated 6 March 2024 (copy enclosed under Annexure A) and generally comprises 2 m wide rock lined swale designed to dissipate flows at the existing natural surface level downstream of the basins.

6.4 Distribution of outflows from the proposed culvert

A 2.1 m wide 0.6 m high culvert is proposed within the 3.5 m wide proposed easement along the site's eastern boundary. The culvert has been designed to convey the full 100 year ARI flows from the upstream catchment. Details of the culvert are depicted on ACOR Consultants Pty Ltd, Reference CC230177, Sheet C3.14, Revision C, dated 6 March 2024 (copy enclosed under Annexure A).



The outlet of the proposed culvert comprises an energy dissipation area including a low flow weir to ensure pre-developed flows are conveyed to Narrabeen Creek via an existing channel during all storm events up to and including the 1 year ARI storm event. When the 1 year ARI flows are exceeded, stormwater will overflow via a weir into the proposed level spreader system and distributed at natural surface levels into the wetland.

7 Watercourse and Corridor Preservation

Based on the foregoing, the proposed water quality systems and on-site detention systems ensure that the proposed development does not result in adverse impacts relating to water quantity and water quality on the downstream wetlands which support a variety of endangered ecological communities.

8 Flood Protection

The following consideration have been made in relation to the management of flood risk on the site:

- All proposed new lots are proposed to be constructed to levels above the applicable Flood Planning Level
 of RL 4.71 m AHD and allow for the construction of future dwellings with habitable floor levels at or above
 the Probable Maximum Flood (PMF) level of RL 4.88 m AHD.
- Construction of the overland flow path along the proposed 3.5 m wide drainage easement has been designed to ensure PMF flows are H1 or H2 hazard (not hazardous to people).

Refer to Flood Impact Assessment prepared by BMT for additional information relating to flood impacts and flood risk management.

9 Stormwater Drainage Concept Plan

The stormwater drainage concept plan is depicted on Civil Engineering Plans prepared by ACOR Consultants Pty Ltd, Reference CC230177, Sheet C1, Revision C, dated 6 March 2024 (copy enclosed under Annexure A). The plans incorporate all of the details covered in this report which have been prepared in line with current industry standards and applicable Council requirements.

9.1 Sydney water sewer pipe

There is an 1800mm dia. Sydney Water sewer pipe traversing the site within a 5 m wide easement. The details of the sewer have been confirmed on site by a qualified contractor considered in the preparation of the stormwater management concept for the site. The proposed stormwater system interacts with the 5 m wide sewer easement in the following locations:

- Minor stormwater pipes must cross over the sewer in three locations. The locations are depicted on ACOR Consultants Pty Ltd, Reference CC230177, Sheet C2.01, Revision C, dated 6 March 2024 (copy enclosed under Annexure A).
- The proposed 2.1 m wide, 0.6 m high culvert must cross **below** the sewer pipe. We note the culvert proposes a 450 mm vertical clearance to the underside of the sewer pipe.

Notwithstanding, the above will form part of an out of scope application to Sydney Water prepared by a suitably qualified Water Services Coordinator during construction certificate stage.



10 Contentions

To address the contentions raised by Council, supplementary civil engineering plans have been prepared. In this regard, we refer to our document reference CC230177, sheets C1.00 to C4.03 (26 sheets), revision C, dated 6 March 2024 (copies enclosed under Annexure A). The DRAINS modelling and MUSIC modelling which forms the basis for our revised design can be forwarded to Council and their representatives for review. A description of the DRAINS modelling parameters are outlined in Addendum Flood Impact Assessment prepared by BMT.

The relevant contentions and associated responses are included in Table 15.

Table 15. Contentions and responses.

No.	Contention	Response
3.a.i	The proposal seeks to discharge stormwater into the wetlands at the rear of the site. The Applicant has not addressed the impacts of the outlet flow upon the wetlands, which contain a groundwater dependent EEC.	We refer to our document CC230177, sheets C1.00 to C4.03 (26 sheets), revision C, dated 6 March 2024 and note that an updated BDAR has been prepared based on these plans. We confirm our revised engineering plans provide an outcome that accommodates the maintenance of base flows currently servicing the EEC.
3.a.ii	The proposal seeks to construct a 900mm RCP stormwater line within the existing drainage reserve (43 Warriewood Road) to cater for the 1 in 100-year AEP upper catchment flow from upslope of the site. Although this approach is supported in principle, the Applicant has not demonstrated how the flow of water from upslope is to be suitably captured/directed into the proposed pipeline	A DRAINS model has been prepared based on our revised civil engineering plans. The model demonstrates that the revised stormwater system is able to capture all of the 100 Year ARI +CC30 flows from Warriewood Road and convey them via the proposed culvert to Narrabeen Creek. The DRAINS modelling includes a 20% blockage applied to all on-grade pits and a 50% blockage to all sag pits.
3.a.iii	The proposed 900mm RCP stormwater line within the existing drainage reserve (43 Warriewood Road) is required to be included in the TUFLOW model, inclusive of all existing and proposed drainage systems in the catchment in order to demonstrate that there are no adverse impacts associated with the proposal up to the PMF Event. The assessment needs to assess the worst-case scenario where there is a coincidental local catchment and Narrabeen Creek event up to the PMF event.	Refer to supplementary information prepared by BMT. We note the proposed overland flow path coveys PMF floodwater falling within the H1-H2 hazard classification which is non-hazardous to people.
3.a.iv	The Applicant has not provided sufficient detail with respect to the design of the overland flow path over the proposed stormwater line within the existing drainage reserve. Cross sections are required along the overland flow path to detail the 1& AEP Top Water Levels and to ensure that all habitable areas are a minimum of 500mm above these levels.	We refer to our response to contention 3.a.ii and note that a revised design and associated DRAINS model has been prepared which demonstrates the system depicted on our revised civil engineering plans can sufficiently capture and convey 100 Year ARI + CC30 flows through the site.



No.	Contention	Response
3.a.v	The overland flow path dissects Lot 1 and limits the available footprint for future development.	We refer to Site Plan prepared by Archidrome, Drawing No.A03, Revision 12, dated 6 March 2024 and note the revised overland flow path does not encroach on the proposed new Lot 1
3.a.vi	The Applicant does not adequately demonstrate suitable infrastructure upgrades on Warriewood Road, with a new 375mm RCP pipe and pit/s required.	We refer to our revised civil engineering plans and note that a revised stormwater drainage system for Warriewood Road has been documented.
3.a.vii	The development application is not supported by sufficient long sections of stormwater lines, required to detail surface levels, the location of the pipe, the hydraulic grade line, velocities, and flows.	We refer to our revised civil engineering plans and note that a revised stormwater drainage long section has been document. The long section includes a hydraulic grade line analysis for the 100 Year ARI flows plus Climate Change.
3.a.viii	It is unclear whether the Overland Flow Study was undertaken in accordance with Australian Rainfall and Runoff 2019 and Book 9 A Guide to Flood Estimation in Urban Areas, or whether climate change was incorporated into the modelling. Further, the modelling needs to be undertaken by an Engineer that is registered under the NSW Design and Building Practitioners Act and Regulation.	We refer to Addendum Flood Impact Assessment prepared by BMT, reference L.N20951.007_Warriewood_Rd_FloodingRFl.docx, dated 8 July 2022. In this regard, we have reviewed the adopted methodology and confirm that climate change has been considered. Additionally, we are of the view that the view the adopted flood modelling is appropriate to allow for an assessment of flood impacts and flood risk associated with the development.
3.a.ix	The development application is not accompanied by a completed/signed Documentation Checklist – Development Application, as required by the Water Management Specification.	We refer to our revised civil engineering plans and note that some of the measures included in our assessment result from an engineering assessment. Whilst these concepts have been discussed with Council's representatives, a merit-based assessment is required which falls outside the items defined within the checklist.
3.a.x	The Applicant has not demonstrated that flood free evacuation from all lots is possible in the PMF event.	We refer to our response to contention 3.a.iii and note supplementary modelling has been undertaken. In this regard, we refer to the revised PMF assessment prepared by BMT.
3.a.xi	The Applicant has failed to consider potential impacts upon the water table, or the groundwater dependent EEC.	We refer to our document CC230177, sheets C1.00 to C4.03 (26 sheets), revision C, dated 6 March 2024 and note that an updated BDAR has been prepared based on these plans. We confirm our revised engineering plans provide an outcome that accommodates the maintenance of base flows currently servicing the EEC.
3.a.xii	The Applicant has not adequately address water quality, with over simplified and generalised statements that are not specific to the unique circumstances of the site.	A detailed MUSIC model has been prepared as part of our revised assessment. The MUSIC model can be provided to Council's representative for assessment. We can confirm that the MUSIC modelling was undertaken in accordance with Council's DCP requirements, and the pollutant reduction targets of Council's Water Management for Development Specification have been met for scenarios representing a dry, wet, and average rainfall year.
3.a.xiii	Insufficient information has been provided in relation to the proposed infiltration basin, with	We refer to our document CC230177, sheets C1.00 to C4.03 (26 sheets), revision C, dated 6 March 2024 and



No.	Contention	Response
	concern that large volumes of runoff will not be treated. A bioretention basin is considered to be more suitable in this instance.	note that two basins have been documented to manage stormwater flows and to provide a compliant water quality outcome. The details of the basin are depicted on our revised documents.
3.a.xiv	The proposed infiltration basin has a weir to control the outflow. The Applicant has not demonstrated that this is an appropriate outlet method in light of the groundwater dependent EEC along the creek line.	We refer to our document CC230177, sheets C1.00 to C4.03 (26 sheets), revision C, dated 6 March 2024 and note that weir overflows are proposed to an energy dissipation swale. This method has been discussed with Council's representatives in concept and are documented on our revised civil engineering plans. We understand this is a satisfactory outcome resulting in minimal impact to the EEC.
3.a.xv	The proposed use of litter baskets is not a practical outcome, with a gross pollutant trap is more appropriate.	We refer to our document CC230177, sheets C1.00 to C4.03 (26 sheets), revision C, dated 6 March 2024 and note two GPTs have been proposed. We acknowledge the limited fall available to provide a large GTP due to the Lorikeet Grove Road surface levels required to tie in with existing road levels. Notwithstanding, we have been liaising with manufacturers who have advised custom solutions are available to suit.
3.a.xvi	The MUSIC Model is to be prepared in accordance with Council's guidelines and is to be supplied to Council as a model file (.sqz).	We can provide our MUSIC model for assessment.
3.a.xvii	The Engineering Report (C&M Consulting Engineers, 4 June 2021) relies upon rainwater tanks on each of the 11 residential lots that are not proposed as part of the proposal, and the assumed rainwater reuse is inconsistent with the submitted BASIX Certificate.	We confirm our revised assessment includes both the ultimate scenario described in the C & M report dated 4 June 2021. We note it is common practice for new dwellings to require reasonably sized rainwater tanks and which will be a requirement of future BASIX for the new lots.
3.a.xviii	The Engineering Report (C&M Consulting Engineers, 4 June 2021) makes assumptions regarding the developed conditions of Catchment 3, being the proposed Torrens Title lots fronting Lorikeet Drive and the access driveway for the residential flat buildings. It is unclear how these assumed calculations have been derived or how the consent authority can ensure consistency with these assumptions in the long term.	We refer to drawing C1.01 which depicts the catchments draining to each of the proposed basins. A DRAINS model has been prepared to size the OSD basins to ensure post-development flows do not exceed pre-developed conditions for a range of storm events from the 1 Year ARI to 100 Year ARI flood event plus climate change.
3.a.xviii	For the purpose of assessing water management and flooding, is it unclear whether the relevant technical consultants have relied upon the architectural plans or the civil plans, which significantly differ with respect to the earthworks proposed and resultant ground levels.	We confirm our revised documents are consistent with the details depicted on Site Plan prepared by Archidrome, Drawing No.A03, Revision 12, dated 6 March 2024.
3.b	Clause C6.1 (Integrated Water Cycle Management) of P21 DCP and the Water Management Specification require a cohesive,	



No.	Contention	Response
	site specific solution for water management across a development site.	We confirm our revised documents are consistent with the details depicted on Site Plan prepared by Archidrome,
3.c	Although individual technical reports have been provided, they are internally inconsistent, and with each other, and do not satisfactorily demonstrate how the water cycle is cohesively managed on the site.	Drawing No.A03, Revision 12, dated 6 March 2024
3.e	Without consistency with the Water Management Specification or adequate consideration of impacts associated with the proposal, it is also inconsistent with the requirements and objectives of cl.5.21 (Flood planning) of PLEP 2014.	
3.d	Without consistency with the Water Management Specification or adequate consideration of impacts associated with the proposal, it is also inconsistent with the requirements and objectives of cl.6.1(3) of PLEP 2014.	

Conclusion 11

Based on the foregoing, we are of the view that the current list of civil engineering contentions before the court can be satisfactorily addressed based on the contents of our revised documents.

Yours faithfully, ACOR Consultants (CC) Pty Ltd

Mbroallsut

Nathan Broadbent BEng(Civil) CPEng NER



Annexure A ACOR Consultants (CC) Pty Ltd Civil Engineering Plans, reference CC230177, sheets C1.00 to C4.03 (26 sheets), Revision C dated 6 March 2024.

PROPOSED DEVELOPMENT (No.43-49) WARRIEWOOD ROAD, WARRIEWOOD

CIVIL ENGINEERING PLANS

GENERAL NOTES

- THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH ALL ARCHITECTURAL AND OTHER CONSULTANTS DRAWINGS AND SPECIFICATIONS AND WITH SUCH OTHER WRITTEN INSTRUCTIONS AS MAY BE ISSUED.
- THE CONTRACTOR BEFORE CONSTRUCTION
- DIMENSIONS SHALL NOT BE OBTAINED BY SCALING THE DRAWINGS
- ALL DIMENSIONS ON DETAILS ARE IN MILLIMETRES UNLESS STATED OTHERWISE. ALL PLANS AND LEVELS ARE EXPRESSED IN METRES.
- DURING CONSTRUCTION THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING THE STRUCTURAL STABILITY OF THE WORKS AND ENSURE NO PARTS BE OVER STRESSED UNDER CONSTRUCTION ACTIVITIES.
- THE APPROVAL OF A SUBSTITUTION SHALL BE SOUGHT FROM THE ENGINEER BUT IS NOT AN AUTHORISATION FOR A VARIATION. ANY VARIATIONS INVOLVED MUST BE TAKEN UP WITH THE ARCHITECT OR PROJECT MANAGER BEFORE THE WORK COMMENCES.
- ANY DISCREPANCIES OR OMISSIONS SHALL BE REFERRED TO THE ENGINEER FOR A
- THE BUILDER SHALL GIVE 48 HOURS NOTICE FOR ALL ENGINEERING INSPECTIONS
- 10. BUILDING FROM THESE DRAWINGS IS NOT TO COMMENCE UNTIL APPROVED BY THE LOCAL
- THE WORD 'ENGINEER' USED IN THESE NOTES REFER TO AN EMPLOYEE OR NOMINATED

ROADWORKS NOTES

- ALL BASECOURSE AND SUB-BASECOURSE MATERIALS SHALL CONFORM WITH AUSTRALIAN
- ALL BASECOURSE AND SUB-BASE MATERIALS SHALL BE COMPACTED TO ACHIEVE A MINIMUL OF 100% STANDARD MAXIMUM DRY DENSITY AT OPTIMUM MOISTURE CONTENT OF +OR- 2% II
- CONCRETE FOR KERB SHALL HAVE A CONCRETE STRENGTH OF 20MPa AT 28 DAYS, MINIMUM SLUMP OF 60mm AND MAXIMUM AGGREGATE SIZE OF 40mm.

EXISTING SERVICES AND FEATURES

- THE CONTRACTOR SHALL ALLOW FOR THE CAPPING OFF, EXCAVATION, REMOVAL AND DISPOSAL IF REQUIRED OF ALL EXISTING SERVICES IN AREAS AFFECTED BY WORKS WITHIN THE CONTRACT AREA, AS SHOWN ON THE DRAWINGS UNLESS DIRECTED OTHERWISE BY THE
- THE CONTRACTOR IS TO CONDUCT A THOROUGH UNDERGROUND SERVICES INVESTIGATION TO LOCATE ALL SERVICES WITHIN THE AREA OF WORKS PRIOR TO ANY DEMOLITION WORKS
- THE CONTRACTOR SHALL ENSURE THAT AT ALL TIMES SERVICES TO ALL BUILDINGS NOT
- PRIOR TO COMMENCEMENT OF ANY WORKS THE CONTRACTOR SHALL GAIN WRITTEN
- CONTRACTOR SHALL CONSTRUCT TEMPORARY SERVICES TO MAINTAIN EXISTING SUPPLY TO BUILDINGS REMAINING IN OPERATION DURING WORKS TO THE SATISFACTION AND APPROVAL OF THE SUPERINTENDENT. ONCE DIVERSION IS IS COMPLETE AND COMMISSIONED THE CONTRACTOR SHALL REMOVE ALL SUCH TEMPORARY SERVICES AND MAKE GOOD TO THE SATISFACTION OF THE SUPERINTENDENT.
- INTERRUPTION TO SUPPLY OF EXISTING SERVICES SHALL BE DONE SO AS NOT TO CAUSE ANY INCONVENIENCE TO THE PRINCIPAL CONTRACTOR TO GAIN APPROVAL OF SUPERINTENDENT FOR TIME OF INTERRUPTION.

SITEWORKS NOTES

- ORIGIN OF LEVELS :- AUSTRALIAN HEIGHT DATUM (A.H.D.)
- CONTRACTOR MUST VERIFY ALL DIMENSIONS AND EXISTING LEVELS ON SITE PRIOR TO
- ALL WORK IS TO BE UNDERTAKEN IN ACCORDANCE WITH THE DETAILS SHOWN ON THE
- EXISTING SERVICES HAVE BEEN PLOTTED FROM SUPPLIED DATA AND AS SLICH THEIR EADS INVO SERVICES TAYE BEEN FLOTTED FROM SUPPLIED DATA AND AS SOUTH THEIR ACCURACY CANNOT BE GUARANTEED. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ESTABLISH THE LOCATION AND LEVEL OF ALL EXISTING SERVICES PRIOR TO THE COMMENCEMENT OF ANY WORK. ANY DISCREPANCIES SHALL BE REPORTED TO THE PAL'S REPRESENTATIVE. CLEARANCES SHALL BE OBTAINED FROM THE RELEVANT
- WHERE NEW WORKS ABUT EXISTING THE CONTRACTOR SHALL ENSURE THAT A SMOOTH EVEN PROFILE, FREE FROM ABRUPT CHANGES IS OBTAINED.
- THE CONTRACTOR SHALL ARRANGE ALL SURVEY SETOUT TO BE CARRIED OUT BY A
- CARE IS TO BE TAKEN WHEN EXCAVATING NEAR EXISTING SERVICES. NO MECHANICAL EXCAVATIONS ARE TO BE UNDERTAKEN OVER COMMUNICATIONS OR ELECTRICAL SERVICES.
- ALL SERVICE TRENCHES LINDER VEHICLILAR PAVEMENTS SHALL BE BACKELLED WITH AN MAXIMUM DRY DENSITY IN ACCORDANCE WITH AS 1289.5.1.1.
- TRENCH BACKFILL MATERIAL SHALL BE COMPACTED TO THE SAME DENSITY AS THE
- ON COMPLETION OF PIPE INSTALLATION ALL DISTURBED AREAS MUST BE RESTORED TO ORIGINAL, INCLUDING KERBS, FOOTPATHS, CONCRETE AREAS, GRAVEL AND GRASSED
- PROVIDE 12mm WIDE EXPANDING CORK JOINTS BETWEEN CONCRETE PAVEMENTS AND ALL BUILDINGS, WALLS, FOOTINGS, COLUMNS, KERBS, DISH DRAINS, GRATED DRAINS, BOLLARD
- CONTRACTOR TO OBTAIN ALL AUTHORITY APPROVALS
- ALL BATTERS TO BE GRASSED LINED WITH MINIMUM 100 TOPSOIL AND APPROVED COUCH LAID AS TURF.
- THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY DIVERSION DRAINS AND MOUNDS TO ENSURE THAT AT ALL TIMES EXPOSED SURFACES ARE FREE DRAINING AND WHERE NECESSARY EXCAVATE SUMPS AND PROVIDE PUMPING EQUIPMENT TO DRAIN EXPOSED
- THESE PLANS SHALL BE READ IN CONJUNCTION WITH APPROVED ARCHITECTURAL
- TRENCHES THROUGH EXISTING ROAD AND CONCRETE PAVEMENTS SHALL BE SAWCUT TO
- INCLUDING, BUT NOT LIMITED TO, KERBS, FOOTPATHS, CONCRETE AREAS, GRASS AND

COMPACTION NOTES

- SUPERINTENDENT AND REPLACED WITH APPROVED MATERIAL SATISFYING THE
- ALL FILL MATERIAL SHALL BE FROM A SOURCE APPROVED BY THE SUPERINTENDENT AND SHALL COMPLY WITH THE FOLLOWING :
 - FREE FROM ORGANIC, PERISHABI F AND CONTAMINATED MATTER
 - MAXIMUM PARTICLE SIZE 75MM PLASTICITY INDEX BETWEEN 2% AND 15%
- THE CONTRACTOR SHALL PROGRAM THE EARTHWORKS OPERATION SO THAT THE WORKING AREAS ARE ADEQUATELY DRAINED DURING THE PERIOD OF CONSTRUCTION. THE SURFACE SHALL BE GRADED AND SEALED OFF TO REMOVE DEPRESSIONS AND SIMILAR WHICH WOULD ALLOW WATER TO POND AND PENETRATE THE UNDERLYING MATERIAL. ANY DAMAGE RECTIFIED BY THE CONTRACTOR AT THEIR COST
- TESTING OF THE SUBGRADE SHALL BE CARRIED OUT BY AN APPROVED NATA REGISTERED

STORMWATER NOTES

- ALL 225 DIA. DRAINAGE PIPES AND LARGER SHALL BE CLASS "2" APPROVED SPIGOT AND
- ALL PIPE JUNCTIONS UP TO AND INCLUDING 450 DIA. AND TAPERS SHALL BE VIA PURPOSE
- MINIMUM GRADE TO STORMWATER LINES TO BE 1%. (U.N.O.)
- CONTRACTOR TO SUPPLY AND INSTALL ALL FITTINGS AND SPECIALS INCLUDING VARIOUS
- ALL CONNECTIONS TO EXISTING DRAINAGE PITS SHALL BE MADE IN A TRADESMAN-LIKE MANNER AND THE INTERNAL WALL OF THE PIT AT THE POINT OF ENTRY SHALL BE CEMENT RENDERED TO ENSURE A SMOOTH FINISH.
- WHERE TRENCHES ARE IN ROCK, THE PIPE SHALL BE BEDDED ON A MIN. 50MM CONCRETE BED (OR 75MM THICK BED OF 12MM BLUE METAL) UNDER THE BARREL OF THE PIPE. THE PIPE COLLAR AT NO POINT SHALL BEAR ON THE ROCK, IN OTHER THAN ROCK, PIPES SHALL BE LAID ON A 75MM THICK SAND BED. IN ALL CASES BACKELL THE TRENCH WITH SAND TO 200MM ABOVE THE PIPE. WHERE THE PIPE IS UNDER PAVEMENTS BACKFILL REMAINDER OF TRENCH WITH SAND OR APPROVED GRANULAR BACKELL COMPACTED IN 150MM LAYERS TO 98%
- BEDDING SHALL BE (U.N.O.) TYPE H1. IN ACCORDANCE WITH CURRENT RELEVANT
- WHERE STORMWATER LINES PASS UNDER FLOOR SLABS SEWER GRADE RUBBER RING
- WHERE SUBSOIL DRAINAGE LINES PASS UNDER FLOOR SLABS AND VEHICULAR PAVEMENTS UNSLOTTED UPVC SEWER GRADE PIPE SHALL BE USED.
- PROVIDE 3.0M LENGTH OF 100 DIA. SUBSOIL DRAINAGE PIPE WRAPPED IN FABRIC SOCK, AT

CONCRETE PAVEMENT NOTES

- - MAXIMUM AGGREGATE SIZE 20mm - FLEXURAL STRENGTH AT 28 DAYS = 3.5MPa
 - FLEXURAL STRENGTH AT 90 DAYS = 3.85 MPa
 - MAXIMUM WATER / CEMENT RATIO = 0.55 MAXIMUM SHRINKAGE I IMIT = 650 MICRON STRAINS
 - (AS 1012 Pt 13)
 MINIMUM CEMENT CONTENT = 300kg/m3

 - CEMENT TO BE TYPE "A" (NORMAL CEMENT) TO AS.1315 SLUMP = 50mm
- JOINT TO BE SAWN AS SOON AS CONCRETE HAS HARDENED SUFFICIENTLY THAT IT WILL NOT BE DAMAGED BY SAWING. IF AN UNPLANNED CRACK OCCURS THE CONTRACTOR SHALL REPLACE WHOLE SLABS EITHER SIDE OF THE UNPLANNED CRACK, UNLESS
- CONSTRUCT JOINTS AS DETAILED
- CONSTRUCTION JOINTS WHERE REQUIRED BUT NOT SHOWN, SHALL BE LOCATED TO THE APPROVAL OF THE ENGINEER AND CONSTRUCTED AT THE CONTRACTORS EXPENSE
- ALL LONGITUDINAL CONSTRUCTION JOINTS SHALL BE FORMED AND INCLUDE DOWE
- BOND BREAKER TO BE TWO (2) UNIFORM COATS OF BITUMEN EMULSION ALL OVER THE EXPOSED SURFACE AND ON END.
- DOWELS AND TIE BARS TO MEET STRENGTH REQUIREMENTS OF STRUCTURAL GRADE STEEL IN ACCORDANCE AS. 1302. DOWELS AND TIE BARS SHALL BE ;-

 - CLEAN AND FREE FROM MILL SCALE, RUST AND OIL.
 - SAWN TO LENGTH NOT CROPPED. DIMENSIONS OF SEALANT RESERVOIR DEPENDANT ON THE SEALANT TYPE ADOPTED.
- DIMENSIONS OF SEALANT RESERVOIR DEPENDANT ON THE SEALANT 11PE ADDPTED.

 ENGINEERS APPROVAL TO BE OBTAINED FOR SEALANT AND RESERVOIR DIMENSIONS
 AND DETAIL PROPOSED BY THE CONTRACTOR. REFER DETAIL "B" FOR TYPICAL
 ARRANGEMENT AND SEALANT.
- PRIOR TO THE PLACEMENT OF CONCRETE IN THE ADJACENT SLAB, SELF EXPANDING CORK FILLER SHALL BE ADHERED TO THE ALREADY CAST AND CLEANED CONCRETE FACE USING AN APPROVED WATERPROOF ADHESIVE. ADHESIVE SHALL BE LIBERALLY APPLIED TO THE FULL FACE OF THE CONCRETE SLAB TO BE COVERED BY THE FILLER, AND ON THE
- REFER TO COMPACTION NOTES FOR PREPARATION OF SUB-BASE AND SUB-GRADE
- ALL WORK TO BE BROOM FINISH.

ASPHALTIC CONCRETE NOTES

- GENERAL

 a) MINERAL AGGREGATES TO COMPLY WITH AUSTRALIAN STANDARDS

 b) MINERAL FILLER TO COMPLY WITH AS 2357 MINERAL FILLERS OR ASPHALT.

 c) BITUMEN BINDER SHALL COMPLY WITH AS 2008

- JOB MIX 10mm NOMINAL SIZE AGGREGATE. MINIMUM BITUMEN CONTENT BY MASS MIX STABILITY - BETWEEN 16kN AND 36kN AS DETERMINED BY AS 289
- AIR VOIDS IN COMPACTED MIX BETWEEN 4% AND 7% OF THE VOI LIME OF THE MIX.
- VOIDS FILLED IN BINDER 65-80% OF AIR VOIDS IN THE TOTAL MINERAL AGGREGATE FILLED BY BINDER IN ACCORDANCE WITH AUSTRALIAN STANDARDS

- PAVEMENT PREPARATION

 a) THE EXISTING SURFACE TO BE SEALED SHALL BE DRY AND BROOMED BEFORE
- COMMENCEMENT OF WORK TO ENSURE COMPLETE REMOVAL OF ALL SUPERFICIAL FOREIGN MATTER.
 ALL DEPRESSIONS OR UNEVEN AREAS ARE TO BE TACK-COATED AND BROUGHT UP TO GENERAL LEVEL OF PAVEMENT WITH ASPHALTIC CONCRETE BEFORE LAYING

TACK COAT

a) THE WHOLE OF THE AREA TO BE SHEETED WITH ASPHALTIC CONCRETE SHALL BE LIGHTLY AND EVENLY COATED WITH RAPID SETTING BITUMEN COMPLYING WITH AUSTRALIAN STANDARDS. APPLICATION RATE FOR RESIDUAL BITUMEN SHALL BE COMPANIONED WITH A SHAPILCATION SHALL BE BY MEANS OF A MECHANICAL SPRAYER WITH SPRAY BAR

- SPREADING

 a) ALL ASPHALTIC CONCRETE SHALL BE SPREAD WITH A SELF PROPELLED PAVING
- THE ASPHALTIC CONCRETE SHALL BE LAID AT A MIX TEMPERATURE AS SHOWN

ROAD SURFACE TEMPERATURE IN SHADE (°C)	MIX TEMPERATURES (°C)
5 - 10	NOT PERMITTED
10 - 15	150
15 - 25	145

- c) ASPHALTIC CONCRETE SHALL NOT BE LAID WHEN THE ROAD SURFACE IS WET OR
- THE MINIMUM COMPACTED THICKNESS IS 30mm OVER EXISTING SEALED.

- THE NUMBER OF JOINTS BOTH LONGITUDINAL AND TRANSVERSE SHALL BE KEPT
- THE ROUBLES OF STATES OF THE DENSITY AND SURFACE FINISH AT JOINTS SHALL BE SIMILAR TO THOSE OF THE REMAINDER OF THE LAYER.

- COMPACTION

 a) ALL COMPACTION SHALL BE UNDERTAKEN USING SELF PROPELLED ROLLERS.
 b) INITIAL ROLLING SHALL BE COMPLETE BEFORE THE MIX TEMPERATURE FALLS
- BELOW 105°C SECONDARY ROLLING SHALL BE COMPLETED BEFORE THE MIX TEMPERATURE
- FALLS BELOW 60°C MINIMUM CHARACTERISTICS VALUE OF RELATIVE COMPACTION OF A LOT WHEN TESTED IN ACCORDANCE WITH AS2150

FINISHED PAVEMENT PROPERTIES

a) FINISHED SURFACES SHALL BE SMOOTH, DENSE AND TRUE TO SHAPE AND SHALL BE SMOOTH, DENSE AND TRUE TO SHAPE AND SHALL BE SMOOTH, DENSE AND TRUE AT ANY DOINT AND THE SMOOTH AND TRUE AT ANY DOINT AND THE SMOOTH AND THE NOT VARY MORE THAN 10mm FROM THE SPECIFIED PLAN LEVEL AT ANY POINT AND SHALL NOT DEVIATE FROM THE BOTTOM OF A 3m STRAIGHT FDGE LAID IN ANY



IMPORTANT: THE CONTRACTOR IS TO MAINTAIN A CURRENT SET OF "DIAL BEFORE YOU DIG" DRAWINGS ON SITE AT ALL TIMES

COVER SHEET & NOTES SHEET C1 00 DRAINAGE CATCHMENT PLAN SHEET C1.0° KEY PLAN - SITE STORMWATER MANAGEMENT SHEET C2.00 STORMWATER MANAGEMENT PLAN SHEET 1 OF 2 SHEET C2.01 STORMWATER MANAGEMENT PLAN SHEET 2 OF 2 SHEET C2.02 SHEET C3.00 KEY PLAN - ROADWORKS & DRAINAGE GENERAL ARRANGEMENT ROADWORKS & DRAINAGE PLAN SHEET 1 OF 5 SHEET C3.01 ROADWORKS & DRAINAGE PLAN SHEET 2 OF 5 SHEET C3.02 ROADWORKS & DRAINAGE PLAN SHEET 3 OF 5 SHEET C3.03 SHEET C3.04 ROADWORKS & DRAINAGE PLAN SHEET 4 OF 5 ROADWORKS & DRAINAGE PLAN SHEET 5 OF 5 SHEET C3.05 ROAD LONGITUDINAL SECTION (MC01) - LORIKEET GROVE SHEET C3.06 ROAD CROSS SECTIONS (MC01) LORIKEET GROVE SHEET 1 OF 3 SHEET C3.07 ROAD CROSS SECTIONS (MC01) LORIKEET GROVE SHEET 2 OF 3 SHEET C3.08 ROAD CROSS SECTIONS (MC01) LORIKEET GROVE SHEET 3 OF 3 SHEET C3.09 ROAD LONGITUDINAL SECTION (MC02) - WARRIEWOOD ROAD SHEET C3 10 ROAD CROSS SECTIONS (MC02) WARRIEWOOD ROAD SHEET 1 OF 2 SHEET C3 1 ROAD CROSS SECTIONS (MC02) WARRIEWOOD ROAD SHEET 2 OF 2 SHEET C3.12 DRAINAGE EASEMENT LONGITUDINAL SECTION (DR01) SHEET C3.13 DRAINAGE PIT & PIPE LONGITUDINAL SECTION SHEET C3.14 SHEET C3.15 TYPICAL DETAIL SHEET 2 OF 2 SHEET C3.16 TYPICAL ROAD SECTIONS SHEET C3.17 BOARDWALK LOCATION PLAN SHEET C4 01 BOARDWALK GENERIC DETAILS SHEET 1 OF 2 SHEET C4 02 BOARDWALK GENERIC DETAILS SHEET 2 OF 2 SHEET C4.03

SHEET INDEX

DIAL BEFORE YOU DIG

DRAWINGS MUST BE PRINTED IN COLOUR

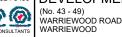
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ISSUED FOR APPROVAL 06.03.24 NB BK ISSUED FOR APPROVAL 28.02.24 SJ BK 15.12.23 RH BK

WARRIEWOOD **DEVELOPERS**

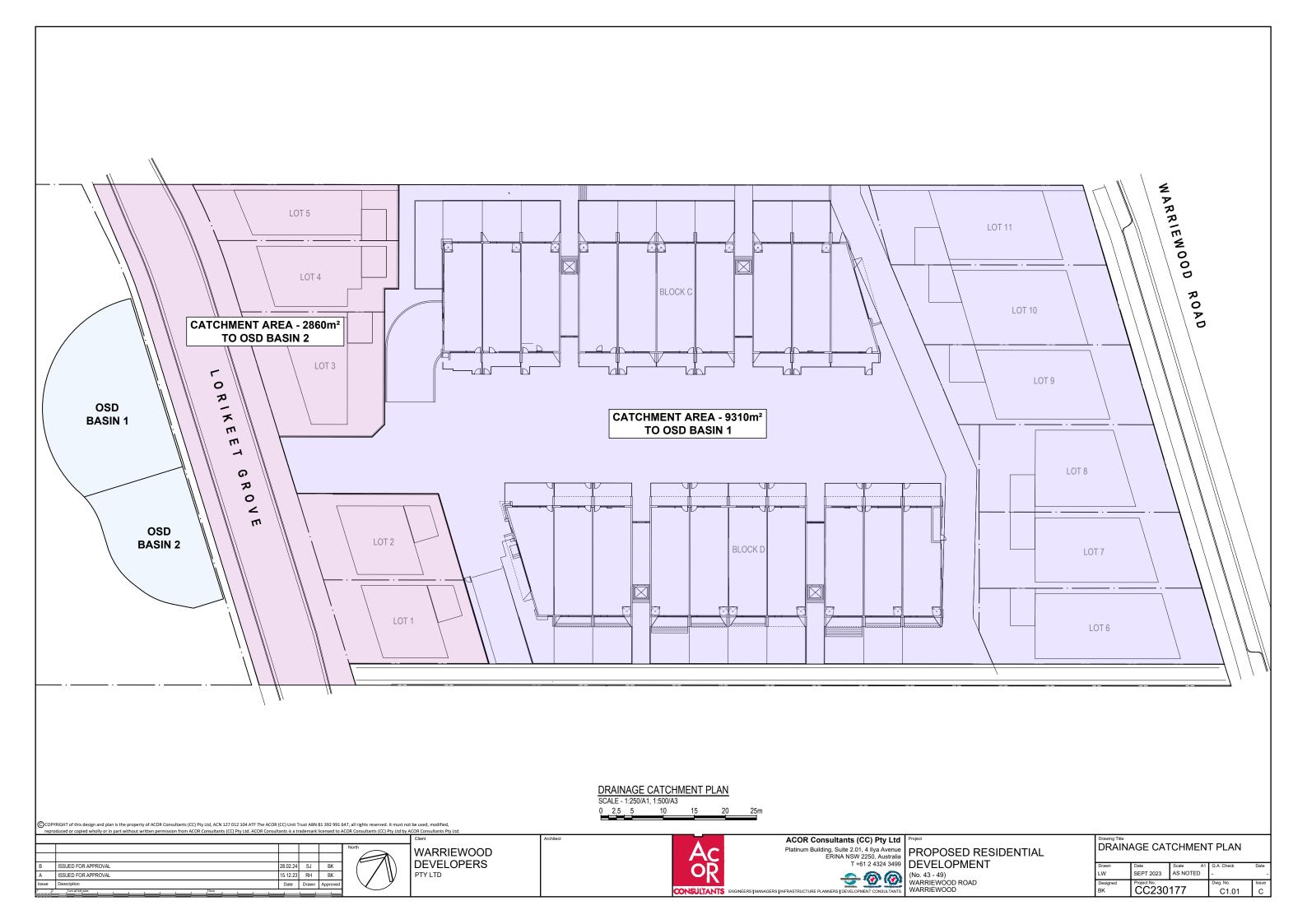
ACOR Consultants (CC) Ptv Ltd Platinum Building, Suite 2.01, 4 Ilya Avenue ERINA NSW 2250, Australia

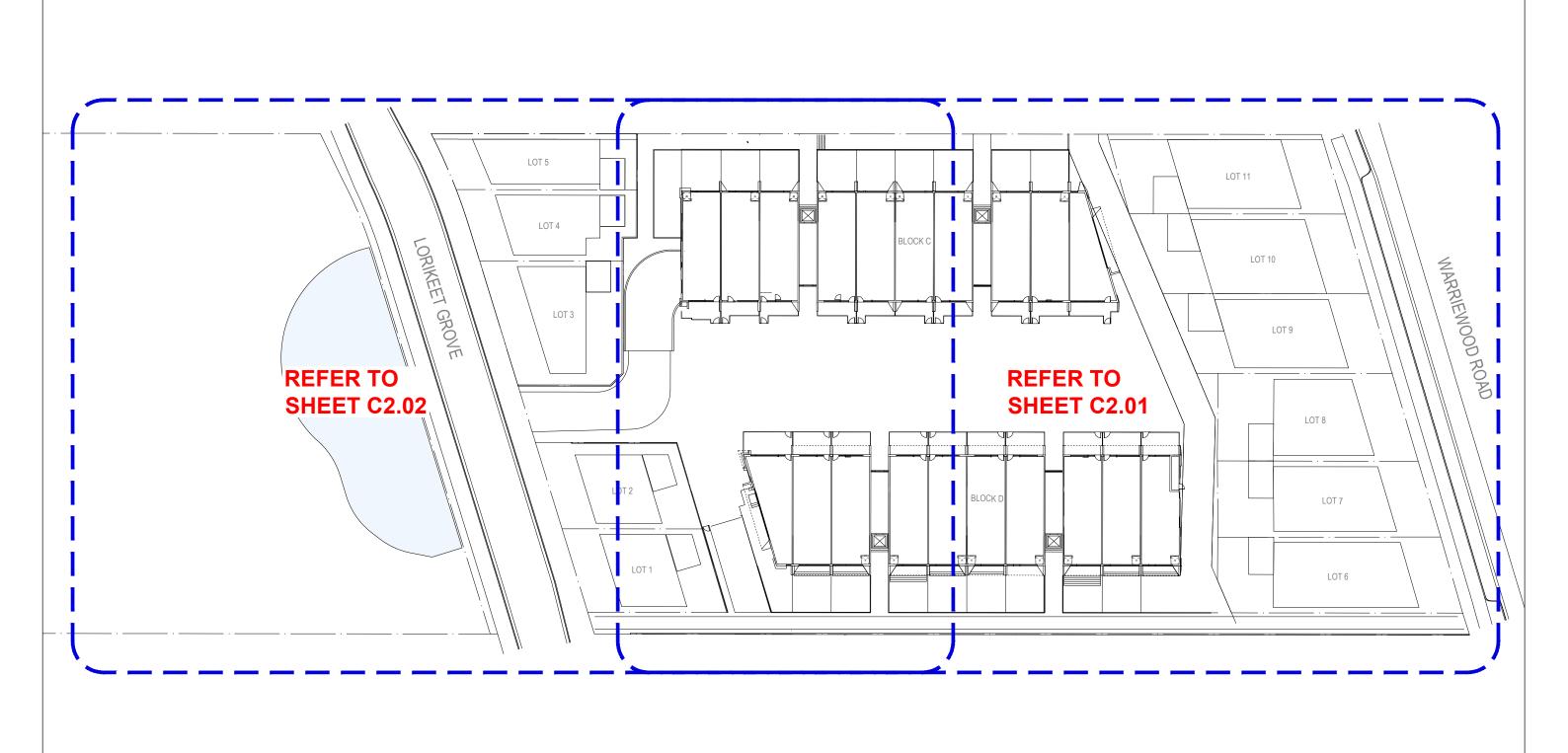
PROPOSED RESIDENTIAL DEVELOPMENT T +61 2 4324 3499



AS NOTED SEPT 23 CC230177

COVER SHEET & NOTES





KEY PLAN SCALE - 1:300/A1, 1:600/A3 0 2 4 8 12 16 20m

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С	ISSUED FOR APPROVAL	06.03.24	NB	BK
В	ISSUED FOR APPROVAL	28.02.24	SJ	BK
Α	ISSUED FOR APPROVAL	15.12.23	RH	BK
Issue	Description	Date	Drawn	Approved

PTY LTD

WARRIEWOOD DEVELOPERS



ACOR Consultants (CC) Pty Ltd Project

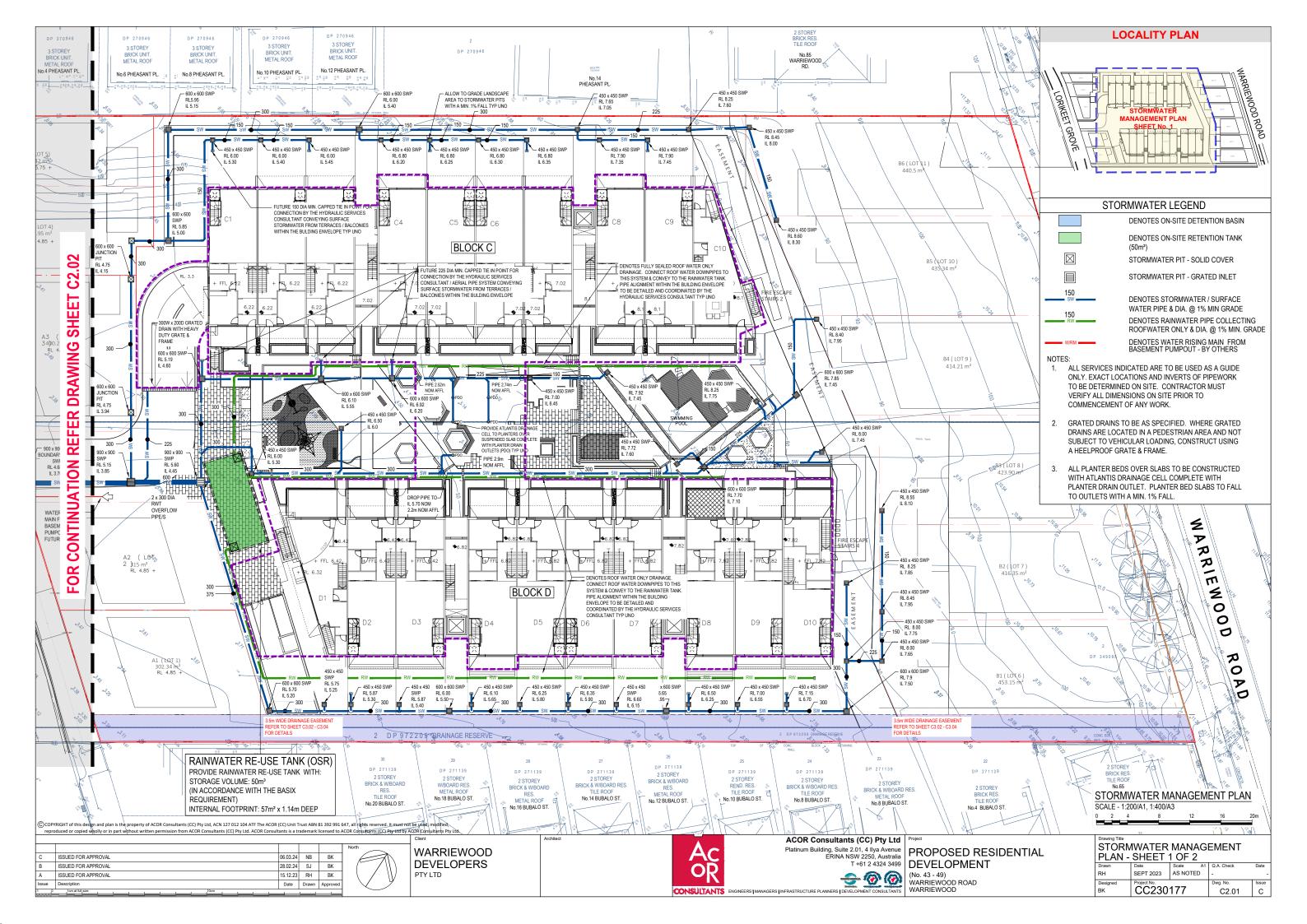


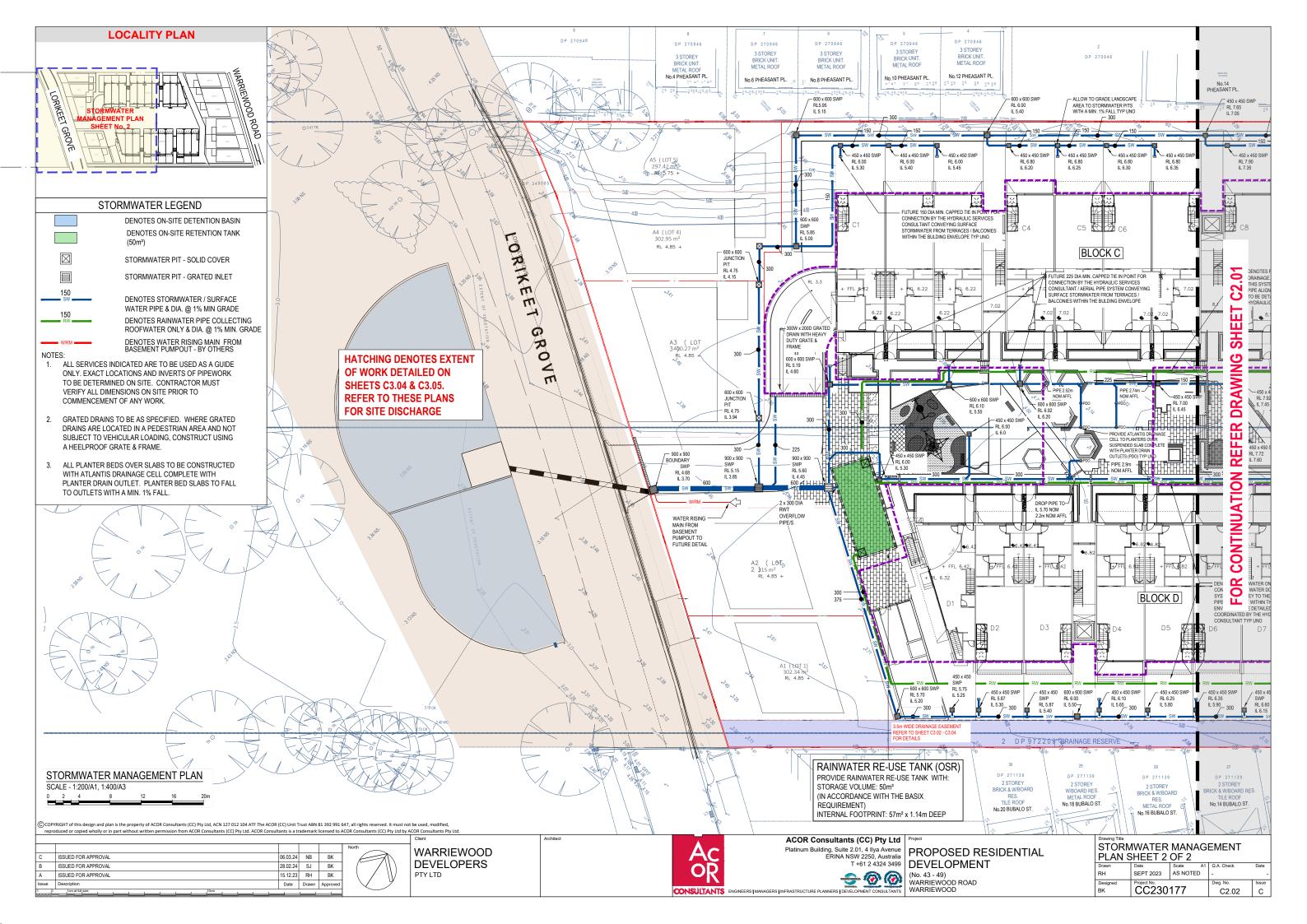
Platinum Building, Suite 2.01, 4 Ilya Avenue
ERINA NSW 2250, Australia
T +61 2 4324 3499
PROPOSED RESIDENTIAL
DEVELOPMENT

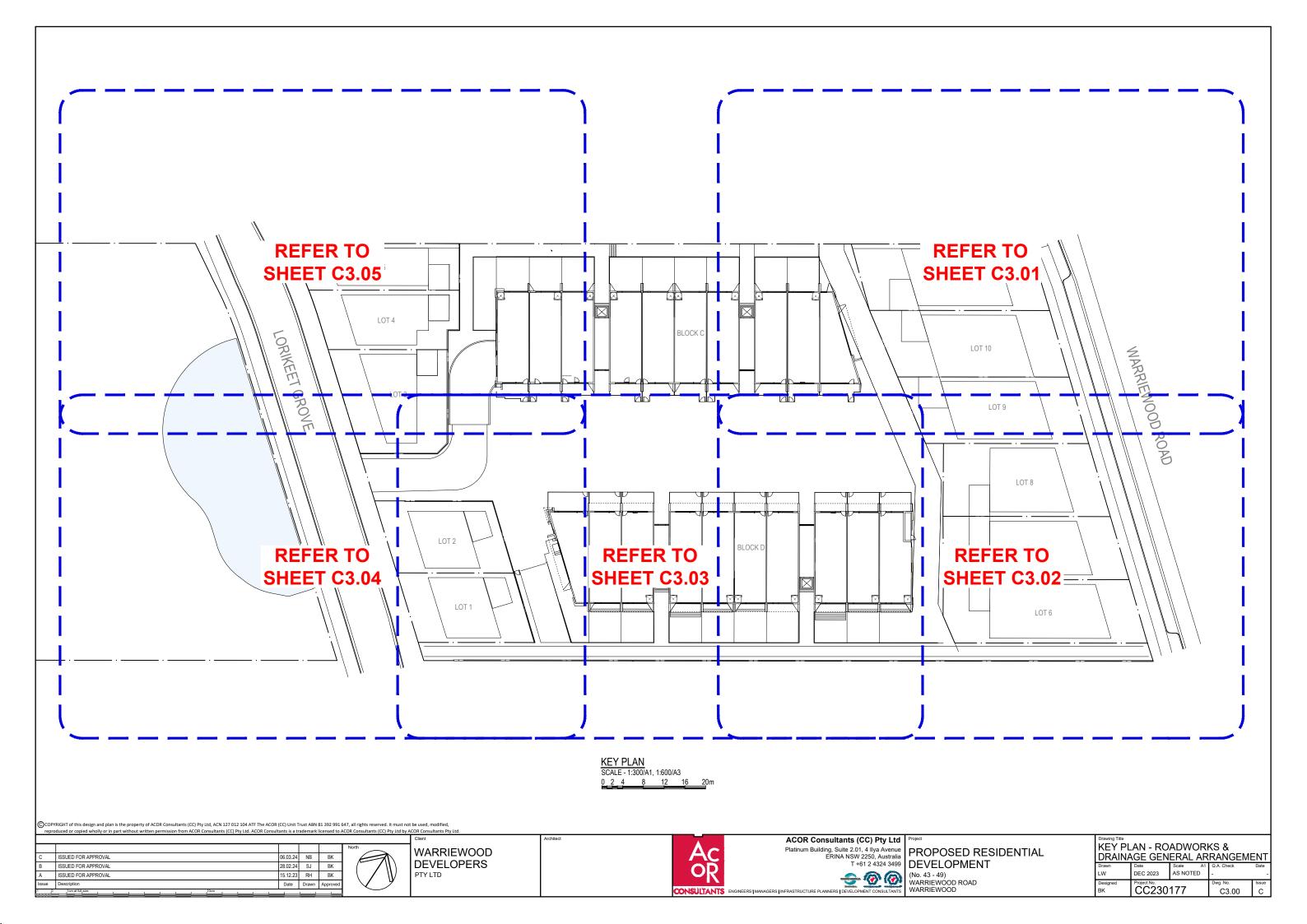
KEY PLAN - SITE
STORMWATER MANAGEMENT

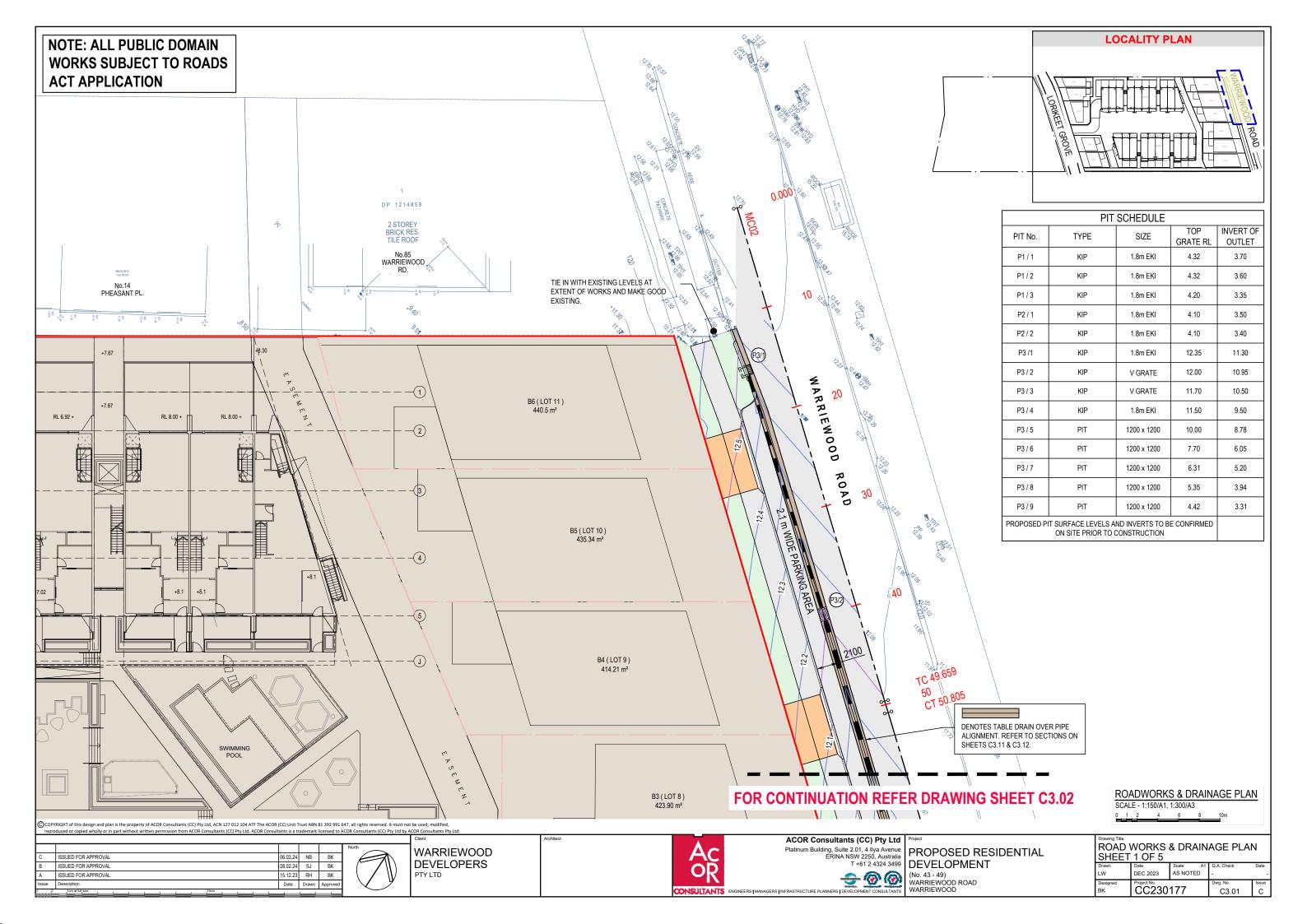
The part of the Date Scale A1 SEPT 2023 AS NOTED Project No. CC230177

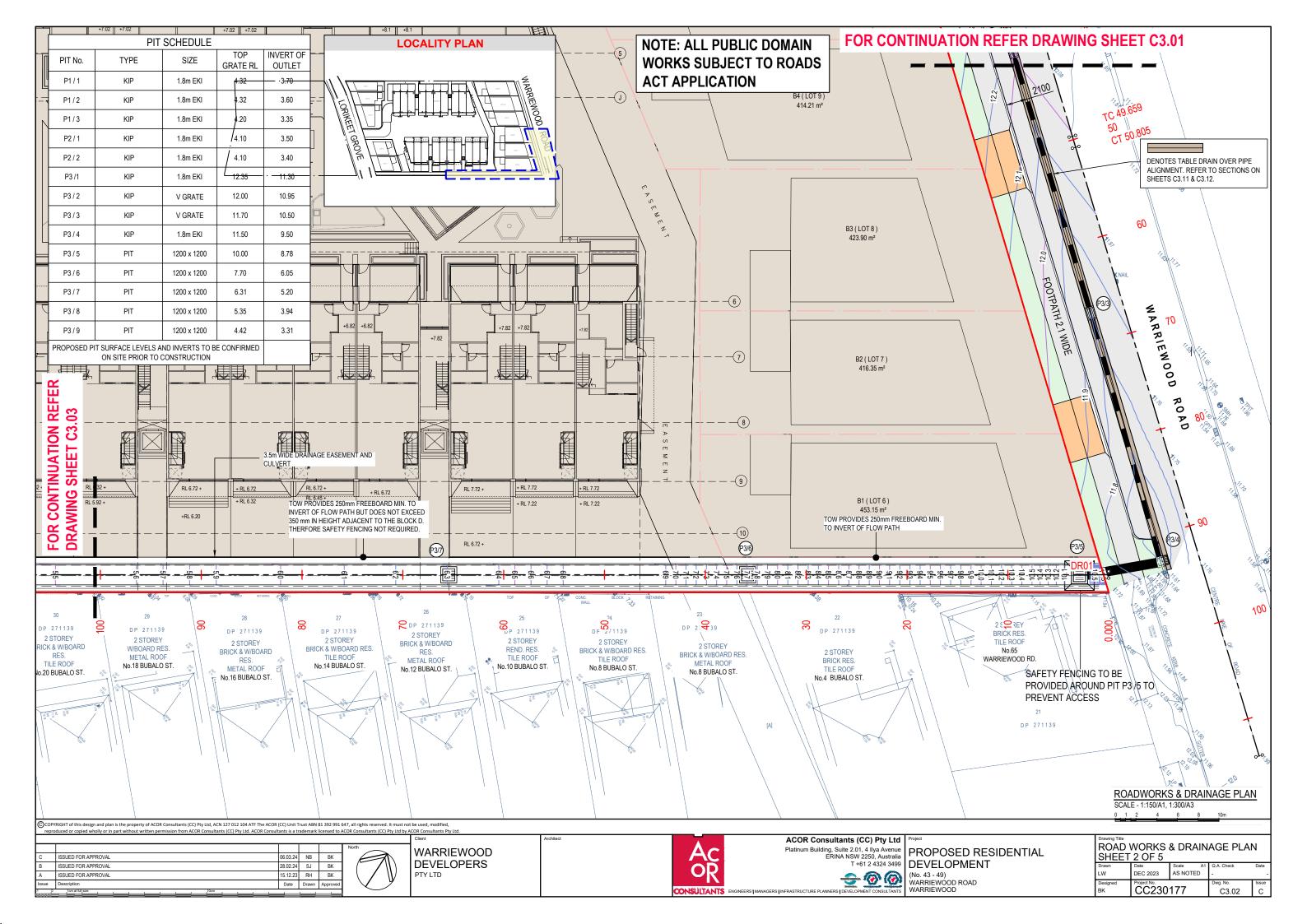
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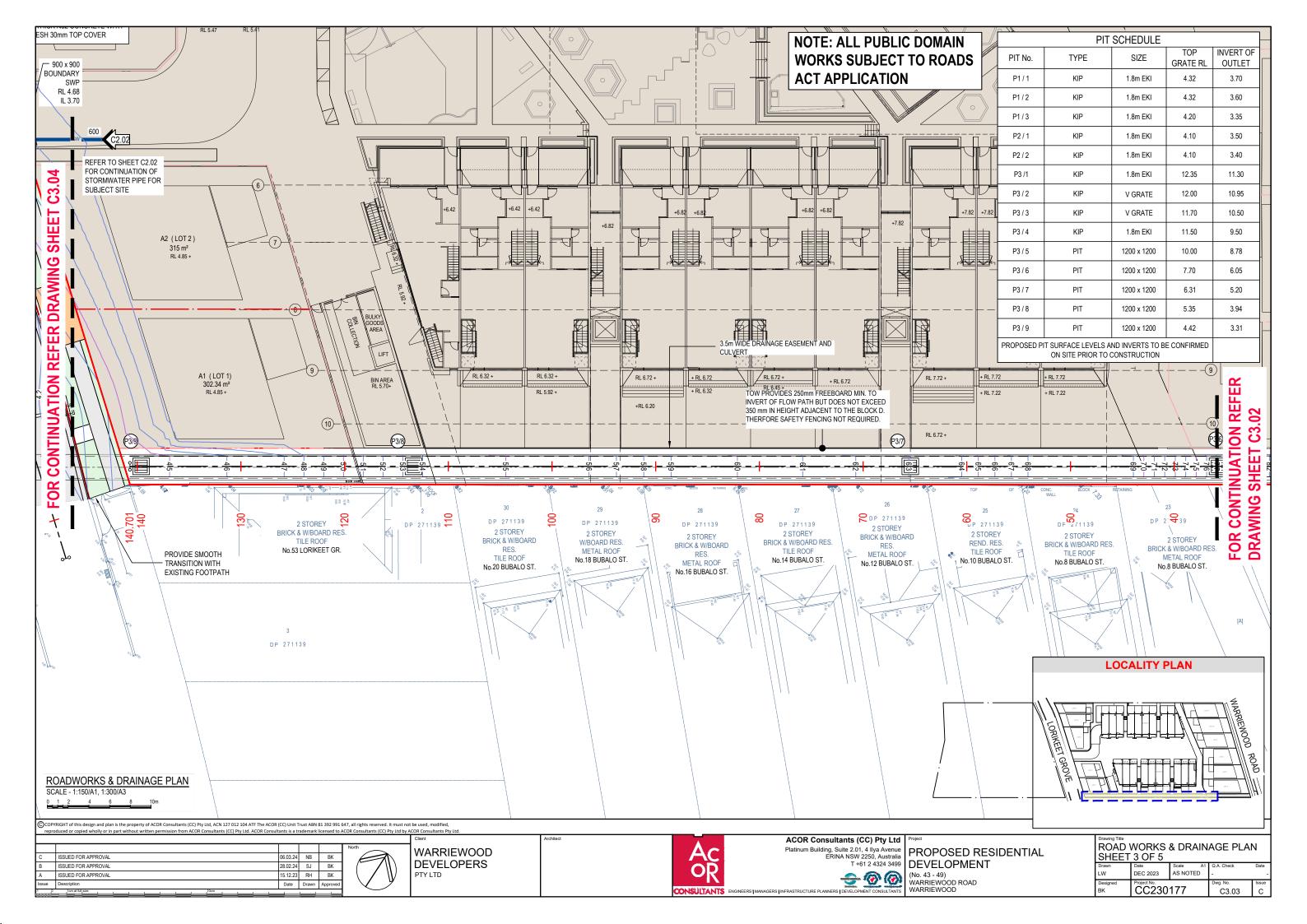


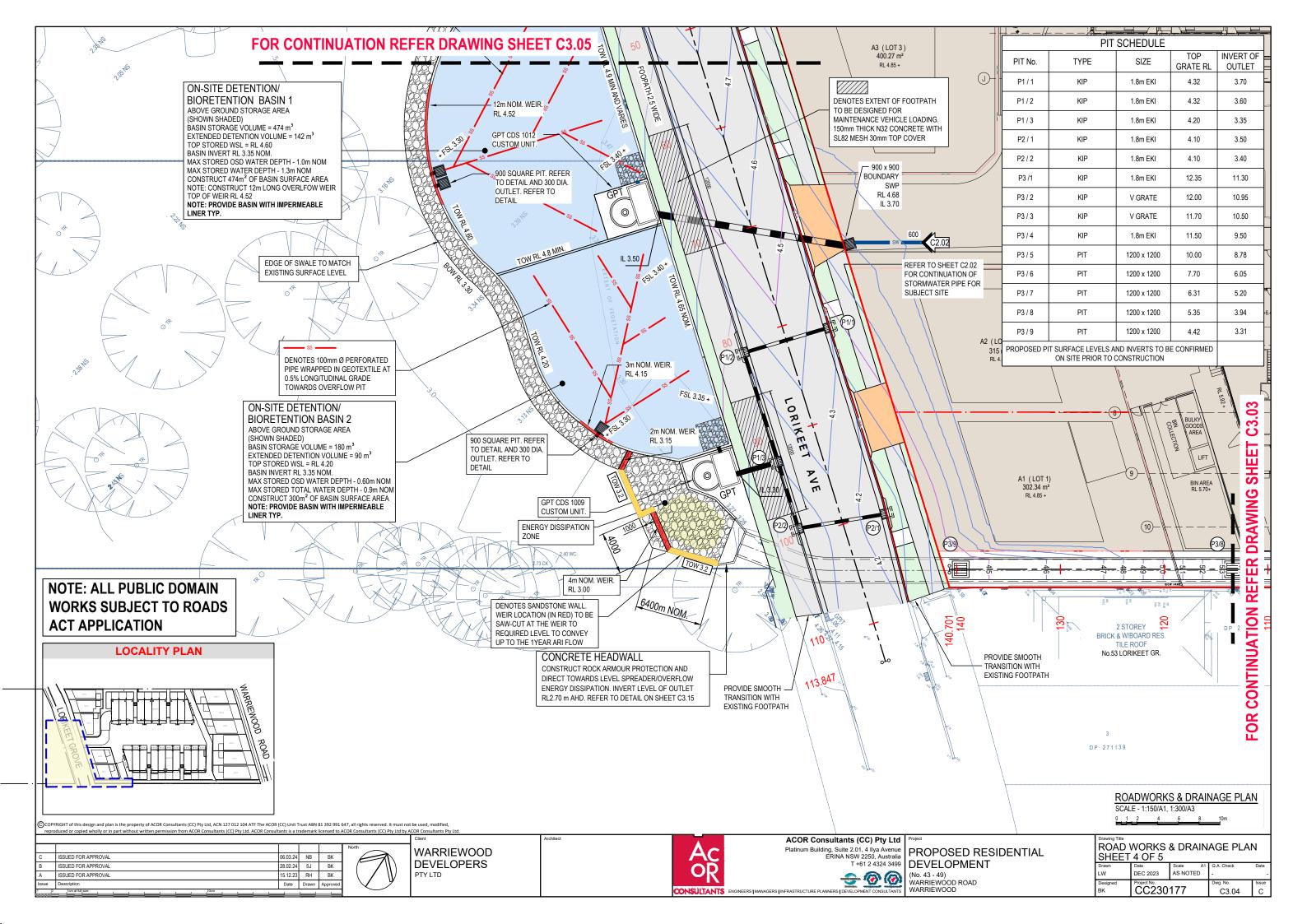


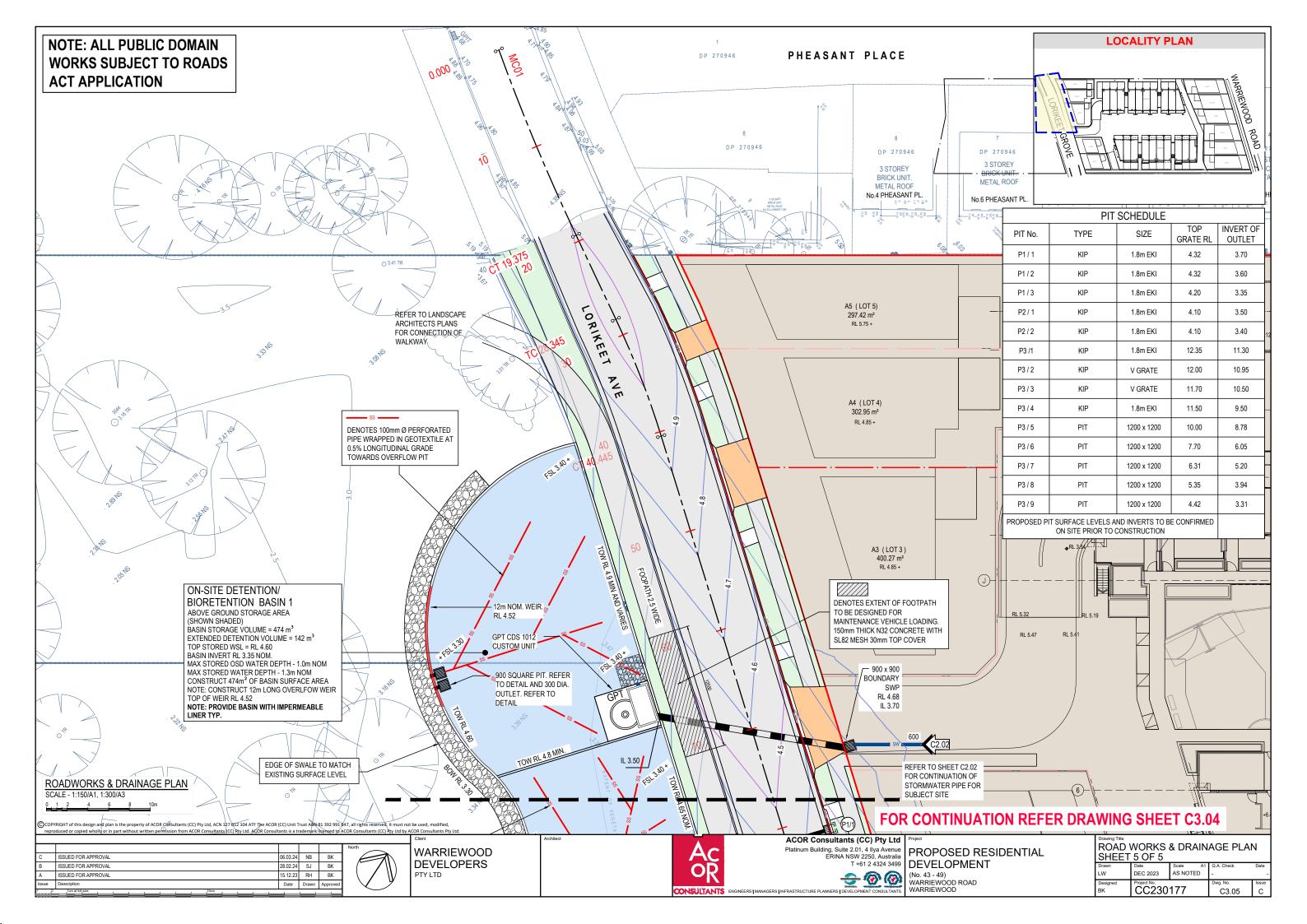


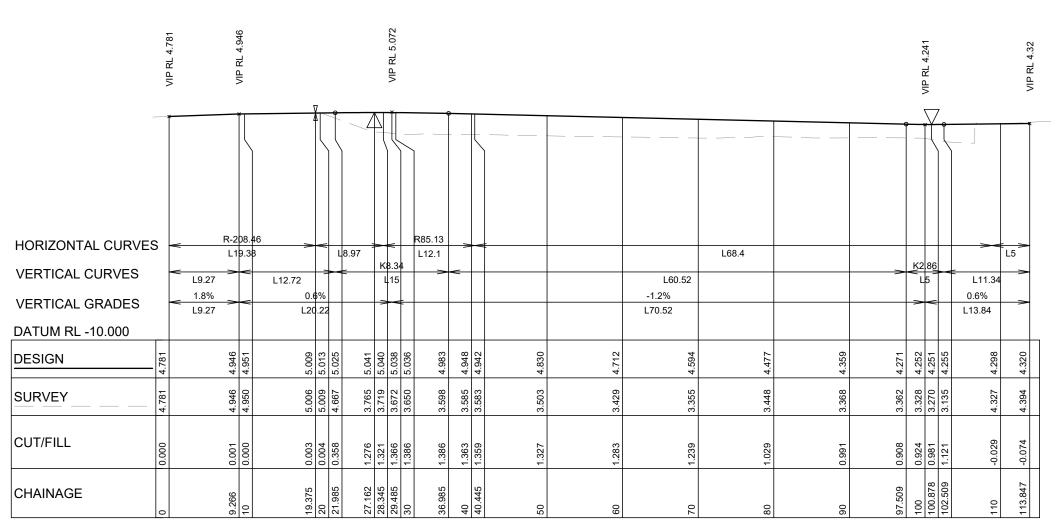












MC01 - LONGITUDINAL SECTION HORZ SCALE 1:500 VERT SCALE 1:250

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					North
С	ISSUED FOR APPROVAL	06.03.24	NB	BK	
В	ISSUED FOR APPROVAL	28.02.24	SJ	BK	
Α	ISSUED FOR APPROVAL	15.12.23	RH	BK	
Issue	Description	Date	Drawn	Approved	
1 ,0	1cm at full size 10cm				

WARRIEWOOD DEVELOPERS



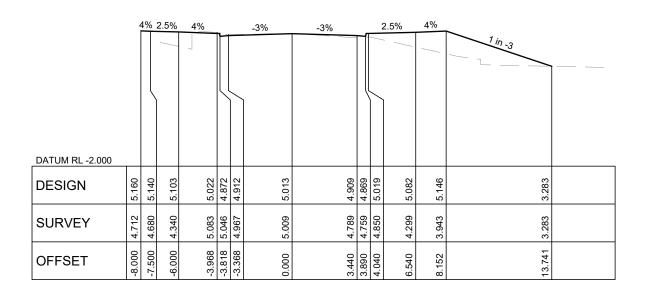
ACOR Consultants (CC) Pty Ltd

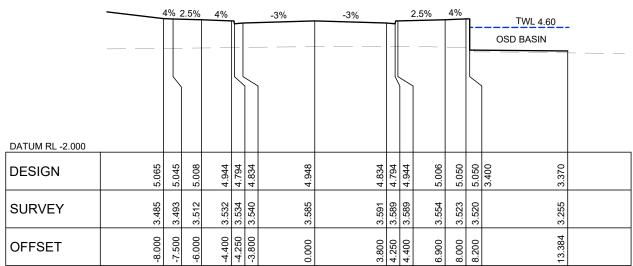
Platinum Building, Suite 2.01, 4 Ilya Avenue ERINA NSW 2250, Australia T +61 2 4324 3499 DEVELOPMENT PROPOSED RESIDENTIAL ROAD LONGITUDINAL SECTION (MC01) - LORIKEET GROVE Date Scale A DEC 2023 AS NOTED

CC230177

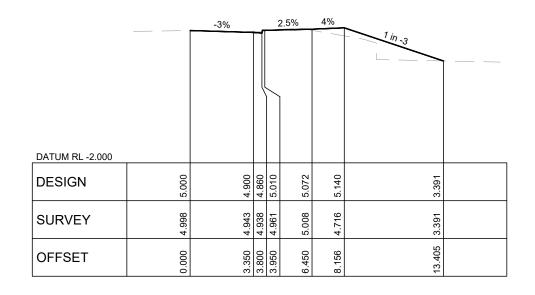
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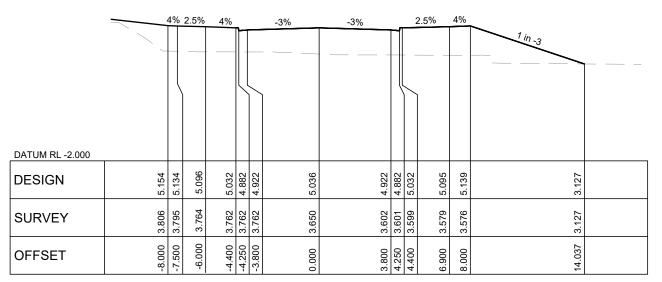
(No. 43 - 49) WARRIEWOOD ROAD WARRIEWOOD





CH 20 CH 40





CH 18 CH 30

> HORZ SCALE 1:200 VERT SCALE 1:400

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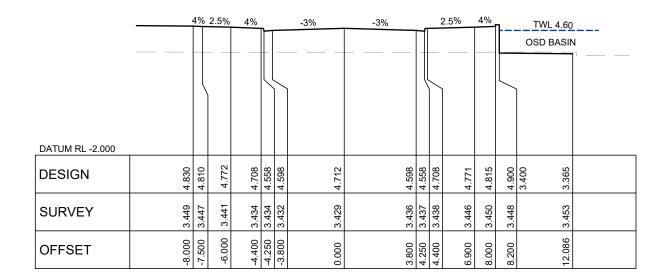
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В	ISSUED FOR APPROVAL	28.02.24	SJ	BK	
A	ISSUED FOR APPROVAL	15.12.23	RH	BK	
ssue	Description	Date	Drawn	Approved	
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WARRIEWOOD DEVELOPERS

ACOR Consultants (CC) Pty Ltd

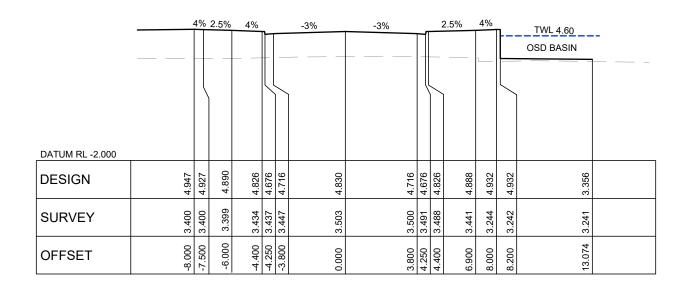
Platinum Building, Suite 2.01, 4 Ilya Avenue
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T +61 2 4324 3499
PROPOSED RES
DEVELOPMENT PROPOSED RESIDENTIAL (No. 43 - 49) WARRIEWOOD ROAD WARRIEWOOD

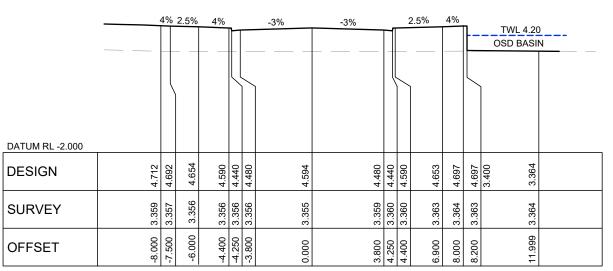
ROAD CROSS SECTIONS (MC01) LORIKEET GROVE SHEET 1 OF 3 DEC 2023 AS NOTED CC230177 C3.07



DATUM RL -2.000		4%	2.5%	4%			-3%	-3%			2	2.5%	4%		OSD BAS	
DESIGN	4 594	4.574	4.537	4.473	4.323	4.363	4.477		4.363	4.323	4.473	4.535	4.579	4.650	3.370	
SURVEY	3 445		3.434	3.424	3.423	3.421	3.448		3.366	3.365	3.365	3.351	3.344	3.344		
OFFSET	000	-7.500	-6.000	-4.400	-4.250	-3.800	0.000		3.800	4.250	4.400	6.900	8.000	8.200	11.768	

CH 60 CH 80





CH 50 CH 70

HORZ SCALE 1:200 VERT SCALE 1:400

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						Client
					North	WARRIEWOOD
	ISSUED FOR APPROVAL	06.03.24	NB	BK		
	ISSUED FOR APPROVAL	28.02.24	SJ	BK		DEVELOPERS
	ISSUED FOR APPROVAL	15.12.23	RH	BK		PTY LTD
e	Description	Date	Drawn	Approved		
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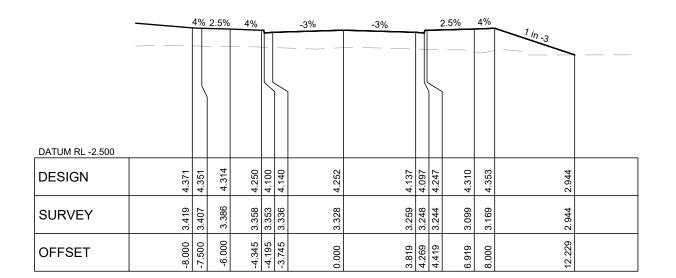


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ERINA NSW 2250, Australia
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DEVELOPMENT

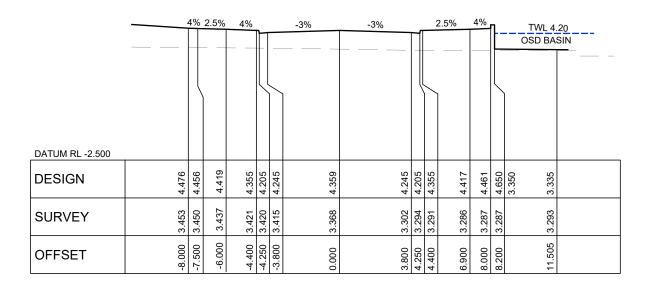
4 Ilya Avenue 250, Australia 2 4324 3499 PROPOSED RESIDENTIAL DEVELOPMENT (No. 43 - 49) WARRIEWOOD ROAD WARRIEWOOD CC230177

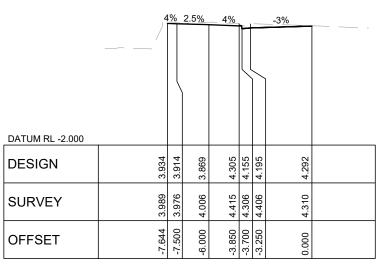
C3.08

ENGINEERS I MANAGERS I INERASTRIUCTURE PI ANNERS I DEVELOPMENT CONSULTANTS



CH 100





CH 90 CH 109

> HORZ SCALE 1:200 VERT SCALE 1:400

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	Client					
					North	WARRIEWOOI
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	ISSUED FOR APPROVAL	28.02.24	SJ	BK		DEVELOPERS
	ISSUED FOR APPROVAL	15.12.23	RH	BK		PTY LTD
ue	Description	Date	Drawn	Approved		

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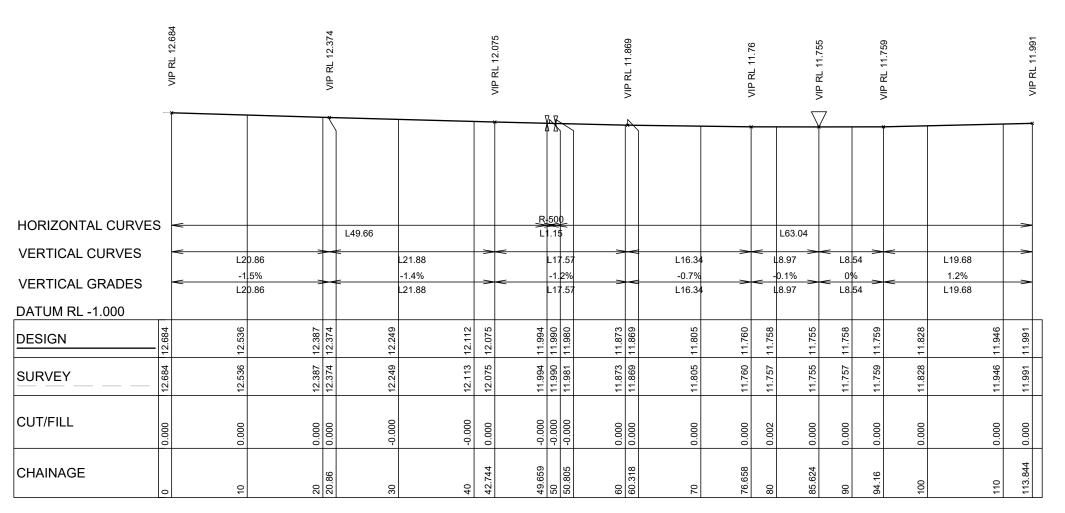


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(No. 43 - 49) WARRIEWOOD ROAD WARRIEWOOD

ROAD CROSS SECTIONS (MC01) LORIKEET GROVE SHEET 3 OF 3

DEC 2023 AS NOTED CC230177 C3.09



MC02 - LONGITUDINAL SECTION HORZ SCALE 1:500 VERT SCALE 1:250

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					North	WARF					
)	ISSUED FOR APPROVAL	06.03.24	NB	BK							
3	ISSUED FOR APPROVAL	28.02.24	SJ	BK		DEVE					
١.	ISSUED FOR APPROVAL	15.12.23	RH	BK		PTY LTD					
sue	Description	Date	Drawn	Approved							
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RIEWOOD ELOPERS



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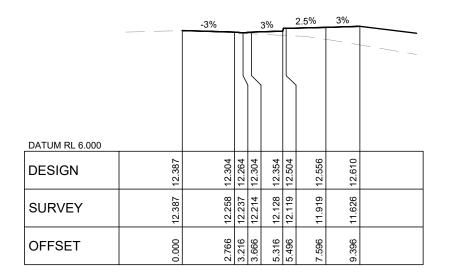


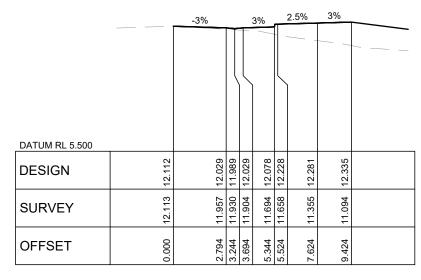
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CC230177

C3.10

(No. 43 - 49) WARRIEWOOD ROAD WARRIEWOOD

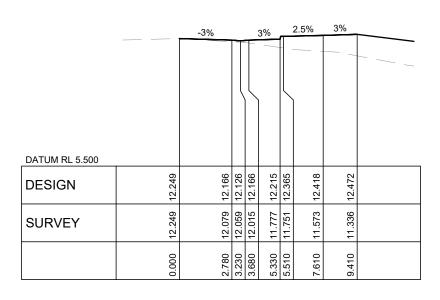




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DATUM RL 6.000									
DESIGN	11.873	11.787	11.747	11.787	11.837	11.987	12.039	12.093	
SURVEY	11.873	11.710	11.684	11.644	11.509	11.494	11.353	11.262	
OFFSET	0.000	2.844	3.294	3.744	5.394	5.574	7.674	9.474	

CH 20 CH 40 CH 60

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DATUM RL 6.000						
DESIGN	12.519	12.417	12.377	12.580	12.677	
SURVEY	12.519	12.408	12.375	12.522	12.040	
OFFSET	0.000	3.400	3.850	6.130	9.383	



		-3%	-	-	3%	<u> </u>	2.5%	3%	
DATUM RL 5.500									
DESIGN	11.990	11.906	11.866	11.906	11.955	12.105	12.158	12.212	
SURVEY	11.990	11.808	11.785	11.763	11.644	11.629	11.350	11.059	
OFFSET	0.000	2.808	3.258	3.708	5.358	5.538	7.638	9.438	

CH 30 CH 11.122 CH 50

HORZ SCALE 1:200 VERT SCALE 1:400

		_			North
С	ISSUED FOR APPROVAL	06.03.24	NB	BK	
В	ISSUED FOR APPROVAL	28.02.24	SJ	BK	
Α	ISSUED FOR APPROVAL	15.12.23	RH	BK	
Issue	Description	Date	Drawn	Approved	
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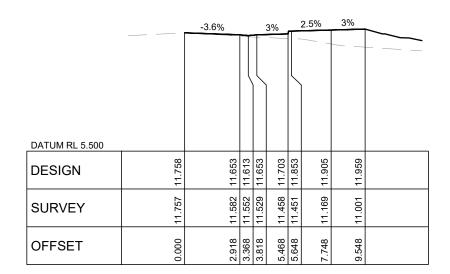
WARRIEWOOD DEVELOPERS

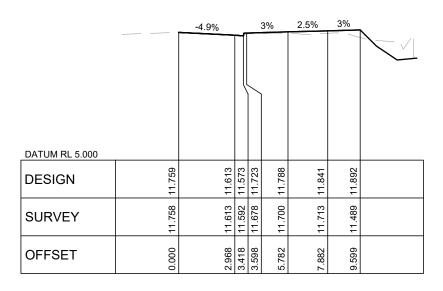
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DEVELOPMENT (No. 43 - 49) WARRIEWOOD ROAD WARRIEWOOD

ROAD CROSS SECTIONS (MC02) WARRIEWOOD ROAD SHEET 1 OF 2 Date Scale A DEC 2023 AS NOTED CC230177 C3.11





CH 80 CH 93.576

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DATUM RL 6.000									
DESIGN	11.805	11.718	11.678	11.718	11.768	11.918	11.970	12.024	
SURVEY	11.805	11.629	11.603	11.577	11.475	11.464	11.360	11.293	
OFFSET	0.000	2.881	3.331	3.781	5.431	5.611	7.711	9.511	

		-7.2%	3.	6%.			2.5%	3%	
DATUM RL 5.000									
DESIGN	11.758	11.546	11.506	11.548	11.698	11.727	11.779	11.833	
SURVEY	11.757	11.502	11.468	11.470	11.466	11.411	11.305	11.132	
OFFSET	0.000	2.955	3.405	4.553	4.733	5.704	7.804	9.585	_

CH 70 CH 90

HORZ SCALE 1:200 VERT SCALE 1:400

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					North	WARR
	ISSUED FOR APPROVAL	06.03.24	NB	BK		WARR
	ISSUED FOR APPROVAL	28.02.24	SJ	BK		DEAF
	ISSUED FOR APPROVAL	15.12.23	RH	BK		PTY LTD
ue	Description	Date	Drawn	Approved		
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RIEWOOD ELOPERS



ACOR Consultants (CC) Pty Ltd Project

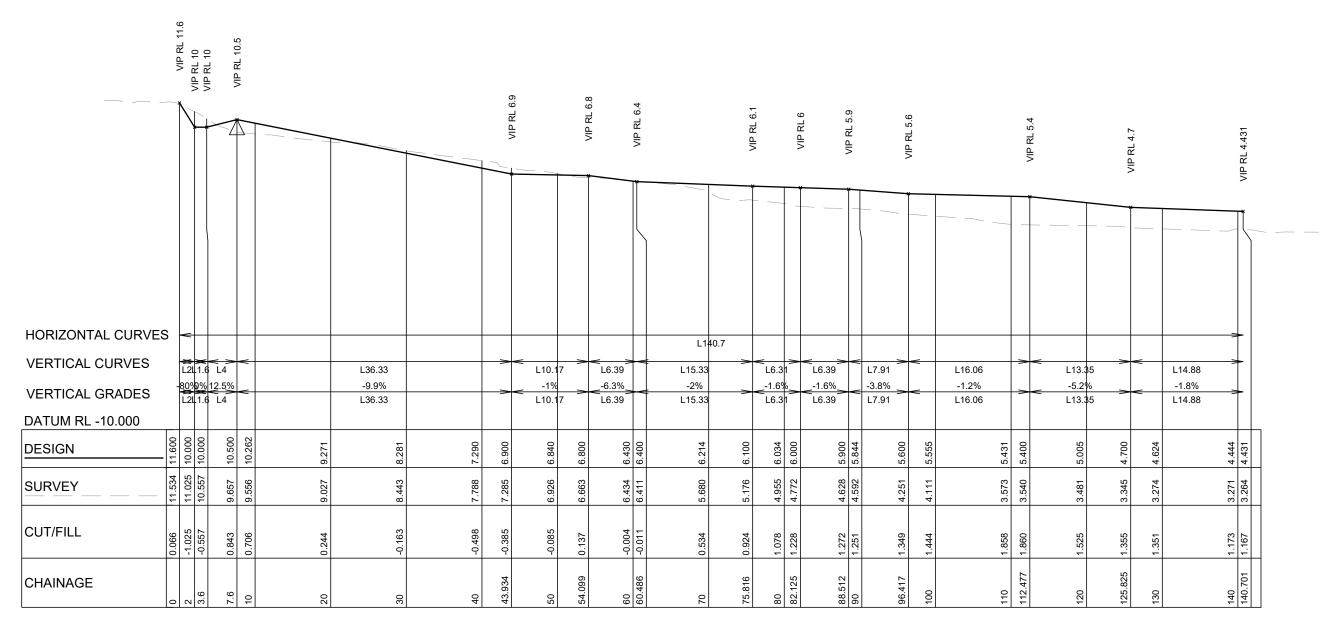
(No. 43 - 49) WARRIEWOOD ROAD WARRIEWOOD

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ROAD CROSS SECTIONS (MC02) WARRIEWOOD ROAD SHEET 2 OF 2 Date Scale A DEC 2023 AS NOTED Project No. CC230177

C3.12



DR01 - LONGITUDINAL SECTION HORZ SCALE 1:500 VERT SCALE 1:250

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		Client				
					North	WARRIEWOOD
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	ISSUED FOR APPROVAL	28.02.24	SJ	BK		DEVELOPERS
	ISSUED FOR APPROVAL	15.12.23	RH	BK		PTY LTD
e	Description	Date	Drawn	Approved		



ACOR Consultants (CC) Pty Ltd

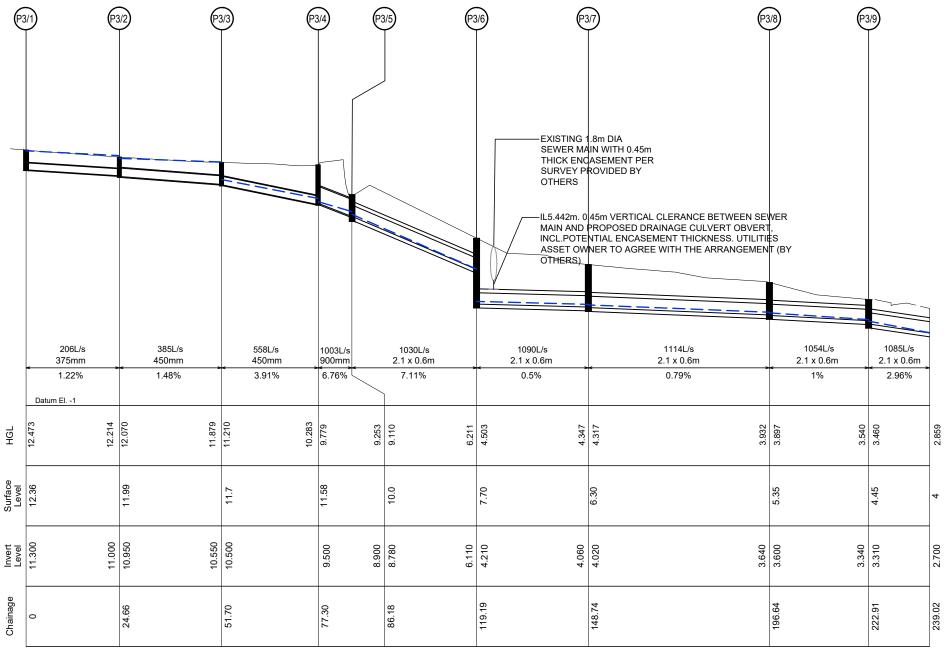
WARRIEWOOD

Platinum Building, Suite 2.01, 4 llya Avenue ERINA NSW 2250, Australia T +61 2 4324 3499 PROPOSED RES PROPOSED RESIDENTIAL (No. 43 - 49) WARRIEWOOD ROAD

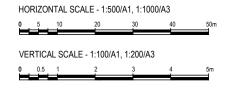
DRAINAGE EASEMENT LONGITUDINAL SECTION (DR01) Date Scale A DEC 2023 AS NOTED

CC230177

C3.13



LONG SECTION (100YR ARI + CC) **DRAINAGE LINE 3**



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						Client
					North	WARRIEWOOD
	ISSUED FOR APPROVAL	06.03.24	NB	BK		
	ISSUED FOR APPROVAL	28.02.24	SJ	BK		DEVELOPERS
	ISSUED FOR APPROVAL	15.12.23	RH	BK		PTY LTD
e	Description	Date	Drawn	Approved		
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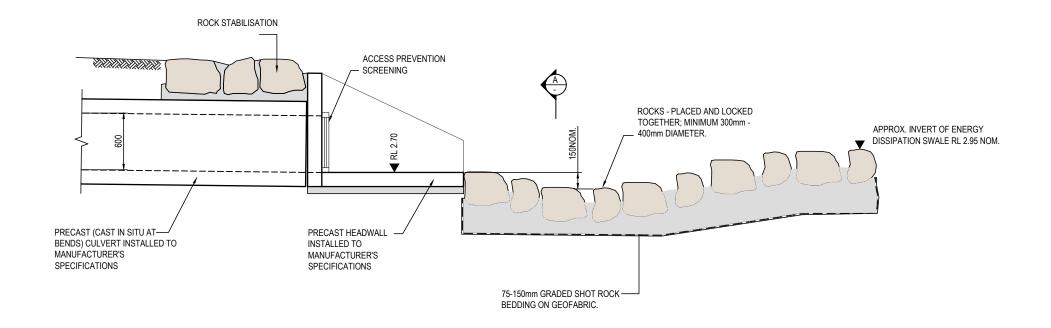
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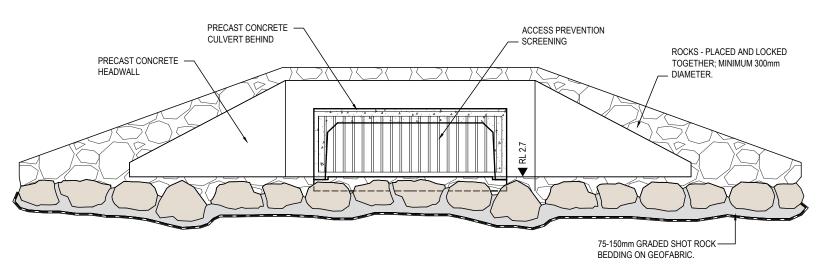
DRAINAGE PIT & PIPE LONGITUDINAL SECTION DEC 2023 AS NOTED (No. 43 - 49) WARRIEWOOD ROAD WARRIEWOOD

CC230177

C3.14



CULVERT HEADWALL SECTION SCALE - 1:20/A1, 1:40/A3



SECTION SCALE: 1:20/A1, 1:40/A3

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ssue	Description	Date	Drawn	Approved		
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WARRIEWOOD DEVELOPERS PTY LTD

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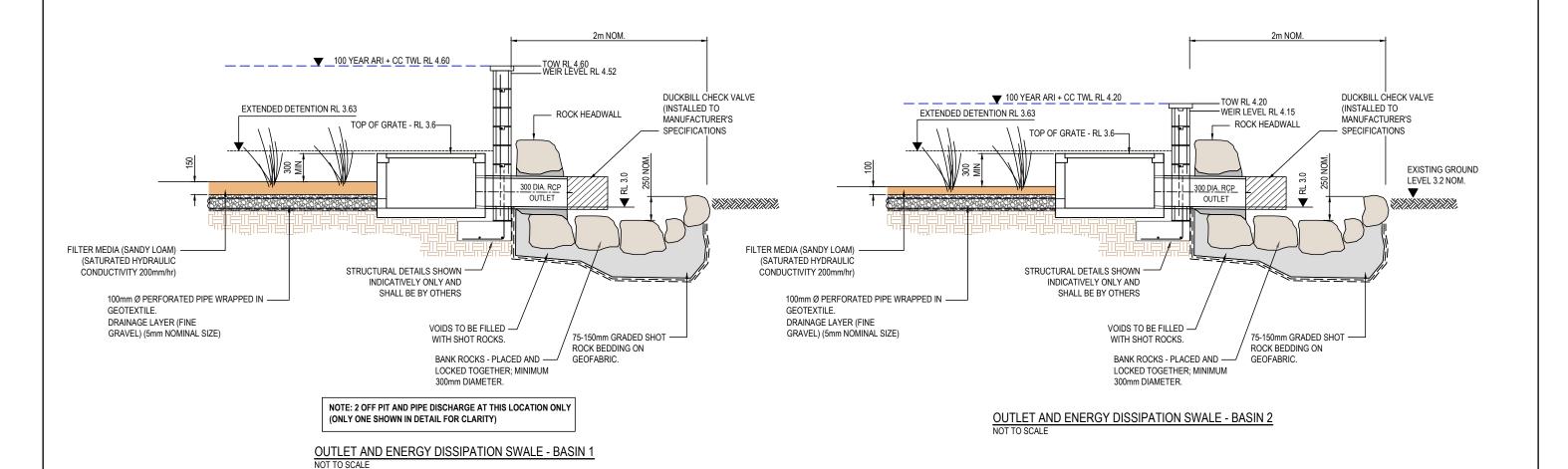
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DEVELOPMENT

PROPOSED RESIDENTIAL

(No. 43 - 49) WARRIEWOOD ROAD WARRIEWOOD

TYPICAL DETAIL SHEET 1 OF 2 AS NOTED DEC 2023

CC230177 C3.15



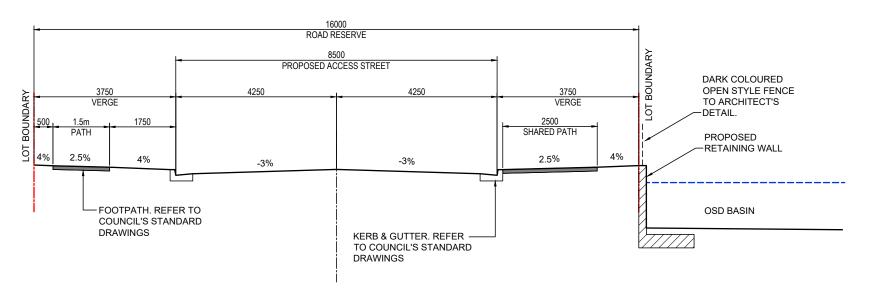
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						Client
					North	WARRIEWOOD
:	ISSUED FOR APPROVAL	06.03.24	NB	BK		
	ISSUED FOR APPROVAL	28.02.24	SJ	BK		DEVELOPERS
	ISSUED FOR APPROVAL	15.12.23	RH	BK		PTY LTD
sue	Description	Date	Drawn	Approved		
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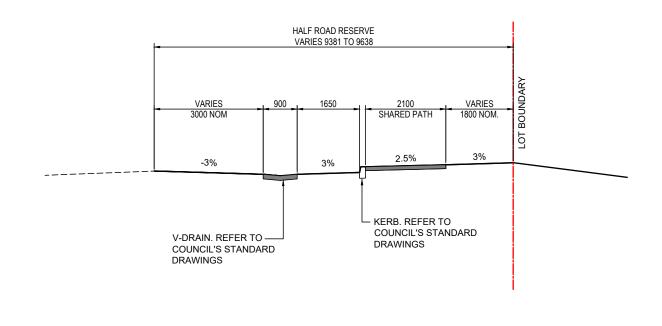


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2.2.1, 4 Ilya Avenue
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(No. 43 - 49)
WARRIEWOOD ROAD



TYPICAL ROAD SECTION - LORIKEET GROVE EXTENSION



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						Client
					North	WARRIEWOOD
	ISSUED FOR APPROVAL	06.03.24	NB	BK		
	ISSUED FOR APPROVAL	28.02.24	SJ	BK		DEVELOPERS
	ISSUED FOR APPROVAL	15.12.23	RH	BK		PTY LTD
ue	Description	Date	Drawn	Approved		



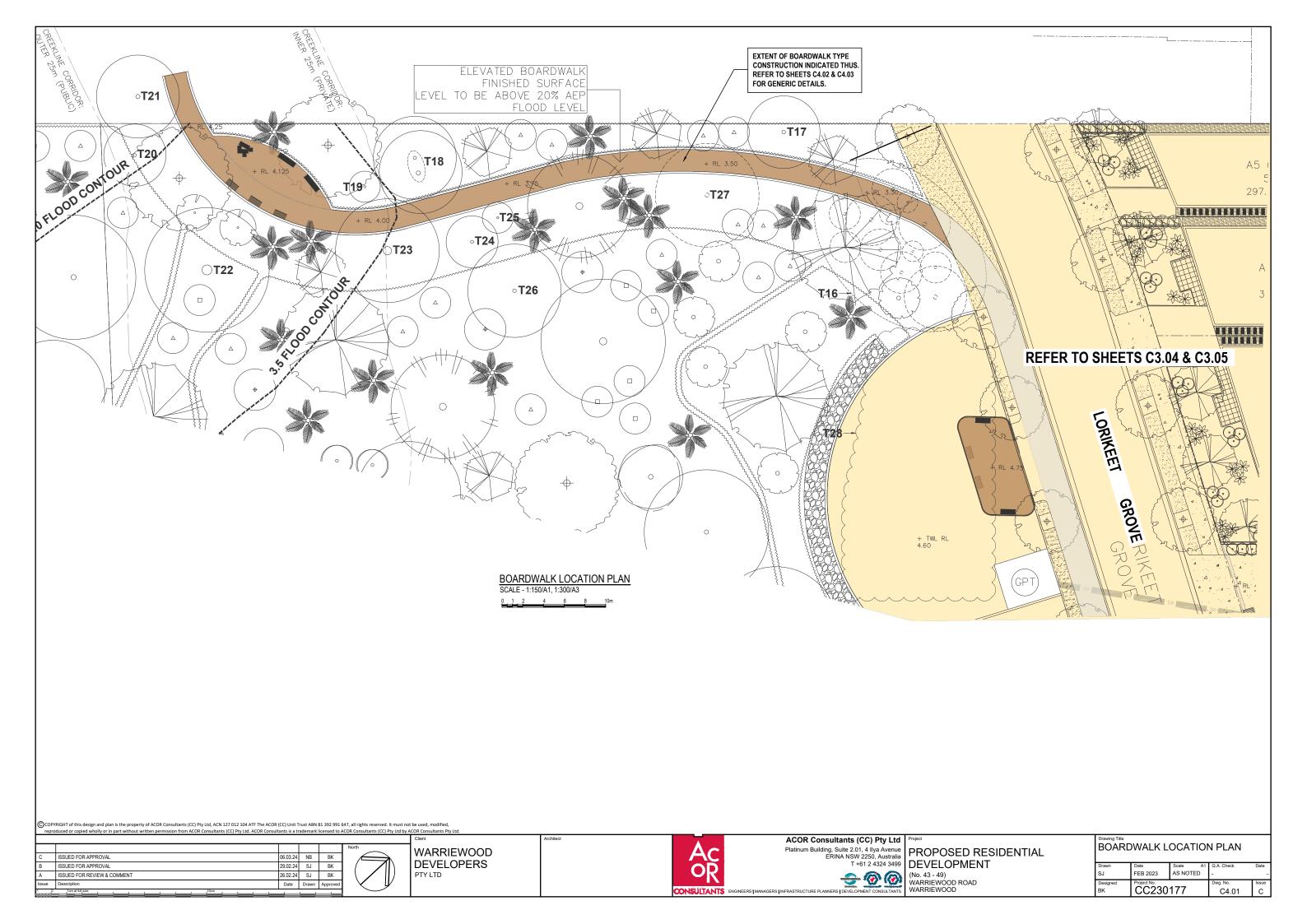
ACOR Consultants (CC) Pty Ltd Project Platinum Building, Suite 2.01, 4 llya Avenue
ERINA NSW 2250, Australia
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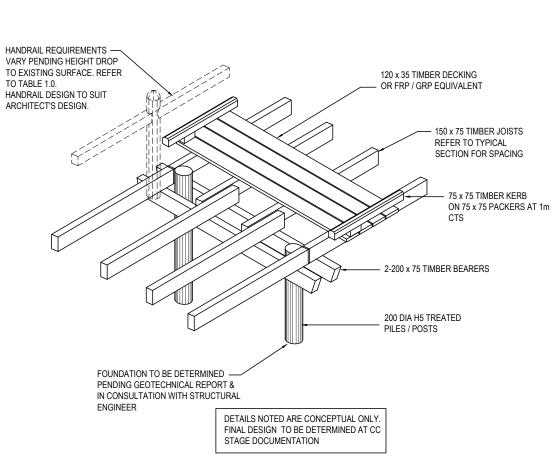
PROPOSED RESIDENTIAL
DEVELOPMENT

Australia	I I KOI OOLD KLO
324 3499	DEVELOPMENT
	(No. 43 - 49) WARRIEWOOD ROAD

Drawing Title
TYPICAL ROAD SECTIONS

Drawn	Date	Scale	A1	Q.A. Check	Date
LW	DEC 2023	AS NOTE	ΞD	-	-
Designed	Project No.			Dwg. No.	Issue
BK	CC230	177		C3.17	С





BOARDWALK GENERAL ARRANGEMENT SCALE - 1:10/A3, 1:20/A1

GEOMETRIC REQUIREMENTS LONGITUDINAL GRADES 3% MAX. GRADE GENERALLY 7% GRADE RAMPS CONNECTING DIFFERENCES IN LEVEL OF LESS THAN 0.63m (IE RAMPS NO LONGER THAN 9m. HORIZONTAL CURVES

AustRoads - Bicycles Approx angle change in 3m chords (degrees Radius Minimum 5m 35 Preferred 8m 22

OVERHEAD VERTICAL CLEARANCE

	9	BCA		AustRoads					
	Rooms	Doorways	Pedestrians Bicycles Bridge Desi				Pedestrians Bicycles Bridg	e Design	
	11001113	& Stairs			Pedestrians	Bicycles			
Minimum	2.1m	2.0m	2.0m	2.4m	2.0m	2.4m			
Preferred	2.4m		2.4m	2.7m	2.4m	2.7m			

2500 120 x 35 TIMBER DECKING 150 x 75 TIMBER JOISTS JOIST SPACINGS TO BE ADJUSTED PENDING DECKING TYPE. OR FRP / GRP EQUIVALENT 75 x 75 TIMBER KERB ON 75 x 75 PACKERS 0 2-200 x 75 TIMBER M20 BOLT (GALV.) FIXING **BEARERS** FOR BEARERS TO PILES. 200 150 440 440 440 150 440 440 20% AEP FLOOD LEVEL 200 DIA H5 TREATED PILES / POSTS FINISHED SURFACE FOUNDATION TO BE DETERMINED PENDING GEOTECHNICAL REPORT & IN CONSULTATION WITH STRUCTURAL ENGINEER. CONSTRUCTION TECHNIQUE MAY VARY FROM PILING TO BORING AND CONCRETE ENCASING PENDING CONDITIONS. 1720 (2100 MAX AT REST AREAS)

TYPICAL BOARDWALK SECTION SCALE - 1:10/A3, 1:20/A1

TABLE 1.0 HANDRAIL HEIGHTS

Bridge Design Code - AustRoads	Min rail height	Preferred rail height
Pedestrian Barrier	1.1m	1.1m
Cyclist Barrier	1.3m	1.3m
AS 1428.2 Design for Access & Mobility	Min rail height	Preferred rail height
Handrails at walkways ramps & landings	0.87 m	1.0 m
Building Code of Australia	Min rail height	Preferred rail height
for balconies between 1 & 3 m above ground. (125 mm max gap)	1.0 m	
for balconies less than 1m	no rail required	

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С	ISSUED FOR APPROVAL	06.03.24	NB	BK
В	ISSUED FOR APPROVAL	29.02.24	SJ	BK
Α	ISSUED FOR REVIEW & COMMENT	26.02.24	SJ	BK
Issue	Description	Date	Drawn	Approved

WARRIEWOOD DEVELOPERS PTY LTD



AT 1m CTS

ACOR Consultants (CC) Pty Ltd Platinum Building, Suite 2.01, 4 Ilya Avenue ERINA NSW 2250, Australia

WARRIEWOOD

PROPOSED RESIDENTIAL T +61 2 4324 3499 | DEVELOPMENT (No. 43 - 49) WARRIEWOOD ROAD

BOARDWALK GENERIC DETAILS SHEET 1 OF 2

AS NOTED FEB 2023 CC230177 C4.02

