

PROJECT: WYVERN HEALTH PRIVATE HOSPITAL

PLANSET: CONCEPT STORMWATER MANAGEMENT

CLIENT: WYVERN HEALTH P/L C/ - PODIA

| DRAWING LIST | | |
|-------------------------------|-----|---|
| DWG NO. | REV | DWG TITLE |
| GENERAL | | |
| PS07-A000 | B | COVER SHEET |
| PS07-A050 | B | DEVELOPMENT OVERVIEW PLAN |
| CONSTRUCTION MANAGEMENT WORKS | | |
| PS07-B300 | B | SOIL AND WATER MANAGEMENT PLAN |
| PS07-B305 | B | SOIL AND WATER MANAGEMENT DETAILS - RUSSLE CALCULATIONS |
| PS07-B310 | A | SOIL AND WATER MANAGEMENT DETAILS |
| DRAINAGE | | |
| PS07-E100 | B | DRAINAGE PLAN - SHEET 1 |
| PS07-E101 | B | DRAINAGE PLAN - SHEET 2 |
| PS07-E102 | B | DRAINAGE PLAN - SHEET 3 |



LOCALITY PLAN
NOT TO SCALE

LGA: NORTHERN BEACHES COUNCIL

4A LAROOL ROAD, TERREY HILLS, NSW
LOT 2 DP 1145029

SECTION 4.55

| REV | DESCRIPTION | DATE | DRAWN | DESIGNED | CHECKED | APPRVD | SCALE | GRID | DATUM | PROJECT MANAGER | CLIENT | DRAWING TITLE | | | | | |
|-----|------------------------------|------------|-------|----------|---------|--------|-------|------|-------|-----------------|---|---|-------------|-------------|-------------|-------------|----------|
| B | ARCHITECTURAL LAYOUT UPDATED | 06/07/2022 | RK/JS | AVG | AVG | JF | | --- | --- | JF | WYVERN HEALTH P/L C/ - PODIA | COVER SHEET | | | | | |
| A | INITIAL RELEASE | 24/06/2022 | NN | AVG | AVG | JF | | | | | WYVERN HEALTH PRIVATE HOSPITAL CONCEPT STORMWATER MANAGEMENT |  Consulting Engineers Environment Water Geotechnical Civil | PROJECT NO. | PLANSET NO. | RELEASE NO. | DRAWING NO. | REVISION |
| | | | | | | | | | | | 4A LAROOL ROAD, TERREY HILLS, NSW LOT 2 DP 1145029 | | P1605687 | PS07 | R03 | PS07-A000 | B |

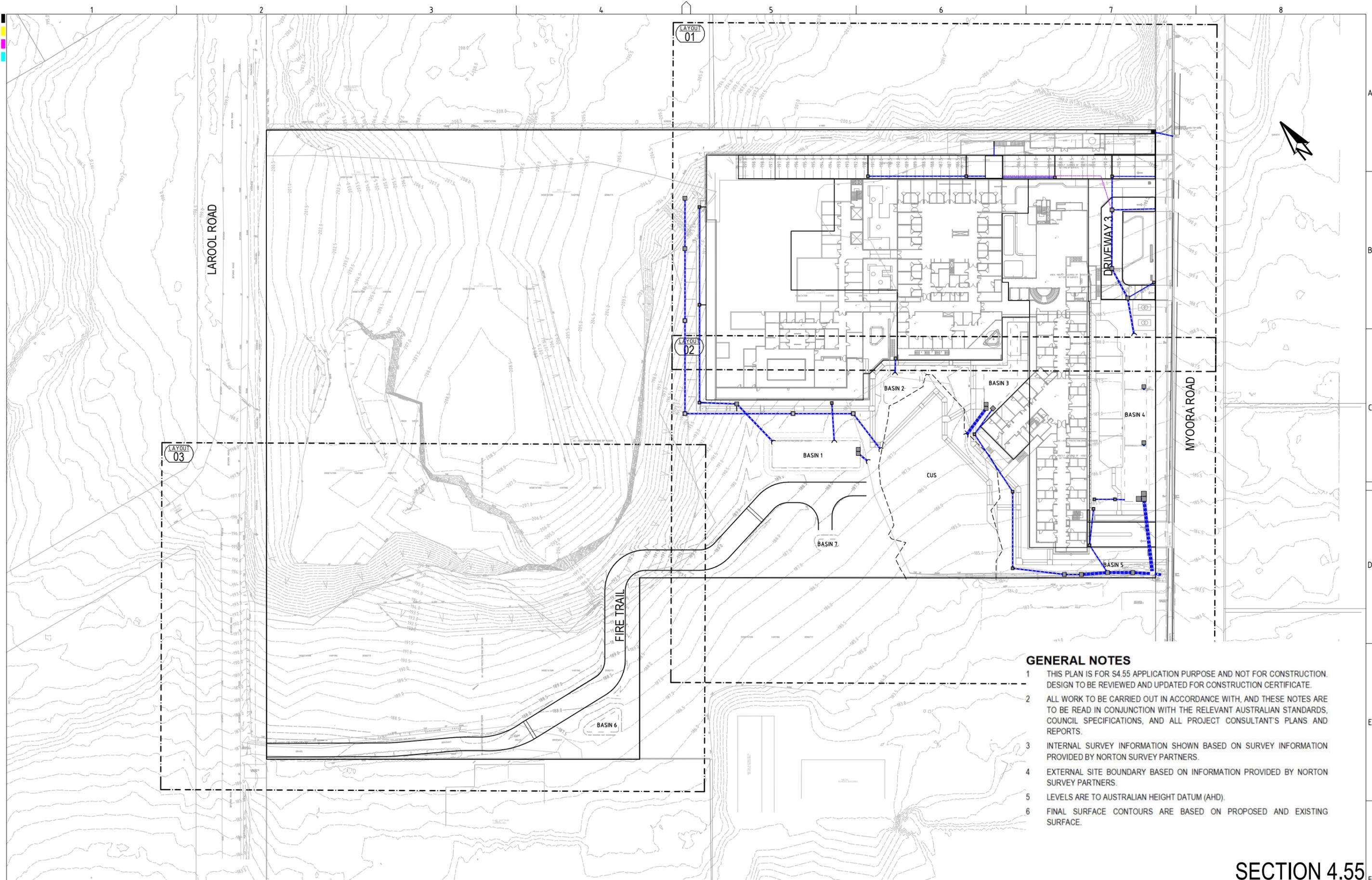
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A1 / A3 LANDSCAPE (A1L_C_022.0.01)



GENERAL NOTES

- 1 THIS PLAN IS FOR S4.55 APPLICATION PURPOSE AND NOT FOR CONSTRUCTION. DESIGN TO BE REVIEWED AND UPDATED FOR CONSTRUCTION CERTIFICATE.
- 2 ALL WORK TO BE CARRIED OUT IN ACCORDANCE WITH, AND THESE NOTES ARE TO BE READ IN CONJUNCTION WITH THE RELEVANT AUSTRALIAN STANDARDS, COUNCIL SPECIFICATIONS, AND ALL PROJECT CONSULTANT'S PLANS AND REPORTS.
- 3 INTERNAL SURVEY INFORMATION SHOWN BASED ON SURVEY INFORMATION PROVIDED BY NORTON SURVEY PARTNERS.
- 4 EXTERNAL SITE BOUNDARY BASED ON INFORMATION PROVIDED BY NORTON SURVEY PARTNERS.
- 5 LEVELS ARE TO AUSTRALIAN HEIGHT DATUM (AHD).
- 6 FINAL SURFACE CONTOURS ARE BASED ON PROPOSED AND EXISTING SURFACE.

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| A | INITIAL RELEASE | 24/06/2022 | NN | AVG | AVG | JF |

| | |
|---------|--------------------------------|
| SCALE | 0 5 10 15 20 25 30 35 40 45 50 |
| A1 (A3) | 1:500 (1:1,000) |
| METRES | |

GRID
GDA 94

DATUM
mAHD

PROJECT MANAGER
JF

CLIENT
WYVERN HEALTH P/L C/ - PODIA

PROJECT NAME/PLANSSET TITLE
WYVERN HEALTH PRIVATE HOSPITAL
CONCEPT STORMWATER MANAGEMENT

4A LAROO ROAD, TERRY HILLS, NSW
LOT 2 DP 1145029

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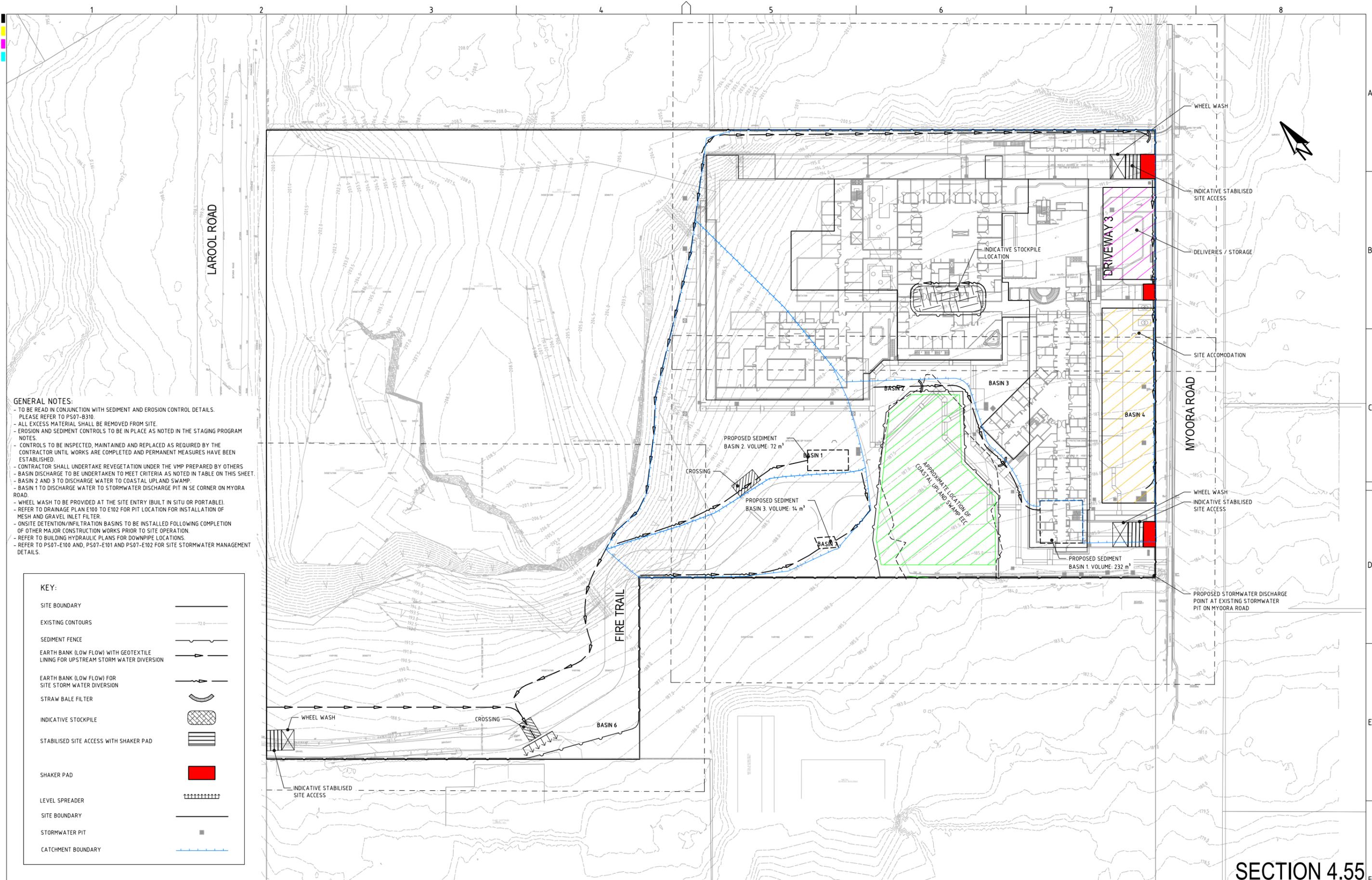
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| | |
|---------------------------|-------------|
| DRAWING TITLE | |
| DEVELOPMENT OVERVIEW PLAN | |
| PROJECT NO. | PLANSET NO. |
| P1605687 | PS07 |
| RELEASE NO. | DRAWING NO. |
| R03 | PS07-A050 |
| REVISION | |
| B | |

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GENERAL NOTES:

- TO BE READ IN CONJUNCTION WITH SEDIMENT AND EROSION CONTROL DETAILS. PLEASE REFER TO PS07-B310.
- ALL EXCESS MATERIAL SHALL BE REMOVED FROM SITE.
- EROSION AND SEDIMENT CONTROLS TO BE IN PLACE AS NOTED IN THE STAGING PROGRAM NOTES.
- CONTROLS TO BE INSPECTED, MAINTAINED AND REPLACED AS REQUIRED BY THE CONTRACTOR UNTIL WORKS ARE COMPLETED AND PERMANENT MEASURES HAVE BEEN ESTABLISHED.
- CONTRACTOR SHALL UNDERTAKE REVEGETATION UNDER THE VMP PREPARED BY OTHERS.
- BASIN DISCHARGE TO BE UNDERTAKEN TO MEET CRITERIA AS NOTED IN TABLE ON THIS SHEET.
- BASIN 2 AND 3 TO DISCHARGE WATER TO COASTAL UPLAND SWAMP.
- BASIN 1 TO DISCHARGE WATER TO STORMWATER DISCHARGE PIT IN SE CORNER ON MYOORA ROAD.
- WHEEL WASH TO BE PROVIDED AT THE SITE ENTRY (BUILT IN SITU OR PORTABLE).
- REFER TO DRAINAGE PLAN E100 TO E102 FOR PIT LOCATION FOR INSTALLATION OF MESH AND GRAVEL INLET FILTER.
- ONSITE DETENTION/INFILTRATION BASINS TO BE INSTALLED FOLLOWING COMPLETION OF OTHER MAJOR CONSTRUCTION WORKS PRIOR TO SITE OPERATION.
- REFER TO BUILDING HYDRAULIC PLANS FOR DOWNPIPE LOCATIONS.
- REFER TO PS07-E100 AND, PS07-E101 AND PS07-E102 FOR SITE STORMWATER MANAGEMENT DETAILS.

KEY:

| | |
|---|--|
| SITE BOUNDARY | |
| EXISTING CONTOURS | |
| SEDIMENT FENCE | |
| EARTH BANK (LOW FLOW) WITH GEOTEXTILE LINING FOR UPSTREAM STORM WATER DIVERSION | |
| EARTH BANK (LOW FLOW) FOR SITE STORM WATER DIVERSION | |
| STRAW BALE FILTER | |
| INDICATIVE STOCKPILE | |
| STABILISED SITE ACCESS WITH SHAKER PAD | |
| SHAKER PAD | |
| LEVEL SPREADER | |
| SITE BOUNDARY | |
| STORMWATER PIT | |
| CATCHMENT BOUNDARY | |

SECTION 4.55

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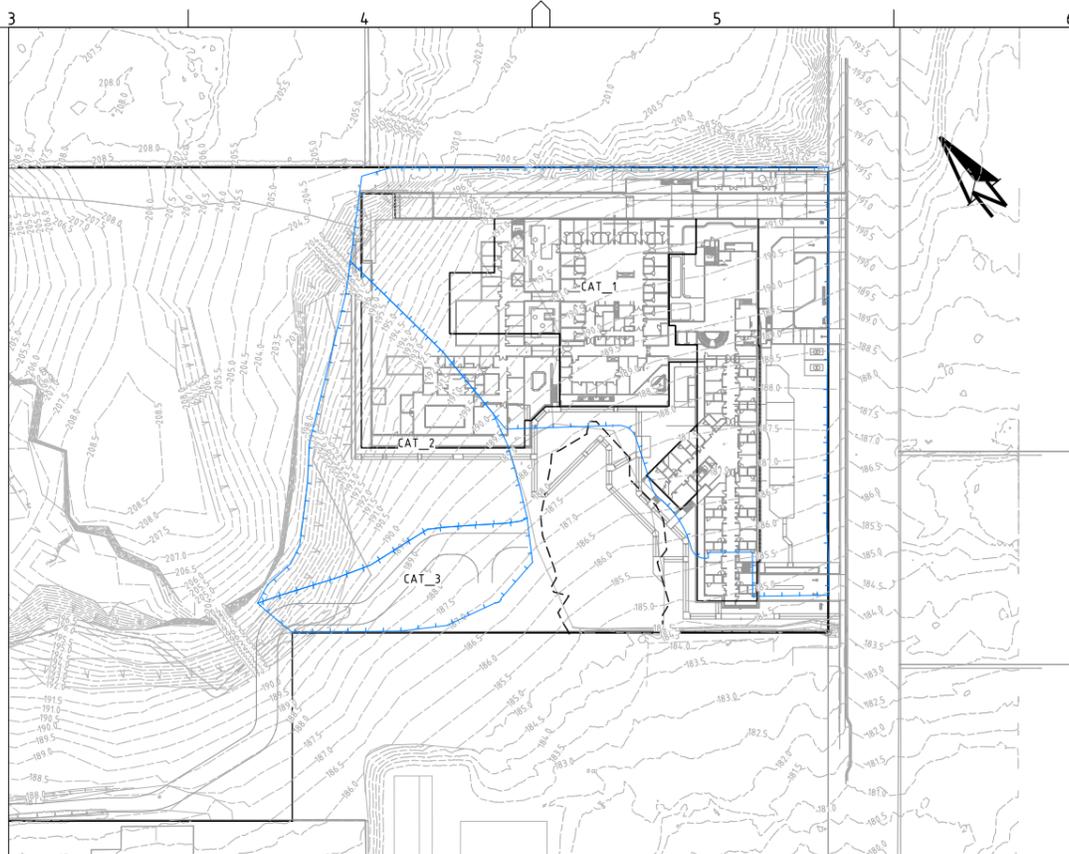
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|--------|-------|-----------------|------------------------------|
| GRID | DATUM | PROJECT MANAGER | CLIENT |
| GDA 94 | mAHD | JF | WYVERN HEALTH P/L C/ - PODIA |

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WYVERN HEALTH PRIVATE HOSPITAL
CONCEPT STORMWATER MANAGEMENT
 4A LAROO ROAD, TERRY HILLS, NSW
 LOT 2 DP 1145029

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|--------------------------------|--------------|-------------|-------------|----------|
| DRAWING TITLE | | | | |
| SOIL AND WATER MANAGEMENT PLAN | | | | |
| PROJECT NO. | PLANSSET NO. | RELEASE NO. | DRAWING NO. | REVISION |
| P1605687 | PS07 | R03 | PS07-B300 | B |



Note: These "Detailed Calculation" spreadsheets relate only to high erosion hazard lands as identified in figure 4.6 or where the designer chooses to use the RUSLE to size sediment basins. The "Standard Calculation" spreadsheets should be used on low erosion hazard lands as identified by figure 4.6 and where the designer chooses not to run the RUSLE in calculations.

1. Site Data Sheet

| | |
|----------------------|------------------------------------|
| Site Name: | P1805687 |
| Site Location: | 4A Larool Rd, Terrey Hills, NSW |
| Precinct: | N/A |
| Description of Site: | Somersby (9130so) & Gymea (9130gy) |

| Site area | Site | | | Remarks |
|-------------------------------|-------|-------|-------|---------|
| | CAT_1 | CAT_2 | CAT_3 | |
| Total catchment area (ha) | 1.07 | 0.34 | 0.14 | |
| Disturbed catchment area (ha) | 0.84 | 0.19 | 0.09 | |

| Soil analysis | | | | |
|---------------------------------------|---------|---------|---------|--|
| % sand (fraction 0.02 to 2.00 mm) | 60 | 60 | 60 | Soil texture should be assessed through mechanical dispersion only. Dispersing agents (e.g. Calgon) should not be used |
| % silt (fraction 0.002 to 0.02 mm) | 15 | 15 | 15 | |
| % clay (fraction finer than 0.002 mm) | 25 | 25 | 25 | |
| Dispersion percentage | 32.5 | 32.5 | 32.5 | E.g. enter 10 for dispersion of 10% |
| % of whole soil dispersible | 10.5625 | 10.5625 | 10.5625 | See Section 6.3.3(e) |
| Soil Texture Group | D | D | D | See Section 6.3.3(c), (d) and (e) |

| Rainfall data | | | | |
|--|------|------|------|--------------------------------|
| Design rainfall depth (days) | 5 | 5 | 3 | See Sections 6.3.4 (d) and (e) |
| Design rainfall depth (percentile) | 85 | 85 | 75 | See Sections 6.3.4 (f) and (g) |
| x-day, y-percentile rainfall event | 40 | 40 | 16 | See Section 6.3.4 (h) |
| Rainfall intensity: 2-year, 6-hour storm | 14.6 | 14.6 | 14.6 | See IFD chart for the site |

| RUSLE Factors | | | | |
|-------------------------------------|-------|-------|-------|---|
| Rainfall erosivity (R-factor) | 4710 | 4710 | 4710 | Automatic calculation from above data |
| Soil erodibility (K-factor) | 0.038 | 0.038 | 0.038 | |
| Slope length (m) | 80 | 50 | 50 | |
| Slope gradient (%) | 8 | 12 | 8 | |
| Length/gradient (LS-factor) | 2.05 | 2.66 | 1.51 | RUSLE data can be obtained from Appendixes A, B and C |
| Erosion control practice (P-factor) | 1.3 | 1.3 | 1.3 | |
| Ground cover (C-factor) | 1 | 1 | 1 | |

| Calculations | | | | |
|---|-----|-----|-----|-------------------------------------|
| Soil loss (t/ha/yr) | 477 | 619 | 351 | |
| Soil Loss Class | 4 | 5 | 4 | See Section 4.4.2(b) |
| Soil loss (m ³ /ha/yr) | 367 | 476 | 270 | |
| Sediment basin storage volume, m ³ | 52 | 15 | 4 | See Sections 6.3.4(i) and 6.3.5 (e) |

4. Volume of Sediment Basins, Type D and Type F Soils

Basin volume = settling zone volume + sediment storage zone volume

Settling Zone Volume

The settling zone volume for Type F and Type D soils is calculated to provide capacity to contain all runoff expected from up to the y-percentile rainfall event. The volume of the basin's settling zone (V) can be determined as a function of the basin's surface area and depth to allow for particles to settle and can be determined by the following equation:

$$V = 10 \times C_v \times A \times R_{x\text{-day}, y\text{-}\%ile} \text{ (m}^3\text{)}$$

where:

10 = a unit conversion factor

C_v = the volumetric runoff coefficient defined as that portion of rainfall that runs off as stormwater over the x-day period

R_{x-day, y-%ile} = is the x-day total rainfall depth (mm) that is not exceeded in y percent of rainfall events. (See Sections 6.3.4(d), (e), (f), (g) and (h)).

A = total catchment area (ha)

Sediment Storage Zone Volume

In the detailed calculation on Soil Loss Classes 1 to 4 lands, the sediment storage zone can be taken as 50 percent of the settling zone capacity. Alternately designers can design the zone to store the 2-month soil loss as calculated by the RUSLE (Section 6.3.4(i)(ii)). However, on Soil Loss Classes 5, 6 and 7 lands, the zone must contain the 2-month soil loss as calculated by the RUSLE (Section 6.3.4(i)(iii)).

Place an "X" in the box below to show the sediment storage zone design parameters used here:

| | |
|-------------------------------------|--|
| <input type="checkbox"/> | 50% of settling zone capacity, |
| <input checked="" type="checkbox"/> | 2 months soil loss calculated by RUSLE |

Total Basin Volume

| Site | C _v | R _{x-day, y-%ile} | Total catchment area (ha) | Settling zone volume (m ³) | Sediment storage volume (m ³) | Total basin volume (m ³) |
|-------|----------------|----------------------------|---------------------------|--|---|--------------------------------------|
| CAT_1 | 0.42 | 40 | 1.07 | 179.76 | 52 | 231.76 |
| CAT_2 | 0.42 | 40 | 0.34 | 57.12 | 15 | 72.12 |
| CAT_3 | 0.42 | 16 | 0.14 | 9.408 | 4 | 13.408 |

SECTION 4.55

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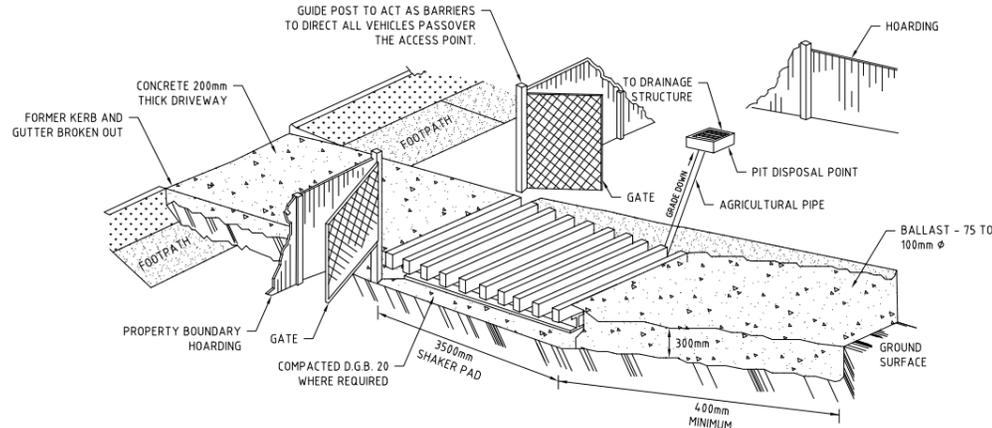
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| DRAWING TITLE | | | | |
| SOIL AND WATER MANAGEMENT DETAILS RUSLE CALCULATIONS | | | | |
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| P1605687 | PS07 | R03 | PS07-B305 | B |

STABILISED ACCESS POINT

TYPE II SAP

THE TYPE II SAP DESIGN IS MORE DEFINED IN THAT IT REQUIRES AN AREA OF BALLAST WITHIN THE SITE COMBINED WITH A SHAKER PAD, ADJACENT TO THE SHAKER PAD AND IN THE PUBLIC WAY IS A TEMPORARY (CONCRETE) VEHICULAR CROSSING. (SEE DIAGRAM)

STABILISED ACCESS POINT - TYPE 2



IN BOTH TYPE I AND TYPE II SAP'S, THE TEMPORARY VEHICULAR CROSSING MUST:

- CONNECT TO AN EXISTING GUTTER LAYBACK (WHERE THE KERB AND GUTTER EXIST). IF A GUTTER LAYBACK DOES NOT EXIST THEN THE CONNECTION MUST BE MADE TO THE GUTTER BY REMOVING THE ADJACENT KERB SECTION ONLY.
- CONNECT TO A DISH CROSSING (WHERE KERB AND GUTTER DOES NOT EXIST). IF A DISH CROSSING DOES NOT EXIST, THEN IT MUST BE CONSTRUCTED IN ACCORDANCE WITH DETAILS CONTAINED IN COUNCIL'S ISSUED FOOTPATH CROSSING LEVELS.

IT SHOULD BE NOTED THAT THESE TYPES OF SAPS ARE CONSIDERED TO BE APPLICABLE FOR THE MAJORITY OF ACTIVITIES HOWEVER SOME SITES MAY REQUIRE SPECIAL CONSIDERATION.

SHAKER PAD (CATTLE GRID)

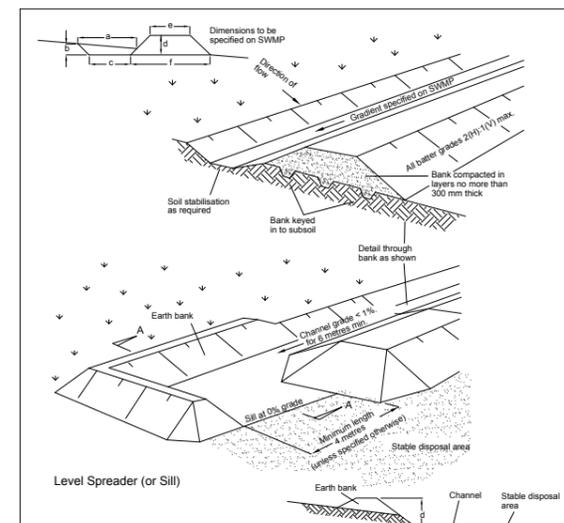
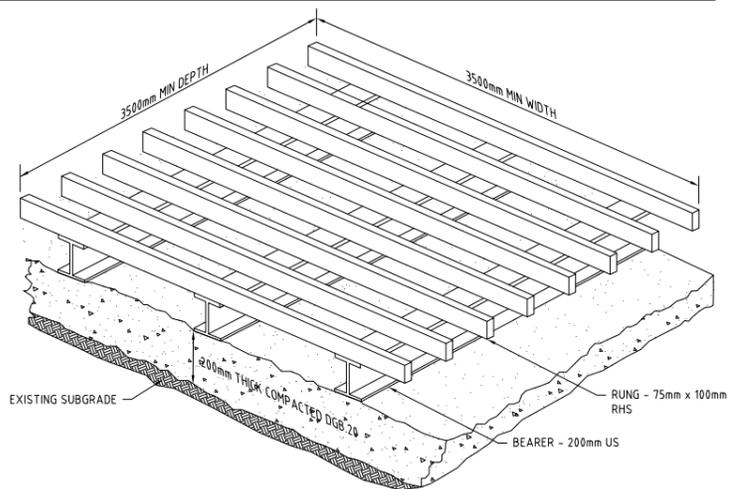
A CORRECTLY DESIGNED AND INSTALLED SHAKER PAD WILL ASSIST IN PREVENTING SEDIMENT TRANSFER FROM A SITE. ANY STABILISED ACCESS POINT (SAP) CAN BE DESIGNED WITH A SHAKER PAD (COMPULSORY IN TYPE II SAP'S)

SHAKER PADS CAN BE DESIGNED AND CONSTRUCTED TO ENABLE RE-USE ON FUTURE PROJECTS.

THE SHAKER PAD:

- MUST BE DESIGNED AND CERTIFIED BY A PRACTISING STRUCTURAL ENGINEER. THE CERTIFIED DESIGN SHOULD BE SUBMITTED WITH THE RELEVANT APPLICATION.
- CAN BE CONSTRUCTED FROM ANY SUITABLE MATERIAL.
- MUST BE LOCATED ON A SUITABLY PREPARED AND COMPACTED SUB-GRADE/BASE MATERIAL.
- MUST BE SITUATED SUCH THAT THE RUNGS OF THE SHAKER PAD ARE LEVEL WITH THE ADJOINING NATURAL SURFACE.
- MUST BE A MINIMUM OF 3.5m IN LENGTH.
- MUST BE A MINIMUM OF 3.5m IN WIDTH.
- MUST HAVE CLEAR SPACING BETWEEN RUNGS OF 200 - 250mm.
- RUNGS MUST HAVE A MAXIMUM WIDTH (BEARING AREA) OF 75mm.
- MUST HAVE A MINIMUM CLEAR DEPTH OF 300mm IE FORM THE TOP OF THE RUNG TO THE FINISHED SUB-GRADE/BASE LEVEL.

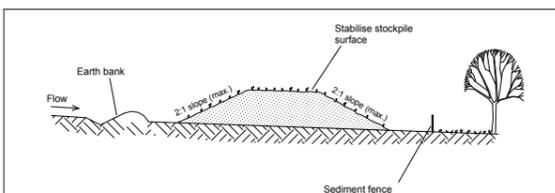
THE SHAKER PAD MUST BE PROVIDED WITH SUITABLE BARRIERS AT THE SIDES TO ENSURE THAT ALL TYERS OF VEHICLES LEAVING THE SITE TRAVERSE THE DEVICE.



Construction Notes

- Construct at the gradient specified on the ESCP or SWMP, normally between 1 and 5 percent.
- Avoid removing trees and shrubs if possible - work around them.
- Ensure the structures are free of projections or other irregularities that could impede water flow.
- Build the drains with circular, parabolic or trapezoidal cross sections, not V-shaped, at the dimensions shown on the SWMP.
- Ensure the banks are properly compacted to prevent failure.
- Complete permanent or temporary stabilisation within 10 days of construction following Table 5.2 in Landcom (2004).
- Where discharging to erodible lands, ensure they outlet through a properly constructed level spreader.
- Construct the level spreader at the gradient specified on the ESCP or SWMP, normally less than 1 percent or level.
- Where possible, ensure they discharge waters onto either stabilised or undisturbed disposal sites within the same subcatchment area from which the water originated. Approval might be required to discharge into other subcatchments.

EARTH BANK HIGH FLOW) SD 5-6

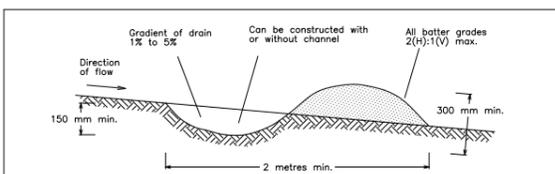


Construction Notes

- Place stockpiles more than 2 (preferably 5) metres from existing vegetation, concentrated water flow, roads and hazard areas.
- Construct on the contour as low, flat, elongated mounds.
- Where there is sufficient area, topsoil stockpiles shall be less than 2 metres in height.
- Where they are to be in place for more than 10 days, stabilise following the approved ESCP or SWMP to reduce the C-factor to less than 0.10.
- Construct earth banks (Standard Drawing 5-5) on the upslope side to divert water around stockpiles and sediment fences (Standard Drawing 6-8) 1 to 2 metres downslope.

STOCKPILES

SD 4-1



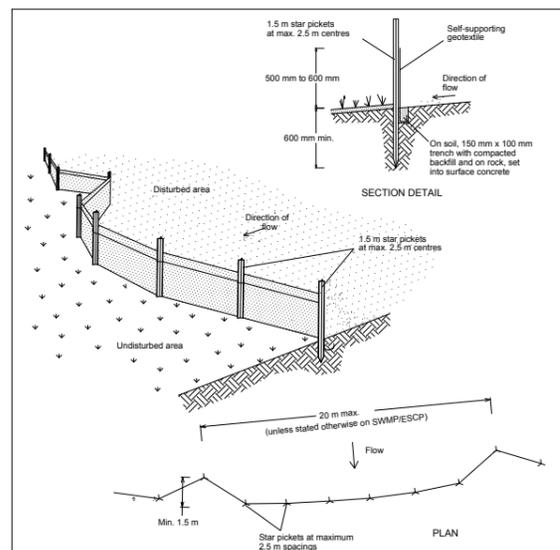
NOTE: Only to be used as temporary bank where maximum upslope length is 80 metres.

Construction Notes

- Build with gradients between 1 percent and 5 percent.
- Avoid removing trees and shrubs if possible - work around them.
- Ensure the structures are free of projections or other irregularities that could impede water flow.
- Build the drains with circular, parabolic or trapezoidal cross sections, not V shaped.
- Ensure the banks are properly compacted to prevent failure.
- Complete permanent or temporary stabilisation within 10 days of construction.

EARTH BANK (LOW FLOW)

SD 5-5

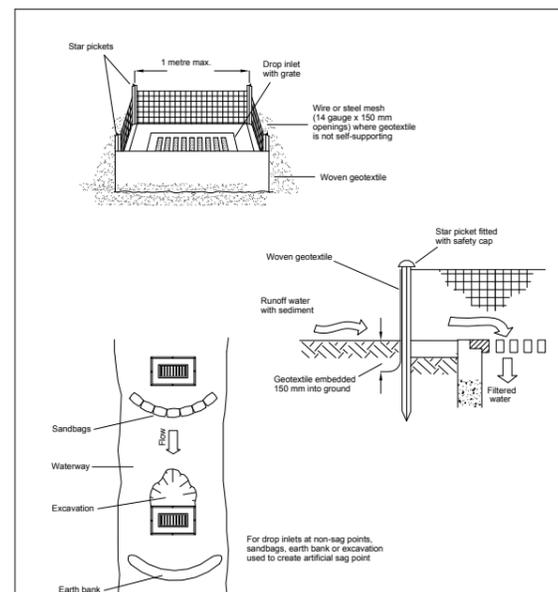


Construction Notes

- Construct sediment fences as close as possible to being parallel to the contours of the site, but with small returns as shown in the drawing to limit the catchment area of any one section. The catchment area should be small enough to limit water flow if concentrated at one point to 50 litres per second in the design storm event, usually the 10-year event.
- Cut a 150-mm deep trench along the upslope line of the fence for the bottom of the fabric to be entrenched.
- Drive 1.5 metre long star pickets into ground at 2.5 metre intervals (max) at the downslope edge of the trench. Ensure any star pickets are fitted with safety caps.
- Fix self-supporting geotextile to the upslope side of the posts ensuring it goes to the base of the trench. Fix the geotextile with wire ties or as recommended by the manufacturer. Only use geotextile specifically produced for sediment fencing. The use of shade cloth for this purpose is not satisfactory.
- Join sections of fabric at a support post with a 150-mm overlap.
- Backfill the trench over the base of the fabric and compact it thoroughly over the geotextile.

SEDIMENT FENCE

SD 6-8

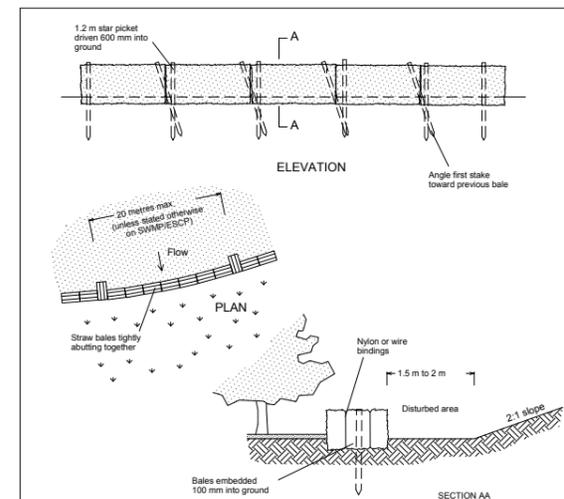


Construction Notes

- Fabricate a sediment barrier made from geotextile or straw bales.
- Follow Standard Drawing 6-7 and Standard Drawing 6-8 for installation procedures for the straw bales or geofabric. Reduce the picket spacing to 1 metre centres.
- In waterways, artificial sag points can be created with sandbags or earth banks as shown in the drawing.
- Do not cover the inlet with geotextile unless the design is adequate to allow for all waters to bypass it.

GEOTEXTILE INLET FILTER

SD 6-12



Construction Notes

- Construct the straw bale filter as close as possible to being parallel to the contours of the site.
- Place bales lengthwise in a row with ends lightly abutting. Use straw to fill any gaps between bales. Straws are to be placed parallel to ground.
- Ensure that the maximum height of the filter is one bale.
- Embed each bale in the ground 75 mm to 100 mm and anchor with two 1.2 metre star pickets or stakes. Angle the first star picket or stake in each bale towards the previously laid bale. Drive them 600 mm into the ground and, if possible, flush with the top of the bales. Where star pickets are used and they protrude above the bales, ensure they are fitted with safety caps.
- Where a straw bale filter is constructed downslope from a disturbed batter, ensure the bales are placed 1 to 2 metres downslope from the toe.
- Establish a maintenance program that ensures the integrity of the bales is retained - they could require replacement each two to four months.

STRAW BALE FILTER

SD 6-7

SECTION 4.55

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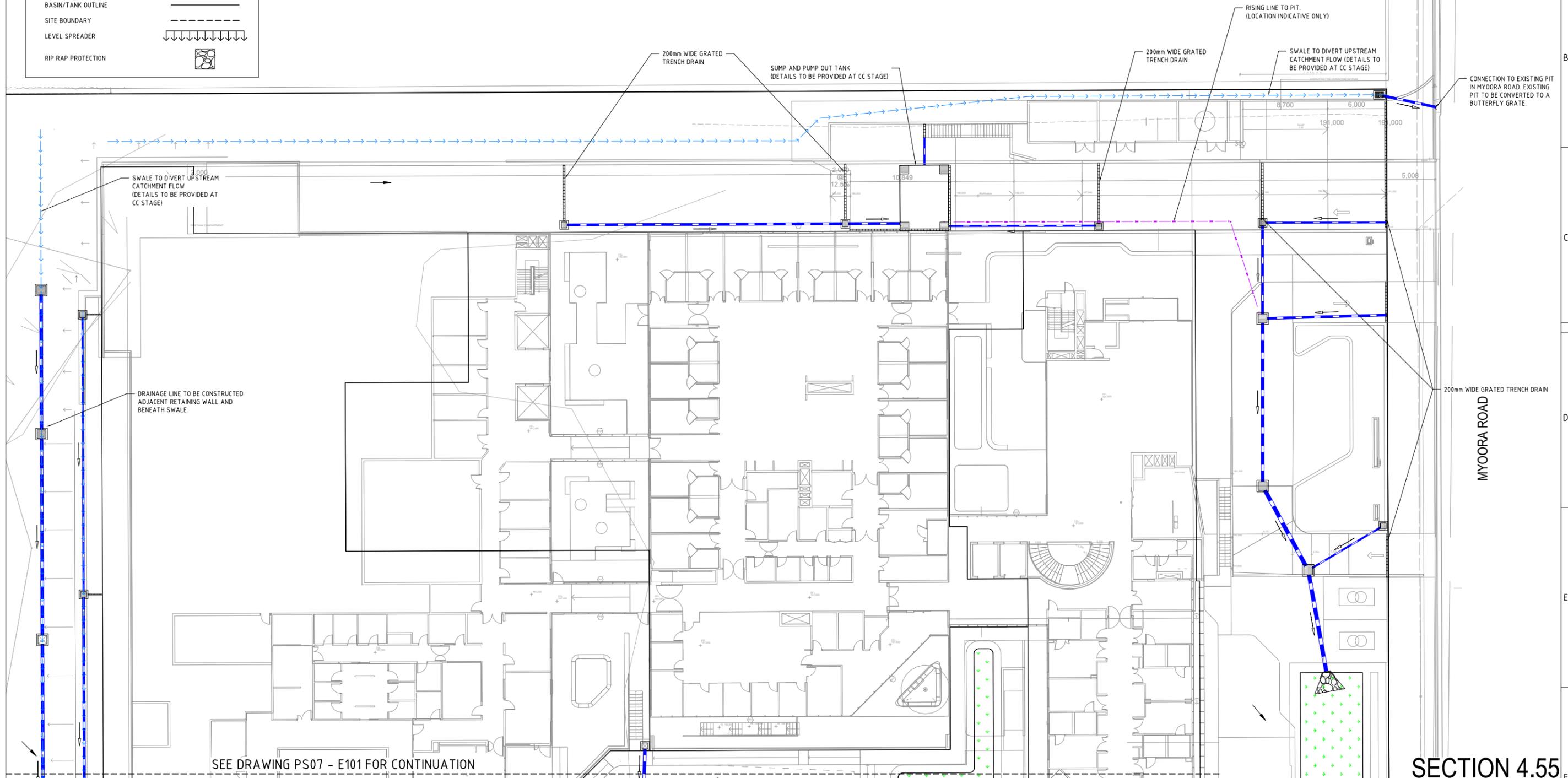


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| DRAWING TITLE | | | | |
|-----------------------------------|--------------|-------------|-------------|----------|
| SOIL AND WATER MANAGEMENT DETAILS | | | | |
| PROJECT NO. | PLANSSET NO. | RELEASE NO. | DRAWING NO. | REVISION |
| P1605687 | PS07 | R03 | PS07-B310 | A |

| KEY | |
|--------------------------|--|
| STORMWATER PIPELINE | |
| RISING MAIN | |
| GRATED SURFACE INLET PIT | |
| JUNCTION PIT | |
| GRATED DRAIN | |
| HEADWALL | |
| FLOW DIRECTION | |
| OVERLAND FLOW PATH | |
| SWALE | |
| BASIN/TANK OUTLINE | |
| SITE BOUNDARY | |
| LEVEL SPREADER | |
| RIP RAP PROTECTION | |

NOTE:
 1- CONCEPT DESIGN ONLY FOR ALL AND PIT AND PIPE LOCATIONS. CONFIRMATION OF FINAL PIPE SIZES AND PIT LOCATIONS TO BE COMPLETED AT DETAILED DESIGN STAGE.
 2- DOWNPIPE AND SLUNG PIPE LOCATIONS TO BE PROVIDED AT DETAILED DESIGN STAGE TO INCORPORATE WITH ALL OTHER BUILDING SERVICES.
 3- REFER TO CC APPROVED MA PLANSET P1605687P505-R05 AND REPORT P1605687JR06V01 (SEPTEMBER 2021) FOR OSD, WSUD, COASTAL UPLAND SWAMP RECHARGE SYSTEM AND WATER BALANCE MODEL DESIGN DETAILS.



SEE DRAWING PS07 - E101 FOR CONTINUATION

SECTION 4.55

| REV | DESCRIPTION | DATE | DRAWN | DESIGNED | CHECKED | APPRVD |
|-----|------------------------------|------------|-------|----------|---------|--------|
| B | ARCHITECTURAL LAYOUT UPDATED | 06/07/2022 | RK/JS | AVG | AVG | JF |
| A | INITIAL RELEASE | 24/06/2022 | NN | AVG | AVG | JF |

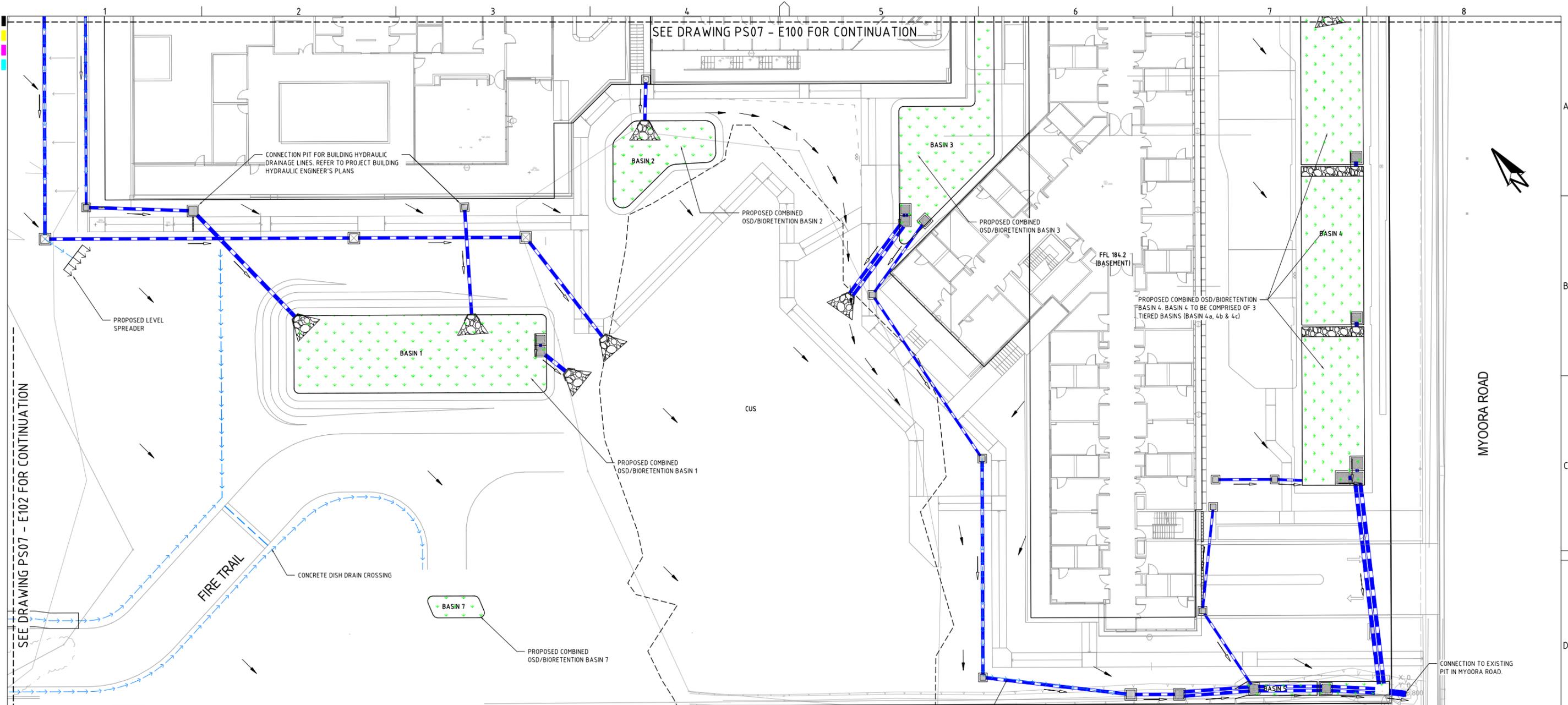
| GRID | DATUM | PROJECT MANAGER | CLIENT |
|--------|-------|-----------------|------------------------------|
| GDA 94 | mAHD | JF | WYVERN HEALTH P/L C/ - PODIA |

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PROJECT NAME/PLANSET TITLE
 WYVERN HEALTH PRIVATE HOSPITAL
 CONCEPT STORMWATER MANAGEMENT
 4A LAROO ROAD, TERRY HILLS, NSW
 LOT 2 DP 1145029

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| DRAWING TITLE | | | | |
|-----------------------|-------------|-------------|-------------|----------|
| DRAINAGE PLAN SHEET 1 | | | | |
| PROJECT NO. | PLANSET NO. | RELEASE NO. | DRAWING NO. | REVISION |
| P1605687 | PS07 | R03 | PS07-E100 | B |



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SEE DRAWING PS07 - E100 FOR CONTINUATION

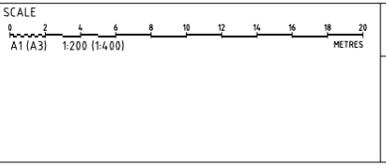
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SECTION 4.55

KEY

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| STORMWATER PIPELINE | |
| RISING MAIN | |
| GRATED SURFACE INLET PIT | |
| JUNCTION PIT | |
| GRATED DRAIN | |
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| SITE BOUNDARY | |
| LEVEL SPREADER | |
| RIP RAP PROTECTION | |

| REV | DESCRIPTION | DATE | DRAWN | DESIGNED | CHECKED | APPRVD |
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| B | ARCHITECTURAL LAYOUT UPDATED | 06/07/2022 | RK/JS | AVG | AVG | JF |
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GRID: GDA 94
 DATUM: mAHD
 PROJECT MANAGER: JF

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 PROJECT NAME/PLANSET TITLE: WYVERN HEALTH PRIVATE HOSPITAL CONCEPT STORMWATER MANAGEMENT
 4A LARROOL ROAD, TERRY HILLS, NSW
 LOT 2 DP 1145029

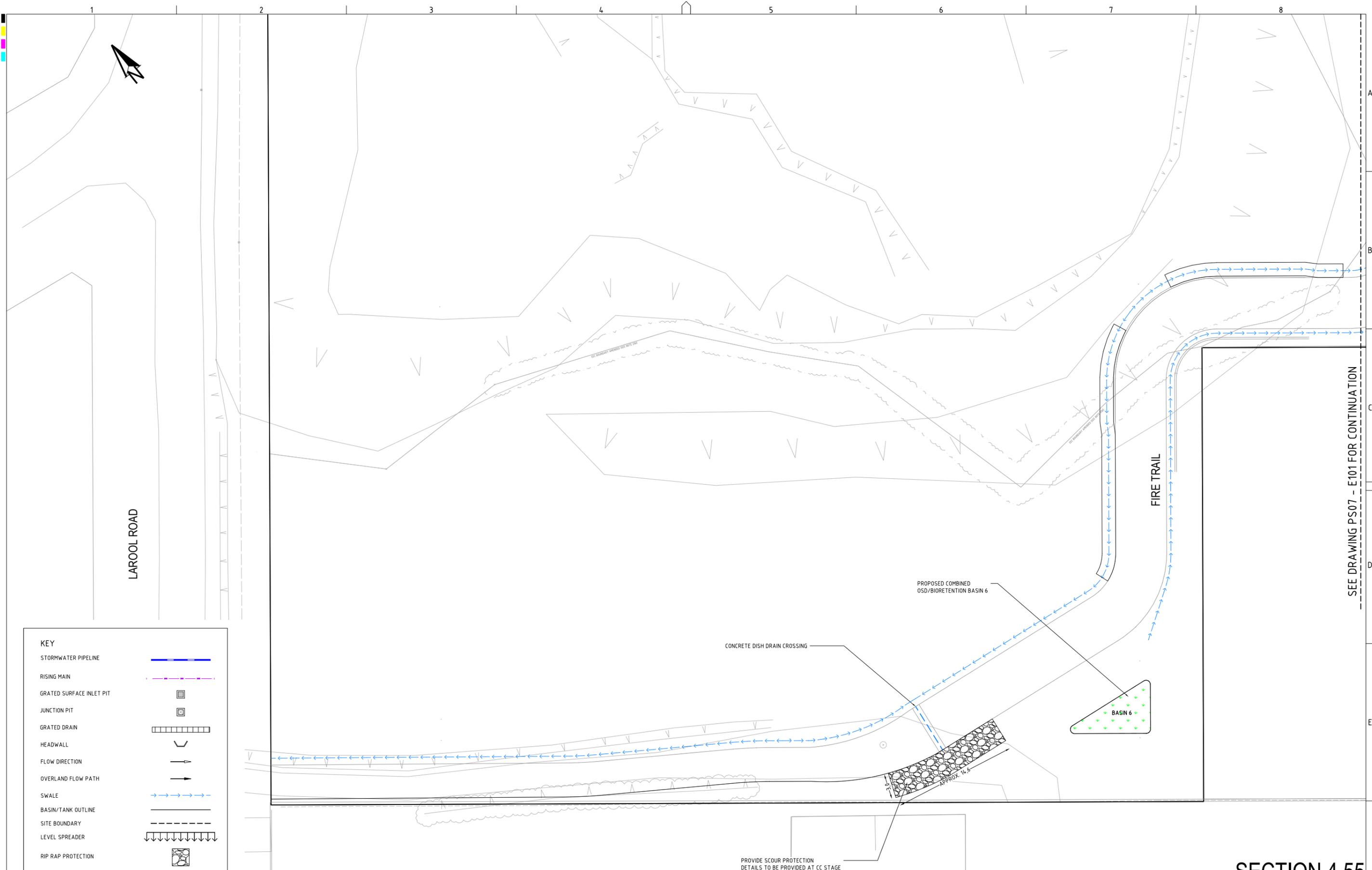
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| DRAWING TITLE: DRAINAGE PLAN SHEET 2 | | | | |
| PROJECT NO. P1605687 | PLANSET NO. PS07 | RELEASE NO. R03 | DRAWING NO. PS07-E101 | REVISION B |

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A1 / A3 LANDSCAPE (A1L_C_02.0.01)

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| KEY | |
|--------------------------|--|
| STORMWATER PIPELINE | |
| RISING MAIN | |
| GRATED SURFACE INLET PIT | |
| JUNCTION PIT | |
| GRATED DRAIN | |
| HEADWALL | |
| FLOW DIRECTION | |
| OVERLAND FLOW PATH | |
| SWALE | |
| BASIN/TANK OUTLINE | |
| SITE BOUNDARY | |
| LEVEL SPREADER | |
| RIP RAP PROTECTION | |

| REV | DESCRIPTION | DATE | DRAWN | DESIGNED | CHECKED | APPRVD |
|-----|------------------------------|------------|-------|----------|---------|--------|
| B | ARCHITECTURAL LAYOUT UPDATED | 06/07/2022 | RK/JS | AVG | AVG | JF |
| A | INITIAL RELEASE | 24/06/2022 | NN | AVG | AVG | JF |

| | |
|----------------------------|---|
| SCALE | 0 2 4 6 8 10 12 14 16 18 20 METRES |
| GRID | GDA 94 |
| DATUM | mAHD |
| PROJECT MANAGER | JF |
| CLIENT | WYVERN HEALTH P/L C/ - PODIA |
| PROJECT NAME/PLANSET TITLE | WYVERN HEALTH PRIVATE HOSPITAL CONCEPT STORMWATER MANAGEMENT |
| CLIENT ADDRESS | 4A LAROO ROAD, TERRY HILLS, NSW LOT 2 DP 1145029 |

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|---------------|-----------------------|-------------|-------------|----------|
| DRAWING TITLE | DRAINAGE PLAN SHEET 3 | | | |
| PROJECT NO. | PLANSET NO. | RELEASE NO. | DRAWING NO. | REVISION |
| P1605687 | PS07 | R03 | PS07-E102 | B |

SECTION 4.55

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