

Geotechnical Investigation Report

For

Sydney Extensions & Designs Pty Ltd

At

16 Noorong Avenue,

Frenchs Forest NSW 2086

Report G11624-1

24th March 2025





Document Control

16 Noorong Avenue,

Frenchs Forest NSW 2086

Prepared for: Sydney Extensions & Designs Pty Ltd

Revision	Date	Author	Reviewer
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1. Introduction

1.1. Overview

Atlas Geotechnical Service Pty Ltd (AGS) was engaged by Sydney Extensions & Designs Pty Ltd (client) to undertake a Geotechnical Investigation for a Proposed Residential Development at 16 Noorong Ave, Frenchs Forest NSW 2086 redevelopment, herein referred to as 'the site'.

It is understood the client requires a Geotechnical Investigation Report in conjunction with the associated laboratory tests to provide a determination of the existing site founding conditions based on investigation findings.

1.2. Objectives

The objectives of this investigation are listed below:

- Determination of in-situ soil conditions via mechanical and manual auger investigation;
- Obtain subsurface soil profile and geotechnical parameters;
- Determination of shallow bearing capacity;
- Determination of site classification;
- Determination of groundwater during investigation (if encountered);
- Determination of Bedrock level depth and classification (if encountered); and
- Provide comments and recommendations on investigation findings.

1.3. Scope of Work

To achieve the above-mentioned objectives, AGS carried out the following scope of work:

- Review of DBYD drawings, geological maps, and other available documents in the area;
- Walkover observation of site conditions;
- Supervise One (1) x mechanical borehole towards the southern perimeter of the proposed development footprint, advanced to a maximum depth of 1.2m;
- Supervise 1 x manual borehole towards the northern perimeter of the proposed development footprint, advanced to a maximum depth of 1.2m;
- Logging of onsite borehole and GINT Logging;
- Undertake 1 x bulk soil sample for the Atterberg Limits test;
- Undertake a total of Two (2) x shallow Dynamic Cone Penetrometer (DCP) within each borehole location; and
- Prepare a Geotechnical Investigation Report of investigation findings along with annotated drawings and geotechnical design parameters.

1.4. Supplied Documents

As part of this investigation, AGS was supplied with the following documentation:



- 'Ground Floor and First Floor Alterations and Additions', issued by: Sydney Extensions & Design Pty Ltd, Ref: Lot 321, DP848146 Rev C, issued: 20/01/2025.
- 'Return of Application', issued by Northern Beaches Council, Application No. DA2025/0197-PAN-210181, issued: 27 February 2025.



2. Site Condition and Description

2.1. Regional Geology

The 1:100,000 scale Geological Series Map of the Sydney region indicates that the subject site is underlain by a Hawkesbury Sandstone (Rh) of the Mesozoic Era. Rh is described as 'Medium to coarsegrained quartz sandstone, very minor shale and laminite lenses'.

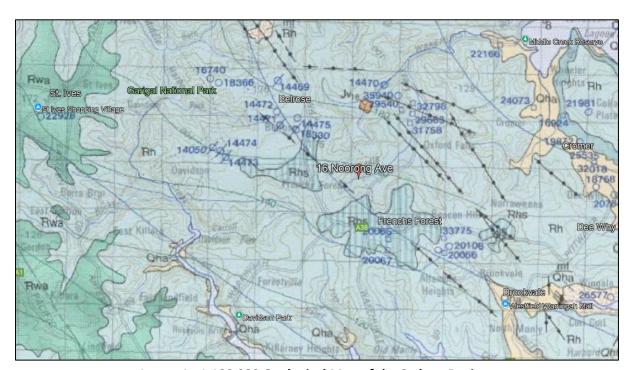


Image 1 - 1:100,000 Geological Map of the Sydney Region

2.2. Proposed Development

Based on client consultation, supplied documentation, and geotechnical observations, AGS has prepared a borehole site plan (refer to Figure 1) presented in the attachments within this report.

It is understood that the proposed first floor is to be constructed above the existing car garage, found at the southwestern portion of the existing residential dwelling footprint. The indicated residential developments concept was extrapolated from the aforementioned supplied documentation (Image 2).





Image 2 - Proposed Construction Area

AGS understands that the following construction has been proposed, with respect to Image 2:

- Demolition of the existing garage roofing;
- Redevelopment of the first floor and garage of the existing structure; and
- Construction of the proposed first floor, including redevelopment of the garage parking space, an additional bedroom, ensuite, staircase and balcony, within the bounds of the proposed works.

2.3. Site Description

The site is located within the Northern Beaches Council, with site access from Noorong Avenue, located on the southwestern perimeter of the site location/boundary. The site maintains an area of 503 m².

The proposed development, as outlined in Section 2.2, will be constructed within the confines of the site's internal boundary (Image 3), the site is located at 16 Noorong Avenue, Frenchs Forest. Topographically, the site was calculated to exhibit a descending terrain with a gradient of approximately 14°, descending from northwest to southeast. The property has landscaped gardens along the front of the property and adjacent to the western fence line, which contains retaining walls. At the front of the property, there are paved and concreted car parking spaces. At the back of the property, there is a swimming pool and timber decking surrounded by concrete paving.



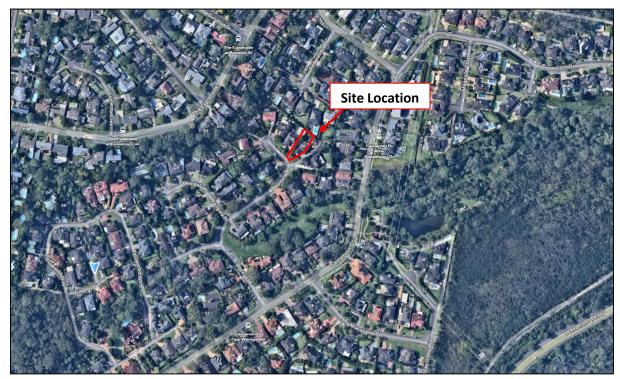


Image 3 - General Site Location



3. Fieldwork

3.1. Drilling Investigation

Fieldwork was undertaken on 7th March 2025 under the full-time supervision of a Geotechnical Engineer from AGS, and it included subsurface investigations at 2 select locations. The investigation comprised 1 mechanical borehole advanced with a 4.1T Pixy Drilling Rig and 1 manual borehole advanced with a hand auger. DCP testing was conducted at each borehole location prior to drilling works to ascertain the bearing capacity of the subsurface soils. The Pixy 4.1T drilling rig was supplied and operated by Precise Drilling Pty Ltd for mechanical borehole drilling and in-situ sample collection.

Proposed borehole locations were confirmed by the client prior to AGS's site visit. AGS was commissioned by the client to undertake a total of 2 boreholes. The proposed boreholes were advanced to practical refusal, with a maximum refusal depth of 1.2m below ground level (bgl).

A retrieved soil sample was collected at the natural profile for the purpose of Atterberg Limits testing, in accordance with AS1289.3.3.1, to ascertain the required geotechnical parameters for subsurface soils' physical properties. Laboratory test results are presented in Appendix D and outlined in Table 3 in section 4.1.

A total of 2 DCP tests were performed during the investigation for the assessment of shallow soil Allowable Bearing Pressure (ABP). DCP test results are summarised in Table 2 of Section 3.3.

AGS did not encounter buried metallic services or utilities during the drilling investigation. Details of underlying soil profiles and descriptions are outlined within the attached borehole logs (Appendix B).

3.2. Soil Profiles

The subsurface conditions observed on-site are summarised in Table 1. For a detailed description, refer to the attached borehole logs and explanatory notes.

Table 1 - Subsurface Soil Profile

Borehole	Borehole depth (m)	Termination /Refusal	Fill¹ (m)	Colluvial ² (m)	Bedrock ³ (m)	
BH01	1.2	Refusal	0.00 - 0.95	-	0.95 – 1.20	
BH02	1.2	Refusal	0.00 - 0.15	0.15 – 1.15	1.15 – 1.20	

NOTE: 1 Soil Horizon Unit 1- FILL: Sandy CLAY, brown-grey, medium plasticity, with fine to medium-grained sand.

 $^{^2}$ Soil Horizon Unit 2 — Colluvial: Clayey SAND, grey, low plasticity, fine to coarse grained sand.

³ Soil Horizon Unit 3 – BEDROCK: SANDSTONE, pale grey, white-brown, extremely weathered, extremely low strength, fine to medium-grained sand, trace fine subrounded gravel fragments.



3.3. Field Dynamic Cone Penetrometer Testing

Field testing using DCP was undertaken within each of the drilled borehole locations for the assessment of shallow soil ABP. The DCP test results are presented in Appendix C and summarised in Table 2 below.

Table 2 - DCP Test Result Summary

	Table 2 Del Test Resalt Sammar						
Test Number:	DCP-1	DCP-2					
Material Description (Soil Horizon):	1	2					
Test Method:	AS1289.6.3.2						
Test Location:	BH01	BH02					
Depth Tested (m):	Blows Pe	r/150mm					
0.00 - 0.15	6	1					
0.15 - 0.30	5	4					
0.30 - 0.45	4	5					
0.45 - 0.60	6	5					
0.60 - 0.75	7	6					
0.75 – 0.90	9	4					
0.90 – 1.05	25/90 - Refusal	25/30 - Refusal					



4. Laboratory Testing

4.1. Atterberg Limits

Laboratory Atterberg Limits tests were conducted in accordance with AS1289.3.3.1 in AGS's NATA accredited laboratory. The Atterberg Limits sample was collected from BH01 at a depth of 0.4m below existing ground level. The Plastic Index Test Report is attached within Appendix D, and the results of the laboratory testing are summarised in Table 3 below.

Table 3 - Atterberg Limits Test Summary

Borehole	Sampling Depth (m)	LL (%)	PL (%)	PI (%)	Material Type
BH01	0.4 – 0.6	35	18	17	Sandy CLAY

Notes: LL: Liquid Limit PL: Plastic Limit PI: Plastic Index



5. Recommendations

5.1. Groundwater Considerations

Groundwater seepage was encountered during AGS's site investigation. Water ingress was encountered at a depth of 0.9m for BH02 and was not encountered for BH01. The proposed footing design was not provided at the time of this report compilation. If additional footings are required, it is anticipated that groundwater seepage may be encountered. Dewatering or inflow redirection procedures may be required should construction activities encounter any groundwater. It is recommended that any potential inflow be controlled by the sump pumping method.

5.2. Site Classification

As part of the requested scope from the client, AGS was commissioned to determine a suitable Site Classification in accordance with AS2870-2011 "Residential Slabs and Footings". Based on analytical laboratory test results, assessed site conditions, and soil profiles, this site may be classified as follows:

Table 4 – Site Classification Summary

Residential Address	Site Classification
16 Noorong Ave, Frenchs Forrest NSW	Class P

This site classification is described as inadequate bearing strength or where ground movement may be significantly affected by factors other than reactive soil movements due to moisture conditions. **Class P** sites include soft or unstable foundations such as soft clays or silt or loose sands, landslip, mine subsidence, collapsing soils and soils subjected to erosion, reactive sites subject to abnormal moisture conditions. The classification of a site with uncontrolled fill not more than 0.8m deep for sand and not more than 0.4m deep for material other than sand shall be Class P, unless all footings are founded on natural soil through the filling.

5.3. Preliminary Bearing Pressure Assessment

Based on onsite DCP testing, it is anticipated that the Proposed Residential Development is to be founded on 2 soil profiles comprising the shallow colluvial profile (Soil Horizon Unit 2) and shallow fill layer (Soil Horizon Unit 1). AGS's drilling investigation indicated that the subject colluvial soil profile consisted of medium-dense sands to a depth of 1.15m and fill material to a depth of 0.95m bgl.

Shallow bedrock strata was encountered in each borehole (BH01 & BH02) locations. It can be anticipated that 800 kPa can be achievable at the encountered bedrock strata (Soil Horizon 3 – Table 1).

Due to the varying depth of the sandstone bedrock profile, it is important to note that all foundations must be founded on consistent material of similar quality and strength (i.e. similar Allowable Bearing Pressure) to eliminate differential settlement due to variations in bearing material. Foundation strata is to be inspected and verified by an experienced and qualified geotechnical engineer post-excavation (HOLD POINT).



For the design and construction of footings, it is also recommended that:

- The base of the excavation is to be cleaned so that no soft, loose, or wet soils are present;
- Before pouring concrete, an experienced geotechnical engineer to inspect the excavation strata and confirm the allowable bearing pressures (HOLD POINT).

Table 5 – Anticipated Allowable Bearing Pressure

Material Description	Allowable Bearing Pressure (kPa)
Clayey SAND	150
Sandstone Class V	800
Sandstone Class IV	1000

5.4. Slope Stability

As indicated by the Northern Beaches Council, the subject site is of landslip potential and must therefore undergo landslip prevention which may arise from the proposed construction. The appraisal is relevant to the proposed development, which is the subject of the current application before the Northern Beaches Council. No historical data, including documents or investigatory findings, were provided regarding the local landslide history at the time of this report.

The subject site has been identified to be within an area where there are risks that slope instability may occur. It was found that the subject site is situated within 'Area B' of the Warringah Landslip Risk Map. It is important to recognise that soil and rock movements are an ongoing geological process, which may be affected by the development and landslip management within the site or on adjoining land. Soil and rock movements may cause visible damage to structures even where the risk of slope failure is considered low. This report is intended to assess the risk of slope failure, apparent at the time of inspection. Our opinion is provided on the risk of slope instability for the land specifically referenced in the title of this report.

No evidence of former land instability was observed within the site and surrounding land during the site visit and walkover survey. Given the shallow depth of bedrock material and the undulating land, with grades in the order of 14° at the steepest sections, we consider the risk of landslides to be low. The need to carry out a further detailed slope risk assessment in accordance with Australian Geomechanics Society (AGS) Practical Note Guidelines for Landslide Risk Management (2007) was deemed necessary.

5.5. Risk Assessment

The assessment has been carried out by:

 Consideration of the likely slope failure mechanisms and likely initiating circumstances that could effect the elements at the site. The type or mode of landslide failure has also been classified.



 For this case, the potential consequences with respect to the proposed additions development have been considered. The current assessed probability of occurrence of each event has been estimated on a quantitative basis. The consequence and probability have been combined to determine the risk assessment.

Table 6 – AGS Risk Assessment (Risk to Property)

	Geotechnic	cal Hazards	Proposed Development Risk to Property								
Site Area	Hazard Type	Element(s) at	Witho	ut Risk Managen	nent	With Risk Management					
	пагаги туре	Risk	Likelihood	Consequence	Risk	Likelihood	Consequence	Risk			
	Landslides	House and surrounding land	Rare	Catastrophic	Modera te	Barely credible	Catastrophic	Low			
	Slumping	House and surrounding land	Rare	Major	Low	Barely credible	Medium	Very low			
16	Subsidence	House and land	Rare	Major	Low	Barely Medium credible		Very low			
Noorong Ave, Frenchs	Topping	House and land	Rare	Catastrophic	Modera te	Barely credible	Major	Very low			
Forest	Uncontrolled Water Flows	House and land	Possible	Insignificant	Very low	Unlikely	Insignificant	Very low			
	Ground vibrations from construction	House and land	Rare	Insignificant	Very low	Barely credible	Insignificant	Very low			
	Retaining wall failure	House and land	Rare	Minor	Very low	Barely credible	Minor	Very low			

As indicated in Table 6, the hazards identified on the site and adjoining land can be effectively managed to maintain a 'Low' or 'Very Low' level of 'risk to property' by following the risk management actions.

The AGS 2007 guidelines provide the following equation to be used for 'risk to life' calculations:

$$R_{(LoL)} = P_{(H)} \times P_{(S:H)} \times P_{(T:S)} \times V_{(D:T)}$$

Where:

 $R_{(LoL)}$ is the risk (annual probability of loss of life (death) of an individual).

 $P_{(H)}$ is the annual probability of the landslide.

P_(S:H) is the probability of spatial impact of the landslide impacting a building (location) taking into account the travel distance and travel direction given the event.

P_(T:S) is the temporal spatial probability (e.g. of the building or location being occupied by the individual) given the spatial impact and allowing for the possibility of evacuation given there is warning of the landslide occurrence.

 $V_{(D:T)}$ is the vulnerability of the individual (probability of loss of life of the individual given the impact).



Table 7 - Summary of Risk to Life Calculations

Hazard	Without Risk Management				With Risk Management					
nazaru	P (H)	P _(S:H)	P _(T:S)	V _(D:T)	R _(LoL)	Р(н)	P _(S:H)	P _(T:S)	V (D:T)	R _(LoL)
Landslides	1 x 10 ⁻³	0.2	0.75	1.0	1.5 x 10 ⁻⁴	1 x 10 ⁻⁴	0.2	0.75	0.5	7.5 x 10 ⁻⁶
Slumping	1 x 10 ⁻⁴	0.2	0.75	1.0	1.5 x 10 ⁻⁵	1 x 10 ⁻⁵	0.2	0.75	0.5	7.5 x 10 ⁻⁷
Toppling	1 x 10 ⁻⁴	0.2	0.75	0.05	7.5 x 10 ⁻⁷	1 x 10 ⁻⁵	0.2	0.75	0.05	7.5 x 10 ⁻⁸

Note: The values of the probability terms in Table 7 have been estimated for the site by engineering judgment, based on previous experience with risk assessment calculations, hillside building developments and landslide stabilisation works.

By reference to the risk assessment set out in Tables 6 and 7, provided that the proposed development is undertaken 'with risk management', AGS has determined that:

- The quantitative risk-to-life calculations meet the 'Acceptable' criteria in accordance with AGS 2007 (i.e. the probability of loss of life for the individual most at risk is less than 1×10^{-6} per annum).
- As the qualitative risk to property assessment for the proposed development indicates a
 'Low' to 'Very low' risk to property, it has been assessed that there is no significant risk
 to life associated with the development.

This outcome meets the 'acceptable risk level' for the proposed development in accordance with Warringah Development Control Plan, section E10 Landslip Risk and Australian Geomechanics Society Practical Note Guidelines for landslide Risk Management (2007).



6. Limitations

AGS has performed its services for this project in accordance with current industry codes and practices. The advice given in this report assumes that the test results are representative of the overall ground conditions. However, it should be noted that actual conditions in some parts of the site might differ from those found. If excavations reveal ground conditions significantly different from those shown in our findings, AGS must be consulted.

The scope and the period of AGS services are described in the report and are subject to restrictions and limitations. AGS did not perform a complete assessment of all possible conditions or circumstances that may exist at the Site. If a service is not expressly indicated, do not assume it has been provided. If a matter is not addressed, do not assume that any determination has been made by AGS regarding it.

Where data has been supplied by the client or a third party, it is assumed that the information is correct unless otherwise stated. No responsibility is accepted by AGS for incomplete or inaccurate data supplied by others. Any drawings or figures presented in this report should be considered only as pictorial evidence of our work. Therefore, unless otherwise stated, any dimensions should not be used for accurate calculations or dimensioning.



7. References

- Geological Series Map of the Penrith region, scale 1:100,000
- Pells, P.J., Douglas, D.J., Rodway, B, Thorne, C. And Mcmahon, B.K "Design Loadings for Foundations on Shale and Sandstone in the Sydney Region". Australian Geomechanics Journal, Vol.3 1978.
- AS1289 Methods of testing soils for engineering purposes









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Project Name:

Proposed Residential Development

Project Address:

16 Noorong Ave, Frenchs Forest NSW 2086

Report No:

G11624-1

Figure Date:

14/03/25

Figure No:

Figure 1

Approximate Borehole & DCP Locations

Figure Title:



Figure 2.1 – General View of Drilling Rig (BH01)



Figure 2.2 – General View of Subsurface Material (BH01)



Figure 2.3 – General View of Subsurface Material (BH01)



Figure 2.4 – General View of Encountered Bedrock Material at BH01



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Project Name: Report No: **Proposed Residential Development** G11624-1 Project Address: Figure Date: 16 Noorong Ave, Frenchs Forest NSW 2086 14/03/2025

Figure No:

Figure Title:

Figure 2

Site Photographs



Figure 3.1 – View of Encountered Topsoil Material (BH02)



Figure 3.3 – View of Colluvial Clayey Sand (BH02)



Figure 3.2 – View of Encountered Silty Clay Fill Material (BH02)

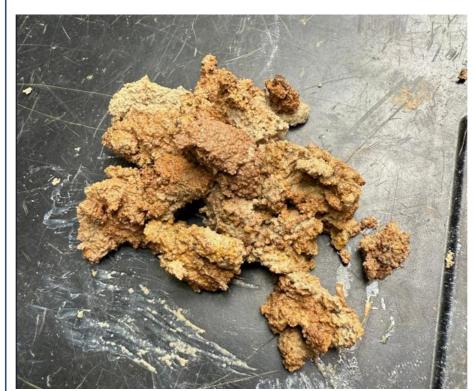


Figure 3.4 - View of In-situ Bedrock Material (BH02)



Figure 3.5 – Closeup View of Augered Material at BH02 Location



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Project Name: Report No: **Proposed Residential Development** G11624-1 Figure Date: Project Address: 16 Noorong Ave, Frenchs Forest NSW 2086 14/03/2025 Figure No:

Figure Title:

Figure 3

Site Photographs







Atlas Geotechnical Services Pty Ltd

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E: info@atlasgeoservice.com.au W: www.atlasgeoservice.com.au BH No: BH01 Sheet: 1 of 1 Job No:G11624-1

Borehole Log

BOREHOLE / TEST PIT SAMPLE LOG.GPJ GINT STD AUSTRALIA.GDT 24/3/25

88.0

2.0

Client: Sydney Extensions & Designs Pty Ltd Started: 7/3/25 Project: Proposed Residential Development Finished: 7/3/25 Location: 16 Noorang Ave, Frenchs Forest NSW 2086 Borehole Size: 110mm Rig Type: 4.1T Pixy Hole Location: Figure 1 Logged: BG Driller: Precise Drilling RL Surface (m): 89.99 Contractor: AGS Checked: PC Bearing: ---Classification Symbol Samples Graphic Log Material Description Additional Observations Tests Remarks RI Depth (m) Clayey SAND, white - yellow, low plasticity, fine to medium grained sand, trace rootlets, appears well compacted $\,$ Sandy CLAY, brown - grey, low plasticity, with fine to medium grained sand, trace rootlets, appears well compacted М FILL No Ground Water Table 89.5 S-2349A SANDSTONE, pale grey, high strength, extremely weathered, fine grained BEDROCK 89.0 Borehole BH01 terminated at 1.2m BH01 Refusal at 1.2m 88.5 1.5



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E: info@atlasgeoservice.com.au W: www.atlasgeoservice.com.au BH No: BH02 Sheet: 1 of 1 Job No:G11624-1

Borehole Log

Client: Sydney Extensions & Designs Pty Ltd

Project: Proposed Residential Development

Finished: 7/3/25

Location: 16 Noorang Ave, Frenchs Forest NSW 2086

Borehole Size: 110mm

. –					renchs Forest NSW 2086					: 110mm
Rig Ty			_		Hole Location: Figure 1	Driller: Atlas Geotechnical Logged: BG				
RL Sur	rface	(m):	90.17		Contractor: AGS	Bearing:	Check	ed:	PC	
Method	RL (m)		Graphic Log	Classification Symbol	Material Description		Samples Tests Remarks	Moisture Condition	Consistency/ Density Index	Additional Observation
₹ E					Sitly Clay, brown, low plasticity, trace rootlets, appea	rs well compacted		М		FILL
	90.0				Sandy CLAY, grey - brown, low to medium plasticity, sands, trace fine subrounded gravels, trace rootlets	fine to medium grained		M	St	COLLUVIAL
Water Seepege at 0.9m	89.5	0.4	5		Clayey SAND, white - grey, low plasticity, fine to me rootlets	dium grained sand, trace		M	MD	
	89.0	1.0	<u> </u>		From 0.9m - Water seepage observed SANDSTONE, white - brown, high strength, extreme	ly weathered, fine to medium		W	MD	Water Seepage — — — — — — — — — — — — — — — — — — —
		+			grained sands, trace fine subrounded sandstone frag Borehole BH02 terminated at 1.2m	gments				BH02 Refusal at 1.2m
	88.5	1. <u>:</u>	55							



EXPLANATORY NOTES – DRILL & EXCAVATION LOGS / SOIL & ROCK DESCRIPTION

GENERAL

Information obtained from site investigation and testing carried out by Atlas Geotechnical Services Pty Ltd (AGS) is recorded on log sheets. Data logged onto "Auger Borehole Logging Sheet" represents the data collected on rock and non-rock material using an auger supervised by an AGS representative. All material description and classification are based on SAA Site Investigation Code AS 1726 – 1993. All rock testing and calculations are in conjunction with AS 4133.4.1 – 2007 accredited by NATA.

DRILLING

Drilling & Casting

_					
WB	Wash- <mark>bore drillin</mark> g				
RR	Rock Roller				
HMLC	HMCL core barrel				
NMLC	NMLC core barrel				
AS	Auger Screwing				
AD/V	Auger Drilling with V-Bit				
AD/T	Auger Drilling with TC-Bit				
HQ	HQ co <mark>re</mark> barrel				
NQ	NQ core barrel				

Drilling Penetration/Drill Depth

Core loss is calculated as core loss per run as a percentage with line and depth identifying core lifts. The following is an abbreviation of the ease of penetration in none-core drilling.

VE	Very Easy
E	Easy
F	Firm
Н	Hard
VH	Very Hard

Samples/Tests

HP	Hand Penetrometer Test		
PBT	Plate Bearing Test		
VS	Vane Shear Test		
SPT	Standard Penetration Test		
PZ	Piezometer Installation		
IMP	Borehole Impression		
	Device		
D	Disturbed		
U	Und isturbed		
С	Core Sample		
N	Results of SPT (*sample		
	taken)		

SOIL DESCRIPTION

Material Description - In accordance with AS 1726-1993, Appendix A2.3

Types of Soil

Observation of the primary, secondary, and minor soil components are used to construct the soil name, which describes the composition of the soil. AS 1726:2017. The following table provides particle size definition.

Fraction	Components
Oversize	BOULDERS
	COBBLES
Coarse Grained Soil	GRAVEL
	SAND
Fine Grained Soil	SILT
	CLAY

The following sizes correspond to the approximate sieve sizes.

Components	Subdivision	Size (mm)
BOULDER		<200
COBBLES		63-200
GRAVEL	Coarse	19-63
	Medium	6.7-19
	Fine	2.36-6.7
SAND	Coarse	0.6-2.36
	Medium	0.21-0.6
	Fine	0.075-0.21
SILT		0.002-0.075
CLAY		<0.002

Moisture Condition

D	Dry, look and f <mark>eel dr</mark> y	
M	Moist, no free water on	
	remoulding	
W	Wet, free water on	
	remouldi <mark>ng</mark>	

Consistency

In accordance with AS 1726-1993, Appendix A2.5

VS	Very Soft	<25kPa
S	Soft	25 – 50kPa
F	Firm	50 – 100kPa
St	Stiff	100 – 200kPa
VSt	Very Stiff	200 – 400kPa
Н	Hard	≥ 400kPa

The Unconfined Compressive Strength range of each class is defined by the approximate strength figures quoted.

Density Index

% is estimated or is based on SPT results. N Value correlation is approximated and is shown in the right column.

-			
VL	Very	< 15%	0 – 4
	Lose		
L	Loose	15 –	4 – 10
		35%	
MD	Medium	35 –	10 – 30
	Dense	36%	
D	Dense	36 –	30 – 50
		85%	

VD	Very	>85%	>50
	Dense		

MATERIAL DESCRIPTION - ROCK Material Description

Rock type, texture and composition are identified based on visual features in accordance with AS 1726-1993, Appendix A3.1-A3.3 and Table A6a, A6b and A7.

Core Loss

Core Loss location will be indicated.

Bedding

Description	Spacing (mm)
Thinly Laminated	< 6
Laminated	6 – 20
Very Thine Bedded	20 – 60
Thinly Bedded	60 – 200
Medium Bedded	200 – 600
Thickly Bedded	600 – 2000
Very Thickly Bedded	>2000

<u>Weathering</u> – Weathering classification are only means of identification and do not contribute to engineering properties.

Fresh (F)	Rock substance unaffected	
	by weathering.	
Slightly	Rock substance partly	
Weathered	stained or discoloured.	
(SW)	Colour and texture of fresh	
	rock recognisable.	
Moderately	Staining or discolouration	
Weathered	extends throughout rock	
(MW)	substance. Fresh rock	
	colour not recognisable.	
Highly	Stained or discoloured	
Weathered	throughout. Signs of	
(HW)	chemical or physical	
	alteration. Rock texture	
	retained.	
Extremely	Rock texture evident but	
Weathered	material has soil properties	
(EW)	and can be remoulded.	

Strength

Rock strength is defined using the following terms:

Rock Strength Class	Abbreviation	Point Load Strength Index, I _{s(50)} (MPa)
Extremely	EL	< 0.03
Low		
Very Low	VL	0.03 to 0.1
Low	L	0.1 to 0.3
Medium	M	0.3 to 1
High	Н	1 to 3
Very High	VH	3 to 10
Extremely	EH	≥ 10
High		

Strengths are supported by Point Load Index Testing of representative samples.

Materials Structure/Fractures Rock

Natural Fracture Spacing – the following classification identifies the spacing of natural fractures such as bedding parting or joints but excludes handling breaks or mechanical breaks.

Visual Log – A diagram in relation to core axis plot of defects showing type, spacing and orientation.

Defects	Breaks through rock
	substance
	Defects closed in-
	 situ
	Defects open in-situ
	or clay sealed

Additional Data – Description of individual defects based on visual observation by type, orientation, in-filling, shape, and roughness in accordance with AS 1726-1993, Appendix A Table A10, notes and Figure A2.

Туре	BP	Bedding Parting
	DL	Drill Lift
	НВ	Handling Break

DB	Drilling Break
JT	Joint
SM	Seam
FZ	Fracture Zone
SZ	Shear Zone
VN	Vein
FL	Foliation
CL	Cleavage

Orientation – Angle relative to the plane normal to the core axis.

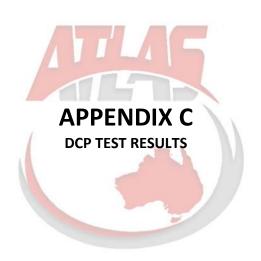
Infilling	CN	Clean
	Clay	Clay
	Fe	Iron Oxide
	Qz	Quarts
	MS	Secondary
		Mineral
	MU	Unidentified
		Mineral
	KT	Chlorite
	CA	Calcite
	X	Carbonaceous
Chana	IR	Irrogular
Shape	IK	Irregular
Snape	ST	stepped
Snape		
Snape	ST	stepped
Snape	ST DIS	stepped Discontinuous
Snape	ST DIS PR	stepped Discontinuous Planar
Roughness	ST DIS PR CU	stepped Discontinuous Planar curved
·	ST DIS PR CU UN	stepped Discontinuous Planar curved Undulose
·	ST DIS PR CU UN VR	stepped Discontinuous Planar curved Undulose Very Rough
·	ST DIS PR CU UN VR RF	stepped Discontinuous Planar curved Undulose Very Rough Rough

Soil

Structures – Fissuring and other defects are described in accordance with AS 1726-1993, Appendix A2.6, using terminology for rock defects.

Origin – Where applicable and assessment is provided of the probable origin of the soil, eg. Fill, topsoil, alluvium, colluvium, residual soil







Atlas Geotechnical Services

Construction Material Testing

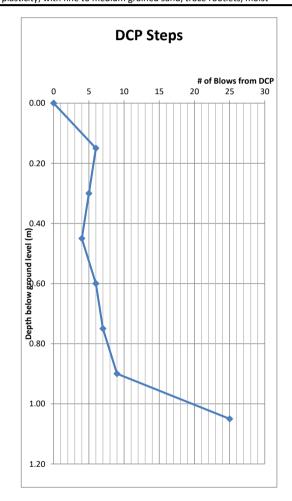
49/93-97 Newton Road Wetherill Park NSW 2164 02 87400494

PENETRATION RESISTANCE REPORT

Client: Report No.: Sydney Extensions & Design Pty Ltd G11624-1 14/03/2025 Project: Proposed Residential Development Date: Location: 16 Noorong Ave, Frenchs Forest NSW 2086 Tested By: BG

DCP No.:	DCP-1	Equipment ID: Kit-2	Date Tested:	7/06/2025
Test Procedure:	>	AS1289.6.3.2 - 1997- Dynamic Cone Penetration Test	AS1289.6.3.3 - 1997 -	Perth Sand Penetrometer Test
Test Location:		Figure 1	✓ Drop Height Check	✓ Tip Sharpness
Soil Description with Moisture:		Sandy CLAY, brown-grey, low plasticity,	with fine to medium gr	ained sand, trace rootlets, moist

Depth (m)	Test RL (m)	Ground water level bgl (m)	# of Blows	Total number of Blows	Penetration Rate	Penetration Resistance Np /300 mm
0.00			0	0	0.00	-
0.15			6	6	25.00	6
0.30			5	11	30.00	11
0.45			4	15	37.50	9
0.60			6	21	25.00	10
0.75			7	28	21.43	13
0.90			9	37	16.67	16
1.05			25	62	6.00	34
		1				
		<u> </u>				
DCP Test:		Termination	1 .	/ efusal at	1.05	m



ABN: 67 626 182 349

* bgl - below ground level

Notes:

Accreditation No.: 20498

Accredited for compliance with ISO/IEC 17025 -Testing.

The results of the tests, calibrations and/or measurements included in this document are traceable to Australian/National standards.

WORLD RECOGNISED ACCREDITATION

Authorised Signatory:

Teterchem

Peter Chen (Xiao Dong Chen) Director | Geotechnical Engineer Adv. Dip. Structural Eng, B. Eng (Civil), MIEAust

Atlas Geotechnical Services Pty Ltd

Tests not covered by NATA are denoted with *.



Atlas Geotechnical Services

Construction Material Testing

49/93-97 Newton Road Wetherill Park NSW 2164 02 87400494

PENETRATION RESISTANCE REPORT

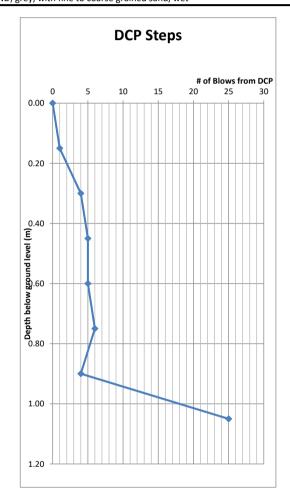
 Client:
 Sydney Extensions & Design Pty Ltd
 Report No.:
 G11624-1

 Project:
 Proposed Residential Development
 Date:
 14/03/2025

 Location:
 16 Noorong Ave, Frenchs Forest NSW 2086
 Tested By:
 BG

DCP No.:	DCP-2	Equipment ID: Kit-2	Date Tested:	7/06/2025
Test Procedure:	<	AS1289.6.3.2 - 1997- Dynamic Cone Penetration Test	AS1289.6.3.3 - 1997 -	Perth Sand Penetrometer Test
Test Location:		Figure 1	✓ Drop Height Check	✓ Tip Sharpness
Soil Description with Moisture:		Clavey SAND, grey, w	ith fine to coarse grain	ed sand, wet

Depth (m)	Test RL (m)	Ground water level bgl (m)	# of Blows	Total number of Blows	Penetration Rate	Penetration Resistance Np /300 mm
0.00			0	0	0.00	-
0.15			1	1	150.00	1
0.30			4	5	37.50	5
0.45			5	10	30.00	9
0.60			5	15	30.00	10
0.75			6	21	25.00	11
0.90			4	25	37.50	10
1.05			25	50	6.00	29
	1					
	1					
DCP Test:		Termination	1	/ efusal at	1.05	m



ABN: 67 626 182 349

Notes:

Accreditation No.: 20498

Accredited for compliance with ISO/IEC 17025 -Testing.

The results of the tests, calibrations and/or measurements included in this document are traceable to Australian/National standards.

NATA
WORLD RECOGNISED
ACCREDITATION

Authorised Signatory:

Teterchem

Peter Chen (Xiao Dong Chen) Director | Geotechnical Engineer Adv. Dip. Structural Eng, B. Eng (Civil), MIEAust

Atlas Geotechnical Services Pty Ltd

Tests not covered by NATA are denoted with *.

^{*} bgl - below ground level





Material Test Report

Report Number: T11624-1

Issue Number:

Date Issued: 12/03/2025

Client: Sydney Extensions and Designs

731 Warringah Rd, Forestville NSW 2087

Contact: Mustafa Varol

Project Number: T11624

Project Name: 16 Noorong Avenue, French Forest
Project Location: 16 Noorong Avenue, French Forest

 Work Request:
 2349

 Sample Number:
 S-2349A

 Date Sampled:
 10/03/2025

Dates Tested: 10/03/2025 - 12/03/2025

Sample Location: 0.4-0.6m

Atterberg Limit (AS1289 3.1.1 & 3.2	2.1 & 3.3.1)	Min	Max
Sample History	Oven Dried / Air Dried / Natural / Unknown		
Preparation Method			
Liquid Limit (%)	35		
Plastic Limit (%)	18		
Plasticity Index (%)	17		



On Budget. On Time. Professional Service

Atlas Geotechnical Services Pty Ltd

49/93-97 Newton Road Wetherill Park NSW 2164 Phone: 0426267115

Email: mh@atlasgeoservice.com.au

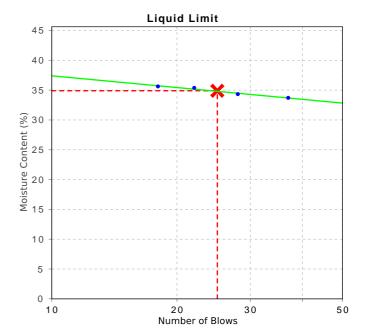
Accredited for compliance with ISO/IEC 17025 - Testing



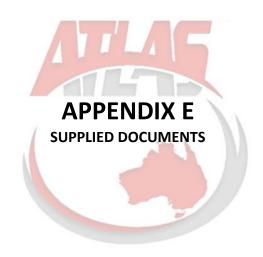
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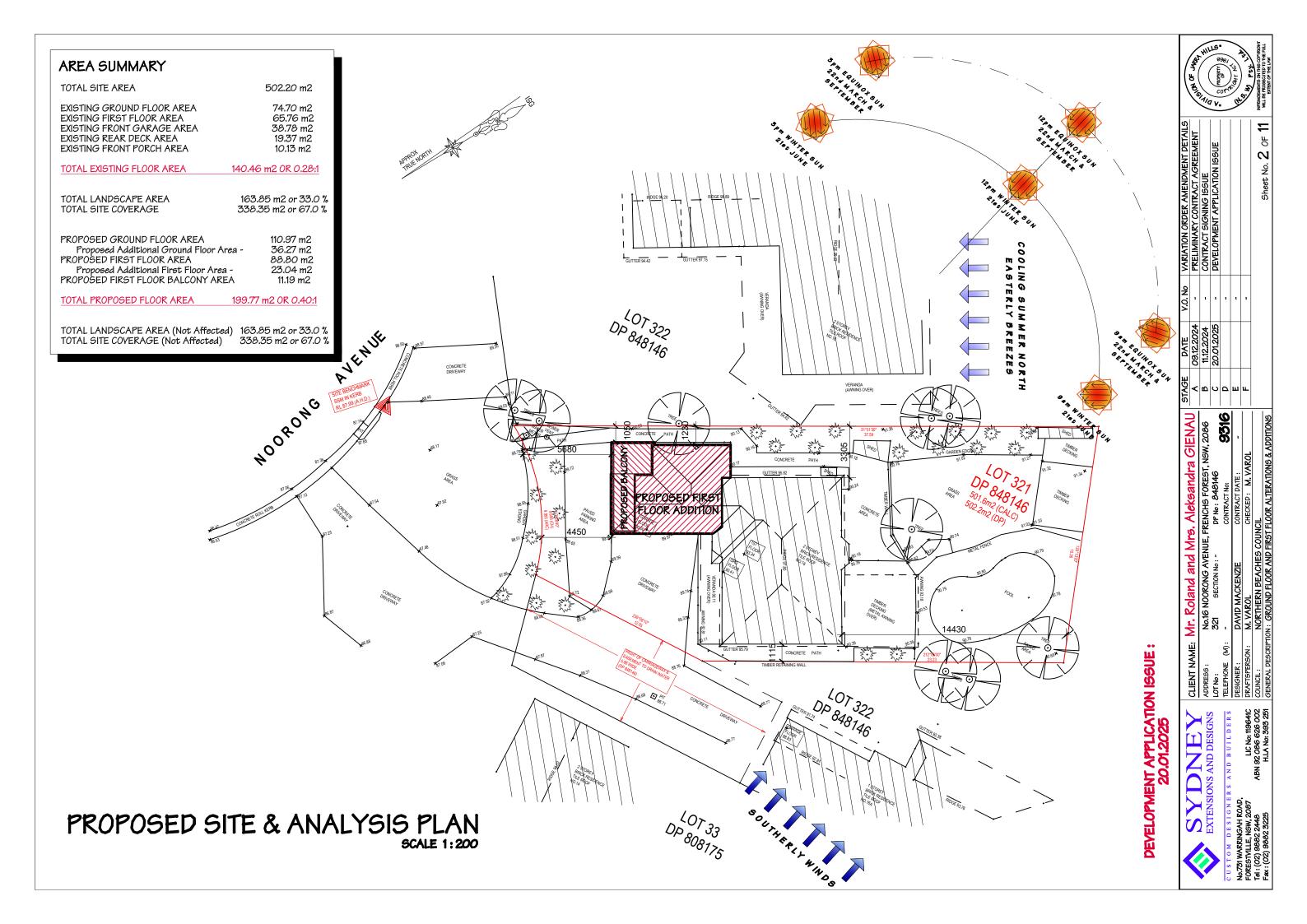
Approved Signatory: Lucas Andrade

NATA Accredited Laboratory Number: 20498









NOTES: (For Council)

- (a) Framing Timbers are to comply with A.S. 1684 (2006).
- (b) Sub Floor Ventilation:
- 150mm minimum bearer to around clearance or 200mm minimum for particleboard.
- ii) Sub floor vents minimum 6000mm/m (External Wall).
- iii) Weep holes at 1200mm maximum centres to comply with A.S. 1640
- (c) Reinforcement:
- (i) Trench mesh for concrete footings lapped minimum 500mm with 50mm minimum cover.
 (ii) Fabric mesh to be lapped minimum 225mm with 25mm minimum cover.
 (d) Brickwork is to comply with A.S. 3700 Flashing as per AS / NZS 2904.
- (e) Wet Areas (refer to Specification for responsible party). Provide impervious floor and wall coverings to all wet areas to AS 3740 (2004).

 (f) Concrete strength to be minimum 20 Mpa at 28 days per A.S. 1480.
- (g) Steps / Stairs: Min Tread
 - 240mm Max Riser Height - 190mm
 - Min Head Clearance - 2000mm. (Recommended)
- Min Width of Stairway
 Min. Handrail Height
 750mm clear of all obstructions eg. handrails, balustrades etc.
 865mm above tread nosing. 1020mm above landing, ramps etc.

 (h) Insulation is to comply with clause 3.12.1.1 (d) & VIC amendment 1.2 of the Building Code
- of Australia.

1. TREES:

Some species of tree in certain soils, (pending size & distance from the building) could in particular circumstances affect the structural integrity of the proposed work. It is suggested that the Client, if concerned, organise an arborist to advise them in this regard. The Builder can, upon written request, organise this report and carry out any recommendations.

2. MATERIALS:

Items specified to match existing as close as possible, does not imply an identical match. The contract allows for materials available from current manufacturer's standard lines (no special tooling allowed for).

3. CHECKS BY CLIENT:

The Client is required to check the dimensions of the work (with the assistance of the Construction Supervisor) at:

- Floor Stage,
- Frame Stage,

This includes levels, heights, widths, window locations, door locations, external and internal wall locations, brick & roof tile selections. Any discrepancies found are to be advised in writing to the Building Manager within three (3) days of the check. The allowance by the Client for the works to continue will be deemed to be acceptance by the Client that the preceding works satisfy the Clients requirements and the Builder's obligations.

4. ASBESTOS:

Unless otherwise specified in these Specifications NO allowance has been made for the removal or handling of any asbestos products. The Builder, prior to commencement of works, will take a sample of all materials in areas affected by new works and have the existing linings tested for the presence of asbestos. If asbestos is found a quote for the safe handling and removal of this material from affected areas will be obtained and this work included in this contract by way of Variation Order at additional cost to Client. Alternatively it will be the Clients responsibility to remove asbestos from affected areas and provide a clearance certificate' for following works to occur.

5. EXISTING STRUCTURE:

Where the Builder is carrying out works in an existing area (e.g. re-tile / re-line floors walls etc.) unless the Client notifies the Builder otherwise the Builder will assume that the Client accepts the existing condition of the structure (ie. Level, plumb, square etc.). Should the Client desire it, the Builder will undertake to provide a detailed existing conditions report in any area of concern for a fee to be agreed upon. The request to undertake this report must be in writing and be received by the Builder in advance of any works being undertaken on site.

Where the Client is responsible for specified works to be carried out in conjunction with this contract, the Client shall be responsible for paying for and obtaining the necessary security deposit / approval as may be required by local council including obtaining a 'Certificate of Final Inspection' upon completion of works.

7. FRAME & LOCK UP ONLY PROJECTS:

The Client will be responsible for:

- obtaining a 'Certificate of Final Inspection' upon completion of works and
- paying for and obtaining any necessary security deposit / approval that may be required by local council.

Where works past frame or lock up stage are required by the Builder (eg. Plaster, Fixing, Fit off etc.) but do not involve completion of the contract, the Builder will claim an extension of time from completion of 'Frame' or 'Lock Up' stage till ten (10) days after receiving written confirmation from the Client that preceding works (eg. Plumbing & electrical rough in, plaster etc) has been completed.

8. COUNCIL CONDITIONS ON APPROVED DOCUMENTS:

The BUILDER has allowed for the standard fees associated with the application of a Development Application and Construction Certificate, unless otherwise specified no allowance has been made for additional works or fees required by Council as per the conditions included within the Development Application and/or Construction Certificate approvals. Any additional costs involved shall be charged to the Client by way of Variation Order.

BASIX (Building Sustainability Index) is a measure of thermal comfort, water saving measures and energy efficiency of a new building. As of 1st July 2007, residential additions and renovations over the value of \$50,000 will need to be assessed and comply with the requirements of BASIX. Works to this effect have been included in this contract, and will be provided for your selection

by way of Variation Order to Contract. The included basix compliance items are: A minimum of 40% of new or altered light fittings are to be fitted with fluorescent, compact fluorescent, or led lamps.

9.BASIX COMPLIANCE:

- All new or altered showerheads must have a flow rate no greater than 9 litres per minute or a 3 star water rating.
- All new or altered toilets have a flow rate no greater than 4 litres per average flush or a minimum
- 3 star water rating.
- All new or altered taps must have a flow rate no greater than 9 litres per minute or a minimum

10. TERMITES:

- CLIENT is advised that due to the nature of termites it is extremely difficult to prevent them gaining access to a building. It is therefore the
- CLIENTS responsibility to ensure regular maintenance and competent inspection of the building (new and existing) is carried out on an ongoing basis.

WINDOW SCHEDULE									
No.	SIZE (HxW)	DESCRIPTION	BASIX No.	GLAZING					
W1	700 x 900	ALUMINIUM AWNING WINDOW	W1	IMPROVED ALUMINIUM, SINGLE CLEAR, (U-VALUE: 6.44, SHGC: 0.75)					
W2	1500 x 2400	ALUMINIUM AWNING WINDOW	W2	IMPROVED ALUMINIUM, SINGLE CLEAR, (U-VALUE: 6.44, SHGC: 0.75)					
W3	1500 x 700	ALUMINIUM FIXED WINDOW	W3	IMPROVED ALUMINIUM, SINGLE CLEAR, (U-VALUE: 6.44, SHGC: 0.75)					
W4	900 x 600	ALUMINIUM AWNING WINDOW	W4	IMPROVED ALUMINIUM, SINGLE CLEAR, (U-VALUE: 6.44, SHGC: 0.75)					
W5 600 x 2420 ALUMINIUM FIXED WINDOW W6 IMPROYED ALUMINIUM, SINGLE CLEAR, (U-YALUE: 6.44, SHGC: 0.75) POOR SCHEDULE									
DI 2100 x 880 ALUMINIUM 820 HINGED DOOR DI IMPROVED ALUMINIUM, SINGLE PYTOLYTIC LOW-E, (U-YALUE: 4.48, SHGC: 0.46)									
DOOR SCHEDULE - CHECK ON-SITE BEFORE ORDERING									
D2	2100 x 2420	ALUMINIUM SLIDING DOOR	D2						

- * SIZES SHOWN ARE NOMINAL SIZES ONLY AND ARE SUBJECT TO MANUFACTURERS STANDARD SIZE AND PROFILE.
- * ALL GLAZING IS TO COMPLY WITH A.S 1288. (2006) AND A.S. 2047 (1999).
- * ALL NEW WINDOWS AND DOORS ARE TO BE CLEAR GLAZED UNLESS NOTED OTHERWISE.
- * ALL NEW WINDOWS TO FIRST FLOOR WINDOWS TO SIT AT 2.1m HEAD HEIGHT
- * BUILDER IS TO DRAFTSEAL ALL NEW WINDOWS
- * ALL NEW WINDOWS ARE TO BE POSITIONED CENTRE TO THE ROOM, UNLESS OTHERWISE NOTED
- st ALL FIRST FLOOR WINDOWS WITH A SILL HEIGHT OF LESS THAN 1.7m HIGH MUST BE RESTRICTED TO AN OPENING OF 125mm ONLY AS PER BCA STANDARDS

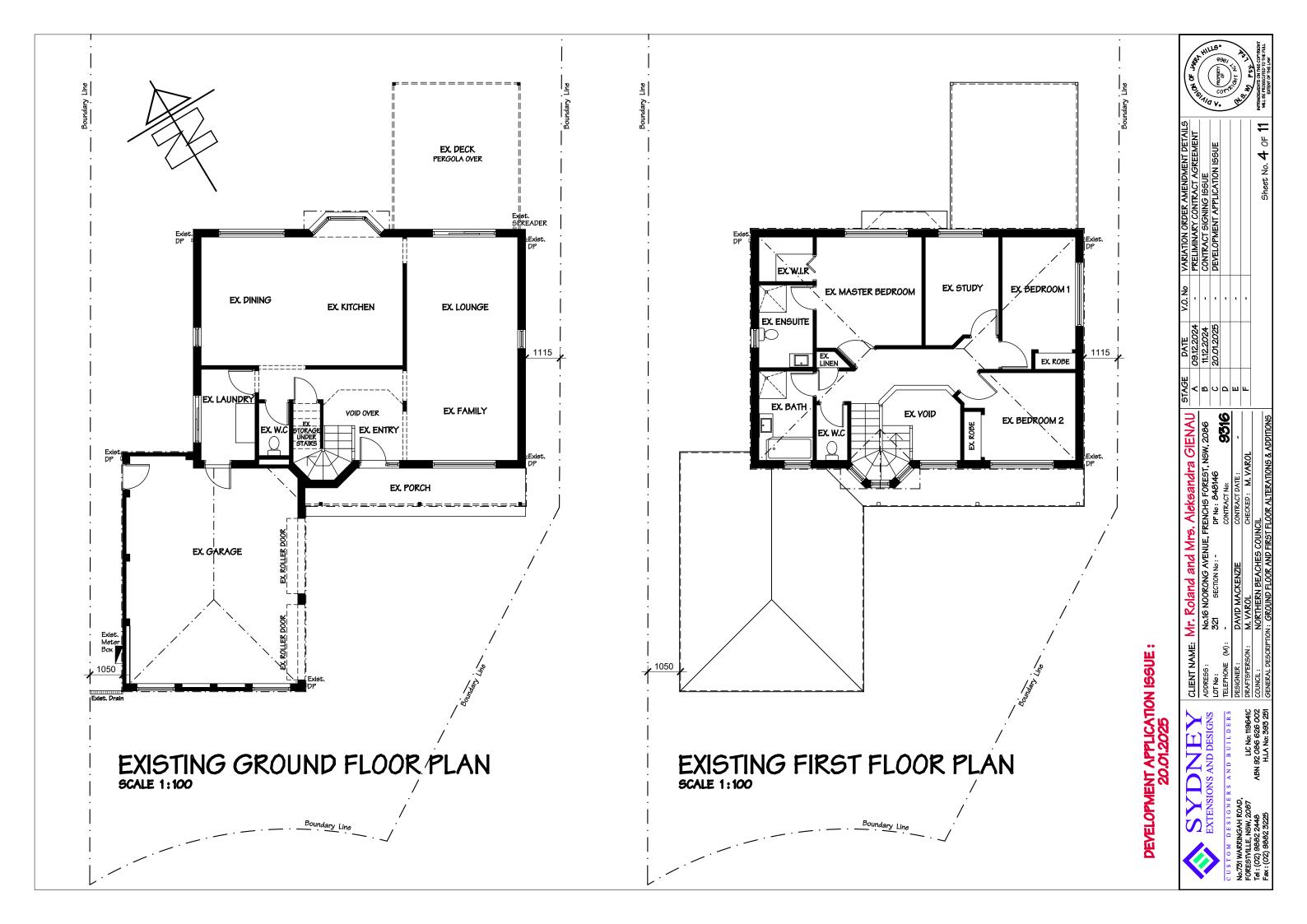
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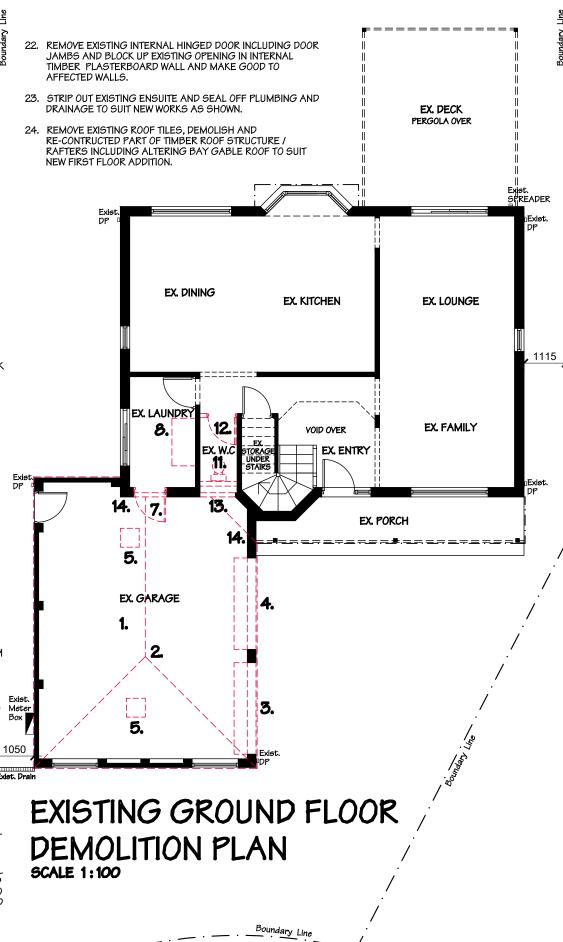
IG NOORONG AVENUE, FRENCHS
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BUILDER TO:

- RAISE / LEVEL EXISTING GARAGE FLOOR LEVEL TO SUIT NEW WORKS AND MAKE GOOD AFFECTED SURFACES AS 3
- DEMOLISH AND REMOVE COMPLETE EXISTING GARAGE TIMBER ROOF STRUCTURE / RAFTERS INCLUDING ROOF
- REMOVE EXISTING GARAGE ROLLER DOOR AND CREATE NEW OPENING TO SUIT 2100H x 2420W ALUMINIUM SLIDING DOOR IN DOUBLE BRICK WALL AND MAKE GOOD AFFECTED SURFACES AS NECESSARY.
- NOTE: OPENING WIDTH TO REMAIN AS IS. SITE CHECK EXACT EXISTING OPENING SIZE BEFORE ORDERING.
- REMOVE EXISTING GARAGE ROLLER DOOR AND CREATE NEW OPENING TO SUIT 600H x 2420W ALUMINIUM AWNING WINDOW AND BLOCK UP PART PART OF OPENING IN DOUBLE BRICK WALL AND MAKE GOOD AFFECTED SURFACES AS NECESSARY.
- NOTE: OPENING WIDTH TO REMAIN AS IS, SITE CHECK EXACT EXISTING OPENING SIZE BEFORE ORDERING.
- 5. REMOVE EXISTING MANHOLE IN GARAGE AND PATCH UP CEILING AND MAKE GOOD AFFECTED SURFACES AS
- 6. DELETED.
- REMOVE EXISTING INTERNAL HINGED DOOR INCLUDING DOOR JAMBS AND BLOCK UP OPENING IN INTERNAL BRICK VENEER WALL AND MAKE GOOD TO AFFECTED WALLS.
- 8. STRIP OUT EXISTING LAUNDRY AND SEAL OFF PLUMBING AND DRAINAGE TO SUIT NEW WORKS AS SHOWN.
- 9. DELETED.
- 10. DELETED.
- 11. STRIP OUT EXISTING GROUND FLOOR W.C AND SEAL OFF PLUMBING AND DRAINAGE TO SUIT NEW WORKS AS
- 12. DEMOLISH AND REMOVE EXISTING INTERNAL TIMBER PLASTERBOARD WALL INCLUDING REMOVAL OF HINGED DOOR AND DOOR JAMBS AND CREATE FULL HEIGHT X FULL WIDTH OPENING AND MAKE GOOD AFFECTED SURFACES AS NECESSARY.
- 13. DEMOLISH AND REMOVE EXISTING BRICK VENEER WALL AND CREATE FULL HEIGHT x FULL WIDTH OPENING AND MAKE GOOD AFFECTED SURFACES AS NECESSARY. BUILDER TO CHECK WALL VOID REGARDING PLUMBING PIPES TO BE ALTER TO SUIT NEW WORKS.
- RE-LINE EXISTING EXTERNAL BRICK WALLS WITH PLASTERBOARD TO NEW GROUND FLOOR GAME ROOM AND FIRST FLOOR BEDROOM 3 TO SUIT NEW WORKS
- 15. RE-LINE EXISTING EXTERNAL BRICK WALLS WITH 6mm VILLABOARD TO NEW FIRST FLOOR ENSUITE WALLS ΤΟ SUIT NEW WORKS AS SHOWN.
- 16. STRIP OUT EXISTING BATHROOM AND SEAL OFF PLUMBING AND DRAINAGE TO SUIT NEW WORKS AS
- 17. STRIP OUT EXISTING FIRST FLOOR W.C AND SEAL OFF PLUMBING AND DRAINAGE TO SUIT NEW WORKS AS
- 18. STRIP EXISTING FLOOR TILES TO FIRST FLOOR W.C, BATHROOM AND ENSUITE AND RELAY NEW SCYON SHEET FLOORING TO SUITE WORKS.
- 19. DEMOLISH AND REMOVE EXISTING FIRST FLOOR INTERNAL PLASTERBOARD TIMBER WALL BETWEEN BATHROOM AND W.C AND CREATE FULL HEIGHT X FULL WIDTH OPENING TO SUIT NEW WORKS AS SHOWN AND MAKE GOOD AFFECTED SURFACES AS NECESSARY.
- 20. CREATE A NEW OPENING IN EXTERNAL BRICK VENEER WALL TO SUIT NEW 900H x 600W ALUMINIUM AWNING WINDOW UNIT AND MAKE GOOD TO AFFECTED WALLS.
- REMOVE EXISTING WINDOW UNIT AND BLOCK UP OPENINĢ IN EXTERNAL BRICK VENEER WALL AND MAKE GOOD TO AFFECTED WALLS.

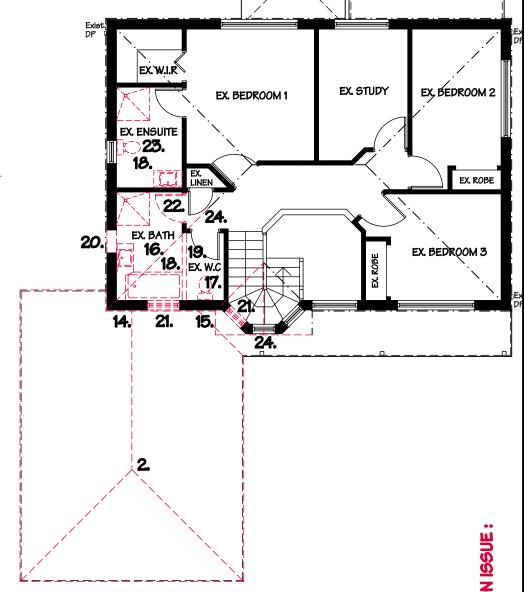




- FULLY PREPARE SITE PRIOR TO COMMENCEMENT OF WORKS BY BUILDER (IE. REMOVAL OF PLANTS, TREES, SHRUBS, FURNITURE, LIGHT FITTINGS, FLOOR COVERINGS, PERSONAL EFFECTS ETC.
- ARRANGE FOR THE DISCONNECTION, REMOVAL & RELOCATION OF ANY EXISTING TV ANTENNAS / SATELLITE DISHES THAT ARE IN THE WAY OF ANY NEW WORKS, IF
- B. ARRANGE FOR THE REMOVAL OF ALL EXISTING AC DUCTS REGISTERS AND AC UNIT THAT'S IN THE WAY OF NEW WORKS.

EXISTING FIRST FLOOR

DEMOLITION PLAN



32.1 32.1

BASIX Certificate

Alterations and Additions

Certificate number: A1784523

This certificate confirms that the proposed development will meet the NSW overment's requirements for sustainability, if it is built in accordance with the commitments set out below. Terms used in this certificate, or in the commitments, have the meaning given by the document entitled "BASIX Definitions" dated 10/09/2020 published by the Department. This document is available at

Secretary
Date of issue: Friday, 21 February 2025
To be valid, this certificate must be lodged within 3 months of the date of issue.



Project name	#9316 GIENAU
Street address	16 NOORONG Avenue FRENCHS FOREST 2086
Local Government Area	Northern Beaches Council
Plan type and number	Deposited Plan DP848146
Lot number	321
Section number	is .
Project type	
Dwelling type	Dwelling house (detached)
Type of alteration and addition	The estimated development cost for my renovation work is \$50,000 or more, and does not include a pool (and/or spa).
N/A	N/A
Certificate Prepared by	lease complete before submitting to Council or PCA)
Name / Company Name: JARRA HI	LLS (N.S.W.) PTY LTD

Fixtures and systems

Lighting

The applicant must ensure a minimum of 40% of new or altered light fixtures are fitted with fluorescent, compact fluorescent, or light-emitting-diode (LED) lamps.

The applicant must ensure new or altered showerheads have a flow rate no greater than 9 litres per minute or a 3 star water rating.

The applicant must ensure new or altered toilets have a flow rate no greater than 4 litres per average flush or a minimum 3 star water

The applicant must ensure new or altered taps have a flow rate no greater than 9 litres per minute or minimum 3 star water rating.

Construction

Insulation requirements

The applicant must construct the new or altered construction (floor(s), walls, and ceilings/roofs) in accordance with the specifications listed in the table below, except that a) additional insulation is not required where the area of new construction is less than 2m2, b) insulation specified is not required for parts of altered construction where insulation already exists.

Construction	Additional insulation required (R-value)	Other specifications
floor above existing dwelling or building.	nil	N/A
external wall: brick veneer	R1.16 (or R1.70 including construction)	
external wall: external insulated façade system (EIFS)(façade panel: 75 mm)	nil	
flat ceiling, pitched roof	ceiling: R2.50 (up), roof: foil/sarking	medium (solar absorptance 0.475 - 0.70)

Glazing requirements

Windows and glazed doors

The applicant must install the windows, glazed doors and shading devices, in accordance with the specifications listed in the table below. Relevant overshadowing specifications must be satisfied for each window and glazed door.

The following requirements must also be satisfied in relation to each window and glazed door:

Each window or glazed door with improved frames, or pyrolytic low-e glass, or clear/air gap/clear glazing, or toned/air gap/clear glazing must have a U-value and a Solar Heat Gain Coefficient (SHGC) no greater than that listed in the table below. Total system U-values and SHGCs must be calculated in accordance with National Fenestration Rating Council (NFRC) conditions. The description is provided for information only. Alternative systems with complying U-value and SHGC may be substituted.

For projections described in millimetres, the leading edge of each eave, pergola, verandah, balcony or awning must be no more than 500 mm above the head of the window or glazed door and no more than 2400 mm above the sill.

Pergolas with polycarbonate roof or similar translucent material must have a shading coefficient of less than 0.35.

Window/door Orientation Area of glass Overshadowing Overshadowing Shading

Pergolas with fixed battens must have battens parallel to the window or glazed door above which they are situated, unless the pergola also shades a perpendicular window. The spacing between battens must not be more than 50 mm.

Glazing requirements

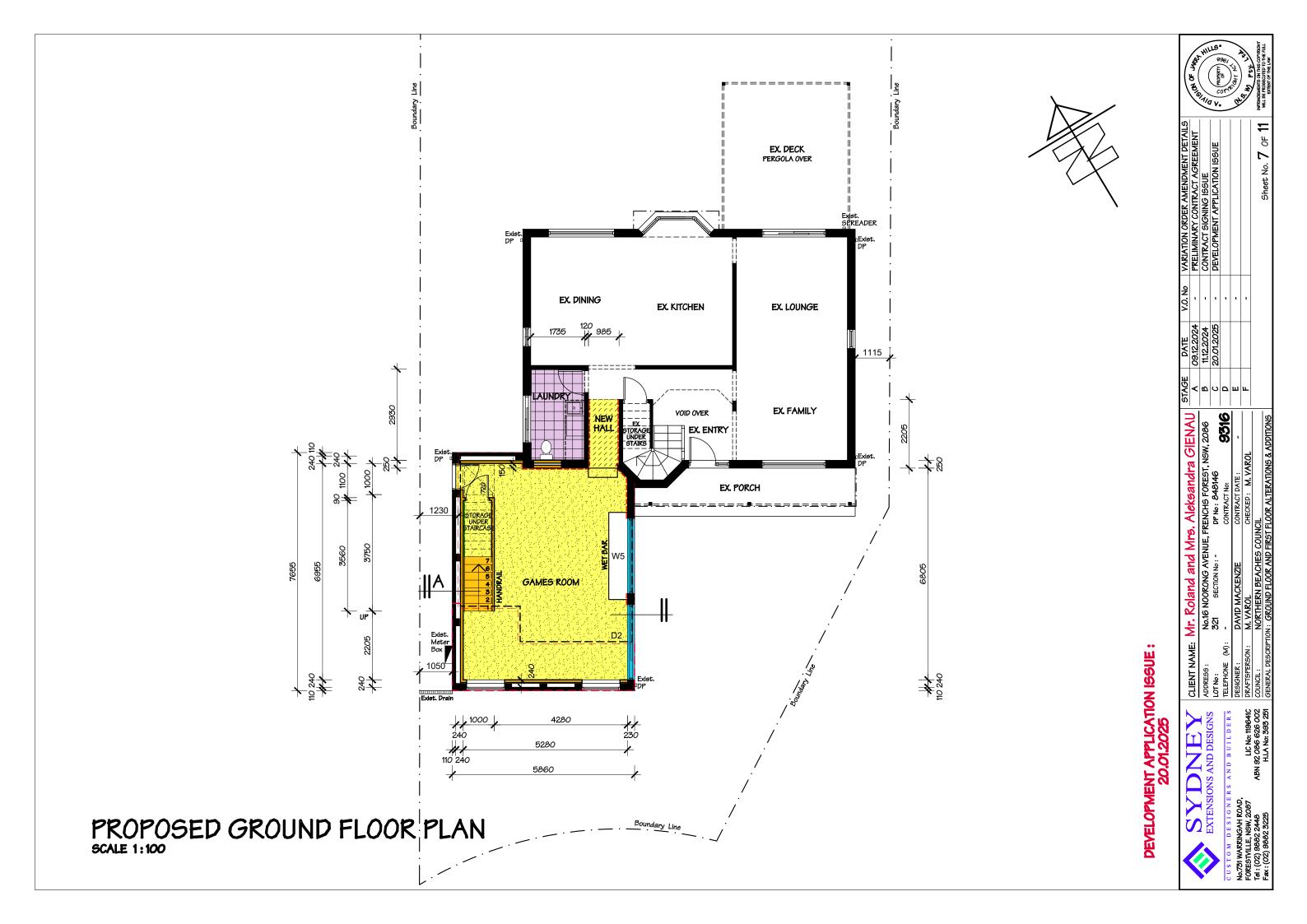
Windows and glazed doors glazing requirements

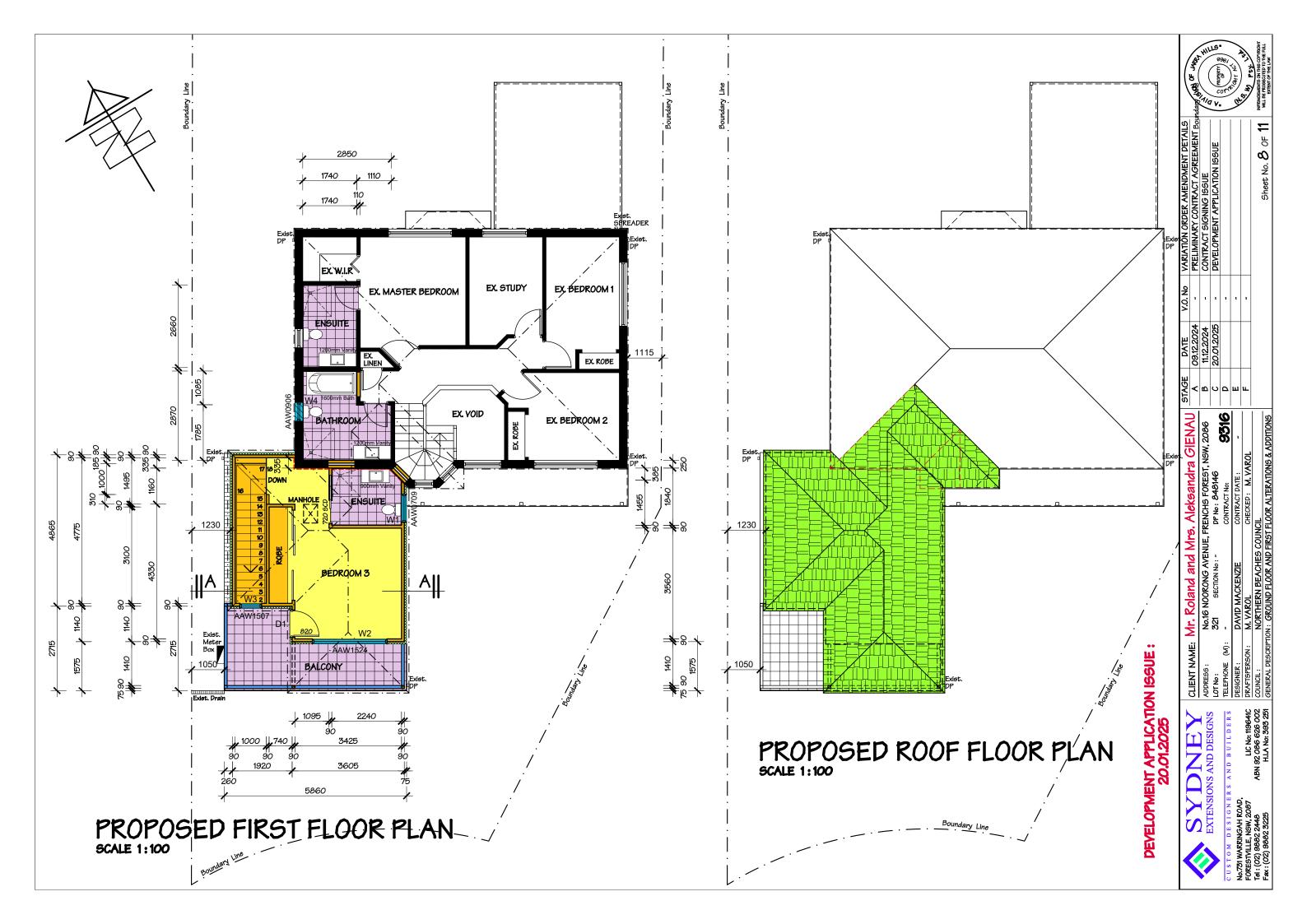
number	Orientation	including frame (m2)	height (m)	distance (m)	device	glass type
W1	SE	0.63	0	0	none	improved aluminium, single clear, (U-value: 6.44, SHGC: 0.75)
W2	SW	3.6	0	0	eave/ verandah/ pergola/balcony >=900 mm	improved aluminium, single clear, (U-value: 6.44, SHGC: 0.75)
W3	sw	1.05	0	0	none	improved aluminium, single clear, (U-value: 6.44, SHGC: 0.75)
W4	NW	0.54	0	0	none	improved aluminium, single clear, (U-value: 6.44, SHGC: 0.75)
W5	SE	1.45	0	0	none	improved aluminium, single clear, (U-value: 6.44, SHGC: 0.75)
D1	NW	1.85	0	0	none	aluminium, double Lo- Tsol/air gap/ clear, (U-value: 4.9, SHGC: 0.33)
D2	SE	5.08	0	0	none	improved aluminium, single clear, (U-value: 6.44, SHGC: 0.75)

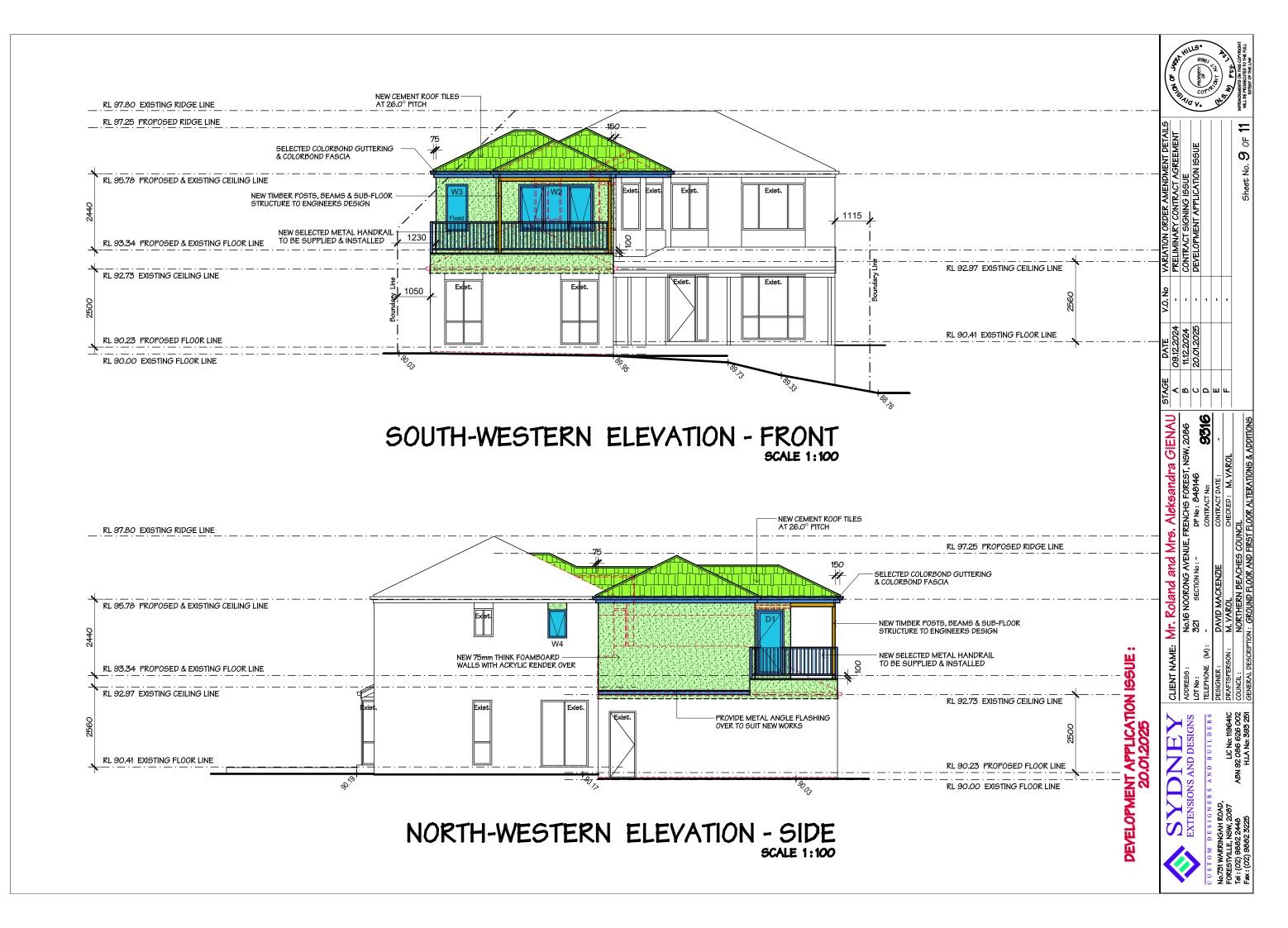
DEVELOPMENT APPLICATION ISSUE 20.01.2025

No.1

CLIENT NAME:
ADDRESS:
LOT No:
TELEPHONE (M):













27 February 2025

Jarra Hills (N.S.W) Pty Ltd 731 Warringah Road FORESTVILLE NSW 2087

Dear Sir/Madam,

Application No. DA2025/0197 - PAN-510171

Address: 16 Noorong Avenue FRENCHS FOREST

Return of Application

Council has conducted a review of your application in accordance with Council's Development Application and Modification Lodgement Requirements (21/22) and is unable to accept the application due to inadequate and/or insufficient information being provided to assess the proposed development.

Accordingly, Council is returning the application to you, which will require you to address the matters listed below and lodge a new application in the NSW Planning Portal, including all relevant plans and documentation as well as the required additional information:

1. Preliminary Geotechnical Report - Landslip Area "B" and "D" (WLEP 2011)

A Preliminary Geotechnical Report (Preliminary Assessment of Site Conditions) as the land is located in Landslip Risk Area "B" or "D" under Council's Landslip Risk Map. The report is to be prepared by a suitably qualified geotechnical consultant.

2. **Landscape Calculation Plan – Warringah**

A Landscape Calculation Plan, which identifies all areas included in the calculation as per the definition of landscaped open space under the Warringah DCP.

Council has adopted this review and checking procedure in the interests of streamlining the processing of applications, ensuring all applications are Assessment Ready and so applications can be processed within reasonable timeframes.

It is very important that you carefully read and understand the reason(s) why your application has been returned, that you refer to the Development Application and Modification Lodgement Requirements (21/22), accessible via Council's forms page, to avoid your application being returned.



Visit our "Lodge your Application" webpage for more information or to access Planning Portal user guides.

Should you wish to speak to an officer to obtain clarification on the above matter(s) prior to relodging your application, please do not hesitate to contact Council's Planning Officer on 1300 434 434 during our business hours of 8.30am to 5.00pm, Monday to Friday.

Your co-operation in this matter is appreciated.

Yours Faithfully **Development Advisory Service Team**