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21st October 2016

D-11-265205

Mr. Daniel Cheetham 21 Waterview Street MONAVALE NSW 2103

Dear Sir & Madam,

<u>Re: Flood Report – Proposed New Retaining wall within the flood flow path at 21</u> <u>Waterview Street, Mona Vale</u> NSW

INTRODUCTION

I, Edward A. Bennett, practicing Civil, Structural, Geotechnical & Environmental Engineer hereby confirm that I inspected the above property for the purpose of carrying out a flood report for this property in respect to flood levels for ARI 1:100 rainfall events (1%AEP).

REPORT

The purpose for this flood report for No. 21 Waterview Street, Mona Vale, is to provide evidence that with the proposed addition of a new retaining wall and raised rear yard levels, its construction being in close proximity of the predicted flow path for the 1%AEP flood, through the property, these flood levels will not rise to affect the neighboring property at No. 19 Waterview Street, Mona Vale or any other properties downstream.

We have relied upon the "Flood information request - common", provided by Northern Beaches Council dated 14th February 2017 refer *Appendix "A"*, the survey details provided by Sydney Surveyors dated 20/12/2016, refer *Appendix "B"*, HEC-RAS 5.0.1, computations, refer *Appendix "C"* and concluded with the Proposed Retaining Wall design, refer Drg. No. C-11-265205, refer *Appendix "D"*

Flood Information:

Site Plan: See Appendix B.

Probable Maximum Flood (PMF): 21.25m AHD

1% AEP Maximum Water Level: 20.55 AHD (Appendix A)

An Approximate Discharge:

The discharge through the property was calculated using the rational method. The assumed catchment area (A) and the rainfall intensity (I) used for the calculation is shown in Fig 1 and Fig 2.

Where, C= 0.9, I = 261 mm/hr & A = 46 ha (approx.)

 $Q = CIA/360 = 0.9*261*70/360 = 30 \text{ m}^3/\text{s}$ (approx.)

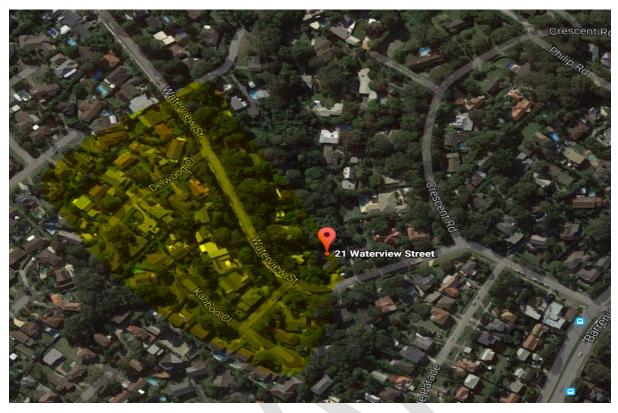


Fig : Assumed Catchment Area

		Intensity.	Frequenci	v-Duration	Table																
Intensity-Frequency-Duration Table Location: 33.6755 151.300E Issued: 22/3/2017 Rainfall intensity in mm/h for various durations and Average Recurrence Interval																					
											Average Recurrence Interval										
											Duration	1 YEAR	2 YEARS	5 YEARS	10 YEARS	20 YEARS	50 YEARS	100 YEARS			
5Mins	98.5	126	159	178	203	236	261														
6Mins	92.2	118	149	167	191	221	245														
10Mins	75.6	96.9	123	138	158	185	204														
20Mins	55.3	71.2	91.6	104	119	140	155														
30Mins	44.9	58.1	75.2	85.3	98.4	116	129														
1Hr	30.6	39.6	51.7	58.9	68.2	80.5	89.8														
2Hrs	20.4	26.4	34.4	39.2	45.4	53.5	59.8														
3Hrs	16.0	20.7	26.9	30.6	35.3	41.6	46.4														
6Hrs	10.5	13.6	17.5	19.9	22.9	26.9	29.9														
12Hrs	6.87	8.86	11.4	12.9	14.9	17.5	19.4														
24Hrs	4.39	5.68	7.39	8.40	9.72	11.5	12.8														
48Hrs	2.70	3.51	4.64	5.33	6.21	7.38	8.28														
72Hrs	1.98	2.58	3.44	3.96	4.63	5.52	6.20														
(Row data: 30.75	. 8.9, 2.58, 81.34, 1	7 48 5 52 skew=0	00 E2=4 3 E50=	15.88)	© Australian	Government Bure	au of Meteorology														

Fig : Rainfall Intesity

From HEC-RAS model in Appendix "C", which includes the new proposed retaining in the cross-sections, the maximum flood water level for the flow of 30 m3/s was found to be RL 20.62 m at Station 33.5m.

It should be noted that the survey information shows that, at the Waterview Street frontage, the N.G.L is RL is 23.16. At the rear boundary, the N.G.L is RL is 16.13. This indicates that the fall from the front boundary to the rear boundary is approx. 7m over. The length of the block is approx. 49 m, making the land sloping at approx. 15%.

This is a substantial slope that will reduce the 1% AEP flood levels substantially over the length of the creek. It is likely that RL 20.55, represents the TWL just inside the boundary such that at Station 33.5, at the overt of the creek, only approx. 2m from the front street boundary alignment.

CONCLUSIONS:

From the drawings, we can confirm that the new addition (retaining wall) is affected by the Flood category 1 but the existing property is not affected by any flood activity. Whereas, the addition of the retaining wall can affect the existing flood level to some extent.

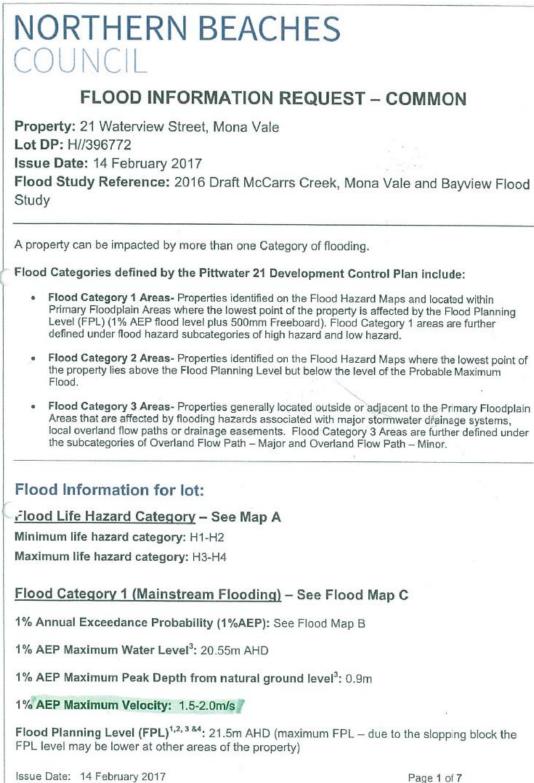
The construction of the proposed new wall at the location shown in Site Plan results in negligible difference in water level, say not greater than 100mm which will not affect the neighboring property.

Yours faithfully,

Caber et

E.A. Bennett M.I.E. Aust. Cp Eng. NPER 198230, Member AGS, BPB 0820

Appendix "A"



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1% AEP Provisional Flood Hazard: High

1% AEP Hydraulic Categorisation: Floodway

Flood Category 3 (Overland Flow) - See Flood Map E

1% AEP Overland Flow Maximum Water Level 384: 20.5m AHD 💉

1% AEP Overland Flow Maximum Depth from Natural Ground Level³⁸⁴: 0.9m

Flood Category 2 (PMF) - See Flood Map D

Probable Maximum Flood (PMF) Level²: 21.25m AHD (See Flood Map D)

PMF Maximum Depth from natural ground level: 1.2m

PMF Maximum Velocity: 2.-2.5m/s

¹Intensification of development requires the consideration of climate change impacts which may result in higher planning levels than those indicated on this flood advice.

²Special Flood Protection developments require a higher planning level using the higher of the PMF or FPL/minimum floor level.

³The flood information does not take into account any local overland flow issues with a depth below 0.15m nor private stormwater drainage systems.

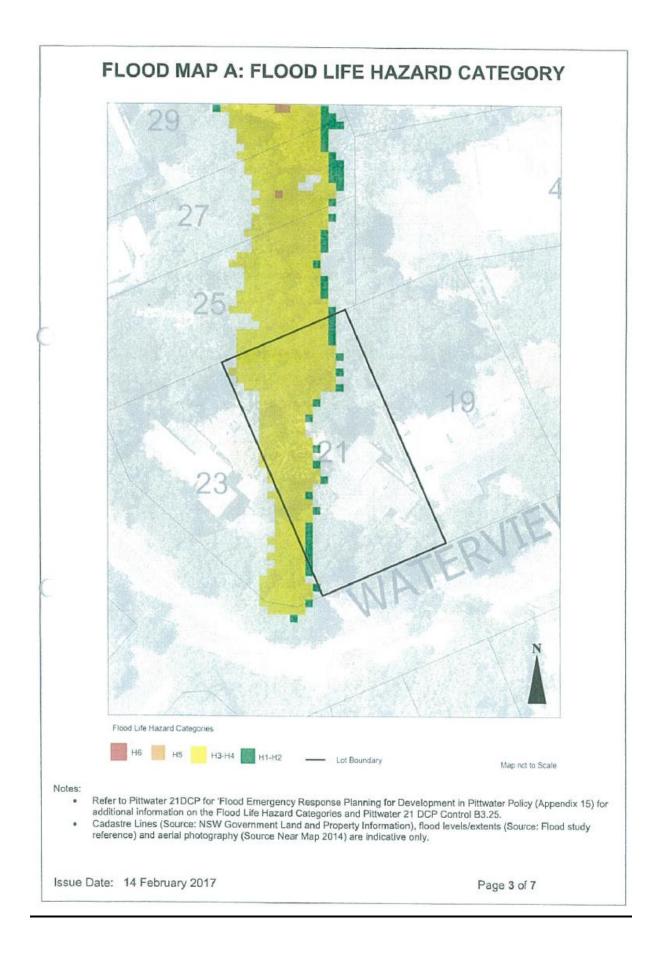
⁴Overland flow/mainstream water levels may vary across a sloping site, resulting in variable minimum levels across the site.

General Notes:

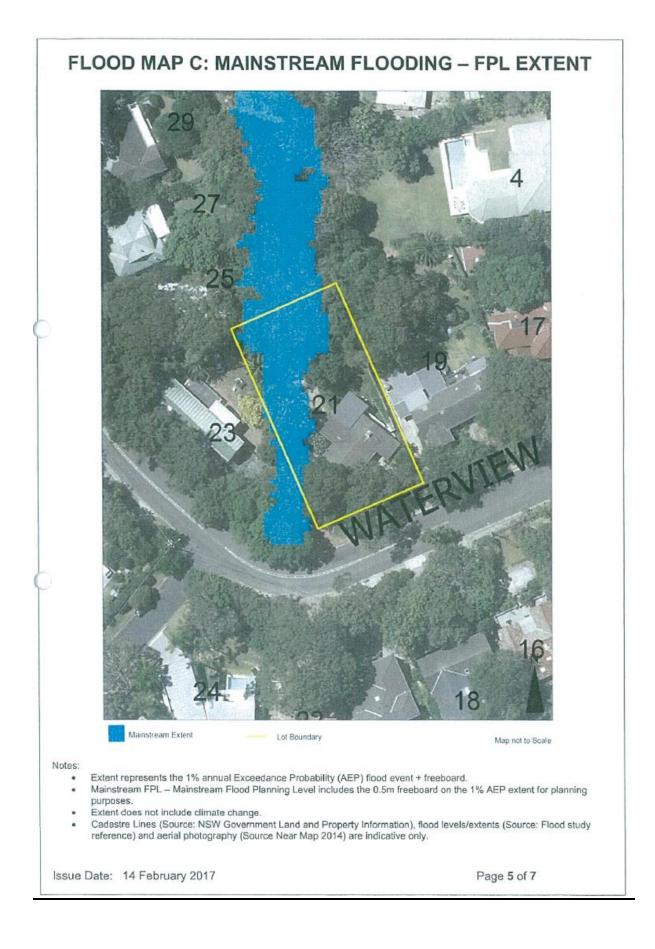
- · All levels are based on Australian Height Datum (AHD) unless otherwise noted.
- The source information on this advice was obtained from numeric modelling prepared by consultants for Pittwater Council for existing site conditions at the time of the flood study. Separate review and flood model verification has not been undertaken by Council.
- The interpolated information is for the purpose of planning only. Detailed flood data for individual land areas were not determined from the exercise.
- Flood models only approximate flood behaviour. Site specific ground and building survey levels should be used to relate flood levels and to assess the impact of flooding. A site specific flood study/risk assessment may be required for any future development. Care and expertise is required in the interpretation of these flood levels. Engage a suitably qualified engineer to assist you in this matter.
- You need to refer to the Pittwater 21 DCP flood development controls, if you are planning to lodge a
 Development Application. The advice may be reviewed and amended by Northern Beaches Council in
 the course of assessment of a specific development application.
- While this advice is periodically updated, it is possible that the Council holds further information dealing
 with the flooding which has not been incorporated into the above advice.
- · Estuarine/coastal inundation has not been taken into account in the flood information.

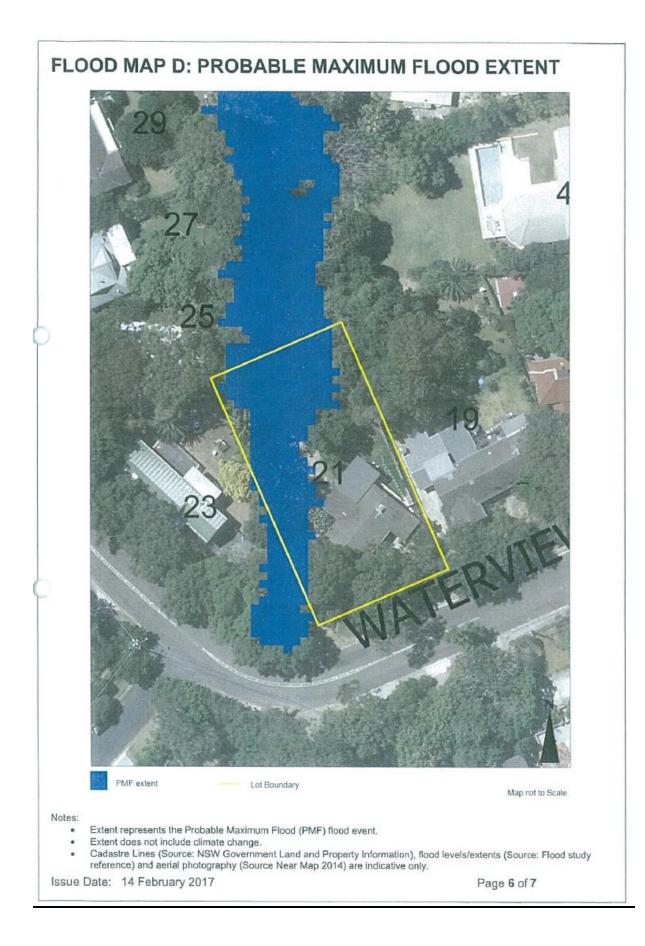
Issue Date: 14 February 2017

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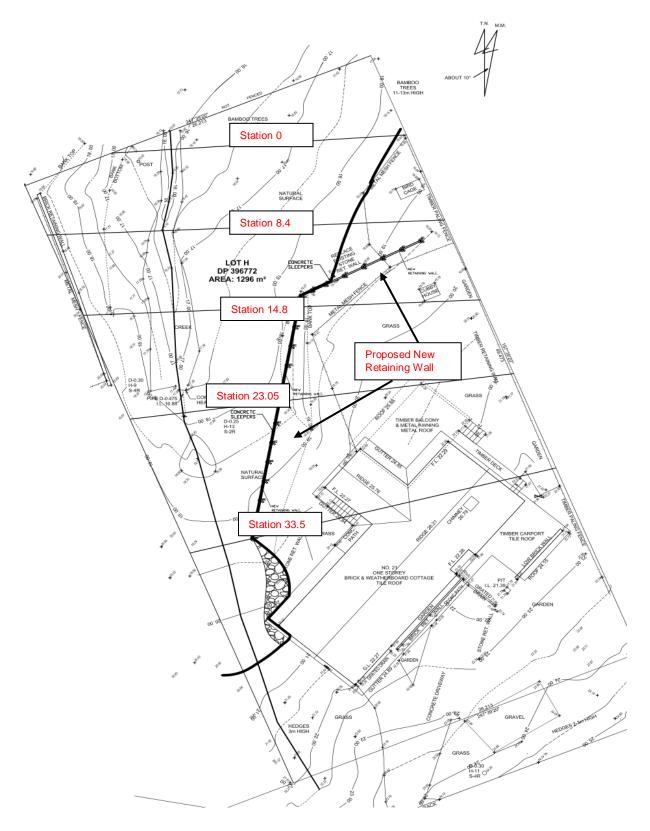






FLOOD MAP E: OVERLAND FLOW EXTENT Overland Flow flowpath Lot Boundary Map not to Scale Notes: Extent represents the 1% annual Exceedance Probability (AEP) flood event. Overland Flow Path Major includes a fixed 5m horizontal planning buffer on the 1% AEP extent for planning purposes. Extent does not include climate change. Areas not identified on the above Flood Map are likely to experience inundation of depths up to 0.14m. Cadastre Lines (Source: NSW Government Land and Property Information), flood levels/extents (Source: Flood study reference) and aerial photography (Source Near Map 2014) are indicative only. Issue Date: 14 February 2017 Page 7 of 7

Appendix "B"



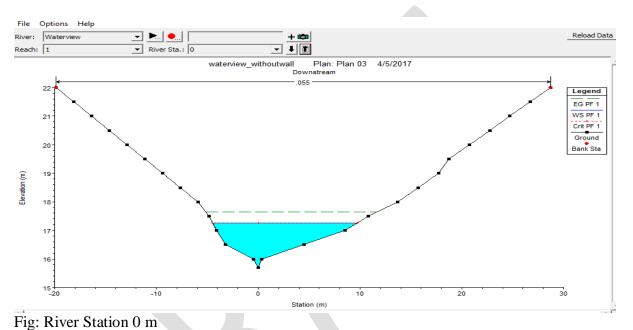


Appendix "C"

HEC-RAS 5.0.1:

HEC-RAS 5.0.1 was used to predict the flood levels for ARI 1:100 (1%AEP) rainfall events. Cross sections in the creek are shown in Appendix "B" and are interpolations of survey levels at certain intervals over a 33.5m reach within the property, for two different cases with and without retaining wall, starting at Station 0 near the rear boundary and working backwards to Station 33.5 which is approx. 2/3rds from the front boundary. These survey level interpolations were used in the HEC-RAS model. The reach profile summaries for these two cases are shown in the table below.

Without wall:



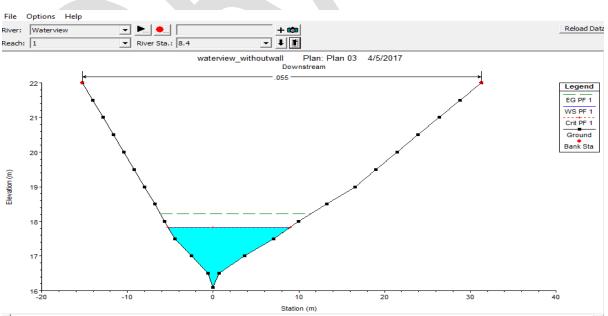
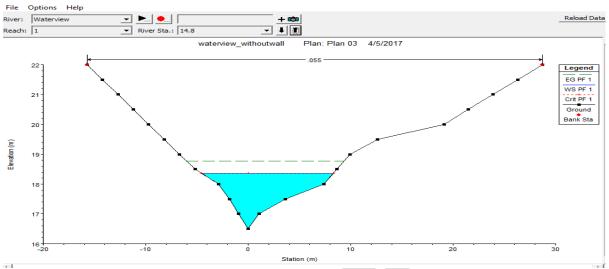
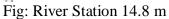
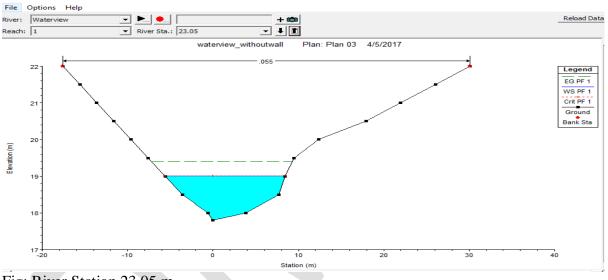


Fig: River Station 8.4 m









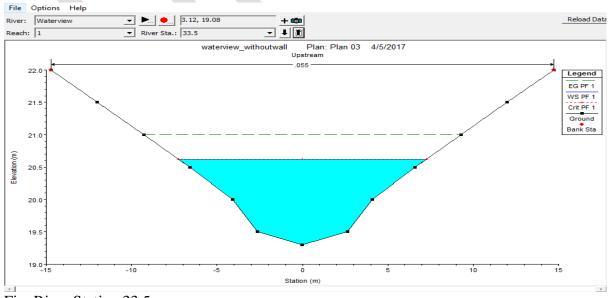


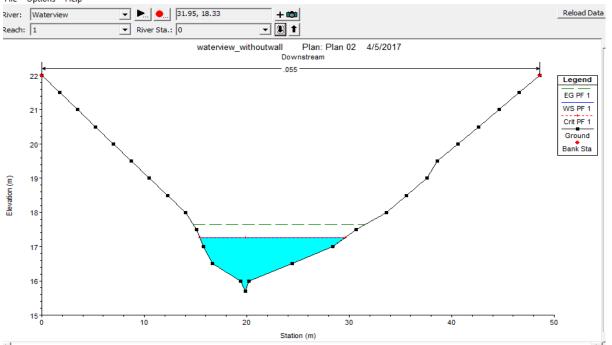
Fig: River Station 33.5 m

Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(m3/s)	(m)	(m)	(m)	(m)	(m/m)	(m/s)	(m2)	(m)	
1	33.5	PF 1	30.00	19.30	20.62	20.62	21.00	0.034022	2.74	10.95	14.52	1.01
1	23.05	PF 1	30.00	17.80	19.01	19.01	19.40	0.033991	2.77	10.82	14.05	1.01
1	14.8	PF 1	30.00	16.50	18.37	18.37	18.78	0.034271	2.85	10.54	12.88	1.01
1	8.4	PF 1	30.00	16.10	17.83	17.83	18.22	0.035093	2.77	10.82	14.21	1.01
1	0	PF 1	30.00	15.70	17.26	17.26	17.65	0.034274	2.75	10.90	14.19	1.00

HEC-RAS Plan: Plan 03 River: Waterview Reach: 1 Profile: PF 1

Table: Profile summary (without wall)







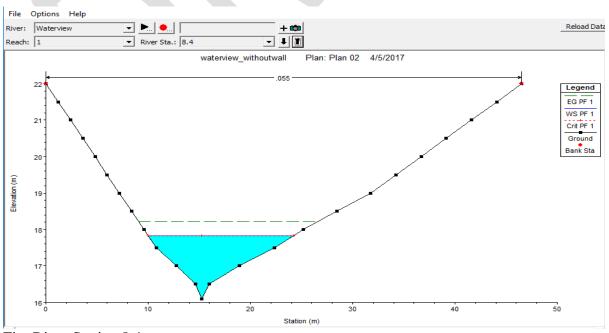
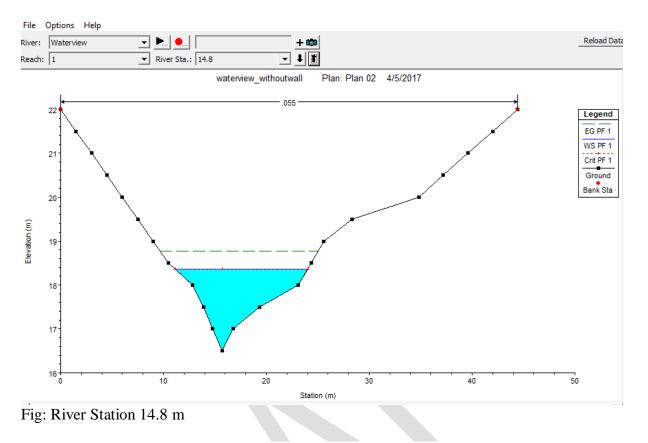


Fig: River Station 8.4 m



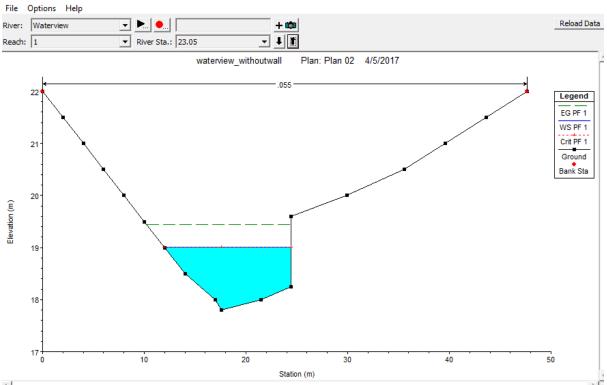


Fig: River Station 23.05 m

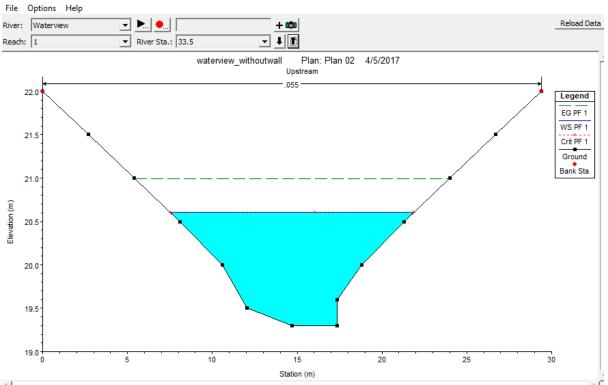


Fig: River Station 33.5 m

HEC-RAS P	lan: Plan 02 R	ver: Waterview	Reach: 1 F	Profile: PF 1								
Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(m3/s)	(m)	(m)	(m)	(m)	(m/m)	(m/s)	(m2)	(m)	
1	33.5	PF 1	30.00	19.30	20.60	20.60	20.99	0.034834	2.75	10.89	14.33	1.01
1	23.05	PF 1	30.00	17.80	19.01	19.01	19.44	0.035193	2.88	10.40	12.46	1.01
1	14.8	PF 1	30.00	16.50	18.37	18.37	18.78	0.034271	2.85	10.54	12.88	1.01
1	8.4	PF 1	30.00	16.10	17.83	17.83	18.22	0.035093	2.77	10.82	14.21	1.01
1	0	PF 1	30.00	15.70	17.26	17.26	17.65	0.034274	2.75	10.90	14.19	1.00

Table: Profile summary (with wall)