



# Flower Power Redevelopment

277 Mona Vale Road, Terrey Hills

PREPARED FOR

Flower Power Pty Ltd 277 Mona Vale Road Terrey Hills NSW 2084 Ref: SY211979 Rev: 1 Date: 15.06.23



# Stormwater Report

## **Revision Schedule**

Date	Revision	Issue	Prepared By	Approved By
15.06.23	1	Issued for Information	S. Sarkis	A. Carvalhaes

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## 1. Introduction

Northrop Consulting Engineers Pty Ltd have been engaged by Statewide Project Management Pty Ltd on behalf of Flower Power to prepare Civil Engineering Drawings for redevelopment works at 277 Mona Vale Road, Terrey Hills. This report will be lodged with Northern Beaches Council to support the Development Application.

## 1.1 Site Description and Proposed works

The subject site is located within Terrey Hills, at 277 Mona Vale Road, and falls within the Northern Beaches Council Local Government Area (LGA). The proposed work will take place throughout the existing Flower Power complex in Terrey Hills. Refer to Appendix A – Survey for site survey.

The site covers an area of approximately 28,288 m<sup>2</sup> and falls from the northwestern side towards the southeastern side at a 4% slope. The site is bounded by the following roads:

- Mona Vale Road to the East.
- Cooyong Road to the North.
- Myoora Road to the West.



Figure 1: Site Aerial Image (Source: Nearmaps)



## 1.2 Project Description

The proposed development for Flower Power Terrey Hills consists of the retention of the existing open nursery, proposed retail stores, open bulk goods display area, café, and external car parking. The architectural site plan is shown in Figure 2.

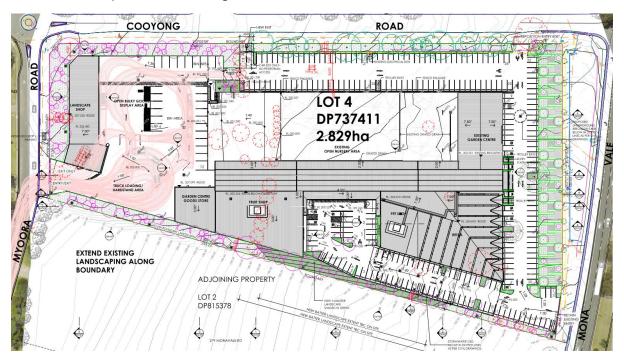


Figure 2: Architectural Site Plan



# 2. Civil Engineering Design

## 2.1 Design Criteria & Methodology

The Civil Engineering design has considered a number of relevant documents and reports applicable to the proposed development. The objectives of this report are to ensure the proposed stormwater management of the site meets the requirements of the following documents:

- Northern Beaches Council Water Management for Development Policy (2021)
- Northern Beaches Council WSUD & MUSIC Modelling Guidelines (2016)
- Managing Urban Stormwater Soil & Construction (2004) by Landcom (The Blue Book)
- AS/NZS 3500.3:2018 (Plumbing and Drainage)

## 2.2 Sediment and Erosion Control

The objectives of the sediment and erosion control for the development site will be to ensure:

- Adequate sediment and erosion control measures are applied prior to the commencement of construction and are maintained throughout construction; and
- Construction site runoff is appropriately treated in accordance with the requirements of Northern Beaches Council, prior to discharge.

Prior to any earthworks commencing on site, the sediment and erosion control will need to be provided during the construction phase of the development in accordance with the requirements of Northern Beaches Council and The Blue Book.

## 2.2.1 Sediment Basin

Due to the size of the proposed development, a temporary sediment basin will be required to capture site runoff during construction. The construction of the basin may be undertaken in stages to enable maximum runoff capture assisted by diversion swales and direct runoff to the basin. The design parameters of this temporary sediment basin are summarised in



### Table 1.

Calculations to determine the required basin size are to be based on available geotechnical information regarding soil types and using The Blue Book.

To ensure the sediment basin is working effectively it will need to be maintained throughout the construction works. Maintenance includes ensuring adequate settlement times or flocculation and pumping of clean water to reach the minimum storage volume at the lower level of the settling zone. The settling zone will be identified by pegs to clearly show the level at which design storage capacity is available.

The pumped water from the sediment basin can be reused for dust control during construction.

Overflow weirs are to be provided to control overflows for rainfall events more than the design criteria.



Table 1: Sediment Basin Sizing Values

Parameter	Adopted value
Total disturbed area (ha)	2.3644
Soil Texture Group	D or F
Design rainfall depth (days)	5
Design rainfall depth (percentile)	80
x-day, y-percentile rainfall event	35.2
Cv	0.42
Settling zone volume (m³)	349.550
Sediment storage volume (m³)	174.775
Total basin volume required (m³)	524.325

#### 2.2.2 Sediment and Erosion Control Measures

Prior to any earthworks commencing on site, sediment and erosion control measure shall be implemented generally in accordance with the engineering drawings, Council requirements, and The Blue Book. The measures are intended to be a minimum treatment only as the contractor will be required to modify and stage the sediment and erosion control measures to suit the construction program, sequencing, and techniques. These measures may include:

- A temporary site security/safety fence is to be constructed around the site, the site office area, and the proposed sediment basin.
- Sediment fencing provided downstream of disturbed areas, including any topsoil stockpiles, shown in Figure 3 (below).
- Dust control measures including covering stockpiles, installing fence hessian, and watering exposed areas.
- Placement of hay bales or mesh and gravel inlet filters around and along proposed catch drains and around stormwater inlets pits
- The construction of a temporary sediment basin as noted above.
- Stabilised site access at the construction vehicle entry/exits.
- Any stockpiled material, including topsoil, shall be located as far away as possible from any
  associated natural watercourses or temporary overland flow paths. All stockpiles and embankment
  formations shall be stabilised by hydroseeding or hydro mulching on formation.



Figure 3: Sediment Fence



## 2.3 Stormwater Infrastructure and modelling

The objectives of this investigation include:

- Management of 'minor' and 'major' flows using piped systems and overland flow paths such that the piped system flow is less than the 20% AEP pre-development runoff scenario.
- OSD design as per the Northern Beaches Water Management for Development Policy.

The proposed stormwater system was modelled using DRAINS. The following parameters were adopted:

- Major Storm = 1% AEP
- Minor Storms = 5% AEP & 20% AEP
- Soil Type 2.5
- Paved (Impervious) Area Depression Storage 1mm
- Supplementary Area Depression Storage 1mm
- Grassed (Pervious) Area Depression Storage 5mm
- Sag pit Blocking Factor 0.5
- On Grade Pit Blocking Factor 0.2

The DRAINS Model Layout is shown below. The DRAINS model can be provided for assessment upon request.

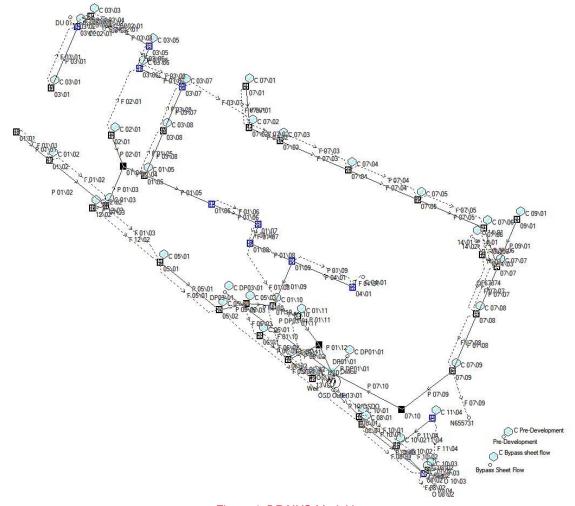


Figure 4: DRAINS Model Layout



### 2.4 On-site Stormwater Detention

## 2.4.1 OSD Requirements

On-site Stormwater Detention (OSD) has been provided for the site. According to the Northern Beaches Water Management for Development Policy, the proposed on-site detention tank should be designed in accordance with the following control standards:

- For all developments, the runoff from the site after development is not to exceed the runoff from the total site prior to the development for all storm durations for the 20% AEP, 5% AEP and a 1% AEP storm event.
- For all developments except single residential dwelling developments the PSD is to be calculated on the maximum allowable impervious fraction of 0%. That is, discharge off the site is to be restricted to the "state of nature" condition.

The PSD and post development discharges were calculated using DRAINS and can be seen in the below table.

Table 2: Site Discharge

Storm Event	Permissible Site Discharge (PSD)	Post-Development Discharge		
20% AEP	0.364 m <sup>3</sup> /s	0.284 m <sup>3</sup> /s		
5% AEP	0.688 m <sup>3</sup> /s	0.495 m <sup>3</sup> /s		
1% AEP	1.115 m <sup>3</sup> /s	0.937 m <sup>3</sup> /s		



## 2.4.2 OSD Design

The tank has been designed to store and attenuate the peak discharge from the site in smaller events (20% and 5% AEP) and in larger events (1% AEP). A 450mm diameter outflow culvert has been sized to control flows for the 1% AEP storm event. A 350mm diameter orifice has also been designed to control flows for the 20% and 5% AEP storm events. The outflow pipe will enter the legal point of discharge located at the southeastern driveway. The assumed tailwater levels at the point of connection are:

- RL 194.140m for 1% AEP (top of kerb)
- RL 194.013m for 5% and 20% AEP (top of grate)

The key OSD design parameters are presented in Table 3. The details of the OSD tank are presented in drawing DAC6.01.

Table 3: Summary of OSD Design Parameters

Item	Design Parameters
Effective OSD Volume	795.90m <sup>3</sup>
1% AEP Top Water Level	196.15m
Footprint Required	420.00m <sup>2</sup>



#### 2.5 Stormwater Overland Flow

According to Section 9.3.2.6 from Northern Beaches Council Water Management for Development Policy, "The total piped flow from the site to a Council drainage pipeline must not exceed the maximum 20% AEP pre-development runoff. The total piped and overland flows from the site must not exceed the 1% AEP pre-development discharge".

The pipe connection council's stormwater system has been designed to limit the piped discharge to be less than the 20% AEP pre-development runoff. The 20% AEP pre-development runoff was calculated to be **0.645m³/s**.

Total post-development pipe flows were modelled and compared with the maximum pipe flow allowed due to the 20% AEP pre-development runoff, the piped flow for the 1% AEP is **0.380** m³/s.

The surface flow paths have been designed for the 1% AEP storm event so that the velocity and depth ratio (v x d) is less than 0.4m<sup>2</sup>/s. The following control measures have been designed to reduce the velocity and depth ratio from the overland flows in the 1% AEP storm event:

## 2.5.1 Landscape Frontage Sheet Flow

The frontage landscape area along Mona Vale Road will not be captured by a piped network. Instead, it will naturally sheet flow off the site to reduce the amount of piped flow that will be discharged to council's drainage network. The flow generated from this landscaped area is 0.067m<sup>3</sup>/s in the 1% AEP storm event.

### 2.5.2 Break in Kerb and Level Spreader

The proposed kerb and gutter of the carpark along Mona Vale Road will have a 4.0m break in the kerb to allow for overflows into the landscaped area. These overflows will then be directed to a 4.0m wide level spreader to reduce the depth of the flow. The flow from this level spreader is  $0.315 \text{m}^3/\text{s}$  in the 1% AEP storm event, with a v x d ratio of  $0.10 \text{ m}^2/\text{s}$ .

### 2.5.3 Sag pit and Weir Wall

The pit at the south-eastern corner of the site will be a raised sag pit in the swale and will be accompanied by a 3.0m long upstanding weir wall along the eastern boundary of site. Once the ponding level of the sag pit exceeds the weir wall height the stormwater will sheet flow to Mona Vale Road's kerb and gutter system. The flow over this weir wall is 0.085m³/s with the v x d ratio of 0.03m²/s.



## 2.6 Stormwater Quality

## 2.6.1 Stormwater Quality Requirements

According to the Northern Beaches Council WSUD & MUSIC Modelling Guidelines, the Stormwater Quality treatment target reduction removal rates are:

- 90% reduction in the post development mean annual load of Total gross pollutant load (>5mm).
- 85% reduction in the post development mean annual load of Total Suspended Solids (TSS).
- 65% reduction in the post development mean annual load of Total Phosphorus (TP).
- 45% reduction in the post development mean annual load of Total Nitrogen (TN).

## 2.6.2 Proposed Stormwater Quality Treatment System

The stormwater pollutant load reduction and water conservation objectives will be met by the use of a rainwater tank and stormwater treatment devices such as filters and pit baskets.

## 2.6.3 Modelling of Stormwater Quality

Stormwater treatment was modelled using Modelling Urban Stormwater Improvement Conceptualisation (MUSIC) software v 6.3.0 for both water quality and quantity. The first MUSIC model uses the Sydney Observatory 6-minute rainfall data from the period of 1981-1985 to analyse the water quality treatment performance of the site. Whilst the second MUSIC model uses Sydney Observatory daily rainfall data from the period of 1925-1974 to accurately analyse the water quantity for the rainwater tank as per Northern Beaches Council WSUD & MUSIC Modelling Guidelines (2016).

The stormwater quality treatment parameters are included below:

- 37 x Ocean Guard Pit Inserts
- 1 x Rainwater Tank (30kL)
- 2 x Irrigation Tank (50kL)
- 1 x StormFilter Chamber (6.0m²)
- 1 x StormFilter Chamber (9.0m²)
- 30 x P<sub>Sorb</sub> StormFilters (690mm)

MUSIC Model Layout and pollutant removal performance is shown below.

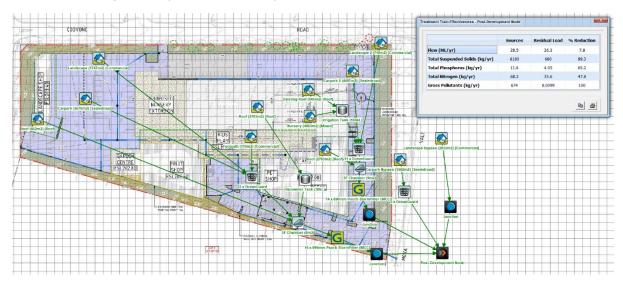


Figure 5: MUSIC Water Quality Model



Table 5: MUSIC Model Results

Item	Target Reduction Removal Rate (%)	% Reduction		
Total Suspended Solids	85	89.3		
Total Phosphorus	65	65.2		
Total Nitrogen	45	47.8		
Gross Pollutants	90	100.0		

As shown in the table above, the stormwater treatment targets appropriate for the site will be met by the treatment measures provided. The pollutant removal performance as calculated by MUSIC modelling meets the Northern Beaches Council targets.

#### 2.7 Water Conservation

## 2.7.1 Water Conservation Requirements

A rainwater tank has been proposed to supply the demand of toilet flushing and landscape irrigation (external to the nursery). It has been assumed that at least 50% of the new retail buildings will be connected to the rainwater tank (2703m²).

With reference to the Architectural drawings, there are 21 toilets in the building and 4497m² of landscaping. Assuming the rates of 0.1 kL/day/toilet and 0.4 kL/year/m² the daily and annual demand adopted in MUSIC are 2.1 kL/day and 1798.8 kL/year, respectively.

Additionally, the site will be fitted with two 50kL irrigation tanks to irrigate the existing 4863m² of open nursery area. Assuming the same irrigation rate of 0.4kL/year/m², the annual demand has been calculated as 1945.2 kL/year.

## 2.7.2 Rainwater Tank Specification

Rainwater tanks of varying sizes were modelled in this design and their rainwater reuse percentages can be seen in Figure 6.



Figure 6: Rainwater % Reuse vs Tank Size



As illustrated, beyond the 30kL, increasing the rainwater tank size provides diminishing returns. A 30kL rainwater tank achieves a reuse demand met value of 35.23% for toilet flushing and irrigation purposes for the landscaped areas. See Figure 7 for the node water balance of the proposed rainwater tank.



Figure 7: MUSIC Water Quantity Model



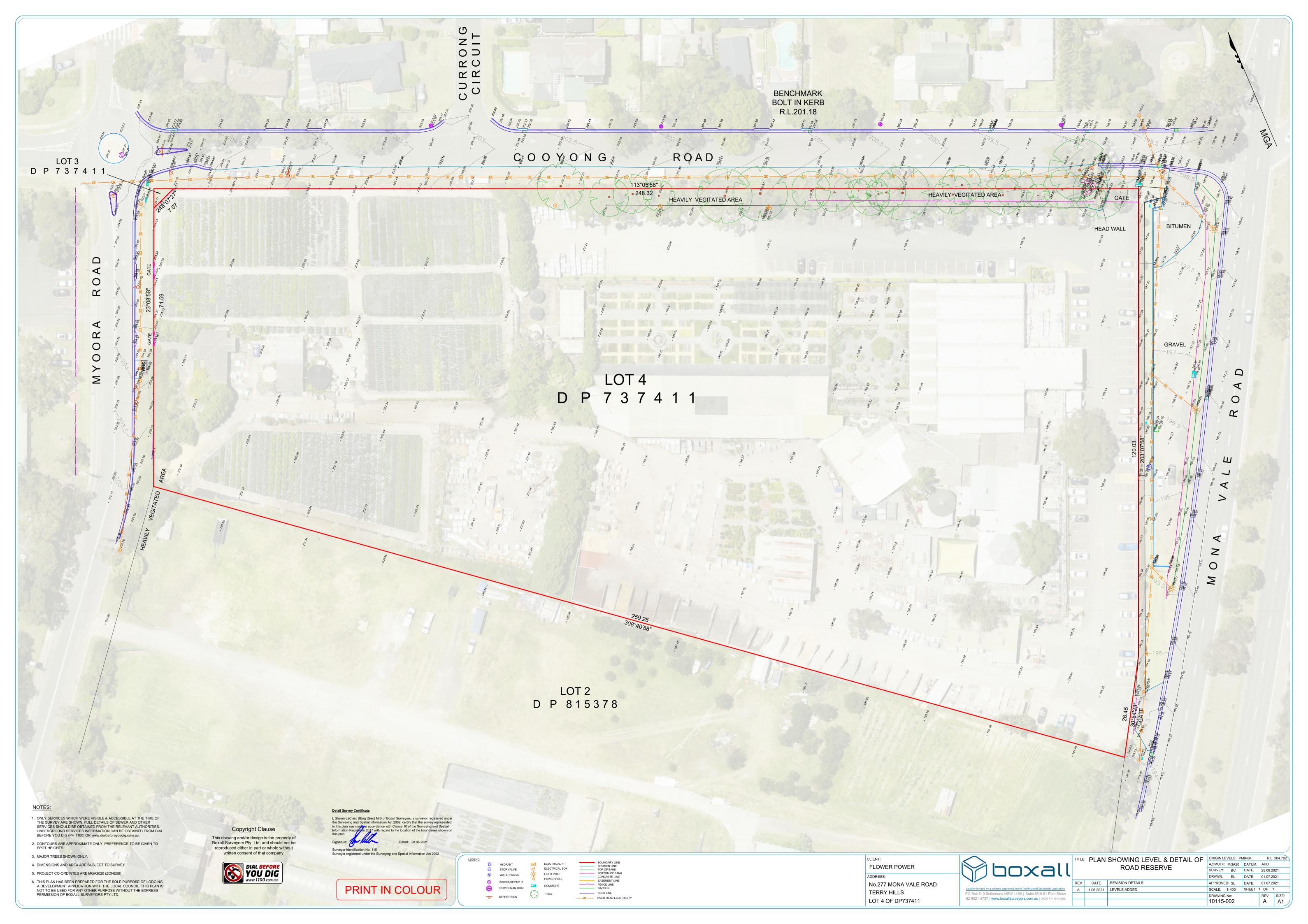
## 3. Conclusion

In summary, the civil engineering requirements for the proposed development are as follows:

- Sediment & Erosion Control A sediment basin is required to effectively capture sediment laden site runoff during siteworks. Sediment and erosion control measures should be installed and maintained for the duration of the construction works.
- Stormwater Infrastructure The stormwater design has considered the below ground drainage network and overflow paths consistent with the requirements of Northern Beaches Council inclusive of on-site stormwater detention.
- Water Quality and Conservation Water quality requirements will be achieved through the
  provision of pit baskets and filter cartridges, in accordance with the requirements of Northern
  Beaches Council. A rainwater tank has been designed to reduce non-potable water demand and
  two irrigation tanks have also been specified for irrigation purposes.



# Appendix A – Survey





# Appendix B – Civil Engineering Drawings

# FLOWER POWER, TERREY HILLS

# CIVIL ENGINEERING PACKAGE **DEVELOPMENT APPLICATION**





# DRAWING SCHEDULE

DRG No. DRAWING TITLE

COVER SHEET, DRAWING SCHEDULE AND LOCALITY PLAN

SPECIFICATION NOTES

SEDIMENT AND SOIL EROSION CONTROL PLAN SEDIMENT AND SOIL EROSION CONTROL DETAILS

BULK EARTHWORKS CUT AND FILL PLAN

SITEWORKS AND GRADING PLAN DAC4.02 STORMWATER MANAGEMENT PLAN

STORMWATER CATCHMENT PLAN

STORMWATER MANAGEMENT DETAILS - SHEET 01

STORMWATER MANAGEMENT DETAILS - SHEET 02 DAC6.03 STORMWATER MANAGEMENT DETAILS - SHEET 03

# NOT FOR CONSTRUCTION

CIVIL ENGINEERING PACKAGE

211979 DRAWING NUMBER

**COVER SHEET, DRAWING SCHEDULE** AND LOCALITY PLAN

DESCRIPTION ISSUED FOR INFORMATION AC 23.05.23 ISSUED FOR INFORMATION AC 15.06.23

DRAWING NOT TO BE USED FOR CONSTRUCTION UNLESS VERIFICATION

LEFFLER SIMES ARCHITECTS



ALL SETOUT TO ARCHITECT'S DRAWINGS, DIMENSIONS TO BE VERIFIED WITH THE ARCHITECT

AND ON SITE BEFORE MAKING SHOP DRAWINGS OR



## GENERAL NOTES

THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH OTHER CONSULTANTS' DRAWINGS AND SPECIFICATIONS AND WITH OTHER SUCH WRITTEN INSTRUCTIONS AS MAY BE ISSUED DURING THE COURSE OF THE CONTRACT. ANY DISCREPANCY SHALL BE REFERRED TO THE ENGINEER BEFORE PROCEEDING WITH THE WORK.

ALL DIMENSIONS ARE IN MILLIMETRES & ALL LEVELS ARE IN METRES, UNO (UNLESS NOTED OTHERWISE).

NO DIMENSION SHALL BE OBTAINED BY SCALING THE DRAWINGS.

ALL LEVELS AND SETTING OUT DIMENSIONS SHOWN ON THE DRAWINGS SHALL BE CHECKED ON SITE PRIOR TO THE COMMENCEMENT OF THE WORK.

DETAIL SURVEY DATA WAS SUPPLIED BY: BOXALL SURVEYORS

REF. NUMBER: 10115-002 REVISION DATE: 01.06.2021

REVISION NUMBER: A GEOCENTRIC DATUM OF AUSTRALIA: GDA94

SURVEYOR: BC APPROVED: SL

EXISTING SERVICES WHERE SHOWN HAVE BEEN PLOTTED FROM SUPPLIED DATA AND SUCH THEIR ACCURACY CAN NOT BE GUARANTEED. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ESTABLISH THE LEVEL OF ALL EXISTING SERVICES PRIOR TO THE COMMENCEMENT OF WORK.

ON COMPLETION OF STORMWATER INSTALLATION, ALL DISTURBED AREAS MUST BE RESTORED TO ORIGINAL CONDITION, INCLUDING KERBS, FOOTPATHS, CONCRETE AREAS, GRAVEL AND GRASSED AREAS AND ROAD PAVEMENTS, UNLESS DIRECTED OTHERWISE.

ALL STORMWATER MANAGEMENT MEASURES SHOWN ON THIS DRAWING HAVE BEEN PREPARED FOR DEVELOPMENT APPLICATION PURPOSES TO DEMONSTRATE FEASIBILITY. ALL MEASURES WILL BE SUBJECT TO DETAIL DESIGN AT THE CONSTRUCTION CERTIFICATE STAGE AND MAY BE SUBJECT TO VARIATION PROVIDED THAT THE DESIGN INTENT IS MAINTAINED.

# STORMWATER DRAINAGE

- 1. ALL DRAINAGE LINES SHALL BE UPVC (CLASS SN4) SEWER GRADE DRAINAGE PIPE, U.N.O.
- 2. ALL DRAINAGE LINES SHALL BE LAID AT 1% MIN. FALL, UNO.
- 3. ALL LEVELS ARE AUSTRALIAN HEIGHT DATUM (AHD).
- 4. ALL DOWNPIPES GUTTERS TO BE DESIGNED IN ACCORDANCE WITH AS/NZS 3500.3.2 - 2003 'STORMWATER' DRAINAGE
- THE STORMWATER DRAINAGE DESIGN HAS BEEN CARRIED OUT IN ACCORDANCE WITH AS/NZS 3500.3.2-2003 'STORMWATER' DRAINAGE.
- 6. ANY VARIATIONS TO THE NOMINATED LEVELS SHALL BE REFERRED TO ENGINEER IMMEDIATELY.
- SUBSOIL DRAINAGE SHALL BE PROVIDED TO ALL RETAINING WALLS & EMBANKMENTS, WITH THE LINES FEEDING INTO THE STORMWATER DRAINAGE SYSTEM.
- 8. ALL GRATES TO BE GALVANISED STEEL WITH HINGES AND CHILD PROOF LOCK.
- 9. THE STORMWATER DRAINAGE IS DESIGNED IN ACCORDANCE WITH NORTHERN BEACHES COUNCIL WATER MANAGEMENT FOR DEVELOPMENT POLICY 2021.

# RAINWATER RE-USE

- PROVIDE RAINWATER RE-USE SYSTEM TO SUPPLY WATER FOR TOILET FLUSHING AND IRRIGATION
- 2. GUTTER GUARD TO BE INSTALLED ON ALL EAVES GUTTERS.
- A PERMANENT SIGN IS TO BE LOCATED IN THE VICINITY OF THE TANK STATING THE WATER IS "NON POTABLE WATER" WITH APPROPRIATE HAZARD IDENTIFICATION.
- 4. PIPEWORK USED FOR RAINWATER SERVICES SHALL BE COLOURED LILAC IN ACCORDANCE WITH AS1345.
- ALL VALVES AND APERTURES SHALL BE CLEARLY AND PERMANENTLY LABELLED WITH SAFETY SIGNS TO COMPLY WITH AS1319.
- 6. RAINWATER TANK RETICULATION SYSTEM AND MAINS WATER BYPASS ARRANGEMENT TO BE INSTALLED IN ACCORDANCE WITH AS/NZS 3500.1.2-2003 AND THE NSW CODE OF PRACTICE: PLUMBING AND DRAINING.
- 7. A FIRST FLUSH FILTRATION DEVICE IS TO BE PROVIDED AT RAINWATER TANK.

# **DESIGN SUMMARY**

SITE DISCHARGE CALCULATIONS:

	<u>20% AEP</u>	<u>5% AEP</u>	<u>1% AEP</u>
PRE-DEVELOPMENT	0.364 m³/s	0.688 m³/s	1.115 m³/s
POST-DEVELOPMENT	0.284 m³/s	0.495 m³/s	0.937 m <sup>3</sup> /s

## ON-SITE DETENTION:

PRE TO POST DEVELOPMENT CONDITIONS

BELOW GROUND BLOCK WORK TANK

ON-SITE DETENTION SUMMARY:

TOP WATER LEVEL = RL196.150 OVERFLOW LEVEL = RL195.900 ORIFICE CENTERLINE = RL194.345

RAINWATER RE-USE:

ORIFICE DIAMETER = Ø350mm

IN ACCORDANCE WITH BASIX/COUNCIL REQUIREMENTS. RAINWATER RE-USE STORAGE PROVIDED IS TO BE DESIGNED

RAINWATER RE-USE TO BE USED FOR THE FOLLOWING;

 TOILET FLUSHING; IRRIGATION.

STORMWATER MANAGEMENT REQUIREMENTS HAVE BEEN CALCULATED IN ACCORDANCE WITH NORTHERN BEACHES COUNCIL WATER MANAGEMENT PLAN FOR DEVELOPMENT POLICY 2021: PART 9.0 ONSITE STORMWATER MANAGEMENT.

## WATER QUALITY:

MUSIC MODEL SUMMARY (REFER NORTHROP REPORT FOR FURTHER DETAILS).

TREATMENT NODES:

RAINWATER RE-USE TANK

'OCEANGUARD' PIT INSERTS (OR SIMILAR)

FILTER CARTRIDGES

MUSIC MODEL PARAMETERS IN ACCORDANCE WITH NORTHERN BEACHES COUNCIL'S MUSIC MODELLING GUIDELINES 2016.

# CONCEPT SOIL & WATER MANAGEMENT

- 1. ALL WORK IS TO BE CARRIED OUT IN ACCORDANCE WITH RELEVANT ORDINANCES AND REGULATIONS; NOTE IN PARTICULAR THE REQUIREMENTS OF LANDCOMS MANAGING URBAN STORMWATER, SOILS AND CONSTRUCTION' (THE 'BLUE BOOK'). THIS SOIL AND WATER MANAGEMENT PLAN DETAILS THE ACTIONS TO BE TAKEN FOR THE MANAGEMENT AND DEWATERING OF STORMWATER DURING CONSTRUCTION OF THE PROPOSED BUILDING.
- INSTALL SEDIMENT PROTECTION FILTERS ON ALL NEW AND EXISTING STORMWATER INLET PITS IN ACCORDANCE WITH EITHER THE MESH AND GRAVEL INLET FILTER DETAIL SD6-11 OR THE GEOTEXTILE INLET
- FILTER DETAIL SD6-12 OF THE 'BLUE BOOK'. 3. ESTABLISH ALL REQUIRED SEDIMENT FENCES IN ACCORDANCE WITH DETAIL SD6-8 OF THE 'BLUE BOOK'.
- 4. INSTALL SEDIMENT FENCING AROUND INDIVIDUAL BUILDING ZONES/AREAS AS REQUIRED AND AS DIRECTED BY THE
- SUPERINTENDENT. 5. ALL TRENCHES INCLUDING ALL SERVICE TRENCHES AND SWALE EXCAVATION SHALL BE SIDE-CAST TO THE HIGH SIDE AND CLOSED AT THE END OF EACH DAYS WORK.
- THE CONTRACTOR SHALL ENSURE THAT ALL VEGETATION (TREE, SHRUB & GROUND COVER) WHICH IS TO BE RETAINED SHALL BE PROTECTED DURING THE DURATION OF CONSTRUCTION. REFER ARCHITECTS PLANS FOR TREES TO BE KEPT.
- 7. ALL VEGETATION TO BE REMOVED SHALL BE MULCHED ONSITE AND SPREAD/STOCKPILED AS DIRECTED BY THE SUPERINTENDENT.
- 8. STRIP TOPSOIL IN AREAS DESIGNATED FOR STRIPPING AND STOCKPILE FOR RE-USE AS REQUIRED. ANY SURPLUS MATERIAL SHALL BE REMOVED FROM SITE AND DISPOSED OF IN ACCORDANCE
- WITH EPA GUIDELINES. 9. CONSTRUCT AND MAINTAIN ALL MATERIAL STOCKPILES IN ACCORDANCE WITH DETAIL SD4-1 OF THE 'BLUE BOOK' (INCLUDING CUT-OFF SWALES TO THE HIGH SIDE AND SEDIMENT FENCES TO THE
- LOW SIDE). 10. ENSURE STOCKPILES DO NOT EXCEED 2.0m HIGH. PROVIDE WIND AND
- RAIN EROSION PROTECTION AS REQUIRED IN ACCORDANCE WITH THE 'BLUE BOOK'.
- 11. PROVIDE WATER TRUCKS OR SPRINKLER DEVICES DURING CONSTRUCTION AS REQUIRED TO SUPPRESS DUST.
- 12. ONCE CUT/FILL OPERATIONS HAVE BEEN FINALIZED ALL DISTURBED AREAS THAT ARE NOT BEING WORKED ON SHALL BE RE-VEGETATED AS SOON AS IS PRACTICAL.
- 13. THE CONTRACTOR SHALL BE RESPONSIBLE FOR KEEPING A DETAILED WRITTEN RECORD OF ALL EROSION & SEDIMENT CONTROLS ON-SITE DURING THE CONSTRUCTION PERIOD. THIS RECORD SHALL BE UPDATED ON A DAILY BASIS & SHALL CONTAIN DETAILS ON THE CONDITION OF CONTROLS AND ANY/ ALL MAINTENANCE, CLEANING & BREACHES. THIS RECORD SHALL BE KEPT ON-SITE AT ALL TIMES AND SHALL BE MADE AVAILABLE FOR INSPECTION BY THE PRINCIPAL CERTIFYING AUTHORITY AND THE SUPERINTENDENT DURING NORMAL WORKING
- 14. GROUNDWATER SEEPAGE RATES AND QUALITY TO BE MONITORED AND TREATED IF REQUIRED DURING CONSTRUCTION IN ACCORDANCE WITH REQUIREMENTS OF SUPERVISING GEOTECHNICAL ENGINEER.

# NOT FOR CONSTRUCTION

CIVIL ENGINEERING PACKAGE

**SPECIFICATION NOTES** 

FLOWER POWER

277 MONA VALE ROAD,

**TERREY HILLS NSW** 

211979

DRAWING NUMBER DRAWING SHEET SIZE = A1

DESCRIPTION ISSUED FOR INFORMATION 23.05.23 AC 15.06.23 ISSUED FOR INFORMATION

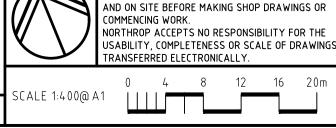
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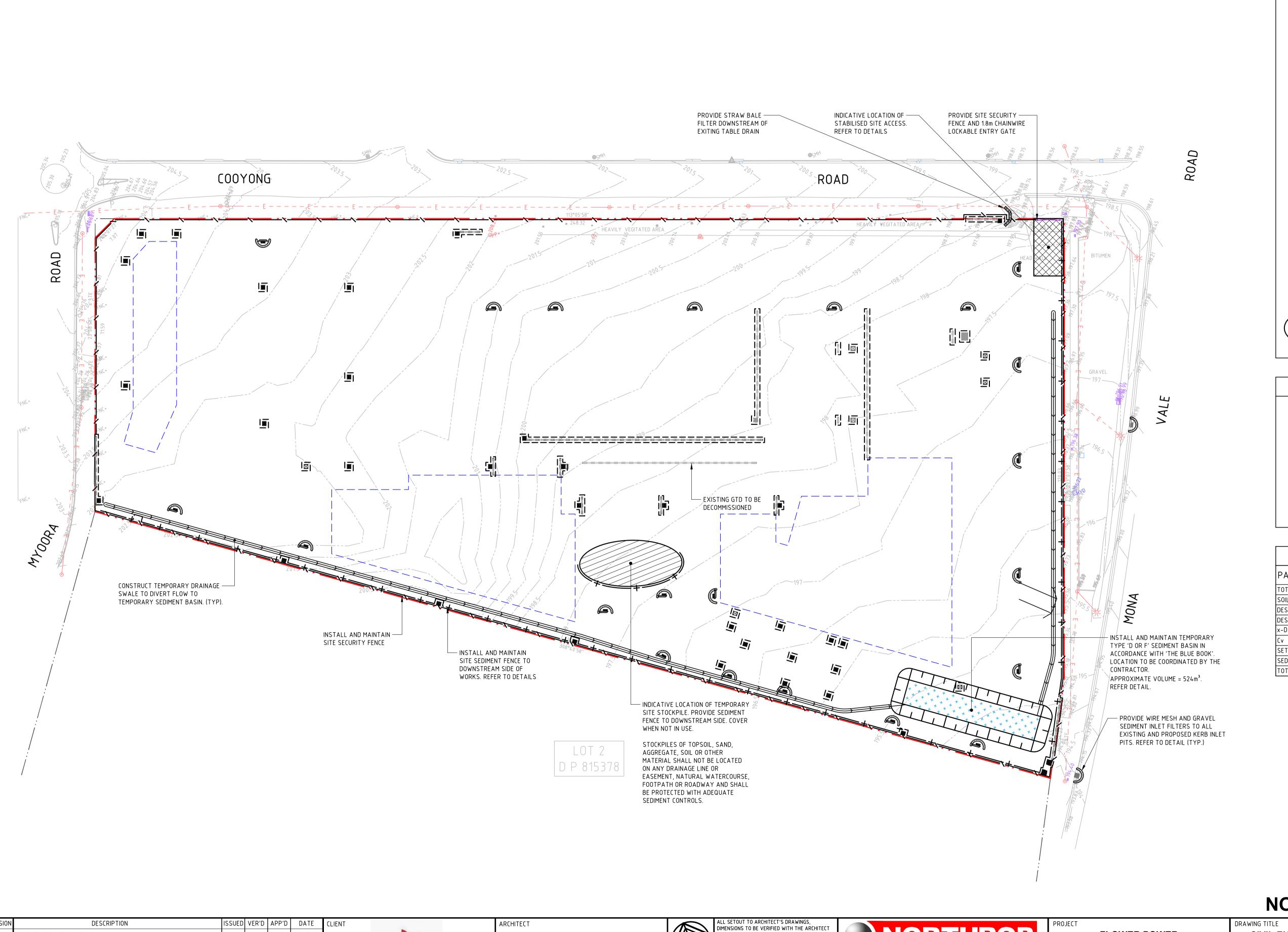
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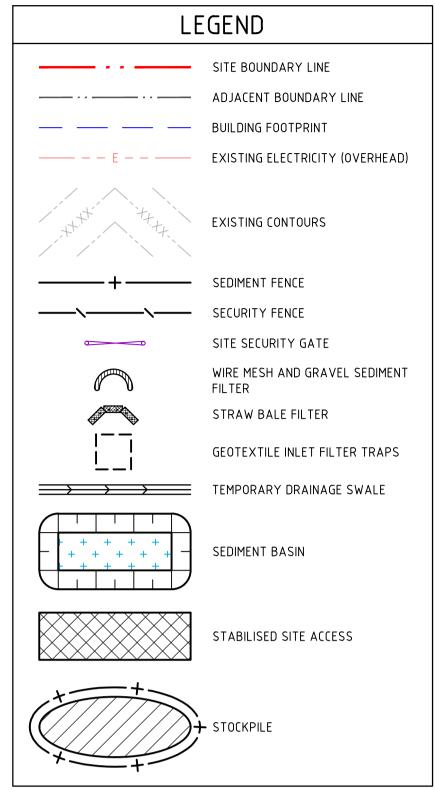


DIMENSIONS TO BE VERIFIED WITH THE ARCHITECT

Level 11 345 George Street, Sydney NSW 2000

Ph (02) 9241 4188 Fax (02) 9241 4324 sydney@northrop.com.au ABN 81 094 433 100





# GENERAL NOTES

- ALL WORKS TO BE CARRIED OUT IN ACCORDANCE WITH COUNCIL / RELEVANT AUTHORITY SPECIFICATIONS AND
- 2. ALL SEDIMENT AND SOIL EROSION CONTROL MEASURES TO BE INSTALLED IN ACCORDANCE WITH THE 'BLUE BOOK'.
  CONTRACTOR TO ENSURE THESE MEASURES ARE IN PLACE AND MAINTAINED AT ALL TIMES DURING CONSTRUCTION
- 3. CONTRACTOR TO PROVIDE 'WIRE MESH AND GRAVEL SEDIMENT FILTER' TO ALL PAVED / ROAD AREAS (BOTH PROPOSED AND EXISTING) IN ACCORDANCE WITH THE 'BLUE
- BOOK'.

  4. CONTRACTOR TO PROVIDE 'GEOTEXTILE INLET FILTER TRAPS'
  TO ALL STORMWATER DRAINAGE INLETS (BOTH PROPOSED

AND EXISTING) IN ACCORDANCE WITH THE 'BLUE BOOK'

SEDIMENT BASIN CALCULATIONS					
PARAMETER	ADOPTED VALUE				
TOTAL DISTURBED AREA (ha)	2.3644				
SOIL TEXTURE GROUP	D OR F				
DESIGN RAINFALL DEPTH (DAYS)	5				
DESIGN RAINFALL DEPTH (PERCENTILE)	80%				
x-DAY, y-PERCENTILE RAINFALL EVENT	35.2				
Cv	0.42				
SETTLING ZONE VOLUME (m³)	349.550				
SEDIMENT STORAGE VOLUME (m³)	174.775				
TOTAL BASIN VOLUME REQUIRED (m³)	524.325				

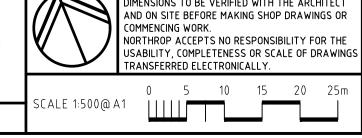
# NOT FOR CONSTRUCTION

					DR.
02	ISSUED FOR INFORMATION	MM	AC	15.06.23	
01	ISSUED FOR INFORMATION	MM	AC	23.05.23	

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FLOWER POWER 277 MONA VALE ROAD, TERREY HILLS NSW CIVIL ENGINEERING PACKAGE

SEDIMENT AND SOIL EROSION CONTROL PLAN

JOB NUMBER
211979

DRAWING NUMBER REV

DRAWING NUMBER REVISION

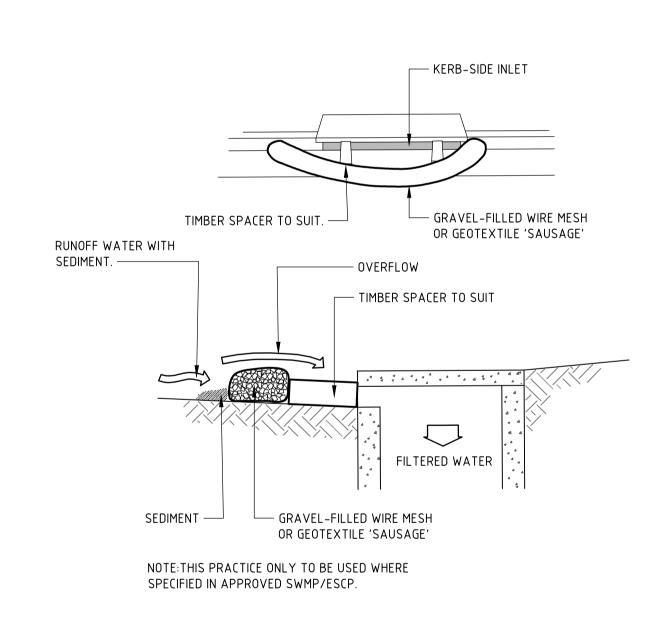
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DRAWING SHEET SIZE = A1

## CONSTRUCTION NOTES

- 1. THE TEMPORARY ACCESS SHALL BE MAINTAINED IN A CONDITION THAT PREVENTS TRACKING OR FLOWING OF
- SEDIMENT ONTO PUBLIC RIGHTS OF WAY, • THIS MAY REQUIRE PERIODIC TOP DRESSING WITH ADDITIONAL GRAVEL AS CONDITIONS DEMAND AND REPAIR AND/OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT,
- 2. ALL SEDIMENT SPILLED, DROPPED, WASHED OR TRACKED ONTO PUBLIC RIGHTS OF WAY MUST BE REMOVED IMMEDIATELY.
- 3. INSTALL BARRIER ON EITHER SIDE OF SHAKER PAD. TO ENSURE VEHICLES ARE GUIDED ON TO THE PAD.
- 4. INVERT OF SHAKER PAD TO BE DRAINED VIA AGRICULTURAL PIPE WRAPPED IN GEOTEXTILE FABRIC.

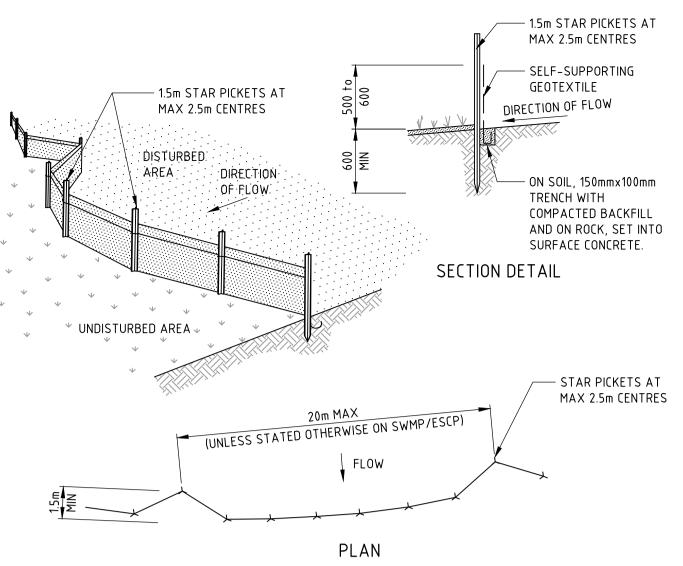
# STABILISED SITE ACCESS



# CONSTRUCTION NOTES

- 1. INSTALL FILTERS TO KERB INLETS ONLY AT SAG POINTS.
- 2. FABRICATE A SLEEVE MADE FROM GEOTEXTILE OR WIRE MESH LONGER THAN THE LENGTH OF THE INLET PIT
- AND FILL IT WITH 25mm TO 50mm GRAVEL.
- 3. FORM AN ELLIPTICAL CROSS-SECTION ABOUT 150mm HIGH x 400mm WIDE.
- 4. PLACE THE FILTER AT THE OPENING LEAVING AT LEAST A 100mm SPACE BETWEEN IT AND THE KERB INLET. MAINTAIN THE OPENING WITH SPACER BLOCKS.
- FORM A SEAL WITH THE KERB TO PREVENT SEDIMENT BYPASSING THE FILTER.
- 6. SANDBAGS FILLED WITH GRAVEL CAN SUBSTITUTE FOR THE MESH OR GEOTEXTILE PROVIDING THEY ARE PLACED SO THAT THEY FIRMLY ABUT EACH OTHER AND SEDIMENT-LADEN WATERS CANNOT PASS BETWEEN.

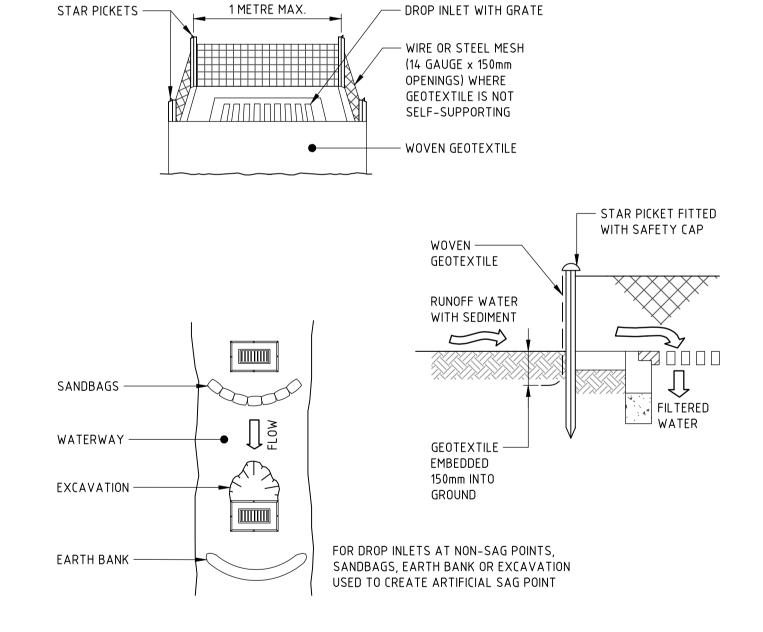
# WIRE MESH AND GRAVEL SEDIMENT FILTER



## CONSTRUCTION NOTES

- CONSTRUCT SEDIMENT FENCES AS CLOSE AS POSSIBLE TO BEING PARALLEL TO THE CONTOURS OF THE SITE, BUT WITH SMALL RETURNS AS SHOWN IN THE DRAWING TO LIMIT THE CATCHMENT AREA OF ANY ONE SECTION. THE CATCHMENT AREA SHOULD BE SMALL ENOUGH TO LIMIT WATER FLOW IF CONCENTRATED AT ONE POINT TO 50 LITRES PER SECOND IN THE DESIGN STORM EVENT, USUALLY THE 10-YEAR EVENT.
- 2. CUT A 150mm DEEP TRENCH ALONG THE UPSLOPE LINE OF THE FENCE FOR THE BOTTOM OF THE FABRIC TO BE ENTRENCHED.
- 3. DRIVE 1.5 METRE LONG STAR PICKETS INTO GROUND AT 2.5 METRE INTERVALS (MAX) AT THE DOWNSLOPE EDGE OF THE TRENCH. ENSURE ANY STAR PICKETS ARE FITTED WITH SAFETY CAPS.
- 4. FIX SELF-SUPPORTING GEOTEXTILE TO THE UPSLOPE SIDE OF THE POSTS ENSURING IT GOES TO THE BASE OF THE TRENCH. FIX THE GEOTEXTILE WITH WIRE TIES OR AS RECOMMENDED BY THE MANUFACTURER. ONLY USE GEOTEXTILE SPECIFICALLY PRODUCED FOR SEDIMENT FENCING. THE USE OF SHADE CLOTH FOR THIS PURPOSE IS NOT SATISFACTORY.
- 5. JOIN SECTIONS OF FABRIC AT A SUPPORT POST WITH A 150mm OVERLAP.
- 6. BACKFILL THE TRENCH OVER THE BASE OF THE FABRIC AND COMPACT IT THOROUGHLY OVER THE GEOTEXTILE.

# SEDIMENT FENCE

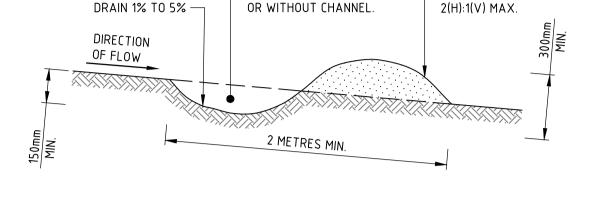


# CONSTRUCTION NOTES

- 1. FABRICATE A SEDIMENT BARRIER MADE FROM GEOTEXTILE OR STRAW BALES.
- 2. FOLLOW STANDARD DRAWING 6-7 AND STANDARD DRAWING 6-8 FOR INSTALLATION PROCEDURES FOR THE STRAW BALES OR GEOFABRIC. REDUCE THE PICKET SPACING TO 1 METRE CENTRES.
- 3. IN WATERWAYS, ARTIFICIAL SAG POINTS CAN BE CREATED WITH SANDBAGS OR EARTH BANKS AS SHOWN IN THE DRAWING.
- 4. DO NOT COVER THE INLET WITH GEOTEXTILE UNLESS THE DESIGN IS ADEQUATE TO ALLOW FOR ALL WATERS TO BYPASS IT.

GEOTEXTILE INLET FILTER TRAPS

ARCHITECT



— CAN BE CONSTRUCTED WITH

─ ALL BATTER GRADES

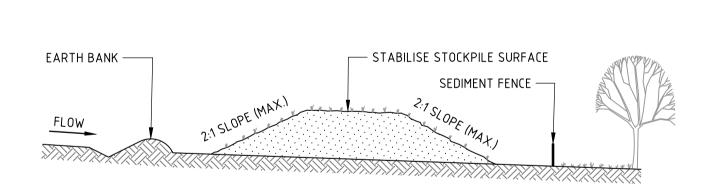
## CONSTRUCTION NOTES

BUILD WITH GRADIENTS BETWEEN 1 AND 5 PERCENT.

GRADIENT OF

- 2. AVOID REMOVING TREES AND SHRUBS IF POSSIBLE WORK AROUND THEM.
- 3. ENSURE THE STRUCTURES ARE FREE OF PROJECTIONS OR OTHER IRREGULARITIES THAT COULD IMPEDE WATER FLOW.
- 4. BUILD THE DRAINS WITH CIRCULAR, PARABOLIC OR TRAPEZOIDAL CROSS SECTIONS, NOT V SHAPED.
- 5. ENSURE THE BANKS ARE PROPERLY COMPACTED TO PREVENT FAILURE.
- 6. COMPLETE PERMANENT OR TEMPORARY STABILISATION WITHIN 10 DAYS OF CONSTRUCTION.

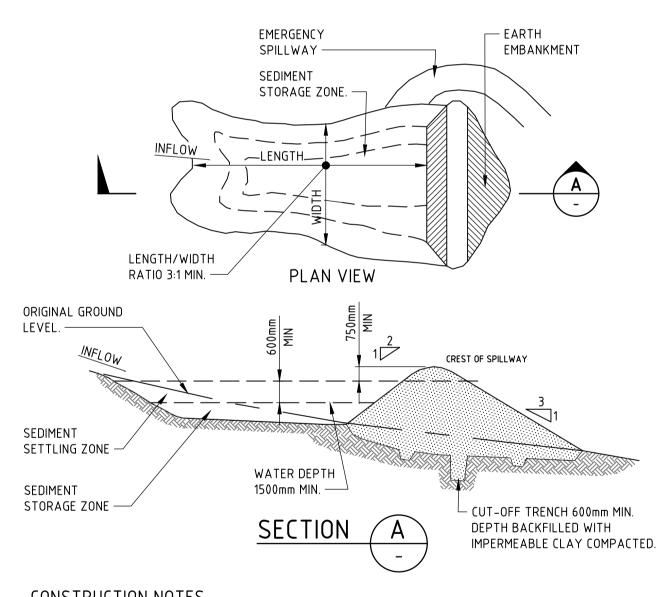
NOTE: ONLY TO BE USED AS TEMPORARY BANK WHERE MAXIMUM UPSLOPE LENGTH IS 80 METRES. TEMPORARY DRAINAGE SWALE



# **CONSTRUCTION NOTES**

- 1. PLACE STOCKPILES MORE THAN 2m (PREFERABLY 5m) FROM EXISTING VEGETATION, CONCENTRATED WATER FLOW, ROADS AND HAZARD AREAS.
- 2. CONSTRUCT ON THE CONTOUR AS LOW, FLAT, ELONGATED MOUNDS.
- 3. WHERE THERE IS SUFFICIENT AREA, TOPSOIL STOCKPILES SHALL BE LESS THAN 2m IN HEIGHT.
- 4. WHERE THEY ARE TO BE IN PLACE FOR MORE THAN 10 DAYS, STABILISE FOLLOWING THE APPROVED ESCP OR SWMP TO REDUCE THE C-FACTOR TO LESS THAN 0.10.
- 5. CONSTRUCT EARTH BANKS (STANDARD DRAWING 5-5) ON THE UPSLOPE SIDE TO DIVERT WATER AROUND STOCKPILES AND SEDIMENT FENCES (STANDARD DRAWING 6-8) 1 TO 2m DOWNSLOPE.

**STOCKPILE** 



# CONSTRUCTION NOTES

- 1. REMOVE ALL VEGETATION AND TOPSOIL FROM UNDER THE DAM WALL AND FROM WITHIN THE STORAGE AREA. 2. CONSTRUCT A CUT-OFF TRENCH 500mm DEEP AND 1200mm WIDE ALONG THE CENTRELINE OF THE EMBANKMENT
- EXTENDING TO A POINT ON THE GULLY WALL LEVEL WITH THE RISER CREST.
- MAINTAIN THE TRENCH FREE OF WATER AND RECOMPACT THE MATERIALS WITH EQUIPMENT AS SPECIFIED IN THE SWMP TO 95 PER CENT STANDARD PROCTOR DENSITY.
- 4. SELECT FILL FOLLOWING THE SWMP THAT IS FREE OF ROOTS, WOOD, ROCK, LARGE STONE OR FOREIGN MATERIAL.
- 5. PREPARE THE SITE UNDER THE EMBANKMENT BY RIPPING TO AT LEAST 100mm TO HELP BOND COMPACTED FILL TO THE EXISTING SUBSTRATE.
- 6. SPREAD THE FILL IN 100mm TO 150mm LAYERS AND COMPACT IT AT OPTIMUM MOISTURE CONTENT FOLLOWING THE
- 7. CONSTRUCT THE EMERGENCY SPILLWAY.
- 8. REHABILITATE THE STRUCTURE FOLLOWING THE SWMP.

(APPLIES TO 'TYPE D' AND 'TYPE F' SOILS ONLY) SEDIMENT BASIN

# NOT FOR CONSTRUCTION

REVISION	DESCRIPTION	ISSUED	VER'D	APP'D	DATE
01	ISSUED FOR INFORMATION	MM		AC	23.05.23
02	ISSUED FOR INFORMATION	MM		AC	15.06.23

SIGNATURE HAS BEEN ADDED

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FLOWER POWER 277 MONA VALE ROAD, **TERREY HILLS NSW** 

CIVIL ENGINEERING PACKAGE

SEDIMENT AND SOIL EROSION CONTROL DETAILS

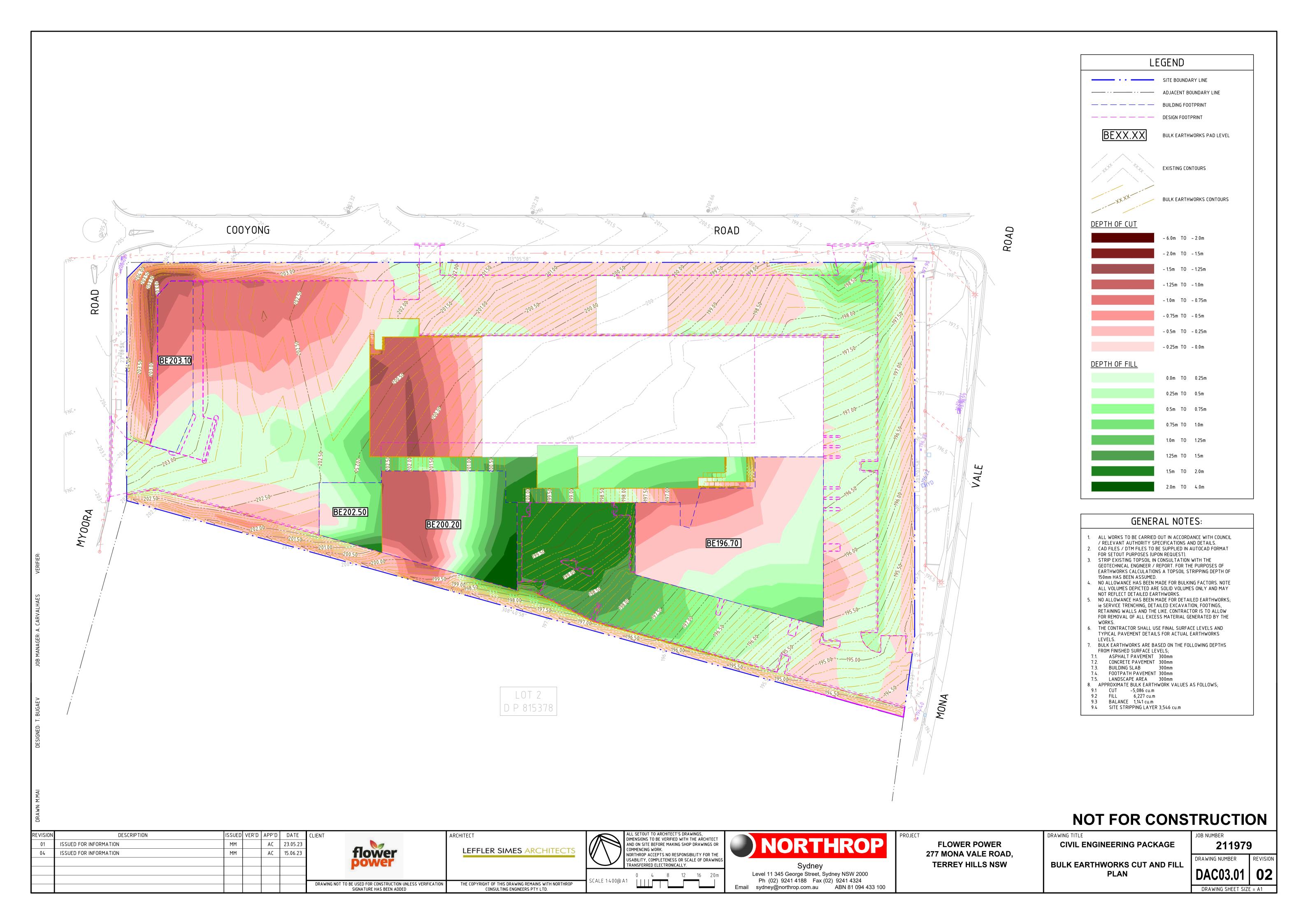
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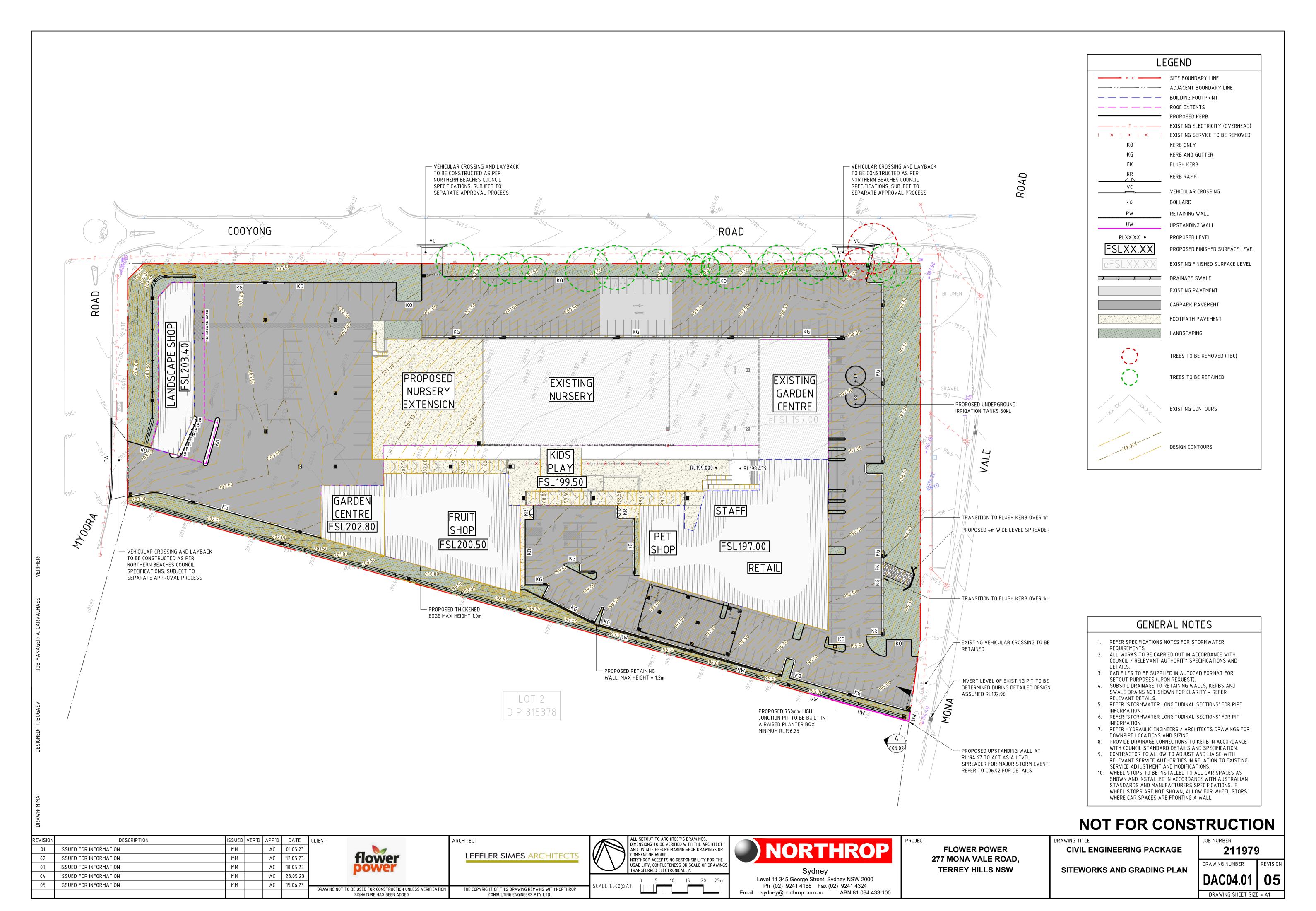
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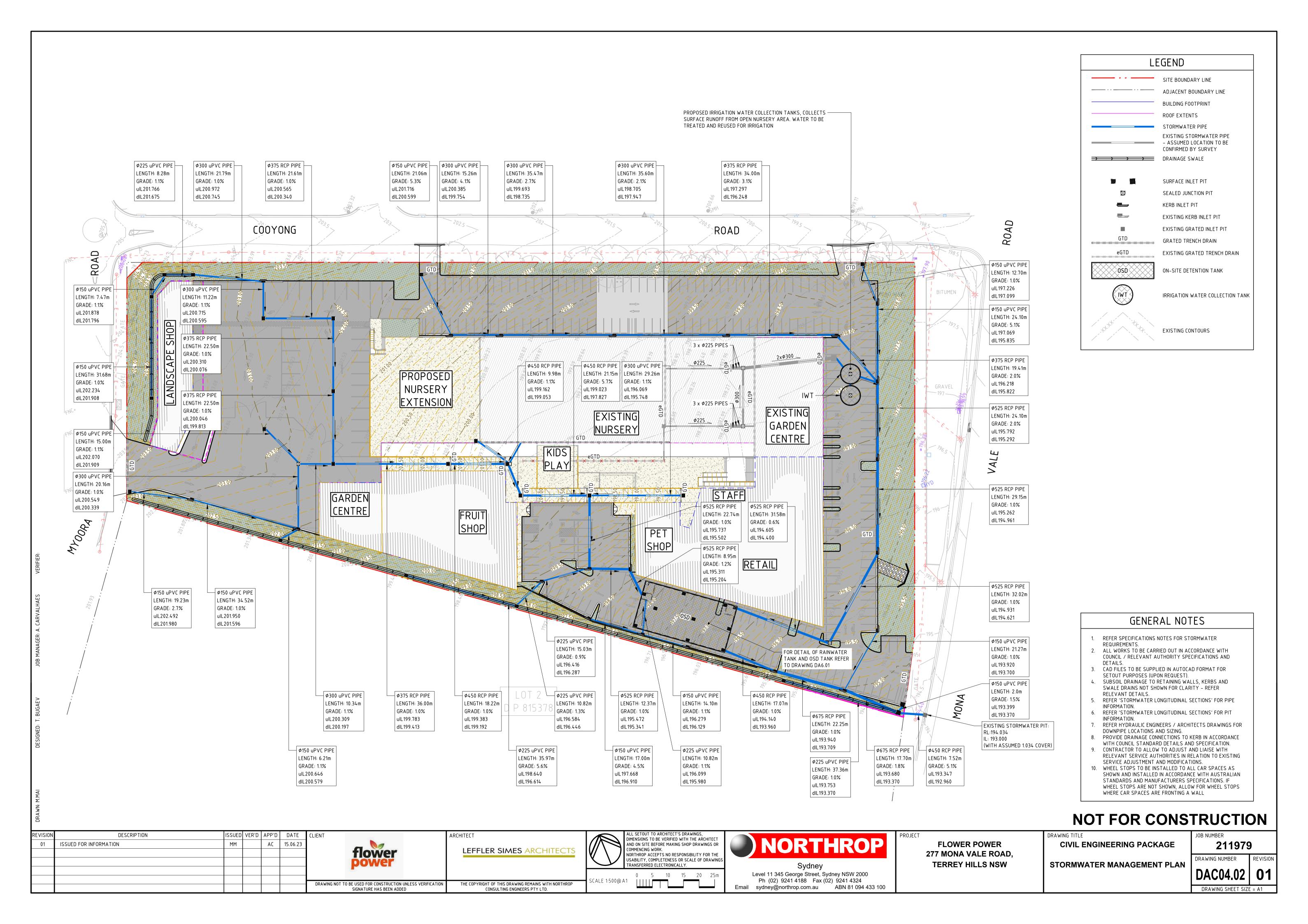
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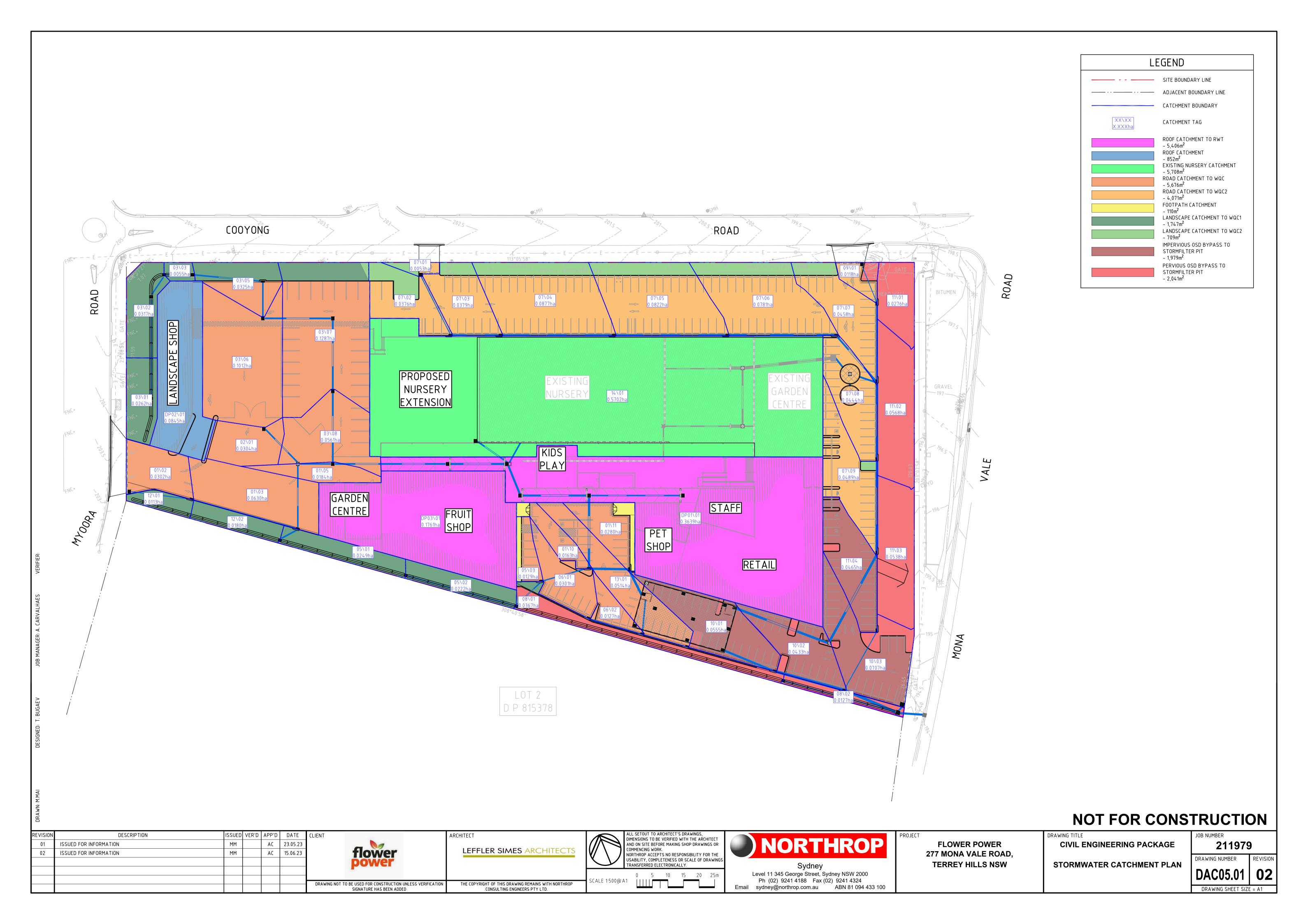
CONSULTING ENGINEERS PTY LTD.

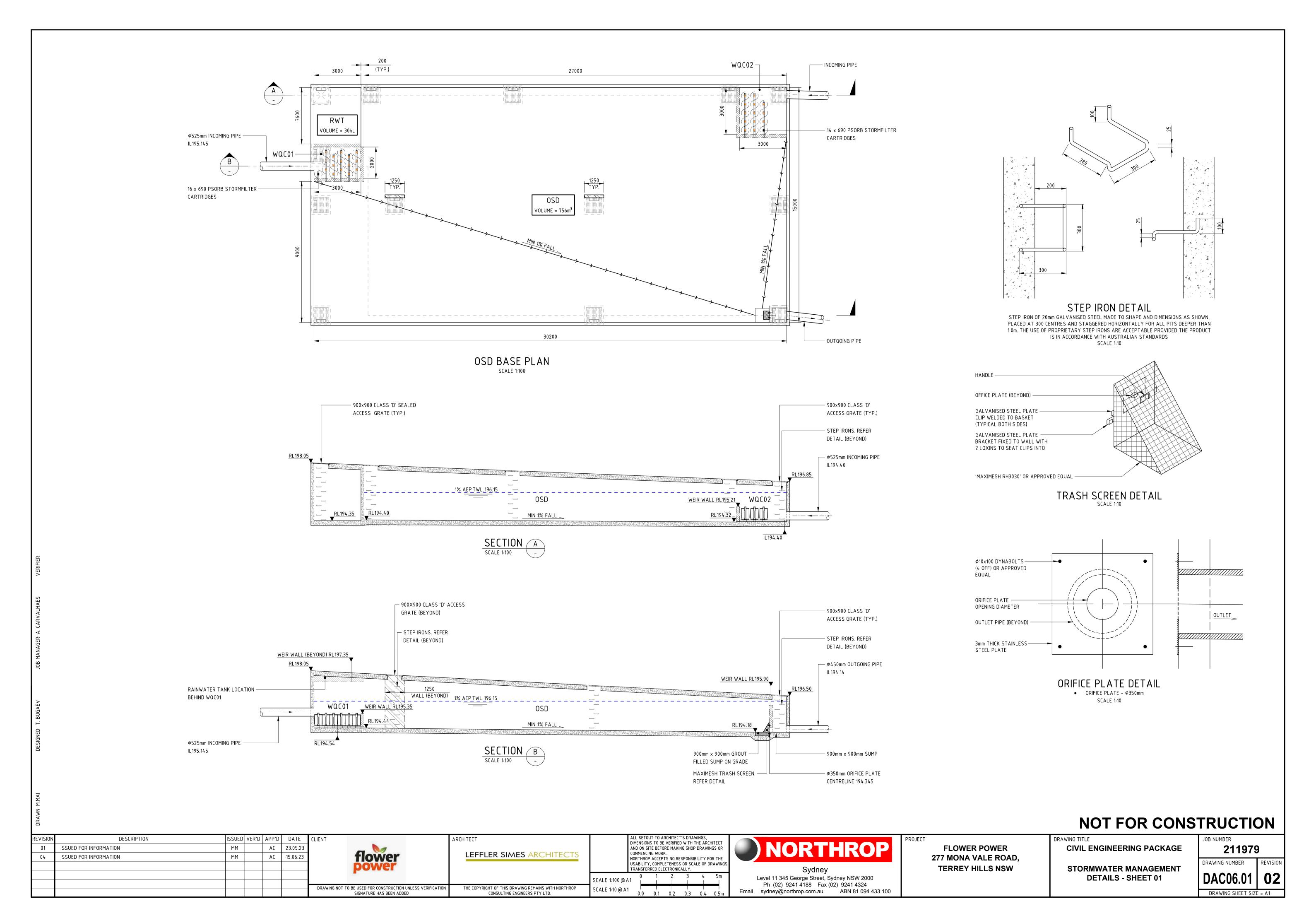
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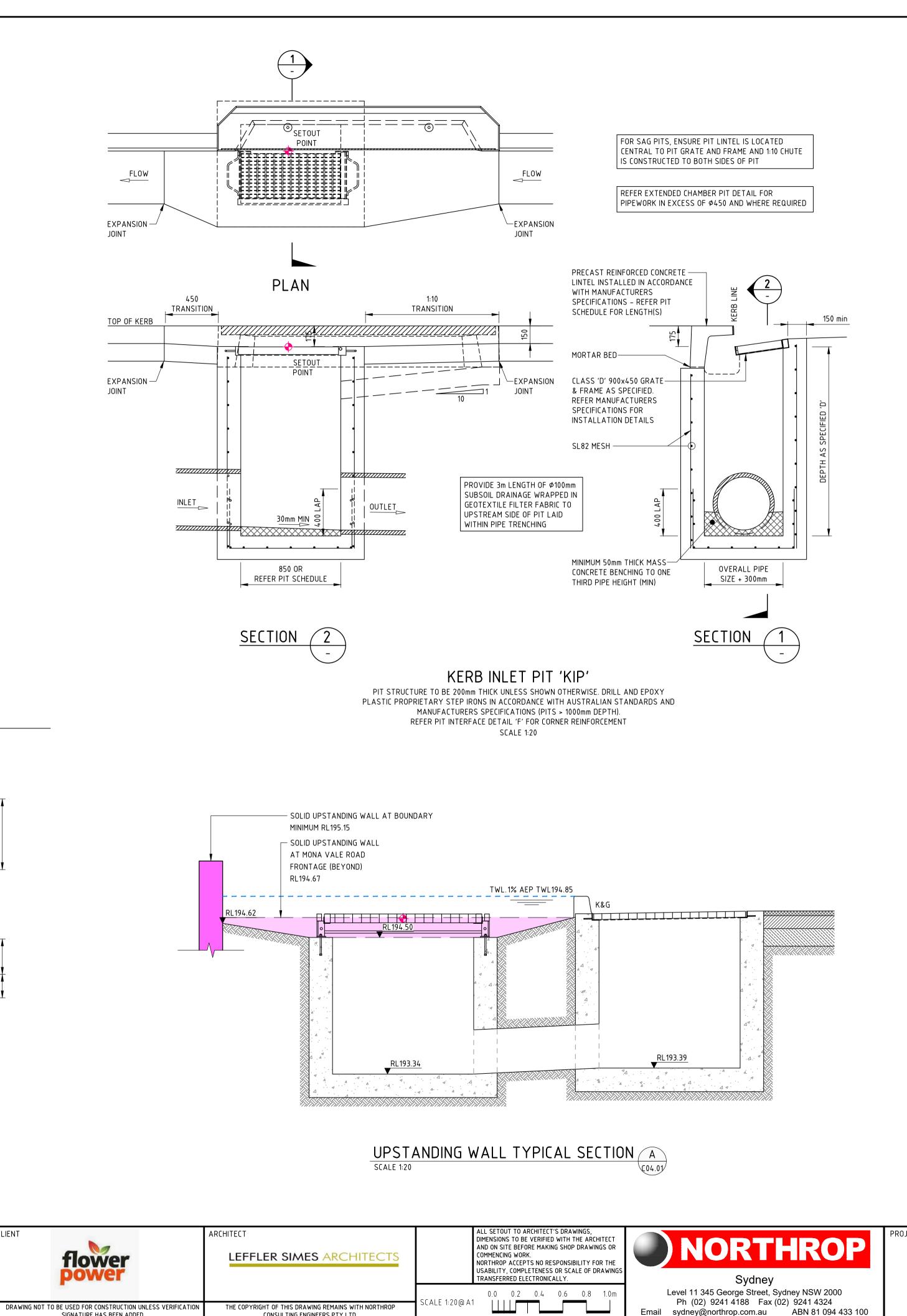












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DRAWING SHEET SIZE = A1

DRAWING NUMBER

DRAWING TITLE

CIVIL ENGINEERING PACKAGE

STORMWATER MANAGEMENT

**DETAILS - SHEET 02** 

**FLOWER POWER** 

277 MONA VALE ROAD,

TERREY HILLS NSW

TRENCH WIDTH = 0.D. + 300mm

TYPICAL PIPE TRENCH - GENERAL AREAS

1. TRENCH WIDTH MAY NEED TO BE INCREASED SUBJECT TO ACHIEVING COMPACTION. ENSURE MINIMUM 300mm CLEARANCE BETWEEN, WHEN USING MULTIPLE PIPES TO

3. THE CONTRACTOR SHALL ENSURE THAT SHORING OF TRENCHES IS INSTALLED AS

SCALE 1:10

DATE

15.06.23

4. ENSURE BACKFILLING COMPACTION MEETS THE FOLLOWING STANDARDS;
4.1. TRENCHES UNDER PAVED AREAS / BUILDING – 100% SMDD

COMPACTED BACKFILL -

OVERLAY ZONE, COMPACTED —

REINFORCED CONCRETE PIPE

- NOM 10mm AGGREGATE

BED ZONE, SELECT FILL -- NOM 10mm AGGREGATE

SUBGRADE MATERIAL -

HAUNCH ZONE, COMPACTED SELECT FILL-

ACHIEVE ADEQUATE COMPACTION.

DESCRIPTION

ISSUED FOR INFORMATION

REQUIRED BY STATUTORY REQUIREMENTS.

4.2. TRENCHES NOT UNDER PAVEMENTS – 95% SMDD

2. MINIMUM PIPE COVER NOT UNDER ROADS TO BE 300mm U.N.O.

ORDINARY FILL

