

GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER
FORM NO. 1 – To be submitted with Development Application

Development Application for Grexx Holdings Pty Ltd, Jonathan Odisho

Name of Applicant

Address of site _____

Declaration made by geotechnical engineer or engineering geologist or coastal engineer (where applicable) as part of a geotechnical report

I, Amir Eslami on behalf of Capital Engineering Consultants
(Insert Name) (Trading or Company Name)

on this the 14/05/25 certify that I am a geotechnical engineer or engineering geologist or coastal engineer as defined by the Geotechnical Risk Management Policy for Pittwater - 2009 and I am authorised by the above organisation/company to issue this document and to certify that the organisation/company has a current professional indemnity policy of at least \$10million.

I:
Please mark appropriate box

- ☒ have prepared the detailed Geotechnical Report referenced below in accordance with the Australia Geomechanics Society's Landslide Risk Management Guidelines (AGS 2007) and the Geotechnical Risk Management Policy for Pittwater - 2009
- ☒ am willing to technically verify that the detailed Geotechnical Report referenced below has been prepared in accordance with the Australian Geomechanics Society's Landslide Risk Management Guidelines (AGS 2007) and the Geotechnical Risk Management Policy for Pittwater - 2009
- ☒ have examined the site and the proposed development in detail and have carried out a risk assessment in accordance with Section 6.0 of the Geotechnical Risk Management Policy for Pittwater - 2009. I confirm that the results of the risk assessment for the proposed development are in compliance with the Geotechnical Risk Management Policy for Pittwater - 2009 and further detailed geotechnical reporting is not required for the subject site.
- ☐ have examined the site and the proposed development/alteration in detail and I am of the opinion that the Development Application only involves Minor Development/Alteration that does not require a Geotechnical Report or Risk Assessment and hence my Report is in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009 requirements.
- ☐ have examined the site and the proposed development/alteration is separate from and is not affected by a Geotechnical Hazard and does not require a Geotechnical Report or Risk Assessment and hence my Report is in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009 requirements.
- ☐ have provided the coastal process and coastal forces analysis for inclusion in the Geotechnical Report

Geotechnical Report Details:

Report Title: Geotechnical Investigation and Landslip Risk Assessment Report
GR24118rev01
Report Date: 01/05/25
:
Author: Dean Mitrevski
Author's Company/Organisation: Capital Engineering Consultants

Documentation which relate to or are relied upon in report preparation:

I am aware that the above Geotechnical Report, prepared for the abovementioned site is to be submitted in support of a Development Application for this site and will be relied on by Pittwater Council as the basis for ensuring that the Geotechnical Risk Management aspects of the proposed development have been adequately addressed to achieve an "Acceptable Risk Management" level for the life of the structure, taken as at least 100 years unless otherwise stated and justified in the Report and that reasonable and practical measures have been identified to remove foreseeable risk.

Signature

Name Amir Eslami

Chartered Professional Status..... CPENG. NER.

Membership No. 6905441

Company Capital Engineering Consultants

GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER
FORM NO. 1(a) - Checklist of Requirements For Geotechnical Risk Management Report for
Development Application

Development Application for Grex Holdings Pty Ltd, Jonathan Odisho

Name of Applicant

Address of site 1-3 Careel Head Rd, Avalon Beach

The following checklist covers the minimum requirements to be addressed in a Geotechnical Risk Management Geotechnical Report. This checklist is to accompany the Geotechnical Report and its certification (Form No. 1).

Geotechnical Report Details:

Report Title: Geotechnical Investigation and Landslip Risk Assessment Report, GR24118rev01

Report Date: 01/05/25

Author: Dean Mitrevski

Author's Company/Organisation: Capital Engineering Consultants

Please mark appropriate box

- 3 Comprehensive site mapping conducted _____
(date)
- 3 Mapping details presented on contoured site plan with geomorphic mapping to a minimum scale of 1:200 (as appropriate)
- 3 ☒ Subsurface investigation required
 - 3 ☐ No Justification
 - 3 ☒ Yes Date conducted 19/06/24
- 3 Geotechnical model developed and reported as an inferred subsurface type-section
- 3 Geotechnical hazards identified
 - 3 Above the site
 - 3 On the site
 - 3 ☒ Below the site
 - 3 ☐ Beside the site
- 3 ☒ Geotechnical hazards described and reported
- 3 ☒ Risk assessment conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
 - 3 Consequence analysis
 - 3 Frequency analysis
- 3 ☒ Risk calculation
- 3 ☒ Risk assessment for property conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
- 3 ☒ Risk assessment for loss of life conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
- 3 ☒ Assessed risks have been compared to "Acceptable Risk Management" criteria as defined in the Geotechnical Risk Management Policy for Pittwater - 2009
- 3 ☒ Opinion has been provided that the design can achieve the "Acceptable Risk Management" criteria provided that the specified conditions are achieved.
- 3 Design Life Adopted:
 - 3 100 years
 - 3 Other specify
- 3 ☒ Geotechnical Conditions to be applied to all four phases as described in the Geotechnical Risk Management Policy for Pittwater - 2009 have been specified
- 3 Additional action to remove risk where reasonable and practical have been identified and included in the report.
- 3 Risk assessment within Bushfire Asset Protection Zone.

I am aware that Pittwater Council will rely on the Geotechnical Report, to which this checklist applies, as the basis for ensuring that the geotechnical risk management aspects of the proposal have been adequately addressed to achieve an "Acceptable Risk Management" level for the life of the structure, taken as at least 100 years unless otherwise stated, and justified in the Report and that reasonable and practical measures have been identified to remove foreseeable risk.

Signature

Name Amir Eslami

Chartered Professional Status..... CPENG. NER.

Membership No. 6905441

Company Capital Engineering Consultants

GEOTECHNICAL INVESTIGATION AND LANDSLIP RISK ASSESSMENT REPORT

Client – Jonathan Odisho & Grex Holdings PTY LTD

Project Title – 1-3 Careel Head Rd, Avalon Beach

Project Type – Proposed Mixed Use Development

Project No. – GR24118rev01

Date Issued – 7/05/2025

Description of Services – Geotechnical Investigation and Landslip Risk Assessment Report

Document Control

Report Title: Geotechnical Investigation and Landslip Risk Assessment Report

Report No: GR24118rev01

Copies	Recipient
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Author		Technical Reviewer	
			
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Revision	Details	Date	Amended By
	Original	13/09/24	
01	Revision 1 Landslip Risk Assessment Updated	01/05/25	DM

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1. Introduction

1.1 Background

CEC Geotechnical Pty Ltd was engaged by the client Jonathan Odisho & Grex Holdings PTY LTD to carry out a geotechnical investigation and landslip risk assessment for the proposed mixed-use development at **1-3 Careel Head Rd, Avalon Beach**.

This report provides geotechnical recommendations regarding the existing ground conditions in order to have a clear guidance for design and construction of ground structures.

The subject site is identified as landslide risk pursuant to the LEP. The report provides a risk assessment in order to comply with the Geotechnical Risk Management Policy for Pittwater and Slope Stability Guidelines.

The results presented in this report are mainly based on the investigation, the laboratory tests and the author's experience. This report is prepared based on the information received from the client. If the site conditions change before, during or after construction, the geotechnical engineer shall be notified.

1.2 Provided Information

- A set of Architectural Drawings including site plans, floor plans, section plans prepared by CDArchitects, Rev: A, Date: 07/2024, Job No: J23587D.
- Survey Plan prepared by Geometra Consulting Ref 12943-1

1.3 Proposed Development

With reference to the information provided by the client, it is understood that the proposed development will comprise the demolition of existing dwelling and construction of mixed-use development with a single basement and associated landscape work.

The lowest proposed basement has a RL -0.25 and hence the maximum excavation depths will be approximately 3.9 m. Deeper excavation is expected locally for the proposed lift shafts, building footings and service pits and trenches.

1.4 Objectives

The objective of this report included:

- Assessment of subsurface profiles and groundwater considerations.
- Excavation methodologies and monitoring.
- Design parameters and allowable bearing capacities.
- Earthquake subsoil classification in accordance with Australian Standards "AS 1170.4-2007".
- Soil salinity and aggressivity assessment.
- Slope stability risk assessment
- General geotechnical recommendations regarding site preparation or further works required.

1.5 Scope of Works

The geotechnical site investigation was carried out on 19/06/2024 by an experienced geotechnical engineer in accordance with "AS 1726". The scope of works included:

- Desktop Study including a review of existing architectural drawings, survey plan, geology and topography of the site and neighbouring properties.

- Drilling of 3 boreholes (BH01 to BH03) by a 4x4 drill rig with 100 mm diameter solid flight auger to depth of 7m or TC-bit refusal and drilling of 1 borehole (BH04) by hand auger (location found in Appendix A and logs found in Appendix B)
- Dynamic Cone Penetration (DCP) testing in accordance with Australian Standards “AS 1289”
- Site classification (see Section 4.1) in accordance with Australian Standards “AS 2870 2011”.
- 4 soil samples were extracted from the borehole, to test the soil/rock salinity and aggressivity (see Appendix C)
- Review of existing documentation
- Hazard Analysis & Risk Assessment
- Remediation Measures

1.6 Constraints

If a more detailed geotechnical investigation regarding soil reactivity is available, it should be provided to CEC Geotechnical Pty Ltd. In addition, any details related to the site's history should be supplied. This report was produced based on a limited geotechnical investigation in line with the requirements of “AS 1726 and AS 2870”.

This classification is based on the findings in this investigation, including visual-tactile identification of the soil profile combined with the author's local knowledge and experience. If the site conditions change from those of the original investigation, the findings of this report may be void.

2. Desktop Assessment

2.1 General Site Description

The site is located within the Local Government Area (LGA) of Northern Beaches and is registered as SP32656 and Lot B DP385973. The site is covering an area of 1777 m² and is bounded by Careel Head Rd to the North and Barrenjoey Rd to the West with surrounding residential properties as shown in Figure 1. During the site visit it was observed that there was an existing single-storey mixed business and a residential dwelling surrounded by concrete pavers and grass.

Figure 1: Site Location (1 Careel Head Rd, Avalon Beach)



There is an existing residential single storey dwelling situated in the Northerly side of the site, surrounded by grass and trees. Within the middle of the site, it is accompanied by single storey mixed businesses, surrounded by concrete pavers as shown in Figure 1.

Figure 2: Site Dimensions



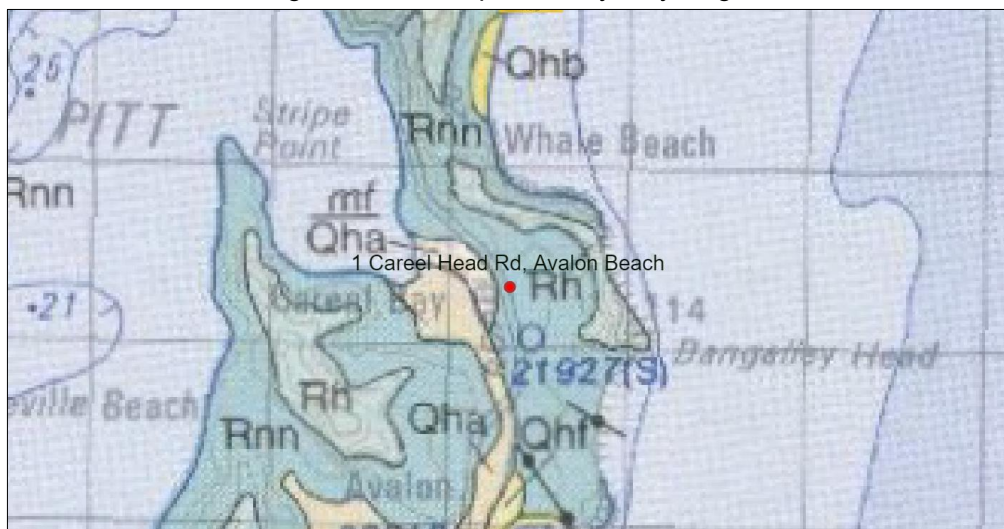
The longitudinal slopes from the highest point of the site (RL 4.52) to the lowest point (RL 2.15) is approximately 4° as shown in Figure 2.

2.2 Topography and Geological Description

Based on the architectural drawings, it can be noted that the site is sloping from Northeast to Southwest.

The 1:100,000 scale Geological Series Sheet 9130 (Edition 1) 1983 of the Sydney region (Figure 3) indicates that the subject site is underlain by Narrabeen group Terrigal Formation (**Rnn**) consists of interbedded laminite, shale, and quartz, to lithic-quartz sandstone. Minor red claystone north of Hawkesbury River. Clay pellet sandstone (Garnie Fm) south of Hawkesbury River.

Figure 3: 1:100,000 scale Geological Series Map of the Sydney Region



3. Geotechnical Investigation Results

3.1 Sub-Surface Conditions

The results of the investigation indicate that the subsurface profile comprises of Sandy CLAY and SAND underlain by SANDSTONE. Based on the borehole information, a summary of subsurface conditions is presented below. The location of each borehole can be found in Appendix A and more detailed logs can be found in Appendix B.

Table 1: Subsurface Conditions

Unit	Description	BH01 (m)*	BH02 (m)*	BH03 (m)*	BH04 (m)*
Unit – 1: Non-Soil	Concrete	0.0-0.3	0.0-0.3	-	-
Unit – 2: Topsoil	Clayey SILT ML: firm, low plasticity, brown	-	-	0.0-0.2	0.0-0.2
Unit – 3: Residual	Clayey SAND SC: loose, low plasticity clay, brown red dark grey	0.3-0.9	0.3-1.0	-	-
Unit – 4: Residual	Sandy CLAY CL-CI: stiff to very stiff, low to medium plasticity, grey brown red	0.9-1.5	1.0-1.4	0.2-1.0	0.2-1.5
Unit – 5: Residual	Sandy CLAY CL-CI: hard, low to medium plasticity, brown yellow red,	1.5-4.3	1.4-4.5	1.0-4.5	1.5-1.8
Unit – 6: Residual	SAND SP: dense to very dense, brown yellow	4.3-10	4.5-7.0	4.5-6.0	-
Unit – 7: Class V SST Rock	Extremely weathered, rock SANDSTONE (SST:) hard, non-plastic, yellow brown	-	-	6.0-7.2	-

*Depths below ground level (BGL) at the location of each borehole. This may vary depending on other areas of the site. BH01, BH02 and BH04 were terminated at depths of 10m, 7.0m and 1.8m respectively and BH03 met refusal at 7.2m.

3.2 Groundwater Observation

Groundwater was encountered within the investigated depths during the geotechnical investigation on the 19th of June 2024. However, due to the usage of water during concrete cutting, the data for the water level was not accurate.

Three groundwater monitoring wells were installed in the boreholes (BH01, BH02 & BH03). The Groundwater levels were measured during additional site visits on 11/07/24, 25/07/24 and 18/08/24 and are summarised in the table below. The measurement was conducted after a rainy week; therefore, the initial collected data was not accurate. Additional fieldwork was completed after two weeks to allow the groundwater levels to stabilise. A geotechnical engineer monitored the water levels after two weeks in order to establish a baseline groundwater level at the proposed site.

Table 2: Observed Groundwater Levels

Groundwater Well	Borehole	Ground RL	Groundwater Level (m bgl)/RL			Approximate depth of Rock Level Encountered bgl
			11/07/24	25/07/24	18/08/24	
GW1	BH01	2.13	0.32m/ RL 1.81	0.37m/ RL 1.76	0.35m/ RL 1.78	-
GW2	BH02	2.15	0.28m/ RL 1.87	0.60m/ RL 1.55	0.57m/ RL 1.58	-
GW3	BH03	4.62	1.64m/ RL 2.98	1.64m/ RL 2.98	1.65m/ RL 2.97	6.0m/ RL -1.38

3.3 Lab Test Results

4 samples were collected at depths of 0.5 m, 1.0 m, 3.0 m and 4.0 m. The samples were submitted to NATA accredited laboratories for further testing. These tests included:

- Chemical testing (Salinity, pH, Chloride (Cl), sulphates (SO₄), and electrical conductivity) to assess soil salinity and aggressivity.

3.3.1 Soil Salinity & Aggressivity Test Results

The soil encountered is Sandy CLAY and was tested as stipulated in the Department of Natural Resources (DNR) publication "Site Investigations for Urban Salinity" (2002).

Results of the laboratory testing are attached to this report in Appendix C and summarised in Tables 3 and 4.

Table 3: Results of Electrical Conductivity Test (Salinity)

Borehole	Depth (m bgl)	Electrical Conductivity (dS/m)	Multiplication Factor ^a	Electrical Conductivity of Saturated Extract (dS/m)	Soil Type
				EC _e	
BH01	0.5	0.069	14	0.966	Sandy Loam
BH02	1.0	0.037	14	0.518	Sandy Loam
BH03	4.0	0.027	14	0.378	Sandy Loam
BH04	3.0	0.044	14	0.616	Sandy Loam

"Site Investigations for Urban Salinity" (2002)

Non-saline <2 dS/m
Saline at >4 dS/m
Slightly saline 2-4 dS/m
Moderately saline 4-8 dS/m
Very saline 8-16 dS/m
Highly saline >16 dS/m

Table 4: Soil pH, Chloride, Sulphate and Electrical Resistivity Test Results (AS 2159-2009)

Borehole	Depth (m bgl)	MC* (%)	pH	Chloride (mg/kg)	Sulphate as SO ₄ ⁴ (mg/kg)	Electrical Resistivity (ohm.cm)
BH01	0.5	21	8.9	19	12	15000
BH02	1.0	24	7.9	36	<10	27000
BH03	4.0	56	6.6	15	87	37000
BH04	3.0	20	6.4	37	38	23000

*MC = Moisture Content

Note: Electrical Resistivity converted from Electrical Conductivity

4. Discussion and Recommendations

4.1 Site Classification

Due to the presence of trees and topsoil, the overall site is classified as **Class P** in accordance with "AS 2870 2011". Once topsoil/fill is removed, this site will then be classified as **CLASS H1** in accordance with "AS 2870 2011". Class H1 is indicative of highly reactive site which can experience **40 to 60mm** movement due to moisture changes.

At basement slab level the site can be classified as **CLASS H1** which is indicative of a highly reactive site which can experience **40 to 60mm** movement due to moisture changes in accordance with "AS 2870 2011".

4.2 Excavation Assessment

Accordance with the proposed basement levels, the excavations for the proposed basement should mostly encounter residual Sandy CLAY and SAND. Removal of materials should be carried out using conventional earthmoving equipment, such as a hydraulic excavator or backhoe. If rock hammers are to be used, such works will need to be completed carefully as there may be direct transmission of ground vibration to existing structures. We recommend that a geotechnical engineer to be present at the site as they may be required to carry out quantitative vibration monitoring to confirm vibration units do not exceed the maximum Peak Particle Velocity (PPV) values provided in Table 5 below.

Table 5: Recommended Maximum PPV

Type of Building or Structure	Maximum PPV (mm/sec)
Historical buildings or structures in sensitive conditions	2
Residential and low-rise buildings	5
Brick or unreinforced structures in good condition	10
Commercial and industrial buildings or structures of reinforced concrete or steel construction	25

It is recommended that monitoring is carried out during demolition and excavation using a vibration monitoring instrument (Vibra) and alarm levels (being the appropriate PPV).

If the vibrations exceed the above values or appear excessive, the excavations should cease, and the project Geotechnical Engineer should be contacted immediately.

4.3 Shoring/Footings Design Parameters

The following parameters established from Rankine's theory would be valid in the design of a retention system. These bearing pressures apply where footings are founded minimum 300mm into the specified material.

Table 6: Geotechnical Design Parameters

Material Description	Unit Weight (kN/m ³)	Allowable Bearing Capacity (kPa)	Ultimate Bearing Capacity (kPa)	C (kPa)	Friction Angle (°)	K _a [#]	K _p [#]	K _o [#]	Modulus of Elasticity (MPa)	Poisson's Ratio v'
Unit 3: Clayey SAND (SC): Loose	18	50	120	0	25	0.41	2.46	0.58	10	0.35
Unit 4: Sandy CLAY (CL-CI): Stiff to Very Stiff	19	150	370	0	28	0.36	2.77	0.53	5	0.3
Unit 5: Sandy CLAY CL-CI: Hard	20	250	620	0	28	0.36	2.77	0.53	10	0.3
Unit 6: SAND (SP): Dense to Very Dense	18	200	500	0	28	0.39	2.56	0.56	10	0.35
Unit 7: Extremely Weathered, Rock SANDSTONE (Class V)	21	750	1500	30	32	0.90	1.11	0.95	100	0.3

*Approximate depth below ground level based on borehole logs completed during geotechnical investigation.

[#]K_a, K_p and K_o are the active, passive and at-rest earth pressure coefficients.

Note: Unit1/Unit 2: Non-Soil/Topsoil design parameters are not applicable.

The following may need to be adopted in response to the design of any retaining wall structures:

- The retaining walls which are propped or restrained by concrete slabs, should be designed using a triangular lateral earth pressure distribution and the K_o, as mentioned in Table 6, for the soil profiles and other backfill materials.

- Free-standing cantilever walls should be designed using a triangular lateral earth pressure distribution and the K_a , as mentioned in Table 6, for the soil profiles and other backfill materials.
- Lateral toe restraint can be achieved through passive resistance adjacent to the wall using a triangular lateral earth pressure and the K_p , as mentioned in Table 6, for the soil profiles and other backfill materials.
- All surcharge loads should be allowed for in the retaining wall design, including building footings and construction related activities, using the appropriate earth pressure coefficient as mentioned in Table 6. (Sloping needs to be considered a surcharge)

4.4 Batter Slopes (Temporary)

The following temporary batter slopes may be considered in areas where sufficient space exists between the basement excavation and the boundary and where an adjacent footing is outside a zone of influence obtained by drawing a line at a 45° angle up from the toe of the proposed excavation.

Table 7: Recommended Batter Slopes (temporary)

Material	Maximum Batter Slope (H:V)
Topsoil / Fill	2:1
Sandy CLAY	1.5:1
SAND	1.5:1
Rock: SANDSTONE	1:1

4.5 Groundwater Management

Based on the investigation at the selected borehole and testing locations, available hydrogeological data and information for the site available at the time of the investigation, it is considered that groundwater was encountered within the depth of excavation and the basement needs to be tanked.

4.6 Earthquake Site Assessment

In accordance with Australian Standard “AS 1170.4-2007” (Reference 2) the site may be classified as a “Shallow Soil Site” (Class Ce).

4.7 Exposure Classification

The lab results show that the electrical conductivity of saturated extracts (EC_e) is approximately 0.378 ds/m to 0.966 ds/m, and hence the samples of residual deposit soil are “Non-Saline”.

- In accordance with “AS 2159” *Piling – Design and Installation*, the lab results indicated that the low permeability soil pH is: “Non-aggressive” for Steel piles and “Non-aggressive” for Reinforced Concrete Piles
- Chloride, Sulphate and Electrical resistivity is “Non-aggressive” for Steel Piles and Reinforced Concrete Piles.

4.8 Sub-grade Preparation

- Fill should be compacted close to its optimum moisture content (+/- 2%) during compaction.
- The compaction method and equipment shall suit the filled material. The compaction of soil shall be tested by a NATA accredited laboratory and Geotechnical Inspection and Testing Authority (GITA) to ensure it meets the requirements of “AS 3798-2007 *Guidelines on earthworks for commercial and residential developments*”.
- Any organic materials (including topsoil) within the proposed building envelope are to be removed.

- The site should be proof rolled after an initial site scrape to unveil any soft spots. Any soft areas are to be removed and backfilled with compacted fill material as described in “AS 2870-2011”, cl 6.4.2.

4.9 Conditions of the Recommendations

- The descriptions of the soils encountered in the boreholes follow those outlined in “AS 1726-2017”, Geotechnical Site Investigations. Colour descriptions can vary with soil moisture content and individual interpretation.
- The advice given in this report assumes that the test results are representative of the overall subsurface conditions. However, it should be noted that actual conditions in some parts of the building site may differ from those found in the boreholes. If excavations reveal soil conditions significantly different from those shown in our attached Borehole Log(s), CEC Geotechnical shall be consulted and the excavations shall be stopped immediately.
- Depths mentioned in this report are measured from the surface during testing and may vary accordingly if any filling or excavation works are carried out. The description of the foundation material has been provided for ease of recognition over the whole building site.
- Any sketches in this report should be considered as only approximate pictorial evidence of our work. Therefore, unless otherwise stated, any dimensions or slope information should not be used for any building cost calculations and/or positioning of the building. Dimensions on logs are correct.

5. Geotechnical Landslide Risk Assessment

The risk assessment process has been carried out for the sites with potential landslide risk. It also considers the likelihood and consequences of particular events using a criterion and matrix, in line with accepted risk management principles.

5.1 Hazard Analysis

According to previous assessments, recent monitoring and the author’s experience, the following hazards have been nominated for the proposed developments:

- Rotational Landslide
- Shoring Wall Failure
- Excavation Instability
- Surface Water Run-Off

5.2 Risk to Property – Qualitative Assessment

The risk to property has been carried out using AGS 2007 Appendix C. The likelihood of each hazard in two separate scenarios (with or without measures) has been estimated as shown in Table 8.

The risk level of each scenario has been derived from the risk matrix (as per AGS 2007 Appendix C), using the consequences to properties for the associated hazards.

Table 8: Hazard Analysis of Potential Failures – For the property

Potential Hazard	Measures	Qualitative Measures of Likelihood	Qualitative Measures of Consequences to Property	Qualitative Risk Analysis – Level of Risk to Property
Landslide & Creep	Without	LIKELY	MAJOR	VERY HIGH
	With	RARE	MAJOR	LOW
Shoring Wall Failure	Without	LIKELY	MAJOR	VERY HIGH
	With	RARE	MEDIUM	LOW
Excavation instability	Without	LIKELY	MAJOR	VERY HIGH
	With	RARE	MEDIUM	LOW
Surface Water Run-Off	Without	LIKELY	MEDIUM	HIGH
	With	UNLIKELY	MEDIUM	LOW

5.3 Risk to Life – Quantitative Assessment

The risk of loss of life has been estimated using the methodology outlined by the AGS, 2007, Section 7. For loss of life, the individual risk can be calculated using the following formula:

$$R(\text{LoL}) = P(\text{H}) \times P(\text{S:H}) \times P(\text{T:S}) \times V(\text{D:T})$$

Where:

R(LoL) = the risk (annual probability of loss of life (death) of an individual)

P(H) = the annual probability of the landslide.

P(S:H) = the probability of spatial impact of the landslide impacting a proposed development with considering the movement distance and direction given particular event.

P(T:S) = is the temporal spatial probability (e.g. of the building or location being occupied by the individual) given the spatial impact and allowing for the possibility of evacuation given there is warning of the landslide occurrence.

V(D:T) = is the vulnerability of the individual (probability of loss of life of the individual given the impact).

Landslide & Creep:

- Likelihood: Classified as **LIKELY** (without measures) and **RARE/VERY UNLIKELY** (with measures).
- Indicative Annual Probability: Assumed at 0.01 (without measures) and 0.00005 (with measures)
- Probability of Spatial Impact: Assumed to be 1
- Temporal Factor: Assuming the neighbouring property to be occupied by a person 16 hours a day, therefore the factor for the person most at risk will be $16/24 = 0.67$. Only 20% of the neighbouring property at the rear falls within the landslide zone, therefore the temporal factor is approx. $0.67 \times 0.2 = 0.134$
- Vulnerability: Assumed to be 0.9, (without measures) and 0.05 (with measures).

Shoring wall failure:

- Likelihood: Classified as **LIKELY** (without measures) and **RARE/VERY UNLIKELY** (with measures).
- Indicative Annual Probability: Assumed at 0.01 (without measures) and 0.00005 (with measures)
- Probability of Spatial Impact: Assumed to be 1
- Temporal Factor: Assuming the neighbouring property to be occupied by a person 16 hours a day, therefore the factor for the person most at risk will be $16/24 = 0.67$. Only 20% of the

neighbouring property at the rear falls within the landslide zone, therefore the temporal factor is approx. $0.67 \times 0.2 = 0.134$

- Vulnerability: Assumed to be 0.7 (may be injured but unlikely to cause death), (without measures) and 0.05 (with measures).

Excavation instability:

- Likelihood: Classified as LIKELY (without measures) and RARE/VERY UNLIKELY (with measures).
- Indicative Annual Probability: Assumed at 0.01 (without measures) and 0.00005 (with measures)
- Probability of Spatial Impact: Assumed to be 1, for people working during construction
- Temporal Factor: Assuming people work 8 hours a day, 6 days a week, for 6 months $8/24 \times 6/7 = 0.57$.
- Vulnerability: Assumed to be 1, (without measures) and 0.05 (with measures).

Uncontrolled or concentrated surface flows with soil erosion:

- Likelihood: Classified as LIKELY (without measures) and UNLIKELY (with measures).
- Indicative Annual Probability: Assumed at 0.01 (without measures) and 0.0001 (with measures)
- Probability of Spatial Impact: Assumed to be 1
- Temporal Factor: Assume to be 0.02
- Vulnerability: Assumed to be 0.5 (may be injured but unlikely to cause death), (without measures) and 0.05 (with measures).

5.4 Risk to Life in Present Condition

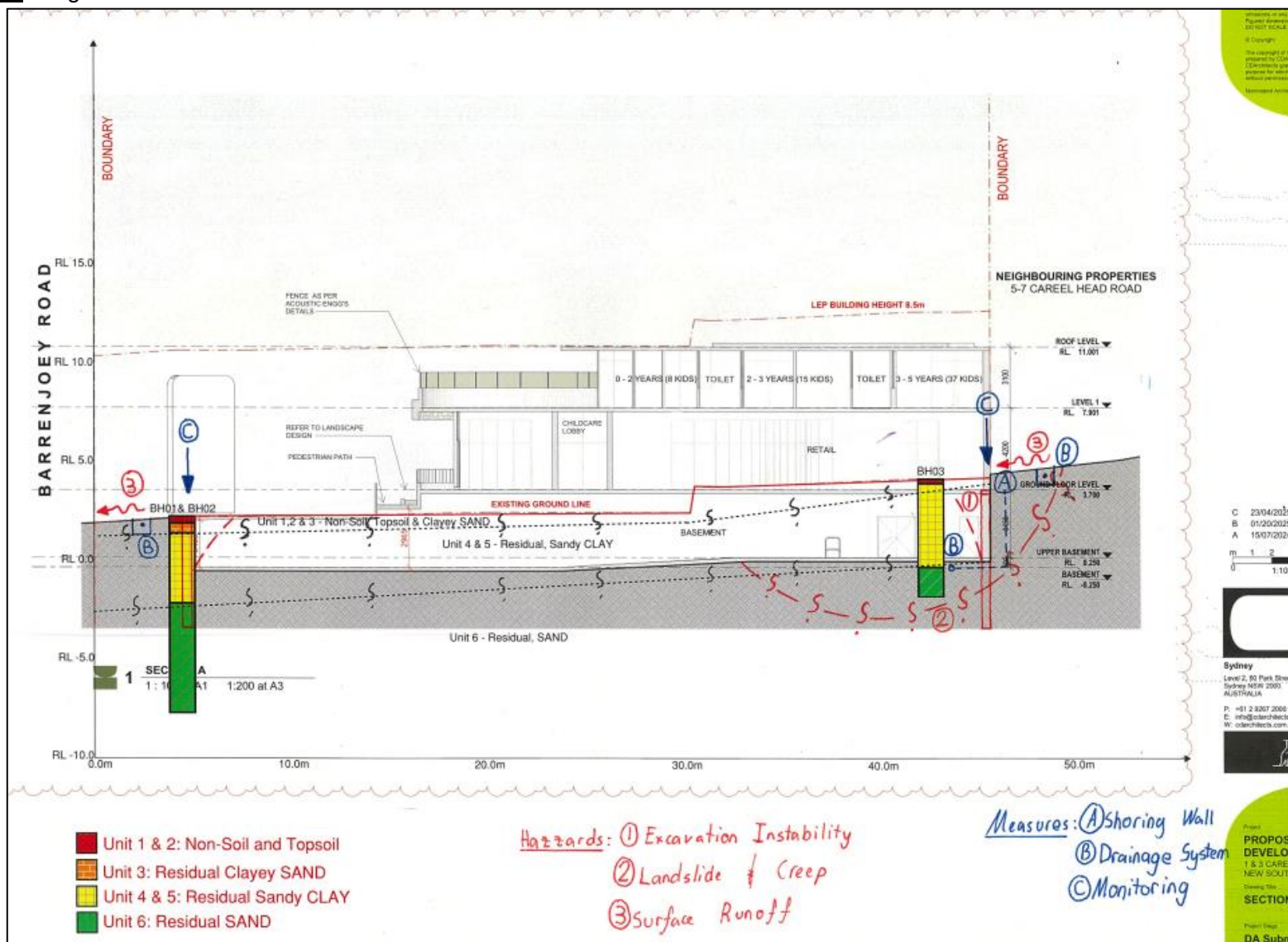
The initial risk assessment is carried out as per the AGS (2007) guidelines for the present condition without any measures and adding engineering works as presented below.

Table 9: Hazard Analysis of Potential Failures – For the person most at risk

Mode of Failure	Measures	Indicative Annual Probability (P_H)	Probability of Spatial ($P(S:H)$)	Temporal Factor ($P(T:S)$)	Vulnerability ($V(D:T)$)	Individual Risk
Landslide & Creep	Without	0.01	1	0.134	0.9	1.206×10^{-3}
	With	0.00005	1	0.134	0.05	3.35×10^{-7}
Shoring wall failure	Without	0.01	1	0.134	0.7	9.38×10^{-4}
	With	0.00005	1	0.134	0.05	3.35×10^{-7}
Excavation instability	Without	0.01	1	0.57	1	5.7×10^{-3}
	With	0.0001	1	0.57	0.05	2.85×10^{-6}
Surface runoff	Without	0.01	1	0.02	0.5	1.0×10^{-4}
	With	0.0001	1	0.02	0.05	1.0×10^{-7}

According to the risk calculation the risk for the person most at risk will be less than the tolerable limits (1×10^{-6} per annum – as mentioned in Pittwater Council, Council Policy No. 178).

Figure 4: Longitudinal Section



5.5 Proposed Remediation Measures

According to the risk assessment, the following measures have been recommended in order to make sure that the stability of the slope is suitable for the future development. A longitudinal section has been developed in order to show the natural ground levels, subsurface conditions, proposed development, possible hazards and proposed measures to mitigate the risks of these hazards (refer to Figure 4)

1. Shoring Wall

- The shoring wall shall be designed by a qualified structural and geotechnical engineer to minimise the impact to the properties and people. This includes but not limited to:
 - Adequate.
 - Design of anchors to assist with lateral movement (if required)

2. Inspections and Monitoring

- Regular site inspections are required by structural and geotechnical engineers in order to the measures are commenced based on the recommendations in this report.

3. Temporary/Permanent Drainage

- The building and associated retaining walls need to have proper surface and subsurface drainage as shown in Figure 4. All drainage work must be designed by a Stormwater Engineer in order to make sure all surface and sub-surface water is drained to an OSD tank.

6. Conclusions

The risk of Landslide and associated hazards shall be minimised to the tolerable limits. To achieve this, CEC Geotechnical has undertaken a landslip risk assessment, and proposed measures as discussed in Section 5.

We further recommend that:

- The measures proposed in this report to reduce the landslip risk shall be designed & detailed by a structural, stormwater & geotechnical engineer at the Construction Certificate (CC) stage.
- Any loose material to be replaced with engineered fill with a geotechnical engineer's supervision.

We, CEC Geotechnical Pty Ltd, confirm and certify that the proposed solutions will reduce the risk to acceptable level if the recommendations in these reports are fully adhered to.

7. Further Geotechnical Recommendations

Furthermore, CEC Geotechnical should be engaged at the following stages:

- If soil conditions encountered differ significantly from those described within this report.
- If the proposed development is altered significantly from what has been assessed and described within this report.
- To confirm safe batter angles and excavation methods during construction.
- To confirm founding materials and allowable bearing capacity.
- If the site conditions at the time of construction differ from those described in this report, then CEC Geotechnical shall be contacted. The owner/builder will be responsible for any fees associated with this additional work.

- The initial groundwater investigation was affected due to the stormwater and runoff. Therefore, additional groundwater monitoring is required to establish an accurate and reliable reading of the groundwater level.

8. Limitations

This report and its associated recommendations have been prepared exclusively for our client who is named on the front page of this report and is the only intended entity to benefit from this report. CEC Geotechnical notes that reliance on the information provided in this report by any third party will be at their own risk. It should be noted that the analysis and conclusions made in this report may rely on works by other consultants and entities and hence, should these documents and investigations be incorrect, CEC Geotechnical must be made aware and the results of this report may be void.

For and on behalf of CEC Geotechnical Pty Ltd

Dean Mitrevski



Geotechnical Engineer
B.E (Civil & Structures) (Hons)

Amir Eslami

Principal Engineer
B.E. (Civil & Structures), M.E. (Geotechnical),
M.I.E. Aust. CPEng, NER, APEC Eng
IntPE(Aus)
NSW Fair Trading Design Practitioner
(DEP0001792)
NSW Fair Trading Professional Engineer
(PRE0001237)

References

- AS 2870 -2011, Residential Slab and Footings – Construction
- AS 1726-2017, Geotechnical Site Investigations
- AS 2159-2009, Piling - Design and Installation
- AS 3798-2007 "Guidelines on earthworks for commercial and residential developments"
- Geological Series Sheet 9030 (EDITION 1) 1991, Map of the Sydney Region, Scale 1:100,000
- Site investigations for urban salinity, Date 1 January 2002, Department of Land and Water Conservation.

APPENDIX A – Site Plan



CEC GEOTECHNICAL

Drawn	DE	CLIENT: JONATHAN ODISHO SITE ADDRESS: 1-3 CAREEL HEAD RD, AVALON BEACH	Figure	1
Checked	SG		Title	Site Plan
Date	19/06/2024		Job No	GR24118
Scale @ A3	NTS			

APPENDIX B – Bore Hole Logs and DCP Test Results

Job Number : GR24118
Client : Jonathan Odisho
Project : Proposed Mixed-Use Development
Location : 1 Careel Head Rd, Avalon Beach NSW
Loc Comment :

Page 1 of 2

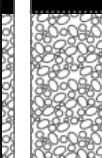
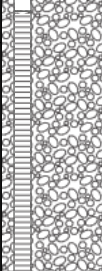
UTM : 56H	Drill Rig : 4x4 drill rig	Job Number : GR24118
Easting (m) : 0.00	Driller Supplier : CEC	Client : Jonathan Odisho
Northing (m) : 0.00	Logged By : DE	Project : Proposed Mixed-Use Development
Ground Elevation : Not Surveyed	Reviewed By :	Location : 1 Careel Head Rd, Avalon Beach NSW
Total Depth : 10 m BGL	Date : 19/06/2024	Loc Comment :

Drilling Method	DCP graph	Depth (m)	Water	Soil Origin	Graphic Log	Classification Code	Material Description	Moisture	Testing	Consistency/Density	Well Diagram
4x4		</									

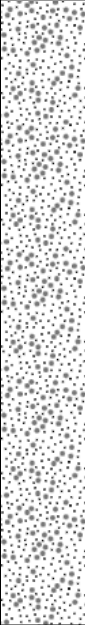
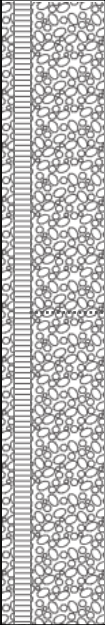
UTM : 56H
Easting (m) : 0.00
Northing (m) : 0.00
Ground Elevation : Not Surveyed
Total Depth : 7 m BGL

Drill Rig : 4x4 drill rig
Driller Supplier : CEC
Logged By : DE
Reviewed By :
Date : 19/06/2024

Job Number : GR24118
Client : Jonathan Odisho
Project : Proposed Mixed-Use Development
Location : 1 Careel Head Rd, Avalon Beach NSW
Loc Comment :

Drilling Method	DCP graph	Depth (m)	Water	Soil Origin	Graphic Log	Classification Code	Material Description	Moisture	Testing	Consistency/Density	Well Diagram		
4x4		0.3		Non-Soil		CCT	Concrete					Concrete Cement	
	1			Residual		SC	Residual Clayey SAND SC: loose to medium dense, low plasticity clay, brown red dark grey, fine grained, slightly moist.	SLM		L-MD		Bentonite	
	2												
	1												
	1												
	3												
	3												
	5												
	6	1	1	Residual		CL-CI	Residual Sandy CLAY CL-CI: very stiff, low to medium plasticity, grey brown red, fine grained sand, organic, moist to slightly moist.	M-SLM		VSt		50mm PVC Solid	
	5												
	7												
	8												
	7	1.4		Residual		CL-CI	Residual Sandy CLAY CL-CI: hard, low to medium plasticity, brown yellow red, fine grained sand, organic, moist.	M		H		Backfill	
	9												
	11												
	14												
	19												
	22												
	20	2											
	18												
	22												
	23												
	19												
	21												
	23												
	28												
	34												
	39	3											
									</				

UTM : 56H	Drill Rig : 4x4 drill rig	Job Number : GR24118
Easting (m) : 0.00	Driller Supplier : CEC	Client : Jonathan Odisho
Northing (m) : 0.00	Logged By : DE	Project : Proposed Mixed-Use Development
Ground Elevation : Not Surveyed	Reviewed By :	Location : 1 Careel Head Rd, Avalon Beach NSW
Total Depth : 7 m BGL	Date : 19/06/2024	Loc Comment :

Drilling Method	DCP graph	Depth (m)	Water	Soil Origin	Graphic Log	Classification Code	Material Description	Moisture	Testing	Consistency/Density	Well Diagram
4x4		6		Residual		SP	Residual SAND SP: dense to very dense, brown yellow, fine grained, with low plasticity clay, wet.	W		D-VD	
		7									
		8					BH02 Terminated at 7m				
		9									

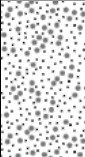
UTM : 56H
 Easting (m) : 0.00
 Northing (m) : 0.00
 Ground Elevation : Not Surveyed
 Total Depth : 7.2 m BGL

Drill Rig : 4x4 drill rig
 Driller Supplier : CEC
 Logged By : DE
 Reviewed By :
 Date : 19/06/2024

Job Number : GR24118
 Client : Jonathan Odisho
 Project : Proposed Mixed-Use Development
 Location : 1 Careel Head Rd, Avalon Beach NSW
 Loc Comment :

Drilling Method	DCP graph	Depth (m)	Water	Soil Origin	Graphic Log	Classification Code	Material Description	Moisture	Testing	Consistency/Density	Well Diagram		
4x4	1	0.2		Top Soil		ML	Top soil Clayey SILT ML: firm, low plasticity, brown, organic, moist.	M		F		Concrete Cement	
	2			Residual		CL	Residual Sandy CLAY CL: stiff, low plasticity, grey brown red, fine grained sand, organic, moist.			St			
	2												
	3	0.7								VSt			
	3	Residual			CL	Residual Sandy CLAY CL: very stiff, low plasticity, grey brown red, fine grained sand, organic, moist.							
	4												
	7	1		Residual		CL-CI	Residual Sandy CLAY CL-CI: hard, low to medium plasticity, yellow brown, fine grained sand, organic, moist.			H			
	6												
	5												
	9	2											
	12												
	14												
	13	3											
	12												
	13												
	14	4											
	15												
	18												
	21	4.5		Residual		SP	Residual SAND SP: very dense, white, fine grained, slightly moist.	SLM		VD			
	17												
	18												
	15												
	14												
	21												
	21												
	26												

UTM : 56H	Drill Rig : 4x4 drill rig	Job Number : GR24118
Easting (m) : 0.00	Driller Supplier : CEC	Client : Jonathan Odisho
Northing (m) : 0.00	Logged By : DE	Project : Proposed Mixed-Use Development
Ground Elevation : Not Surveyed	Reviewed By :	Location : 1 Careel Head Rd, Avalon Beach NSW
Total Depth : 7.2 m BGL	Date : 19/06/2024	Loc Comment :

Drilling Method	DCP graph	Depth (m)	Water	Soil Origin	Graphic Log	Classification Code	Material Description	Moisture	Testing	Consistency/Density	Well Diagram
4x4		5.5		Residual		SP	Residual SAND SP: very dense, white, fine grained, slightly moist.	SLM		VD	 Backfill 50mm PVC Slotted
		6		Residual		SC	Residual Clayey SAND SC: very dense, low to medium plasticity clay, red brown, fine grained, wet.	W			
		7		Rock		SST	Extremely weathered, rock SANDSTONE (SST): hard, non-plastic, yellow brown, medium to coarse grained sand, inorganic, slightly moist.			H	
							BH03 refusal at 7.2m				
		8									
		9									



Job Number : GR24118
Client : Jonathan Odisho
Project : Proposed Mixed-Use Development
Location : 1 Careel Head Rd, Avalon Beach NSW
Loc Comment :

Page 1 of 1

EXPLANATION OF NOTES, ABBREVIATIONS & TERMS USED ON BOREHOLE AND TEST PIT LOGS - SOIL DESCRIPTION (AS1726 - 2017)

SOIL CLASSIFICATION SYSTEM

Coarse Grained Soil

GW	Well graded gravels, gravel-sand mixtures, little or no fines
GP	Poorly-graded gravels, gravel-sand mixtures, little or no fines, uniform gravels
GM	Silty gravels, Gravel-sand-silt mixtures
GC	Clayey gravels, gravel-sand-clay mixtures
SW	Well-graded sands, gravelly sands, little or no fines
SP	Poorly-graded sands, gravelly sand, little or no fines
SM	Silty sands, sand-silt mixtures
SC	Clayey sands, sand-clay mixtures

Fine Grained Soils

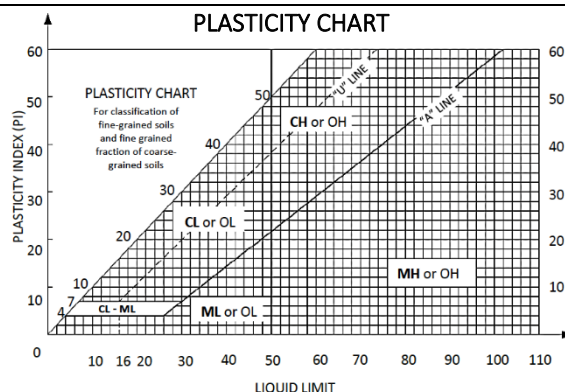
ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or silts with low plasticity
CL, CI	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays
OL	Organic silts and organic silty clays or low plasticity
MH	Inorganic silts, micaceous or diatomaceous fine sand for silty soils
CH	Inorganic clays of high plasticity
OH	Organic clays of medium to high plasticity, organic silts
PT	Peat, humus, swamp soils with high organic contents

First Letter: G = Gravel, S = Sand, M = Silt, C = Clay; Second Letter: W = Well graded, P = Poorly-graded, M = Mixture, O = Organic, L = Low plasticity, H = High plasticity
Soils may be a combination of multiple soil classifications where borderline

PARTICLE SIZE

Soil	Major Division	Sub-Division	Particle Size (mm)
Coarse	Boulders		>200
	Cobbles		63 – 200
	Gravel	Coarse	20 – 63
		Medium	6 – 20
		Fine	2.36 – 6
	Sand	Coarse	0.6 – 2.36
		Medium	0.2 – 0.6
		Fine	0.075 – 0.2
Fine	Silt		0.002 – 0.075
	Clay		<0.002

0.075 mm is the approximate minimum particle size discernible by eye



MOISTURE CONDITION

Coarse	D	Dry	Sands and gravels are free flowing.
	M	Moist	Soils are darker than in the dry condition and may feel cool. Sands and gravels tend to cohere.
	W	Wet	Soils exude free water. Sands and gravels tend to cohere
Fine	PL	Plastic Limit	Moisture content of fine grained soils are described; as below plastic limit (<PL), near to plastic limit (=PL), above plastic limit(>PL), near to the liquid limit (=LL), or above the liquid limit (>LL)
	LL	Liquid Limit	

CONSISTENCY AND DENSITY

Fine Grained Soils

VS	Very Soft	Exudes between fingers when squeezed
S	Soft	Can be moulded by light finger pressure
F	Firm	Can be moulded by strong finger pressure
St	Stiff	Cannot be moulded by fingers. Can be indented by thumb
VSt	Very Stiff	Can be indented by thumb nail
H	Hard	Can be indented by thumb nail with difficulty

Pocket Penetrometer Reading (kPa)

<25
20 – 50
50 – 100
100 – 200
200 – 400
>400

Coarse Grained Soils

VL	Very Loose	Density Index %	'N' Value
L	Loose	≤15	0 – 4
MD	Medium Dense	15 – 35	4 – 10
D	Dense	35 – 65	10 – 30
VD	Very Dense	65-85	30 – 50
		>85	>50

SECONDARY OR MINOR SOIL COMPONENTS

Designation of Components	In Coarse Grained Soils				In Fine Grained Soils	
	% Fines	Terminology	% Accessory Coarse Fraction	Terminology	% Sand/gravel	Terminology
Minor	≤5	'trace' clay/silt	≤15	'trace' sand/gravel	≤15	'trace' sand/gravel
	5 – 12	'with' clay/silt	15 – 30	'with' sand/gravel	15 – 30	'with' sand/gravel
Secondary	>15	Prefix silty or clayey	>30	Prefix sandy or gravelly	>30	Prefix sandy or gravelly

EXPLANATION OF NOTES, ABBREVIATIONS & TERMS USED ON BOREHOLE AND TEST PIT LOGS - SOIL DESCRIPTION (AS1726 - 2017)

STRENGTH OF INTACT ROCK

Symbol	Term	Point Load Index, (I_{50}) MPa	Field Guide to Strength
VL	Very Low	$0.03 \leq I_{50} < 0.1$	Material crumbles under firm blows with sharp end of pick; can be peeled with knife; pieces up to 30 mm thick can be broken by finger pressure
L	Low	$0.1 \leq I_{50} < 0.3$	Easily scored with knife; indentations 1 mm to 3 mm after firm blow with pick point; core 150mm long and 50 mm diameter can be broken by hand; sharp edges of core friable
M	Medium	$0.3 \leq I_{50} < 1.0$	Readily scored with knife; core 150 mm long and 50 mm diameter can be broken by hand with difficulty
H	High	$1.0 \leq I_{50} < 3$	Core 150 mm long and 50 mm diameter cannot be broken by hand but can be broken by single firm blow of pick; rock rings under hammer
VH	Very High	$3 \leq I_{50} < 10$	Hand held specimen breaks with pick after more than one blow; rock rings under hammer
EH	Extremely High	$10 \leq I_{50}$	Specimen requires many pick blows to break intact rock, rock rings under hammer

Material with rock strength less than “Very Low” is to be described using soil properties

DEGREE OF ROCK WEATHERING

DEGREE OF ROCK WEATHERING				
Term		Symbol	Definition	
Residual Soil		RS	Soil derived from the weathering of rock; the mass structure and material fabric are no longer evident the soil has not been significantly transported.	
Extremely Weathered		XW	Material is weathered to such an extent that it has soil properties, i.e. it either disintegrates or can be remoulded in water. Fabric of original rock still visible.	
Highly Weathered	Distinctly Weathered	HW	DW	Rock strength is changed by weathering. The whole of the rock material is discoloured, usually by iron staining or bleaching to the extent that the colour of the original rock is not recognizable. Some minerals are decomposed to clay minerals. Porosity may be increased by leach, or may be decreased due to deposition or weathering products in pores.
Moderately Weathered		MW		The whole of the rock material is discoloured, usually by iron staining or bleaching to the extent that the colour of the original rock is not recognizable, but shows little or no change of strength from fresh rock.
Slightly Weathered		SW	Rock is slightly discoloured but shows little or no change of strength from fresh rock.	
Fresh		FR	Rock shows no sign of decomposition or staining.	

Distinctly Weathered is to be used when it is not possible to differentiate between highly and moderately weathered.

Extremely Weathered material is to be described using soil properties

ROCK MASS PROPERTIES

Term	Separation of Stratification Planes	Term	Description
Thinly laminated	< 6 mm	Fragmented	Primarily fragments < 20 mm length and mostly of width < core diameter
Laminated	6 mm to 20 mm	Highly fractured	Core lengths generally less than 20 mm to 40 mm with occasional fragments
Very thinly bedded	20 mm to 60 mm		
Thinly bedded	60 mm to 200 mm	Fractured	Core lengths mainly 30 mm to 100 mm with occasional shorter and longer pieces
Medium bedded	0.2 m to 0.6 m	Slightly fractured	Core lengths generally 0.3 m to 1.0 m with occasional longer and shorter sections
Thickly bedded	0.6 m to 2.0 m		
Massive	> 2 m	Unbroken	Core has no fractures

DEFECT TYPES AND DESCRIPTIONS

Defect Type	Defect Shape	Surface Roughness	Defect Coatings
BR Bedding parting	PL Planar	VR Very rough	CL Clean
JT Joint	ST Stepped	RO Rough	ST Stained
SR Sheared surface	CR Curved	SM Smooth	VN Veneer
SZ Sheared zone	IR Irregular	PO Polished	CT Coating
SS Sheared seam	UN Undulating	SL Slickenside	
CS Crushed seam			
IS Infill seam			
XS Extremely Weathered Seam			

Vertical Boreholes – The dip of the defect is given from the horizontal
Inclined Boreholes – The angle of the defect is given from the core axis

APPENDIX C – Laboratory Test Results

CEC Geotechnical
Unit 4 83 Grose Street
North Paramatta
NSW 2151



NATA Accredited
Accreditation Number 1261
Site Number 18217

Accredited for compliance with ISO/IEC 17025 – Testing
NATA is a signatory to the ILAC Mutual Recognition
Arrangement for the mutual recognition of the
equivalence of testing, medical testing, calibration,
inspection, proficiency testing scheme providers and
reference materials producers reports and certificates.

Attention: **Diego**

Report **1110583-S**

Project name

Project ID **GR241118**

Received Date Jun 20, 2024

Client Sample ID			BH01	BH02	BH03	BH04
Sample Matrix			Soil	Soil	Soil	Soil
Eurofins Sample No.			S24-Jn0057494	S24-Jn0057495	S24-Jn0057496	S24-Jn0057497
Date Sampled			Jun 19, 2024	Jun 19, 2024	Jun 19, 2024	Jun 19, 2024
Test/Reference	LOR	Unit				
Chloride	10	mg/kg	19	36	15	37
Conductivity (1:5 aqueous extract at 25 °C as rec.)	10	uS/cm	69	37	27	44
pH (1:5 Aqueous extract at 25 °C as rec.)	0.1	pH Units	8.9	7.9	6.6	6.4
Resistivity*	0.5	ohm.m	150	270	370	230
Sulphate (as SO4)	10	mg/kg	12	< 10	87	38
Sample Properties						
% Moisture	1	%	21	24	56	20

Sample History

Where samples are submitted/analysed over several days, the last date of extraction is reported.

If the date and time of sampling are not provided, the Laboratory will not be responsible for compromised results should testing be performed outside the recommended holding time.

Description	Testing Site	Extracted	Holding Time
Chloride - Method: LTM-INO-4270 Anions by Ion Chromatography	Sydney	Jun 26, 2024	28 Days
Conductivity (1:5 aqueous extract at 25 °C as rec.) - Method: LTM-INO-4030 Conductivity	Sydney	Jun 26, 2024	7 Days
pH (1:5 Aqueous extract at 25 °C as rec.) - Method: LTM-GEN-7090 pH by ISE	Sydney	Jun 26, 2024	7 Days
Sulphate (as SO ₄) - Method: In-house method LTM-INO-4270 Sulphate by Ion Chromatograph	Sydney	Jun 26, 2024	28 Days
% Moisture - Method: LTM-GEN-7080 Moisture	Sydney	Jun 22, 2024	14 Days



web: www.eurofins.com.au
email: EnviroSales@eurofins.com

ABN: 50 005 085 521

Melbourne 6 Monterey Road Dandenong South VIC 3175 +61 3 8564 5000 NATA# 1261 Site# 1254	Geelong 19/8 Lewalan Street Grovedale VIC 3216 +61 3 8564 5000 NATA# 1261 Site# 25403	Sydney 179 Magowar Road Girraween NSW 2145 +61 2 9900 8400 NATA# 1261 Site# 18217	Canberra Unit 1,2 Dacre Street Mitchell ACT 2911 +61 2 6113 8091 NATA# 1261 Site# 25466	Brisbane 1/21 Smallwood Place Murarie QLD 4172 T: +61 7 3902 4600 NATA# 1261 Site# 20794 & 2780	Newcastle 1/2 Frost Drive Mayfield West NSW 2304 +61 2 4968 8448 NATA# 1261 Site# 25079 & 25289
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ABN: 91 05 0159 898

Perth 46-48 Banksia Road Welshpool WA 6106 +61 8 6253 4444 NATA# 2377 Site# 2370

ABN: 47 009 120 549

Perth ProMicro 46-48 Banksia Road Welshpool WA 6106 +61 8 6253 4444 NATA# 2561 Site# 2554
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NZBN: 9429046024954

Auckland 35 O'Rorke Road Penrose, Auckland 1061 +64 9 526 4551 IANZ# 1327	Auckland (Focus) Unit C1/4 Pacific Rise, Mount Wellington, Auckland 1061 +64 9 525 0568 IANZ# 1308	Christchurch 43 Detroit Drive Rolleston, Christchurch 7675 +64 3 343 5201 IANZ# 1290	Tauranga 1277 Cameron Road, Gate Pa, Tauranga 3112 +64 9 525 0568 IANZ# 1402
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Company Name: CEC Geotechnical
Address: Unit 4 83 Grose Street
North Paramatta
NSW 2151

Project Name:
Project ID: GR241118

Order No.:
Report #: 1110583
Phone: 02 9630 0121
Fax:

Received: Jun 20, 2024 12:50 PM
Due: Jun 26, 2024
Priority: 5 Day
Contact Name: Diego

Eurofins Analytical Services Manager : Adam Bateup

Sample Detail						Aggressivity Soil Set	Moisture Set
Sydney Laboratory - NATA # 1261 Site # 18217						X	X
External Laboratory							
No	Sample ID	Sample Date	Sampling Time	Matrix	LAB ID		
1	BH01	Jun 19, 2024		Soil	S24-Jn0057494	X	X
2	BH02	Jun 19, 2024		Soil	S24-Jn0057495	X	X
3	BH03	Jun 19, 2024		Soil	S24-Jn0057496	X	X
4	BH04	Jun 19, 2024		Soil	S24-Jn0057497	X	X
Test Counts						4	4

Internal Quality Control Review and Glossary

General

1. Laboratory QC results for Method Blanks, Duplicates, Matrix Spikes, and Laboratory Control Samples follow guidelines delineated in the National Environment Protection (Assessment of Site Contamination) Measure 1999, as amended May 2013. They are included in this QC report where applicable. Additional QC data may be available on request.
2. Unless otherwise stated, all soil/sediment/solid results are reported on a dry weight basis.
3. Unless otherwise stated, all biota/food results are reported on a wet weight basis on the edible portion.
4. For CEC results where the sample's origin is unknown or environmentally contaminated, the results should be used advisedly.
5. Actual LORs are matrix dependent. Quoted LORs may be raised where sample extracts are diluted due to interferences.
6. Results are uncorrected for matrix spikes or surrogate recoveries except for PFAS compounds where annotated.
7. SVOC analysis on waters is performed on homogenised, unfiltered samples unless noted otherwise.
8. Samples were analysed on an 'as received' basis.
9. Information identified in this report with **blue** colour indicates data provided by customers that may have an impact on the results.
10. This report replaces any interim results previously issued.

Holding Times

Please refer to the 'Sample Preservation and Container Guide' for holding times (QS3001).

For samples received on the last day of holding time, notification of testing requirements should have been received at least 6 hours before sample receipt deadlines as stated on the SRA.

If the Laboratory did not receive the information in the required timeframe, and despite any other integrity issues, suitably qualified results may still be reported.

Holding times apply from the sampling date; therefore, compliance with these may be outside the laboratory's control.

For VOCs containing vinyl chloride, styrene and 2-chloroethyl vinyl ether, the holding time is seven days; however, for all other VOCs, such as BTEX or C6-10 TRH, the holding time is 14 days.

Units

mg/kg: milligrams per kilogram	mg/L: milligrams per litre	ppm: parts per million
µg/L: micrograms per litre	ppb: parts per billion	%: Percentage
org/100 mL: Organisms per 100 millilitres	NTU: Nephelometric Turbidity Units	MPN/100 mL: Most Probable Number of organisms per 100 millilitres
CFU: Colony Forming Unit	Colour: Pt-Co Units (CU)	

Terms

APHA	American Public Health Association
CEC	Cation Exchange Capacity
COC	Chain of Custody
CP	Client Parent - QC was performed on samples pertaining to this report
CRM	Certified Reference Material (ISO17034) - reported as percent recovery.
Dry	Where moisture has been determined on a solid sample, the result is expressed on a dry weight basis.
Duplicate	A second piece of analysis from the same sample and reported in the same units as the result to show comparison.
LOR	Limit of Reporting.
LCS	Laboratory Control Sample - reported as percent recovery.
Method Blank	In the case of solid samples, these are performed on laboratory-certified clean sands and in the case of water samples, these are performed on de-ionised water.
NCP	Non-Client Parent - QC performed on samples not pertaining to this report, QC represents the sequence or batch that client samples were analysed within.
RPD	Relative Percent Difference between two Duplicate pieces of analysis.
SPIKE	Addition of the analyte to the sample and reported as percentage recovery.
SRA	Sample Receipt Advice
Surr - Surrogate	The addition of a similar compound to the analyte target is reported as percentage recovery. See below for acceptance criteria.
TBTO	Tributyltin oxide (<i>bis</i> -tributyltin oxide) - individual tributyltin compounds cannot be identified separately in the environment; however, free tributyltin was measured, and its values were converted stoichiometrically into tributyltin oxide for comparison with regulatory limits.
TCLP	Toxicity Characteristic Leaching Procedure
TEQ	Toxic Equivalency Quotient or Total Equivalence
QSM	US Department of Defense Quality Systems Manual Version 6.0
US EPA	United States Environmental Protection Agency
WA DWER	Sum of PFBA, PFPeA, PFHxA, PFHpA, PFOA, PFBS, PFHxS, PFOS, 6:2 FTSA, 8:2 FTSA

QC - Acceptance Criteria

The acceptance criteria should only be used as a guide and may be different when site-specific Sampling Analysis and Quality Plan (SAQP) have been implemented.

RPD Duplicates: Global RPD Duplicates Acceptance Criteria is ≤30%; however, the following acceptance guidelines are equally applicable:

Results <10 times the LOR:	No Limit
Results between 10-20 times the LOR:	RPD must lie between 0-50%
Results >20 times the LOR:	RPD must lie between 0-30%

NOTE: pH duplicates are reported as a range, not as RPD

Surrogate Recoveries: Recoveries must lie between 20-130% for Speciated Phenols & 50-150% for PFAS. SVOCs recoveries 20 – 150%, VOC recoveries 50 – 150%

PFAS field samples containing surrogate recoveries above the QC limit designated in QSM 6.0, where no positive PFAS results have been reported or reviewed, and no data was affected.

QC Data General Comments

1. Where a result is reported as less than (<), higher than the nominated LOR, this is due to either matrix interference, extract dilution required due to interferences or contaminant levels within the sample, high moisture content or insufficient sample provided.
2. Duplicate data shown within this report that states the word "BATCH" is a Batch Duplicate from outside of your sample batch but within the laboratory sample batch at a 1:10 ratio. The Parent and Duplicate data shown are not data from your samples.
3. pH and Free Chlorine analysed in the laboratory - Analysis on this test must begin within 30 minutes of sampling. Therefore, laboratory analysis is unlikely to be completed within holding time. Analysis will begin as soon as possible after sample receipt.
4. Recovery Data (Spikes & Surrogates) - where chromatographic interference does not allow the determination of recovery, the term "INT" appears against that analyte.
5. For Matrix Spikes and LCS results, a dash "-" in the report means that the specific analyte was not added to the QC sample.
6. Duplicate RPDs are calculated from raw analytical data; thus, it is possible to have two sets of data.

Quality Control Results

Test				Units	Result 1			Acceptance Limits	Pass Limits	Qualifying Code
Method Blank										
Conductivity (1:5 aqueous extract at 25 °C as rec.)				uS/cm	< 10			10	Pass	
Method Blank										
Chloride				mg/kg	< 10			10	Pass	
Sulphate (as SO4)				mg/kg	< 10			10	Pass	
LCS - % Recovery										
Conductivity (1:5 aqueous extract at 25 °C as rec.)				%	99			70-130	Pass	
Resistivity*				%	99			70-130	Pass	
LCS - % Recovery										
Chloride				%	100			70-130	Pass	
Sulphate (as SO4)				%	103			70-130	Pass	
Test	Lab Sample ID	QA Source		Units	Result 1			Acceptance Limits	Pass Limits	Qualifying Code
Duplicate										
Sample Properties					Result 1	Result 2	RPD			
% Moisture	S24-Jn0057490	NCP		%	12	12	<1	30%	Pass	
Duplicate										
					Result 1	Result 2	RPD			
Conductivity (1:5 aqueous extract at 25 °C as rec.)	S24-Jn0057495	CP		uS/cm	37	39	5.8	30%	Pass	
pH (1:5 Aqueous extract at 25 °C as rec.)	S24-Jn0057495	CP		pH Units	7.9	7.7	<1	30%	Pass	
Resistivity*	S24-Jn0057495	CP		ohm.m	270	260	5.8	30%	Pass	

Comments

Sample Integrity

Custody Seals Intact (if used)	N/A
Attempt to Chill was evident	Yes
Sample correctly preserved	Yes
Appropriate sample containers have been used	Yes
Sample containers for volatile analysis received with minimal headspace	Yes
Samples received within HoldingTime	Yes
Some samples have been subcontracted	No

Authorised by:

Adam Bateup	Analytical Services Manager
Dilani Samarakoon	Senior Analyst-Inorganic
Roopesh Rangarajan	Senior Analyst-Sample Properties



Glenn Jackson
Managing Director

Final Report – this report replaces any previously issued Report

- Indicates Not Requested

* Indicates NATA accreditation does not cover the performance of this service

Measurement uncertainty of test data is available on request or please [click here](#).

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1 of

Location: AVALON BEACH

11105 83

APPENDIX D – Lake Macquarie City Council (Council) Geotechnical Assessment and Declaration

PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007
APPENDIX C: LANDSLIDE RISK ASSESSMENT
QUALITATIVE TERMINOLOGY FOR USE IN ASSESSING RISK TO PROPERTY

QUALITATIVE MEASURES OF LIKELIHOOD

Approximate Annual Probability		Implied Indicative Landslide Recurrence Interval		Description	Descriptor	Level
Indicative Value	Notional Boundary					
10 ⁻¹	5x10 ⁻²	10 years	20 years	The event is expected to occur over the design life.	ALMOST CERTAIN	A
10 ⁻²		100 years		The event will probably occur under adverse conditions over the design life.	LIKELY	B
10 ⁻³	5x10 ⁻³	1000 years	200 years	The event could occur under adverse conditions over the design life.	POSSIBLE	C
10 ⁻⁴	5x10 ⁻⁴	10,000 years	2000 years	The event might occur under very adverse circumstances over the design life.	UNLIKELY	D
10 ⁻⁵	5x10 ⁻⁵	100,000 years	20,000 years	The event is conceivable but only under exceptional circumstances over the design life.	RARE	E
10 ⁻⁶	5x10 ⁻⁶	1,000,000 years	200,000 years	The event is inconceivable or fanciful over the design life.	BARELY CREDIBLE	F

Note: (1) The table should be used from left to right; use Approximate Annual Probability or Description to assign Descriptor, not *vice versa*.

QUALITATIVE MEASURES OF CONSEQUENCES TO PROPERTY

Approximate Cost of Damage		Description	Descriptor	Level
Indicative Value	Notional Boundary			
200%	100%	Structure(s) completely destroyed and/or large scale damage requiring major engineering works for stabilisation. Could cause at least one adjacent property major consequence damage.	CATASTROPHIC	1
60%		Extensive damage to most of structure, and/or extending beyond site boundaries requiring significant stabilisation works. Could cause at least one adjacent property medium consequence damage.	MAJOR	2
20%	40%	Moderate damage to some of structure, and/or significant part of site requiring large stabilisation works. Could cause at least one adjacent property minor consequence damage.	MEDIUM	3
5%	10%	Limited damage to part of structure, and/or part of site requiring some reinstatement stabilisation works.	MINOR	4
0.5%	1%	Little damage. (Note for high probability event (Almost Certain), this category may be subdivided at a notional boundary of 0.1%. See Risk Matrix.)	INSIGNIFICANT	5

- Notes:** (2) The Approximate Cost of Damage is expressed as a percentage of market value, being the cost of the improved value of the unaffected property which includes the land plus the unaffected structures.
- (3) The Approximate Cost is to be an estimate of the direct cost of the damage, such as the cost of reinstatement of the damaged portion of the property (land plus structures), stabilisation works required to render the site to tolerable risk level for the landslide which has occurred and professional design fees, and consequential costs such as legal fees, temporary accommodation. It does not include additional stabilisation works to address other landslides which may affect the property.
- (4) The table should be used from left to right; use Approximate Cost of Damage or Description to assign Descriptor, not *vice versa*

PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007

APPENDIX C: – QUALITATIVE TERMINOLOGY FOR USE IN ASSESSING RISK TO PROPERTY (CONTINUED)

QUALITATIVE RISK ANALYSIS MATRIX – LEVEL OF RISK TO PROPERTY

LIKELIHOOD		CONSEQUENCES TO PROPERTY (With Indicative Approximate Cost of Damage)				
	Indicative Value of Approximate Annual Probability	1: CATASTROPHIC 200%	2: MAJOR 60%	3: MEDIUM 20%	4: MINOR 5%	5: INSIGNIFICANT 0.5%
A – ALMOST CERTAIN	10 ⁻¹	VH	VH	VH	H	M or L (5)
B – LIKELY	10 ⁻²	VH	VH	H	M	L
C – POSSIBLE	10 ⁻³	VH	H	M	M	VL
D – UNLIKELY	10 ⁻⁴	H	M	L	L	VL
E – RARE	10 ⁻⁵	M	L	L	VL	VL
F – BARELY CREDIBLE	10 ⁻⁶	L	VL	VL	VL	VL

Notes: (5) For Cell A5, may be subdivided such that a consequence of less than 0.1% is Low Risk.

(6) When considering a risk assessment it must be clearly stated whether it is for existing conditions or with risk control measures which may not be implemented at the current time.

RISK LEVEL IMPLICATIONS

Risk Level		Example Implications (7)
VH	VERY HIGH RISK	Unacceptable without treatment. Extensive detailed investigation and research, planning and implementation of treatment options essential to reduce risk to Low; may be too expensive and not practical. Work likely to cost more than value of the property.
H	HIGH RISK	Unacceptable without treatment. Detailed investigation, planning and implementation of treatment options required to reduce risk to Low. Work would cost a substantial sum in relation to the value of the property.
M	MODERATE RISK	May be tolerated in certain circumstances (subject to regulator's approval) but requires investigation, planning and implementation of treatment options to reduce the risk to Low. Treatment options to reduce to Low risk should be implemented as soon as practicable.
L	LOW RISK	Usually acceptable to regulators. Where treatment has been required to reduce the risk to this level, ongoing maintenance is required.
VL	VERY LOW RISK	Acceptable. Manage by normal slope maintenance procedures.

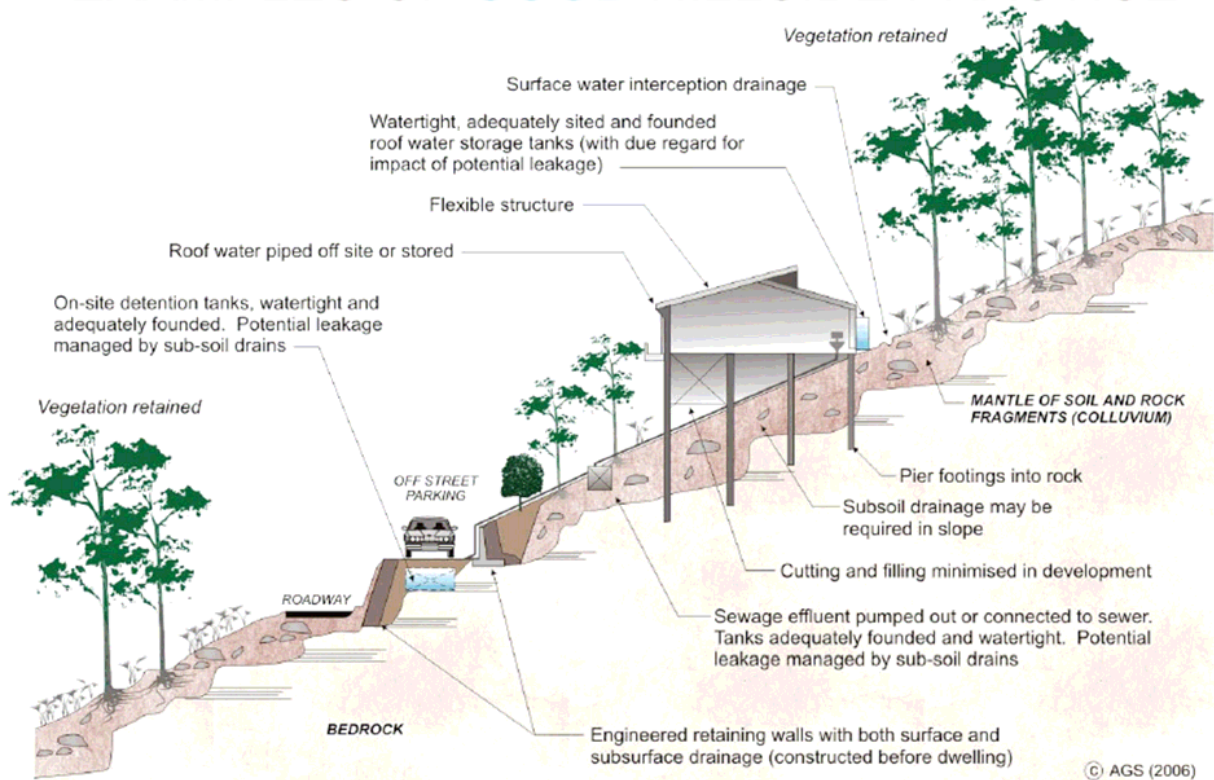
Note: (7) The implications for a particular situation are to be determined by all parties to the risk assessment and may depend on the nature of the property at risk; these are only given as a general guide.

PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007

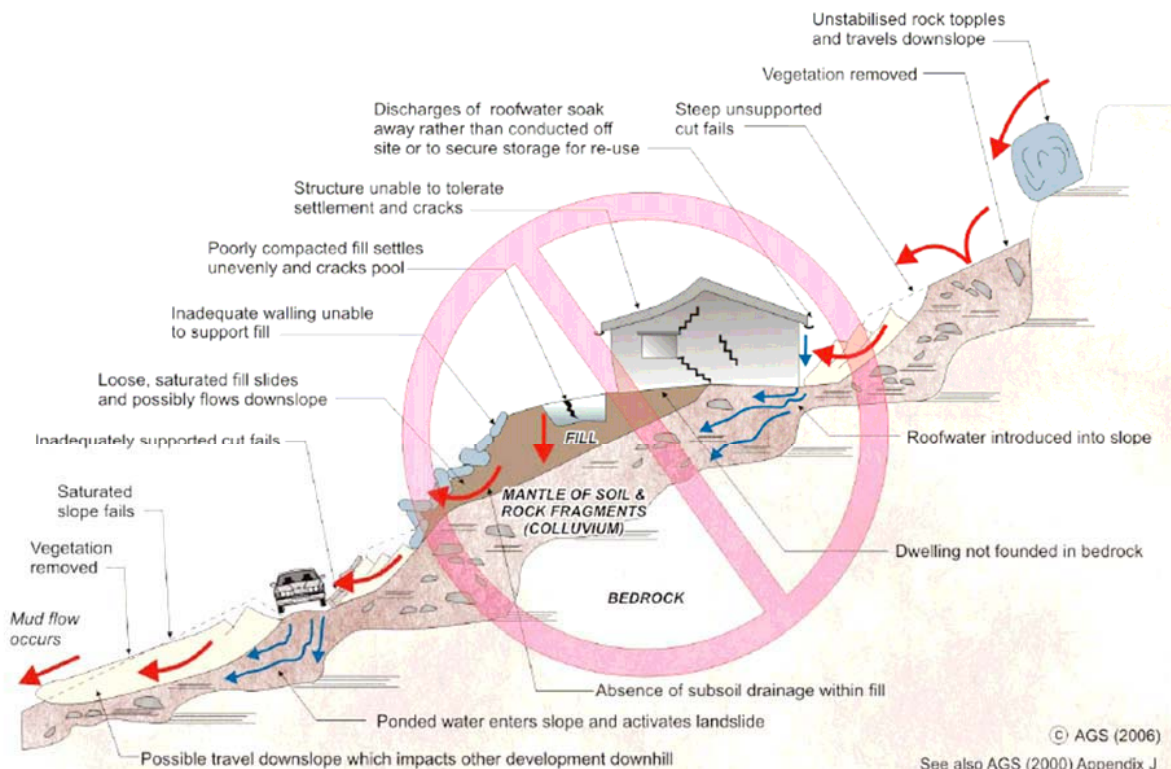
APPENDIX - SOME GUIDELINES FOR HILLSIDE CONSTRUCTION

ADVICE		GOOD ENGINEERING PRACTICE	POOR ENGINEERING PRACTICE
GEOTECHNICAL ASSESSMENT		Obtain advice from a qualified, experienced geotechnical practitioner at early stage of planning and before site works.	Prepare detailed plan and start site works before geotechnical advice.
PLANNING			
SITE PLANNING		Having obtained geotechnical advice, plan the development with the risk arising from the identified hazards and consequences in mind.	Plan development without regard for the Risk.
DESIGN AND CONSTRUCTION			
HOUSE DESIGN		Use flexible structures which incorporate properly designed brickwork, timber or steel frames, timber or panel cladding. Consider use of split levels. Use decks for recreational areas where appropriate.	Floor plans which require extensive cutting and filling. Movement intolerant structures.
SITE CLEARING		Retain natural vegetation wherever practicable.	Indiscriminately clear the site.
EARTHWORKS		Retain natural contours wherever possible.	Indiscriminatory bulk earthworks.
CUTS		Minimise depth. Support with engineered retaining walls or batter to appropriate slope. Provide drainage measures and erosion control.	Large scale cuts and benching. Unsupported cuts. Ignore drainage requirements
FILLS		Minimise height. Strip vegetation and topsoil and key into natural slopes prior to filling. Use clean fill materials and compact to engineering standards. Batter to appropriate slope or support with engineered retaining wall. Provide surface drainage and appropriate subsurface drainage.	Loose or poorly compacted fill, which if it fails, may flow a considerable distance including onto property below. Block natural drainage lines. Fill over existing vegetation and topsoil. Include stumps, trees, vegetation, topsoil, boulders, building rubble etc in fill.
ROCK OUTCROPS & BOULDERS		Remove or stabilise boulders which may have unacceptable risk. Support rock faces where necessary.	Disturb or undercut detached blocks or boulders.
RETAINING WALLS		Found on rock where practicable. Provide subsurface drainage within wall backfill and surface drainage on slope above. Construct wall as soon as possible after cut/fill operation.	Construct a structurally inadequate wall such as sandstone flagging, brick or unreinforced blockwork. Lack of subsurface drains and weepholes.
FOOTINGS		Found within rock where practicable. Use rows of piers or strip footings oriented up and down slope. Design for lateral creep pressures if necessary. Backfill footing excavations to exclude ingress of surface water.	Found on topsoil, loose fill, detached boulders or undercut cliffs.
SWIMMING POOLS		Engineer designed. Support on piers to rock where practicable. Provide with under-drainage and gravity drain outlet where practicable. Design for high soil pressures which may develop on uphill side whilst there may be little or no lateral support on downhill side.	
DRAINAGE		Provide at tops of cut and fill slopes. Discharge to street drainage or natural water courses. Provide general falls to prevent blockage by siltation and incorporate silt traps. Line to minimise infiltration and make flexible where possible. Special structures to dissipate energy at changes of slope and/or direction.	Discharge at top of fills and cuts. Allow water to pond on bench areas.
SURFACE			
SUBSURFACE		Provide filter around subsurface drain. Provide drain behind retaining walls. Use flexible pipelines with access for maintenance. Prevent inflow of surface water.	Discharge roof runoff into absorption trenches.
SEPTIC & SULLAGE		Usually requires pump-out or mains sewer systems; absorption trenches may be possible in some areas if risk is acceptable. Storage tanks should be water-tight and adequately founded.	Discharge sullage directly onto and into slopes. Use absorption trenches without consideration of landslide risk.
EROSION CONTROL & LANDSCAPING		Control erosion as this may lead to instability. Revegetate cleared area.	Failure to observe earthworks and drainage recommendations when landscaping.
DRAWINGS AND SITE VISITS DURING CONSTRUCTION			
DRAWINGS		Building Application drawings should be viewed by geotechnical consultant	
SITE VISITS		Site Visits by consultant may be appropriate during construction/	
INSPECTION AND MAINTENANCE BY OWNER			
OWNER'S RESPONSIBILITY		Clean drainage systems; repair broken joints in drains and leaks in supply pipes. Where structural distress is evident see advice. If seepage observed, determine causes or seek advice on consequences.	

EXAMPLES OF **GOOD** HILLSIDE PRACTICE



EXAMPLES OF **POOR** HILLSIDE PRACTICE



APPENDIX E – Site Classification General Information

Foundation Maintenance and Footing Performance: A Homeowner's Guide



CSIRO

BTF 18
replaces
Information
Sheet 10/91

Buildings can and often do move. This movement can be up, down, lateral or rotational. The fundamental cause of movement in buildings can usually be related to one or more problems in the foundation soil. It is important for the homeowner to identify the soil type in order to ascertain the measures that should be put in place in order to ensure that problems in the foundation soil can be prevented, thus protecting against building movement.

This Building Technology File is designed to identify causes of soil-related building movement, and to suggest methods of prevention of resultant cracking in buildings.

Soil Types

The types of soils usually present under the topsoil in land zoned for residential buildings can be split into two approximate groups – granular and clay. Quite often, foundation soil is a mixture of both types. The general problems associated with soils having granular content are usually caused by erosion. Clay soils are subject to saturation and swell/shrink problems.

Classifications for a given area can generally be obtained by application to the local authority, but these are sometimes unreliable and if there is doubt, a geotechnical report should be commissioned. As most buildings suffering movement problems are founded on clay soils, there is an emphasis on classification of soils according to the amount of swell and shrinkage they experience with variations of water content. The table below is Table 2.1 from AS 2870, the Residential Slab and Footing Code.

Causes of Movement

Settlement due to construction

There are two types of settlement that occur as a result of construction:

- Immediate settlement occurs when a building is first placed on its foundation soil, as a result of compaction of the soil under the weight of the structure. The cohesive quality of clay soil mitigates against this, but granular (particularly sandy) soil is susceptible.
- Consolidation settlement is a feature of clay soil and may take place because of the expulsion of moisture from the soil or because of the soil's lack of resistance to local compressive or shear stresses. This will usually take place during the first few months after construction, but has been known to take many years in exceptional cases.

These problems are the province of the builder and should be taken into consideration as part of the preparation of the site for construction. Building Technology File 19 (BTF 19) deals with these problems.

Erosion

All soils are prone to erosion, but sandy soil is particularly susceptible to being washed away. Even clay with a sand component of say 10% or more can suffer from erosion.

Saturation

This is particularly a problem in clay soils. Saturation creates a bog-like suspension of the soil that causes it to lose virtually all of its bearing capacity. To a lesser degree, sand is affected by saturation because saturated sand may undergo a reduction in volume – particularly imported sand fill for bedding and blinding layers. However, this usually occurs as immediate settlement and should normally be the province of the builder.

Seasonal swelling and shrinkage of soil

All clays react to the presence of water by slowly absorbing it, making the soil increase in volume (see table below). The degree of increase varies considerably between different clays, as does the degree of decrease during the subsequent drying out caused by fair weather periods. Because of the low absorption and expulsion rate, this phenomenon will not usually be noticeable unless there are prolonged rainy or dry periods, usually of weeks or months, depending on the land and soil characteristics.

The swelling of soil creates an upward force on the footings of the building, and shrinkage creates subsidence that takes away the support needed by the footing to retain equilibrium.

Shear failure

This phenomenon occurs when the foundation soil does not have sufficient strength to support the weight of the footing. There are two major post-construction causes:

- Significant load increase.
- Reduction of lateral support of the soil under the footing due to erosion or excavation.
- In clay soil, shear failure can be caused by saturation of the soil adjacent to or under the footing.

GENERAL DEFINITIONS OF SITE CLASSES

Class	Foundation
A	Most sand and rock sites with little or no ground movement from moisture changes
S	Slightly reactive clay sites with only slight ground movement from moisture changes
M	Moderately reactive clay or silt sites, which can experience moderate ground movement from moisture changes
H	Highly reactive clay sites, which can experience high ground movement from moisture changes
E	Extremely reactive sites, which can experience extreme ground movement from moisture changes
A to P	Filled sites
P	Sites which include soft soils, such as soft clay or silt or loose sands; landslip; mine subsidence; collapsing soils; soils subject to erosion; reactive sites subject to abnormal moisture conditions or sites which cannot be classified otherwise

Tree root growth

Trees and shrubs that are allowed to grow in the vicinity of footings can cause foundation soil movement in two ways:

- Roots that grow under footings may increase in cross-sectional size, exerting upward pressure on footings.
- Roots in the vicinity of footings will absorb much of the moisture in the foundation soil, causing shrinkage or subsidence.

Unevenness of Movement

The types of ground movement described above usually occur unevenly throughout the building's foundation soil. Settlement due to construction tends to be uneven because of:

- Differing compaction of foundation soil prior to construction.
- Differing moisture content of foundation soil prior to construction.

Movement due to non-construction causes is usually more uneven still. Erosion can undermine a footing that traverses the flow or can create the conditions for shear failure by eroding soil adjacent to a footing that runs in the same direction as the flow.

Saturation of clay foundation soil may occur where subfloor walls create a dam that makes water pond. It can also occur wherever there is a source of water near footings in clay soil. This leads to a severe reduction in the strength of the soil which may create local shear failure.

Seasonal swelling and shrinkage of clay soil affects the perimeter of the building first, then gradually spreads to the interior. The swelling process will usually begin at the uphill extreme of the building, or on the weather side where the land is flat. Swelling gradually reaches the interior soil as absorption continues. Shrinkage usually begins where the sun's heat is greatest.

Effects of Uneven Soil Movement on Structures

Erosion and saturation

Erosion removes the support from under footings, tending to create subsidence of the part of the structure under which it occurs. Brickwork walls will resist the stress created by this removal of support by bridging the gap or cantilevering until the bricks or the mortar bedding fail. Older masonry has little resistance. Evidence of failure varies according to circumstances and symptoms may include:

- Step cracking in the mortar beds in the body of the wall or above/below openings such as doors or windows.
- Vertical cracking in the bricks (usually but not necessarily in line with the vertical beds or perpend).

Isolated piers affected by erosion or saturation of foundations will eventually lose contact with the bearers they support and may tilt or fall over. The floors that have lost this support will become bouncy, sometimes rattling ornaments etc.

Seasonal swelling/shrinkage in clay

Swelling foundation soil due to rainy periods first lifts the most exposed extremities of the footing system, then the remainder of the perimeter footings while gradually permeating inside the building footprint to lift internal footings. This swelling first tends to create a dish effect, because the external footings are pushed higher than the internal ones.

The first noticeable symptom may be that the floor appears slightly dished. This is often accompanied by some doors binding on the floor or the door head, together with some cracking of cornice mitres. In buildings with timber flooring supported by bearers and joists, the floor can be bouncy. Externally there may be visible dishing of the hip or ridge lines.

As the moisture absorption process completes its journey to the innermost areas of the building, the internal footings will rise. If the spread of moisture is roughly even, it may be that the symptoms will temporarily disappear, but it is more likely that swelling will be uneven, creating a difference rather than a disappearance in symptoms. In buildings with timber flooring supported by bearers and joists, the isolated piers will rise more easily than the strip footings or piers under walls, creating noticeable doming of flooring.

Trees can cause shrinkage and damage



As the weather pattern changes and the soil begins to dry out, the external footings will be first affected, beginning with the locations where the sun's effect is strongest. This has the effect of lowering the external footings. The doming is accentuated and cracking reduces or disappears where it occurred because of dishing, but other cracks open up. The roof lines may become convex.

Doming and dishing are also affected by weather in other ways. In areas where warm, wet summers and cooler dry winters prevail, water migration tends to be toward the interior and doming will be accentuated, whereas where summers are dry and winters are cold and wet, migration tends to be toward the exterior and the underlying propensity is toward dishing.

Movement caused by tree roots

In general, growing roots will exert an upward pressure on footings, whereas soil subject to drying because of tree or shrub roots will tend to remove support from under footings by inducing shrinkage.

Complications caused by the structure itself

Most forces that the soil causes to be exerted on structures are vertical – i.e. either up or down. However, because these forces are seldom spread evenly around the footings, and because the building resists uneven movement because of its rigidity, forces are exerted from one part of the building to another. The net result of all these forces is usually rotational. This resultant force often complicates the diagnosis because the visible symptoms do not simply reflect the original cause. A common symptom is binding of doors on the vertical member of the frame.

Effects on full masonry structures

Brickwork will resist cracking where it can. It will attempt to span areas that lose support because of subsided foundations or raised points. It is therefore usual to see cracking at weak points, such as openings for windows or doors.

In the event of construction settlement, cracking will usually remain unchanged after the process of settlement has ceased.

With local shear or erosion, cracking will usually continue to develop until the original cause has been remedied, or until the subsidence has completely neutralised the affected portion of footing and the structure has stabilised on other footings that remain effective.

In the case of swell/shrink effects, the brickwork will in some cases return to its original position after completion of a cycle, however it is more likely that the rotational effect will not be exactly reversed, and it is also usual that brickwork will settle in its new position and will resist the forces trying to return it to its original position. This means that in a case where swelling takes place after construction and cracking occurs, the cracking is likely to at least partly remain after the shrink segment of the cycle is complete. Thus, each time the cycle is repeated, the likelihood is that the cracking will become wider until the sections of brickwork become virtually independent.

With repeated cycles, once the cracking is established, if there is no other complication, it is normal for the incidence of cracking to stabilise, as the building has the articulation it needs to cope with the problem. This is by no means always the case, however, and monitoring of cracks in walls and floors should always be treated seriously.

Upheaval caused by growth of tree roots under footings is not a simple vertical shear stress. There is a tendency for the root to also exert lateral forces that attempt to separate sections of brickwork after initial cracking has occurred.

The normal structural arrangement is that the inner leaf of brickwork in the external walls and at least some of the internal walls (depending on the roof type) comprise the load-bearing structure on which any upper floors, ceilings and the roof are supported. In these cases, it is internally visible cracking that should be the main focus of attention, however there are a few examples of dwellings whose external leaf of masonry plays some supporting role, so this should be checked if there is any doubt. In any case, externally visible cracking is important as a guide to stresses on the structure generally, and it should also be remembered that the external walls must be capable of supporting themselves.

Effects on framed structures

Timber or steel framed buildings are less likely to exhibit cracking due to swell/shrink than masonry buildings because of their flexibility. Also, the doming/dishing effects tend to be lower because of the lighter weight of walls. The main risks to framed buildings are encountered because of the isolated pier footings used under walls. Where erosion or saturation cause a footing to fall away, this can double the span which a wall must bridge. This additional stress can create cracking in wall linings, particularly where there is a weak point in the structure caused by a door or window opening. It is, however, unlikely that framed structures will be so stressed as to suffer serious damage without first exhibiting some or all of the above symptoms for a considerable period. The same warning period should apply in the case of upheaval. It should be noted, however, that where framed buildings are supported by strip footings there is only one leaf of brickwork and therefore the externally visible walls are the supporting structure for the building. In this case, the subfloor masonry walls can be expected to behave as full brickwork walls.

Effects on brick veneer structures

Because the load-bearing structure of a brick veneer building is the frame that makes up the interior leaf of the external walls plus perhaps the internal walls, depending on the type of roof, the building can be expected to behave as a framed structure, except that the external masonry will behave in a similar way to the external leaf of a full masonry structure.

Water Service and Drainage

Where a water service pipe, a sewer or stormwater drainage pipe is in the vicinity of a building, a water leak can cause erosion, swelling or saturation of susceptible soil. Even a minuscule leak can be enough to saturate a clay foundation. A leaking tap near a building can have the same effect. In addition, trenches containing pipes can become watercourses even though backfilled, particularly where broken rubble is used as fill. Water that runs along these trenches can be responsible for serious erosion, interstrata seepage into subfloor areas and saturation.

Pipe leakage and trench water flows also encourage tree and shrub roots to the source of water, complicating and exacerbating the problem.

Poor roof plumbing can result in large volumes of rainwater being concentrated in a small area of soil:

- Incorrect falls in roof guttering may result in overflows, as may gutters blocked with leaves etc.

- Corroded guttering or downpipes can spill water to ground.
- Downpipes not positively connected to a proper stormwater collection system will direct a concentration of water to soil that is directly adjacent to footings, sometimes causing large-scale problems such as erosion, saturation and migration of water under the building.

Seriousness of Cracking

In general, most cracking found in masonry walls is a cosmetic nuisance only and can be kept in repair or even ignored. The table below is a reproduction of Table C1 of AS 2870.

AS 2870 also publishes figures relating to cracking in concrete floors, however because wall cracking will usually reach the critical point significantly earlier than cracking in slabs, this table is not reproduced here.

Prevention/Cure

Plumbing

Where building movement is caused by water service, roof plumbing, sewer or stormwater failure, the remedy is to repair the problem. It is prudent, however, to consider also rerouting pipes away from the building where possible, and relocating taps to positions where any leakage will not direct water to the building vicinity. Even where gully traps are present, there is sometimes sufficient spill to create erosion or saturation, particularly in modern installations using smaller diameter PVC fixtures. Indeed, some gully traps are not situated directly under the taps that are installed to charge them, with the result that water from the tap may enter the backfilled trench that houses the sewer piping. If the trench has been poorly backfilled, the water will either pond or flow along the bottom of the trench. As these trenches usually run alongside the footings and can be at a similar depth, it is not hard to see how any water that is thus directed into a trench can easily affect the foundation's ability to support footings or even gain entry to the subfloor area.

Ground drainage

In all soils there is the capacity for water to travel on the surface and below it. Surface water flows can be established by inspection during and after heavy or prolonged rain. If necessary, a grated drain system connected to the stormwater collection system is usually an easy solution.

It is, however, sometimes necessary when attempting to prevent water migration that testing be carried out to establish watertable height and subsoil water flows. This subject is referred to in BTF 19 and may properly be regarded as an area for an expert consultant.

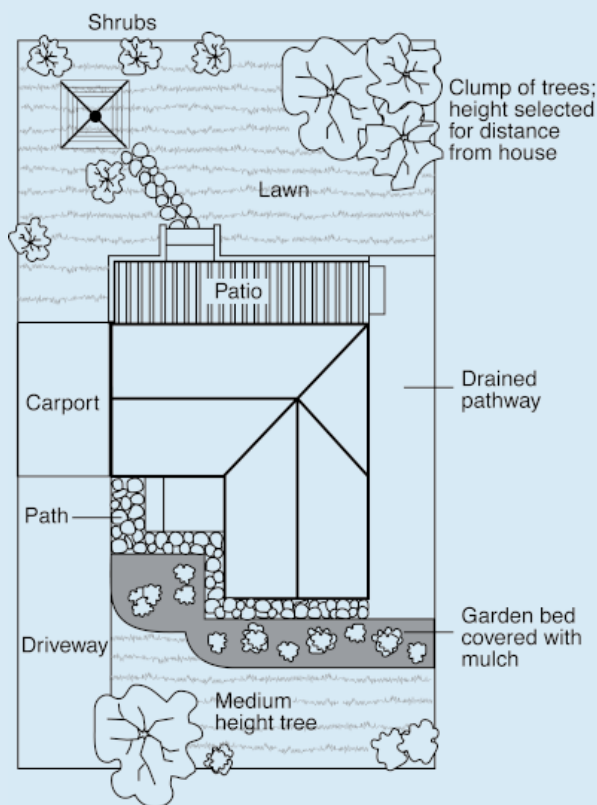
Protection of the building perimeter

It is essential to remember that the soil that affects footings extends well beyond the actual building line. Watering of garden plants, shrubs and trees causes some of the most serious water problems.

For this reason, particularly where problems exist or are likely to occur, it is recommended that an apron of paving be installed around as much of the building perimeter as necessary. This paving

CLASSIFICATION OF DAMAGE WITH REFERENCE TO WALLS		
Description of typical damage and required repair	Approximate crack width limit (see Note 3)	Damage category
Hairline cracks	<0.1 mm	0
Fine cracks which do not need repair	<1 mm	1
Cracks noticeable but easily filled. Doors and windows stick slightly	<5 mm	2
Cracks can be repaired and possibly a small amount of wall will need to be replaced. Doors and windows stick. Service pipes can fracture. Weathertightness often impaired	5–15 mm (or a number of cracks 3 mm or more in one group)	3
Extensive repair work involving breaking-out and replacing sections of walls, especially over doors and windows. Window and door frames distort. Walls lean or bulge noticeably, some loss of bearing in beams. Service pipes disrupted	15–25 mm but also depend on number of cracks	4

Gardens for a reactive site



- Water that is transmitted into masonry, metal or timber building elements causes damage and/or decay to those elements.
- High subfloor humidity and moisture content create an ideal environment for various pests, including termites and spiders.
- Where high moisture levels are transmitted to the flooring and walls, an increase in the dust mite count can ensue within the living areas. Dust mites, as well as dampness in general, can be a health hazard to inhabitants, particularly those who are abnormally susceptible to respiratory ailments.

The garden

The ideal vegetation layout is to have lawn or plants that require only light watering immediately adjacent to the drainage or paving edge, then more demanding plants, shrubs and trees spread out in that order.

Overwatering due to misuse of automatic watering systems is a common cause of saturation and water migration under footings. If it is necessary to use these systems, it is important to remove garden beds to a completely safe distance from buildings.

Existing trees

Where a tree is causing a problem of soil drying or there is the existence or threat of upheaval of footings, if the offending roots are subsidiary and their removal will not significantly damage the tree, they should be severed and a concrete or metal barrier placed vertically in the soil to prevent future root growth in the direction of the building. If it is not possible to remove the relevant roots without damage to the tree, an application to remove the tree should be made to the local authority. A prudent plan is to transplant likely offenders before they become a problem.

Information on trees, plants and shrubs

State departments overseeing agriculture can give information regarding root patterns, volume of water needed and safe distance from buildings of most species. Botanic gardens are also sources of information. For information on plant roots and drains, see Building Technology File 17.

Excavation

Excavation around footings must be properly engineered. Soil supporting footings can only be safely excavated at an angle that allows the soil under the footing to remain stable. This angle is called the angle of repose (or friction) and varies significantly between soil types and conditions. Removal of soil within the angle of repose will cause subsidence.

Remediation

Where erosion has occurred that has washed away soil adjacent to footings, soil of the same classification should be introduced and compacted to the same density. Where footings have been undermined, augmentation or other specialist work may be required. Remediation of footings and foundations is generally the realm of a specialist consultant.

Where isolated footings rise and fall because of swell/shrink effect, the homeowner may be tempted to alleviate floor bounce by filling the gap that has appeared between the bearer and the pier with blocking. The danger here is that when the next swell segment of the cycle occurs, the extra blocking will push the floor up into an accentuated dome and may also cause local shear failure in the soil. If it is necessary to use blocking, it should be by a pair of fine wedges and monitoring should be carried out fortnightly.

This BTF was prepared by John Lewer FAIB, MIAMA, Partner, Construction Diagnosis.

should extend outwards a minimum of 900 mm (more in highly reactive soil) and should have a minimum fall away from the building of 1:60. The finished paving should be no less than 100 mm below brick vent bases.

It is prudent to relocate drainage pipes away from this paving, if possible, to avoid complications from future leakage. If this is not practical, earthenware pipes should be replaced by PVC and backfilling should be of the same soil type as the surrounding soil and compacted to the same density.

Except in areas where freezing of water is an issue, it is wise to remove taps in the building area and relocate them well away from the building – preferably not uphill from it (see BTF 19).

It may be desirable to install a grated drain at the outside edge of the paving on the uphill side of the building. If subsoil drainage is needed this can be installed under the surface drain.

Condensation

In buildings with a subfloor void such as where bearers and joists support flooring, insufficient ventilation creates ideal conditions for condensation, particularly where there is little clearance between the floor and the ground. Condensation adds to the moisture already present in the subfloor and significantly slows the process of drying out. Installation of an adequate subfloor ventilation system, either natural or mechanical, is desirable.

Warning: Although this Building Technology File deals with cracking in buildings, it should be said that subfloor moisture can result in the development of other problems, notably:

The information in this and other issues in the series was derived from various sources and was believed to be correct when published.

The information is advisory. It is provided in good faith and not claimed to be an exhaustive treatment of the relevant subject.

Further professional advice needs to be obtained before taking any action based on the information provided.

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