

Reference: 16.367r04v2

traffic & transport planners

Suite 2.08 50 Holt Street Surry Hills NSW 2010 PO Box 1124 Strawberry Hills NSW 2012 t; +61 2 8324 8700 f; +61 2 9380 4481 W: www.traffix.com.au director Graham Pindar acn: 065132961 abn: 66065132961

28 November 2017

Meriton Group Level 11, Meriton Tower 528 Kent Street SYDNEY NSW 2000

Attention: Ms Norelle Jones, Senior Development Planner

Re: Dee Why Town Centre – Child Care Centre (Eastern Tower) Proposed Expansion – Traffic Impact Statement

Dear Norelle,

We refer to the subject proposal and confirm that TRAFFIX has been commissioned to prepare this Traffic Impact Statement to accompany a Section 96 Application. We understand that approval is sought to increase the capacity of the approved child care centre at the eastern tower of Dee Why Town Centre from 130 placements to 159 placements.

Having reviewed the parking requirements and assessed the traffic impacts of the proposal, we now advise as follows.

Existing Conditions

Site and Location

The site accommodates the approved Dee Why Town Centre, which is situated between Oaks Avenue and Howard Avenue, immediately east of Pittwater Road. When fully constructed, the development will comprise of two attached buildings up to 17 storeys and accommodates residential, retail, commercial and child care land uses.

Dee Why Town Centre will be afforded three levels of basement car parking containing a total of 1,035 parking spaces. Vehicular access is to be provided by a left-in / left-out driveway at Howard Street, adjacent to the eastern site boundary, and by an unrestricted access at Oaks Avenue, towards the western site boundary. The site will also accommodate separate loading facilities accessed independently from Oaks Avenue.

The subject development area consists of a tenancy on the eastern tower of Dee Why Town Centre which has approval for operation of a child care centre with capacity for 130 placements. This tenancy was also afforded an allocation for 24 car parking spaces situated within the basement car park.

A Location Plan is presented in Figure 1, with a Site Plan presented in Figure 2.





Figure 1: Location Plan





Figure 2: Site Plan



Road Network

The road hierarchy in the vicinity of the site is shown in **Figure 3** with the following roads of particular interest:

• Pittwater Road: an RMS Main Road (MR164) that generally runs in a north-south direction between Barrenjoey Road in the north and Condamine Street to the south (it continues northwest of Barrenjoey Road as a local road). In the vicinity of the site, Pittwater Road accommodates three lanes of traffic in each direction within a divided carriageway, where auxiliary turn lanes are provided for key intersections. Approval has been granted to upgrade the intersection of Oaks Avenue that would increase the right turn bay length from Fisher Road to Sturdee Parade.

Bus lanes are operational on the eastern and western kerbside lanes during morning and evening peak periods respectively, with parking generally permissible during other times.

- Howard Avenue: a local road that runs in an east-west direction between The Strand in the east and Pittwater Road to the west. It generally accommodates a single lane of traffic in each direction within an undivided carriageway. Howard Avenue forms a signalised intersection with Pittwater Road and St David Avenue, where right movements from Pittwater Road into Howard Avenue are prohibited. A median is to be constructed on Howard Avenue that would restrict site specific traffic to a left-in / left-out arrangement.
- Oaks Avenue: a local road that runs in an east-west direction between Monash Parade in the east and Pittwater Road to the west. It generally accommodates a single lane of traffic in each direction within an undivided carriageway. Two approach lanes are provided on Oaks Avenue at the intersection of Pittwater Road which must turn left only. It is understood that Council intends to relocate a pedestrian crossing adjacent to the Dee Why Town Centre access (from approximately 130m east of the access location).

It can be seen from **Figure 3** that the site is conveniently located with respect to the arterial and local road systems serving the region. It is therefore able to effectively distribute traffic onto the wider road network, minimising traffic impacts.

Public Transport

The public transport network operating in the locality is shown illustrated in Figure 4. It is evident that bus stops on Pittwater Road and Howard Avenue are within 200 metres walking distance from the site, which are serviced by an extensive number of routes. Connections are provided to key employment centres such as the Sydney central business district and Chatswood as well as the surrounding region.

As part of the Northern Beaches Transport Access Plan, Transport for NSW has announced that new public transport interchanges will be built, including at Dee Why, opposite the subject site. This interchange will be served by the introduction new services that will be known as the Bus Rapid Transit, which will be supported by bus bays and continuous bus lanes. These initiatives are anticipated to encourage the uptake of public transport and reduce private car dependency.





Figure 3: Road Hierarchy





Figure 4: Public Transport



Proposal

A full description of the proposed development can be found in the Statement of Environmental Effects prepared separately. In summary, a Section 96 Application seeks approval to increase the capacity of the approved child care centre at the eastern tower from 130 placements to 159 placements. The tenancy will be afforded an additional six (6) parking spaces and will have access to a total pool of 30 spaces.

The approved operating hours for the development will be retained, which are from 7:00am to 7:00pm.

The parking requirements and traffic impacts associated with the additional 39 placements are discussed below.

Parking Requirements

Car Parking

The Warringah Development Control Plan (DCP) 2011 requires parking for child care centre uses to be provided at the rates shown listed in **Table 1**.

Table 1: Council Parking Rates and Provision

Туре	Number	Parking Rate	Spaces Required
Child Care Centre	29	1 space for every 4 children	7

It can be seen that under the DCP, the proposed addition of 29 placements will result in a nominal requirement for seven (7) parking spaces. However, the prior approval for the child care centre took into consideration its location within the Dee Why Town Centre, whereby a reduction rate of 25% was agreed having regard for accessibility of the development to nearby residents as well as proximity to public transport services.

On this basis, the reduction would reduce the DCP requirement to six (6) parking spaces (when rounding up to the nearest whole number). In this regard, the allocation of an additional six (6) parking spaces within the basement car park is expected to satisfactorily accommodate all additional demands generated by the additional 29 placements.

Accessible Parking

The DCP does not stipulate any specific requirements to provide accessible parking for child care centres. Nonetheless, Dee Why Town Centre contains 11 accessible parking spaces that are non-residential and would be available for use by parents.

Bicycle Parking

The DCP does not stipulate any specific requirements to provide bicycle parking for child care centres. Nonetheless the Dee Why Town Centre development provides bicycle parking facilities which may be of benefit to staff for the proposed child care centre.



Service Vehicle Parking

It is anticipated that servicing and waste collection for the proposed child care centre will be accommodated by the in-house loading facilities provided for the Dee Why Town Centre, which include a separate and dedicated loading area accessed off Oaks Avenue. It is proposed that any additional demands of the expanded child care centre be catered for by these facilities.

Traffic Impacts

Trip Generation

The RMS *Guide to Traffic Generating Developments* provides traffic generation rates for various types of child care centres in the Sydney region. For a long-day care centre, it recommends an hourly trip generation rate of 0.8 vehicle trips per child during the AM peak period and 0.7 vehicle trips per child during the PM peak period.

When previously assessing the approved child care centre, Council has stipulated a site specific trip generation rate of 0.6 vehicle trips per child during the AM peak period and 0.525 vehicle trips per child during the PM peak period. This 25% reduction over the RMS guideline rate still remains appropriate, having regard for trips either being redundant with parents residing at Dee Why Town Centre, or by trips already being accounted for by employees of other developments on-site.

Application of these reduced rates results in the following traffic generation associated with the additional 29 placements:

- 17 vehicle trips per hour during the AM peak period; and
- 15 vehicle trips per hour during the PM peak period.

During both AM and PM peak periods, it is assumed that these volumes would be split with equal parents arriving and departing.

Distributions

After observing the local road network, it is apparent that Howard Avenue would provide the only convenient means to access streets west of Pittwater Road, continuing as St David Avenue (a right turn is prohibited onto Fisher Road, south of Oaks Avenue). This is relevant for parents of the child care centre, whom would typically reside within close proximity of the site, as opposed to staff for Dee Why Town Centre, whom would likely use Oaks Avenue to access Pittwater Road.

Before dropping off or immediately after picking up a child, it is thus expected that the majority of parents would enter/exit the site from the Howard Avenue access, or exit left on Oaks Avenue if they reside east of the site. Reliance on the Oaks Avenue access to or from Pittwater Road would thus be mostly limited to parents departing to work in the morning after dropping of their child, or only when arriving to pick-up their child in the evening.

The net changes in traffic volumes arising from the additional 29 placements have subsequently been distributed as shown in **Figure 1** and are based on the percentage splits outlined in **Attachment 1**.





Figure 5: Distributions

Intersection Performance

Based on previous modelling undertaken for the Dee Why Town Centre, the intersections considered most sensitive are:

- The intersection of Pittwater Road and Oaks Avenue
- The site access at Oaks Avenue

SIDRA Intersection (Version 7.0) software been used to model these two intersections has a network for the following two scenarios:

- An approved scenario which includes base case traffic volumes generated by the approved Dee Why Centre (inclusive of a subsequent approved change of use application for a child care centre at the western tower).
- A future scenario which adds the development traffic associated with the additional 29 placements as illustrated in Figure 5.

As previously requested by Council, a planned relocation of pedestrian crossing adjacent to the Oaks Avenue site access has been incorporated into the network model.

A summary of the critical parameters as per RMS guidelines are provided below in **Table 2**. Reference should also be made to the SIDRA outputs provided in **Attachment 2** which provide detailed results for individual lanes and approaches.



Intersection Description	Control Type	Model	Period	Degree of Saturation	Intersection Delay* (sec)	Level of Service
			AM	0.097	6.2	А
Oaks Avenue /	Priority*	Approved	PM	0.571	12.5	А
Site Access		Futuro	AM	0.116	6.4	А
		Future	PM	0.605	13.3	А
		Anne neu ve d	AM	0.883	17.3	В
Pittwater Road /	Cinnal	Approved	PM	0.924	19.9	В
Oaks Avenue**	Signal	Future	AM	0.912	18.8	В
		Future	PM	0.940	21.0	В

Table 2: Intersection Performance: AM and PM Peak Hour

* Movement with largest delay shown in accordance with RMS guidelines.

** Intersection assessed under present arrangement, prior to planning upgrade for right turn bay at Pittwater Road.

It can be seen from Table 2 that the addition of 29 placements will have minimal effects on delays to these intersections, with a maximum increase of 1.5 seconds. Furthermore, both intersections will continue to operate at a Level of Service of B or greater, indicating acceptable overall performance.

While detailed SIDRA outputs indicate that average delays for right turns from Pittwater Road into Oaks Avenue will increase by 9.0 seconds and 6.2 seconds for AM and PM peak periods respectively, it is understood that plans to increase the length of the right turn bay from Fisher Road to Sturdee Parade to approximately 270 metres. This would account for the largest 95th percentile queue experienced during the PM peak period for a future scenario, and as such the Level of Service for this individual movement is expected to improve substantially and with the additional delays to be less pronounced.

It is also noted that all movements within the Oaks Avenue site access perform at a Level of Service of A and will experience a 95th percentile queue length that is equivalent to two vehicle lengths.

Furthermore, the assessed volumes are considered to be conservative in light of the following site specific operating characteristics:

- The child care centre will close at 7:00pm, thereby spreading the evening peak traffic across a three-hour period (from 4:00pm).
- The base case volumes are representative of a traditional Thursday late night trading scenario despite the major tenancy being a supermarket, which operating late every day, has more even customer demands throughout the week.

Accordingly, the traffic impacts associated with the proposed additional 29 placements are considered to have minimal impacts on the local road network.



Summary

In summary, the proposed expanded child care centre is expected to operate satisfactorily, with parking demands accommodated on-site and with minimal traffic impacts to the local road network. Support is therefore given on transport planning grounds.

We trust the above is of assistance and please contact the undersigned should you have any queries or require any further information.

Yours faithfully,

traffix

Kedar Ballurkar Senior Engineer

Attachments:

Distributions
SIDRA Outputs



Attachment 1

Distributions

Land Use

Child Care Centre		
Capacity:	29 children	
AM Rate:	0.6 vehicle t	rips per child
PM Rate:	0.525 vehicle t	rips per child
Traffic Generation		- St d A V
AM Traffic Generation:	17 vehicle t	rips per hour
PM Traffic Generation:	15 vehicle t	rips per hour
<u>Split</u>		
AM Split	50% IN 50% OUT	
PM Split	50% IN 50% OUT	
AM IN	9	
AM OUT	9	
PM IN	8	
PM OUT	8	
Distributions - AM IN		
Pittwater Road - North (Oaks Avenue)	25%	2 vehicle trips per hour
Pittwater Road - South (Oaks Avenue)	25%	2 vehicle trips per hour
Oaks Avenue - East	25%	2 vehicle trips per hour
Howard Avenue - West	25%	2 vehicle trips per hour
Distributions - AM OUT		
Pittwater Road - North (Howard Avenue)	20%	2 vehicle trips per hour
Pittwater Road - South (Oaks Avenue)	60%	5 vehicle trips per hour
Oaks Avenue - East	10%	1 vehicle trips per hour
St David Avenue - West	10%	1 vehicle trips per hour

Distributions - PM IN

Pittwater Road - North (Oaks Avenue)	20%	2 vehicle trips per hour
Pittwater Road - South (Oaks Avenue)	60%	5 vehicle trips per hour
Oaks Avenue - East	10%	1 vehicle trips per hour
Howard Avenue - West	10%	1 vehicle trips per hour

Distributions - PM OUT

Pittwater Road - North (Howard Avenue)	25%	2 vehicle trips per hour
Pittwater Road - South (Oaks Avenue)	25%	2 vehicle trips per hour
Oaks Avenue - East	25%	2 vehicle trips per hour
St David Avenue - West	25%	2 vehicle trips per hour





Attachment 2 SIDRA Outputs

traffic impact studies | expert witness | local govt. liaison | traffic calming | development advice | parking studies pedestrian studies | traffic control plans | traffic management studies | intersection design | transport studies

SITE LAYOUT Site: 101 [1. Pittwater Road / Oaks Avenue - Approved AM]

Signals - Fixed Time Isolated



SITE LAYOUT V Site: 1 [3. Zebra Crossing / Oaks Avenue - Approved PM]

Giveway / Yield (Two-Way)



SITE LAYOUT

Site: 101 [2. Oaks Avenue / Site Access - Approved AM]

Stop (Two-Way)



SITE LAYOUT

 ∇ Site: 1 [3. Site Access / Pedestrian Footpath - Approved AM]

Giveway / Yield (Two-Way)



Site: 101 [1. Pittwater Road / Oaks Avenue - Approved AM] 🛛 👎 Netw

♦♦ Network: N101 [Approved AM]

Signals - Fixed Time Isolated Cycle Time = 110 seconds (User-Given Phase Times)

Move	ment	Performar	nce - \	/ehicle	s								
Mov ID	OD Mov	Demand Total veh/h	ΗV	Arrival Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
East:	Oaks A		/0	VOII/II	/0		000		Voll				
4a	L1	191	2.7	191	2.7	0.233	40.5	LOS C	4.2	30.4	0.86	0.73	11.7
Appro	ach	191	2.7	191	2.7	0.233	40.5	LOS C	4.2	30.4	0.86	0.73	11.7
North	East: Pi	ttwater Roa	d										
24b	L3	134	2.8	134	2.8	0.459	13.5	LOS A	1.5	12.7	0.39	0.59	24.0
25	T1	1753	5.0	1753	5.0	0.745	11.5	LOS A	33.1	238.4	0.65	0.61	29.4
Appro	ach	1886	4.8	1886	4.8	0.745	11.6	LOS A	33.1	238.4	0.64	0.61	29.0
South	West: F	Pittwater Ro	ad										
31	T1	860	5.0	860	5.0	0.232	0.4	LOS A	1.5	10.7	0.14	0.13	57.6
32a	R1	444	3.5	444	3.5	0.883	64.3	LOS E	26.5	191.3	1.00	1.20	4.1
Appro	ach	1304	4.5	1304	4.5	0.883	22.2	LOS B	26.5	191.3	0.44	0.49	17.2
All Vel	hicles	3381	4.6	3381	4.6	0.883	17.3	LOS B	33.1	238.4	0.57	0.57	21.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 1.0 % Number of Iterations: 6 (maximum specified: 10)

Move	Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow	Average Delay		Average Back Pedestrian	of Queue Distance	Prop. Queued	Effective Stop Rate					
		ped/h	sec		ped	m		per ped					
P2	East Full Crossing	53	8.4	LOS A	0.1	0.1	0.39	0.39					
P6	NorthEast Full Crossing	53	48.3	LOS E	0.2	0.2	0.94	0.94					
All Pe	destrians	105	28.4	LOS C			0.67	0.67					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

V Site: 1 [3. Zebra Crossing / Oaks Avenue - Approved PM]

Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles												
Mov ID	OD Mov	Demand I Total	ΗV	Total	ΗV	Deg. Satn	Average Delay	Level of Service		Distance	Prop. Queued	Rate	Speed
East:	Oaks Av	veh/h /enue	70	veh/h	%	v/c	Sec	_	veh	m	_	per veh	km/h
8	T1	117	2.9	117	2.9	0.094	2.8	LOS A	0.3	2.5	0.29	0.43	36.9
Appro	ach	117	2.9	117	2.9	0.094	2.8	LOS A	0.3	2.5	0.29	0.43	36.9
West:	Oaks A	venue											
2	T1	266	3.9	266	3.9	0.216	1.8	LOS A	0.9	6.4	0.32	0.43	38.4
Appro	ach	266	3.9	266	3.9	0.216	1.8	LOS A	0.9	6.4	0.32	0.43	38.4
All Ve	hicles	383	3.6	383	3.6	0.216	2.1	NA	0.9	6.4	0.31	0.43	37.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: Akçelik M1.

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 1.0 % Number of Iterations: 6 (maximum specified: 10)

🦥 Site: 101 [2. Oaks Avenue / Site Access - Approved AM]

Stop (Two-Way)

Move	Movement Performance - Vehicles												
Mov ID	OD Mov	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h
East:	Oaks Av	venue E											
5	T1	84	4.0	84	4.0	0.076	0.8	LOS A	0.3	1.9	0.26	0.15	23.1
6	R2	33	0.0	33	0.0	0.076	3.7	LOS A	0.3	1.9	0.26	0.15	23.1
Appro	ach	117	2.9	117	2.9	0.076	1.6	NA	0.3	1.9	0.26	0.15	23.1
North:	Site Ac	ccess N											
7	L2	9	0.0	9	0.0	0.097	4.3	LOS A	0.4	2.9	0.50	0.90	7.0
9	R2	60	0.0	60	0.0	0.097	6.2	LOS A	0.4	2.9	0.50	0.90	7.0
Appro	ach	69	0.0	69	0.0	0.097	5.9	LOS A	0.4	2.9	0.50	0.90	7.0
West:	Oaks A	venue W											
10	L2	182	0.0	182	0.0	0.254	3.5	LOS A	0.0	0.0	0.00	0.19	35.3
11	T1	257	4.0	257	4.0	0.254	0.0	LOS A	0.0	0.0	0.00	0.19	35.3
Appro	ach	439	2.3	439	2.3	0.254	1.4	NA	0.0	0.0	0.00	0.19	35.3
All Ve	hicles	625	2.2	625	2.2	0.254	2.0	NA	0.4	2.9	0.10	0.26	32.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Venicie movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 1.0 % Number of Iterations: 6 (maximum specified: 10)

Site: 1 [3. Site Access / Pedestrian Footpath - Approved AM]

Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles												
Mov ID	OD Mov	Demand I Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: Site Ad	ccess											
2	T1	215	0.0	215	0.0	0.215	2.1	LOS A	0.8	5.9	0.48	0.46	21.0
Appro	ach	215	0.0	215	0.0	0.215	2.1	LOS A	0.8	5.9	0.48	0.46	21.0
North:	Site Ac	cess											
8	T1	69	0.0	69	0.0	0.070	1.7	LOS A	0.2	1.7	0.43	0.33	28.7
Appro	ach	69	0.0	69	0.0	0.070	1.7	NA	0.2	1.7	0.43	0.33	28.7
All Ve	hicles	284	0.0	284	0.0	0.215	2.0	NA	0.8	5.9	0.46	0.43	22.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: Akçelik M1.

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 1.0 % Number of Iterations: 6 (maximum specified: 10)

Site: 101 [2. Oaks Avenue / Site Access - Approved PM]

Stop (Two-Way)

Move	Movement Performance - Vehicles												
Mov ID	OD Mov	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h
East:	Oaks Av	venue E											
5	T1	122	4.0	122	4.0	0.080	0.4	LOS A	0.1	1.1	0.13	0.05	29.0
6	R2	15	0.0	15	0.0	0.080	4.2	LOS A	0.1	1.1	0.13	0.05	29.0
Appro	ach	137	3.6	137	3.6	0.080	0.8	NA	0.1	1.1	0.13	0.05	29.0
North:	: Site Ad	cess N											
7	L2	57	0.0	57	0.0	0.571	9.4	LOS A	1.4	9.9	0.73	1.41	4.1
9	R2	287	0.0	287	0.0	0.571	12.5	LOS A	1.4	9.9	0.73	1.41	4.1
Appro	ach	344	0.0	344	0.0	0.571	12.0	LOS A	1.4	9.9	0.73	1.41	4.1
West:	Oaks A	venue W											
10	L2	126	0.0	126	0.0	0.296	3.5	LOS A	0.0	0.0	0.00	0.11	37.0
11	T1	394	4.0	394	4.0	0.296	0.0	LOS A	0.0	0.0	0.00	0.11	37.0
Appro	ach	520	3.0	520	3.0	0.296	0.8	NA	0.0	0.0	0.00	0.11	37.0
All Ve	hicles	1001	2.1	1001	2.1	0.571	4.7	NA	1.4	9.9	0.27	0.55	23.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 % Number of Iterations: 7 (maximum specified: 10)

SIDRA INTERSECTION 7.0 | Copyright © 2000-2017 Akcelik and Associates Pty Ltd | sidrasolutions.com Organisation: TRAFFIX PTY LTD | Processed: Thursday, 23 November 2017 6:14:02 PM

Project: T:\Synergy\Projects\16\16.367\Modelling\16.367m02v06 - Proposed Child Care Centre.sip7

V Site: 1 [3. Zebra Crossing / Oaks Avenue - Approved AM]

Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles												
Mov ID	OD Mov	Demand I Total	ΗV	Total	ΗV	Deg. Satn	Average Delay	Level of Service		Distance	Prop. Queued	Rate	Speed
East:	Oaks Av	veh/h	%	veh/h	%	v/c	Sec		veh	m		per veh	km/h
8	T1		26	107	2.6	0.111	2.0	LOS A	0.4	2.0	0.29	0.43	36.9
0		137	3.6	137	3.6	0.111	2.8	LUSA	0.4	3.0	0.29	0.43	30.9
Appro	ach	137	3.6	137	3.6	0.111	2.8	LOS A	0.4	3.0	0.29	0.43	36.9
West:	Oaks A	venue											
2	T1	451	3.5	451	3.5	0.364	2.0	LOS A	1.8	12.7	0.38	0.46	38.3
Appro	ach	451	3.5	451	3.5	0.364	2.0	LOS A	1.8	12.7	0.38	0.46	38.3
All Ve	hicles	587	3.5	587	3.5	0.364	2.2	NA	1.8	12.7	0.36	0.46	37.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: Akçelik M1.

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 % Number of Iterations: 7 (maximum specified: 10)

Site: 101 [1. Pittwater Road / Oaks Avenue - Approved AM] 🛛 🖶 Network

♦♦ Network: N101 [Approved PM]

Signals - Fixed Time Isolated Cycle Time = 110 seconds (User-Given Phase Times)

Move	ment l	Performar	nce - \	/ehicle	s								
Mov ID	OD Mov	Demand Total veh/h	ΗV	Arrival Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
East: 0	Oaks Av		,,,	Voluin	/0	1,0							
4a	L1	452	1.8	452	1.8	0.599	43.7	LOS D	10.9	77.6	0.94	0.80	11.1
Appro	ach	452	1.8	452	1.8	0.599	43.7	LOS D	10.9	77.6	0.94	0.80	11.1
North	East: Pi	ttwater Roa	ıd										
24b	L3	174	4.1	174	4.1	0.201	13.5	LOS A	3.4	24.4	0.39	0.70	22.9
25	T1	1164	5.0	1164	5.0	0.795	12.6	LOS A	38.2	272.7	0.72	0.67	28.1
Appro	ach	1338	4.9	1338	4.9	0.795	12.7	LOS A	38.2	272.7	0.68	0.67	27.4
South	West: F	Pittwater Ro	ad										
31	T1	1380	5.0	1380	5.0	0.395	0.5	LOS A	3.1	22.3	0.18	0.16	57.1
32a	R1	489	4.0	489	4.0	0.924	72.2	LOS F	31.1	225.6	1.00	1.26	3.7
Appro	ach	1869	4.7	1869	4.7	0.924	19.3	LOS B	31.1	225.6	0.39	0.45	19.7
All Vel	hicles	3659	4.4	3659	4.4	0.924	19.9	LOS B	38.2	272.7	0.57	0.57	19.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 % Number of Iterations: 7 (maximum specified: 10)

Move	Novement Performance - Pedestrians													
Mov	Description	Demand	Average		Average Back		Prop.	Effective						
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate						
		ped/h	sec		ped	m		per ped						
P2	East Full Crossing	53	8.4	LOS A	0.1	0.1	0.39	0.39						
P6	NorthEast Full Crossing	53	48.3	LOS E	0.2	0.2	0.94	0.94						
All Pe	destrians	105	28.4	LOS C			0.67	0.67						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 1 [3. Site Access / Pedestrian Footpath - Approved PM]

Giveway / Yield (Two-Way)

Move	ement F	Performan	nce - V	/ehicle	s								
Mov ID	OD Mov	Demand I Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: Site Ad	ccess											
2	T1	238	0.0	238	0.0	0.243	2.5	LOS A	0.9	6.6	0.48	0.52	22.4
Appro	ach	238	0.0	238	0.0	0.243	2.5	LOS A	0.9	6.6	0.48	0.52	22.4
North:	Site Ac	cess											
8	T1	47	0.0	47	0.0	0.048	1.6	LOS A	3.2	22.6	0.42	0.31	22.3
Appro	ach	47	0.0	47	0.0	0.048	1.6	NA	3.2	22.6	0.42	0.31	22.3
All Ve	hicles	285	0.0	285	0.0	0.243	2.3	NA	3.2	22.6	0.47	0.49	22.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 % Number of Iterations: 7 (maximum specified: 10)

Site: 101 [1. Pittwater Road / Oaks Avenue - Future AM]

♦♥ Network: N101 [Future AM]

Signals - Fixed Time Isolated Cycle Time = 110 seconds (User-Given Phase Times)

Mouro	Movement Performance - Vehicles													
Mov	OD	Demand			Flows	Deg.	Average	Level of	95% Back		Prop.	Effective	0	
ID	Mov	Total	ΗV	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed	
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h	
East:	Oaks A	venue												
4a	L1	200	2.6	200	2.6	0.244	40.6	LOS C	4.5	32.0	0.87	0.73	11.7	
Appro	ach	200	2.6	200	2.6	0.244	40.6	LOS C	4.5	32.0	0.87	0.73	11.7	
North	NorthEast: Pittwater Road													
24b	L3	142	2.7	142	2.7	0.484	13.5	LOS A	1.6	12.8	0.39	0.59	23.9	
25	T1	1753	5.0	1753	5.0	0.748	11.5	LOS A	33.4	240.7	0.66	0.61	29.4	
Appro	ach	1895	4.8	1895	4.8	0.748	11.6	LOS A	33.4	240.7	0.64	0.61	29.0	
South	West: F	Pittwater Ro	ad											
31	T1	860	5.0	860	5.0	0.232	0.4	LOS A	1.5	10.7	0.14	0.13	57.6	
32a	R1	459	3.4	459	3.4	0.912	73.3	LOS F	29.6	213.2	1.00	1.25	3.6	
Appro	ach	1319	4.4	1319	4.4	0.912	25.8	LOS B	29.6	213.2	0.44	0.52	15.4	
All Ve	hicles	3414	4.5	3414	4.5	0.912	18.8	LOS B	33.4	240.7	0.57	0.58	20.8	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 % Number of Iterations: 9 (maximum specified: 10)

Move	Novement Performance - Pedestrians													
Mov		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective						
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate						
		ped/h	sec		ped	m		per ped						
P2	East Full Crossing	53	8.4	LOS A	0.1	0.1	0.39	0.39						
P6	NorthEast Full Crossing	53	48.3	LOS E	0.2	0.2	0.94	0.94						
All Pe	destrians	105	28.4	LOS C			0.67	0.67						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

101 [2. Oaks Avenue / Site Access - Future AM]

♦ Network: N101 [Future AM]

Stop (Two-Way)

Move	ement l	Performa	nce - V	/ehicle	S								
Mov ID	OD Mov	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A Stop Rate	Average Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h
East:	Oaks Av	venue E											
5	T1	84	4.0	84	4.0	0.085	1.0	LOS A	0.3	2.2	0.29	0.17	21.6
6	R2	38	0.0	38	0.0	0.085	3.9	LOS A	0.3	2.2	0.29	0.17	21.6
Appro	ach	122	2.8	122	2.8	0.085	1.9	NA	0.3	2.2	0.29	0.17	21.6
North:	: Site Ac	cess N											
7	L2	12	0.0	12	0.0	0.116	4.3	LOS A	0.5	3.5	0.51	0.91	6.9
9	R2	69	0.0	69	0.0	0.116	6.4	LOS A	0.5	3.5	0.51	0.91	6.9
Appro	ach	81	0.0	81	0.0	0.116	6.1	LOS A	0.5	3.5	0.51	0.91	6.9
West:	Oaks A	venue W											
10	L2	211	0.0	211	0.0	0.286	3.5	LOS A	0.0	0.0	0.00	0.21	34.9
11	T1	257	4.0	257	4.0	0.286	0.0	LOS A	0.0	0.0	0.00	0.21	34.9
Appro	ach	467	2.2	467	2.2	0.286	1.6	NA	0.0	0.0	0.00	0.21	34.9
All Ve	hicles	671	2.0	671	2.0	0.286	2.2	NA	0.5	3.5	0.12	0.29	31.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 % Number of Iterations: 9 (maximum specified: 10)

Site: 1 [3. Site Access / Pedestrian Footpath - Future PM] + Network: N101 [Future AM]

Giveway / Yield (Two-Way)

Move	ement F	Performar	nce - \	/ehicle	es								
Mov	OD	Demand	Flows	Arriva	l Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
ID	Mov	Total	ΗV	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: Site Ac	cess											
2	T1	248	0.0	248	0.0	0.249	2.5	LOS A	1.0	7.0	0.49	0.54	23.2
Appro	ach	248	0.0	248	0.0	0.249	2.5	LOS A	1.0	7.0	0.49	0.54	23.2
North	Site Ac	cess											
8	T1	81	0.0	81	0.0	0.082	1.7	LOS A	0.3	2.0	0.43	0.34	30.3
Appro	ach	81	0.0	81	0.0	0.082	1.7	NA	0.3	2.0	0.43	0.34	30.3
All Ve	hicles	329	0.0	329	0.0	0.249	2.3	NA	1.0	7.0	0.47	0.49	24.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: Akçelik M1.

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 % Number of Iterations: 9 (maximum specified: 10)

V Site: 1 [3. Zebra Crossing / Oaks Avenue - Future PM]

Giveway / Yield (Two-Way)

Move	ement F	Performan	ice - V	/ehicle	s								
Mov ID	OD Mov	Demand I Total	ΗV	Arrival Total	ΗV	Deg. Satn	Average Delay	Level of Service		of Queue Distance	Prop. Queued	Effective Stop Rate	Speed
East:	Oaks Av	veh/h venue	%	veh/h	%	v/c	sec	-	veh	m	-	per veh	km/h
8	T1	122	2.8	122	2.8	0.098	2.8	LOS A	0.4	2.6	0.29	0.43	36.9
Appro	ach	122	2.8	122	2.8	0.098	2.8	LOS A	0.4	2.6	0.29	0.43	36.9
West:	Oaks A	venue											
2	T1	268	3.8	268	3.8	0.217	1.8	LOS A	0.9	6.5	0.32	0.43	38.4
Appro	ach	268	3.8	268	3.8	0.217	1.8	LOS A	0.9	6.5	0.32	0.43	38.4
All Ve	hicles	391	3.5	391	3.5	0.217	2.1	NA	0.9	6.5	0.31	0.43	37.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: Akçelik M1.

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 % Number of Iterations: 9 (maximum specified: 10)

Site: 101 [1. Pittwater Road / Oaks Avenue - Future PM]

♦ Network: N101 [Future PM]

Signals - Fixed Time Isolated Cycle Time = 110 seconds (User-Given Phase Times)

Move	Movement Performance - Vehicles														
Mov ID	OD Mov	Demand Total veh/h	Flows HV		Flows HV %	Deg. Satn v/c	Average Delay	Level of Service	95% Back Vehicles	Distance	Prop. Queued	Rate	Speed		
East:	Oaks A		70	ven/m	70	V/C	Sec	_	veh	m	_	per veh	km/h		
4a	L1	464	1.8	464	1.8	0.634	43.9	LOS D	11.3	80.0	0.94	0.80	11.1		
Appro	ach	464	1.8	464	1.8	0.634	43.9	LOS D	11.3	80.0	0.94	0.80	11.1		
NorthEast: Pittwater Road															
24b	L3	177	4.0	177	4.0	0.205	13.5	LOS A	3.4	24.8	0.39	0.70	22.9		
25	T1	1164	5.0	1164	5.0	0.795	12.6	LOS A	38.2	273.0	0.72	0.67	28.1		
Appro	ach	1341	4.9	1341	4.9	0.795	12.8	LOS A	38.2	273.0	0.68	0.67	27.4		
South	West: F	Pittwater Ro	ad												
31	T1	1380	5.0	1380	5.0	0.395	0.5	LOS A	3.1	22.3	0.18	0.16	57.1		
32a	R1	498	4.0	498	4.0	0.940	78.4	LOS F	33.2	240.2	1.00	1.29	3.4		
Appro	ach	1878	4.7	1878	4.7	0.940	21.2	LOS B	33.2	240.2	0.40	0.46	18.4		
All Ve	hicles	3683	4.4	3683	4.4	0.940	21.0	LOS B	38.2	273.0	0.57	0.58	19.1		

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 % Number of Iterations: 7 (maximum specified: 10)

Move	Movement Performance - Pedestrians													
Mov	Description	Demand	Average		Average Back		Prop.	Effective						
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate						
		ped/h	sec		ped	m		per ped						
P2	East Full Crossing	53	8.4	LOS A	0.1	0.1	0.39	0.39						
P6	NorthEast Full Crossing	53	48.3	LOS E	0.2	0.2	0.94	0.94						
All Pe	destrians	105	28.4	LOS C			0.67	0.67						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

11 Site: 101 [2. Oaks Avenue / Site Access - Future PM]

+ Network: N101 [Future PM]

Stop (Two-Way)

Move	ement	Performar	nce - V	/ehicle	s								
Mov ID	OD Mov	Demand Total	Flows HV	Arrival Total	l Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h
East:	Oaks A	venue E											
5	T1	122	4.0	122	4.0	0.083	0.5	LOS A	0.2	1.2	0.15	0.06	27.9
6	R2	17	0.0	17	0.0	0.083	4.3	LOS A	0.2	1.2	0.15	0.06	27.9
Appro	ach	139	3.5	139	3.5	0.083	1.0	NA	0.2	1.2	0.15	0.06	27.9
North:	: Site Ad	ccess N											
7	L2	61	0.0	61	0.0	0.605	10.0	LOS A	1.4	9.9	0.75	1.47	3.9
9	R2	300	0.0	300	0.0	0.605	13.3	LOS A	1.4	9.9	0.75	1.47	3.9
Appro	ach	361	0.0	361	0.0	0.605	12.8	LOS A	1.4	9.9	0.75	1.47	3.9
West:	Oaks A	venue W											
10	L2	138	0.0	138	0.0	0.309	3.5	LOS A	0.0	0.0	0.00	0.12	36.9
11	T1	394	4.0	394	4.0	0.309	0.0	LOS A	0.0	0.0	0.00	0.12	36.9
Appro	ach	532	3.0	532	3.0	0.309	0.9	NA	0.0	0.0	0.00	0.12	36.9
All Ve	hicles	1032	2.0	1032	2.0	0.605	5.1	NA	1.4	9.9	0.28	0.59	22.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 % Number of Iterations: 7 (maximum specified: 10)

Site: 1 [3. Site Access / Pedestrian Footpath - Future PM] + Network: N101 [Future PM]

Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles														
Mov	OD	Demand I	Flows	Arrival	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average		
ID	Mov	Total	ΗV	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed		
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h		
South	: Site Ac	cess													
2	T1	252	0.0	252	0.0	0.259	2.7	LOS A	1.0	7.1	0.49	0.55	23.0		
Appro	ach	252	0.0	252	0.0	0.259	2.7	LOS A	1.0	7.1	0.49	0.55	23.0		
North	: Site Ac	cess													
8	T1	64	0.0	64	0.0	0.064	1.6	LOS A	3.8	26.5	0.42	0.33	27.7		
Appro	ach	64	0.0	64	0.0	0.064	1.6	NA	3.8	26.5	0.42	0.33	27.7		
All Ve	hicles	316	0.0	316	0.0	0.259	2.4	NA	3.8	26.5	0.48	0.51	23.8		

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 % Number of Iterations: 7 (maximum specified: 10)

V Site: 1 [3. Zebra Crossing / Oaks Avenue - Future AM]

Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles													
Mov ID	OD Mov	Demand I Total	ΗV	Arrival Total	HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed	
Foot	Ooko Av	veh/h	%	veh/h	%	v/c	sec	_	veh	m	_	per veh	km/h	
East: Oaks Avenue														
8	T1	139	3.5	139	3.5	0.112	2.8	LOS A	0.4	3.0	0.29	0.43	36.9	
Appro	ach	139	3.5	139	3.5	0.112	2.8	LOS A	0.4	3.0	0.29	0.43	36.9	
West: Oaks Avenue														
2	T1	455	3.5	455	3.5	0.368	2.0	LOS A	1.8	12.9	0.38	0.46	38.2	
Appro	ach	455	3.5	455	3.5	0.368	2.0	LOS A	1.8	12.9	0.38	0.46	38.2	
All Ve	hicles	594	3.5	594	3.5	0.368	2.2	NA	1.8	12.9	0.36	0.46	37.9	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: Akçelik M1.

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 % Number of Iterations: 7 (maximum specified: 10)