

PROPOSED FENCE

**At: Lot 7 Wakehurst Parkway,
Seaforth N.S.W. 2092**

Architect: Zen Scapes

For: Michael Eror

Prepared By:

NB NORTHERN BEACHES
Consulting Engineers P/L.

A.C.N. 076 121 616 A.B.N. 24 076 121 616
Suite 207, 30 FISHER ROAD
DEE WHY N.S.W. 2099
Ph: (02) 9984 7000 Fax: (02) 9984 7444
e-mail : nb@nbconsulting.com.au
web page : www.nbconsulting.com.au

DRAWING SCHEDULE:

S01 - GENERAL NOTES
S02 - SITE PLAN & DETAILS
S03 - PIER /FOOTING DETAILS
S04 - FOOTING STEP DETAILS



110665

05.07.2011

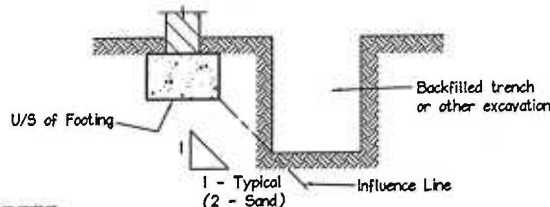
GENERAL NOTES:

GENERAL

- G1. The drawings are to be read together with all Architects drawings and specifications.
- G2. Dimensions shall not be obtained by scaling from the drawings. All setting out dimensions shall be verified and discrepancies shall be referred to the Engineer prior to commencement of work.
- G3. Care is required during construction so that structural elements are not over stressed and that the works and excavations required therefore are kept stable at all times.
- G4. Design, materials and workmanship are to be in accordance with current S.A.A standards and statutory authority regulations except where varied by these documents.
- G5. Design live loads are in accordance with AS 1170.1
- G6. Builder to ensure stability of existing structures in the vicinity of excavation works.

FOOTINGS

- F1. FOUNDATION STRATA IS ASSUMED FOR DESIGN PURPOSES IN ACCORDANCE WITH AS 2870-1996 "RESIDENTIAL SLAB AND FOOTINGS-CONSTRUCTION". SEE FOOTNOTE. CLASSIFICATION TO BE VERIFIED BY A GEOTECHNICAL ENGINEER COMMISSIONED BY THE CLIENT FOR CERTIFICATION OF FOUNDATIONS.
- F2. Footings to be constructed and back filled as soon as possible following excavation to avoid softening by rain or drying out by exposure.
- F3. Footings must bear into undisturbed natural ground clear of organic material. Refer to details.
- F4. If rock or variable bearing strata is encountered during excavation of the footings all footings/piers are to be excavated to similar material of greater bearing capacity. The Engineer is to be contacted at that time for approval or review.
- F5. Footings to be cast in approved material having an allowable capacity as follows:
- Sand Foundations:
- SA1. Required bearing capacity 100 kPa.
- SA2. Trenches must be cleaned of all debris and hand compacted prior to placement of reinforcement.
- Clay Foundations:
- CL1. Required bearing capacity 150 kPa.
- CL2. Trenches must be cleaned of all debris. Soft spots must be cut out and filled as per compacted fill notes, prior to placement of reinforcement.
- Shale Foundations:
- SH1. Required bearing capacity 400 kPa.
- SH2. Excavation for footings into shale must be cast or capped with plain concrete on the same day as excavation.
- Sandstone Foundations:
- SSI. Required bearing capacity 600 kPa.
- SS2. Scrape weathered surface to remove cleaved sandstone under footings. Refer adjacent for assumed Design bearing strata.
- F6. Future development of neighboring properties may affect ground water conditions on this site. Consequently, reactivity in subgrade beneath footings may be locally altered therefore putting footing at risk of differential settlement. We recommend that, particularly in clay subgrades, agricultural drainage is installed to the upstream perimeter of the building at a distance from the building which is outside the zone of influence of the footings. The agricultural drain must be installed below the fluctuating seasonal zone which should be identified by geotechnical investigation.
- F7. UNLESS OTHERWISE APPROVED. Excavations near new or existing footings shall not be within the footing influence line.



CONCRETE

- C1. All workmanship and materials shall be in accordance with AS 3600.
- C2. Concrete quality shall be as follows and shall be verified by tests.
- C3. All concrete unless otherwise noted shall have a slump of 80mm at point of placement, a max. aggregate size of 20 mm. No water shall be added to the mix prior to or during placement of concrete. Strength as specified on plans.
- C4. Clear concrete cover to reinforcement shall be as follows unless otherwise shown-

ELEMENT	INTERIOR	EXTERIOR	EXTERIOR CAST AGAINST GROUND
FOOTINGS	-	-	50
COLUMNS/PEDESTALS	30 UNO	REFER TO PLAN	-
SLABS/WALLS	25	REFER TO PLAN	40 ON MEMBRANE
BEAMS	25 UNO	REFER TO PLAN	50
BLOCKWORK	55 FROM APPROPRIATE FACE		

- C5. Sizes of concrete elements do not include thickness of applied finishes.
- C6. All Construction Joints locations shall be approved by the Structural Engineer.
- C7. Beam depths are written first and include slab thickness, if any.
- C8. No holes or chases other than those shown on the structural drawings shall be made in concrete elements without the prior approval of the engineer.
- C9. Shrinkage reducing admixtures such as 'Eclipse' or approved equivalent, if specified, must be added to mix prior to pour.
- C10. Water reducing agents, if specified, must be added to mix prior to pour. No extra water is to be added to increase slump.
- C11. Where vertical slab/beam surfaces are formed against a masonry (or other) wall, provide 10 mm styrene separation material.
- C12. Water must not be added to concrete mix prior to placement of concrete.
- C13. Above covers may have to be adjusted if fire rating is a requirement.

REINFORCEMENT

- R1. All reinforcement specified is Grade D500 unless noted otherwise.
- R2. Reinforcement is represented diagrammatically it is not necessarily shown in true projection.
- R3. Top reinforcement is to be continuous over supports. Bottom reinforcement to be lapped at supports.
- R4. Welding of reinforcement shall not be permitted unless shown on the structural drawings.
- R5. Pipes or conduits shall not be placed within the zone of concrete cover to the reinforcement without the approval of the engineer.
- R6. All reinforcing bars and fabric shall comply with AS 4671-2001.
- R7. Reinforcement symbols:
- N - Grade 500N deformed bar (D500) Normal Ductility
- R - Grade 250N plain round bar (R250) Normal Ductility.
- SL - Grade 500L welded deformed ribbed mesh (D500) Square Low Ductility.
- RL - Grade 500L welded deformed ribbed mesh (D500) Rectangular Low Ductility.
- The number immediately following these symbols is the number of millimeters in the bar diameter.
- Example : 8 N12-250
- Denotes 8, Grade 500N deformed bars, 12 mm diameter at 250 cts.
- R8. Fabric reinforcement to be lapped 1 complete square + 25 mm unless noted otherwise.
- R9 All reinforcement shall be firmly supported on bar chairs spaced at a maximum of 750 centres both ways under rod and fabric reinforcement. Reinforcement shall be tied at alternate intersections.

FORMWORK

- FW1. Formwork must be cleaned of all debris prior to casting of concrete.
- FW2. Minimum stripping times for form work shall be as recommended in AS 3610 - 1990 or as directed by the engineer.
- FW3. The finished concrete shall be a dense homogeneous mass, completely filling the form work, thoroughly embedding the reinforcement and free of stone pockets. All concrete elements including slabs on ground and footings shall be compacted with mechanical vibrators.
- FW4. Curing of all concrete is to be achieved by keeping surfaces continuously wet for a period of 3 days, followed by prevention of loss of moisture for seven days followed by a gradual drying out. Approved sprayed on curing compounds may be used where no floor finishes are proposed. Polythene sheeting or wet hessian may be used if protected from wind and traffic.

BRICKWORK

- BR1. Brickwork is to be constructed to AS 3700.
- BR2. Two layers of approved greased metal based slip material shall be used over all load bearing walls that support concrete slabs and placed on smooth brickwork or trowelled mortar finish. Non load-bearing walls shall have 10 mm compressible material and ties to the slab soffit.

ASSUMED FOUNDATION CLASSIFICATION FOR DESIGN PURPOSES - CLASS 1A
ASSUMED BEARING STRATA FOR DESIGN PURPOSES - Rock 600 kPa
CONTRACTOR TO ENGAGE GEOTECHNICAL CONSULTANT TO VERIFY FOUNDATION CLASSIFICATION

- BR3. No brickwork shall be constructed on suspended slabs until all propping has been removed from the underside of the slab and the concrete has the specified 28 day cylinder strength verified by tests.
- BR4. Control joints to be placed at a maximum of 8m centres or in accordance with AS 3700.
- BR5. Exposure grade bricks to be used below damp proof course.
- BR6. Vertical control joint material where specified on plan between slabs and brick walls shall be: 10 mm Spandex External UNO. Bitumastic fibreboard internal UNO.
- BR7. Provide stainless steel wall ties below DPC to AS 3700. Provide galvanized wall ties above DPC to AS 3700 & Local Council Specifications.
- BR8. Dry Pressed Bricks should always be used for brick retaining walls. In addition we recommend that dry pressed bricks be used for all types of construction where possible. Dry pressed bricks grow only half as much as extruded bricks. Extruded bricks are difficult to fix to and excessive brick growth leads to cracking in walls and render.

BLOCKWORK

- BL1. Concrete blocks shall have a minimum compressive strength of 15 MPa and conform to AS 1500. Masonry to be constructed to AS 3700.
- BL2. Where cores of hollow blocks are to be filled, properly compacted 20MPa concrete with 10 mm aggregate and 230 mm slump shall be used. Clean out openings must be utilized for all cores.
- BL3. Location of actual starters is critical to suit block cores, allow 55 mm cover from the outside face of blockwork. All reinforcement lap lengths to conform to AS 3600.
- BL4. Control joints to be placed at a maximum of 8 m centres or in accordance with AS 3700.
- BL5. Vertical control joint material where specified on plan between slabs and brick walls shall be: 10 mm Spandex External UNO. Bitumastic fibreboard internal UNO.
- BL6. Retaining walls or any reinforced and concrete core filled block walls to be of Double 'U' Block Construction.
- BL7. No blockwork shall be constructed on suspended slabs until all propping has been removed from the underside of the slab and the concrete has the specified 28 day cylinder strength verified by tests. Unless approved by the Structural Engineer.
- BL8. Max. pour height for unrestrained blockwork is 2000.

STEEL

- S1. All Structural steelwork to be Grade 300 or greater. Design, fabrication and erection to be in accordance with AS 4100.
- S2. Materials and workmanship shall comply with AS 1250 - 1981, SAA Steel Structures Code and the specification for Structural Steel.
- S3. Rolled steel sections including steel plates shall comply with AS 3678-1990.
- S4. Cold formed steel sections shall be Grade 450 Zinc coated in accordance with AS 1538-1988.
- S5. Welded and seamless steel hollow sections shall comply with AS 1163 Grade 350.
- S6. Bolt Designation:
- 4.6S - Commercial bolts Grade 4.6, snug tightened.
- 8.8S - High Strength structural bolts Grade 8.8, snug tightened.
- 8.8TB - High Strength structural bolts Grade 8.8, fully tightened to AS 1511 and acting as a Bearing Joint.
- 8.8TF - High Strength structural bolts Grade 8.8, fully tensioned to AS 1511 and acting as a Bearing Joint.
- Unless noted otherwise, all bolts will be 8.8S.
- S7. Unless shown otherwise, minimum connection shall be 2M16 bolts, 10 thick gusset plates, 6mm continuous fillet welds.
- S8. Load indicating washers shall be used in all fully tensioned joints. (8.8TF & 8.8TB).
- S9. All welding shall be carried out in accordance with AS 1554 SAA Structural Steel Welding Code.
- S10. Unless noted otherwise all welds shall be category SP using E41xx Electrodes. All butt welds shall be complete penetration butt welds category SP.
- S11. Grouting of anchor bolt sleeves and base plates shall be completed by the contractor using High Strength, Non-Shrink grout.
- S12. Fabrication and erection tolerances for Structural Steelwork shall be in accordance with AS 4100.
- S13. Purlin bolts shall be M12 - 4.6S galvanised.
- S14. Steel work shall have one of the following grades of corrosion protection:
- INTERNAL
- a. Thoroughly cleaned wire brushing, followed by two coats of zinc phosphate primer equivalent to Dulux Luxaprim applied by hand using brushes to achieve a total dry film thickness of 70 microns.
- EXTERNAL ELEMENTS, 4 ELEMENTS WITHIN EITHER SKIN OF EXTERNAL CAVITY WALLS GREATER THAN 2 km FROM SEA WATER:
- b. Preparation Blast clean to a minimum standard Class 2.5 in accordance with AS 1627 Part 4.
- Primer 2-pack epoxy phosphate at dft 75 microns (Dulux Duremax P14).
- Barrier Coat 2-pack epoxy micaceous iron oxide, dft 100 microns.
- Finish Coat 2-pack epoxy high gloss acrylic to dft 75 microns. (e.g. Dulux Acrathane 1 F)

- c. Hot dipped galvanized to AS 4680. Where the galvanic (Hot Dip Galvanized) coating is compromised by welding, bolting or damage, two pack zinc rich epoxy primer (Dulux Zincanode 202) is to be applied after wire brushing affected area (use 3 coats minimum) or Hot Metal Spray in accordance with AS 4680.
- PAINING OVER HOT DIP GALVANISED STEEL:
- Degrease and preparation whip blast. Application of a general purpose epoxy (Dulux Duremax GPE) thickness 125 microns. Application of a high build polyurethane (Dulux Weathermax HBR) thickness 100 microns
- EXTERNAL ELEMENTS, 4 ELEMENTS WITHIN EITHER SKIN OF EXTERNAL CAVITY WALLS LESS THAN 2 km FROM SEA WATER:
- d. Preparation blast clean to minimum Class 2.5
- Application of a two pack zinc rich epoxy primer (Dulux Zincanode 402) thickness 75 microns. Application of a general purpose epoxy (Dulux Duremax GPE) thickness 125 microns. Application of a high build polyurethane (Dulux Weathermax HBR) thickness 100 microns
- S15. Workshop drawings shall be prepared and two copies submitted to the engineer for review prior to fabrication commencement.

TIMBER

- T1. All workmanship and materials to be in accordance with AS 1684, AS 1720 and as 3459. All soft wood to be Grade F7 unless noted otherwise. All hardwood to be minimum Grade F14 unless otherwise noted. Exposed timber to be CCA treated (to AS 1604) redried after full impregnation, or durability class 1, 2 or 3. ALL SOFTWOOD TIMBER FRAMING TO HAVE A MINIMUM TREATMENT PROTECTION OF H2 or T2 TREATED FOR TERMITE PROTECTION UNLESS NOTED OTHERWISE.
- T2. All joists deeper than 150 to have blocking over support bearers and at a maximum 3000 centres.
- T3. Roof trusses to be designed by the manufacturer to the relevant standards. Pre camber to be an amount equal to dead load deflection unless otherwise noted..
- T4. All holes for bolts to be exact size. Washers to be used under all heads and nuts and to be at least 2.5 times the bolt diameter. Bolts to be M16 grade 4.6 unless noted otherwise.
- T5. Treat all exposed cut ends with Reseal by Protim to manufacturers specification to achieve required Hazard Level Exposure Classification.
- T6. Battens for T & G to be Kiln Dried to 12 %. 38mm minimum, deep treated pine or as recommended by supplier. Flooring to be installed no sooner than 28 days after slab pour.
- T7. Hot dip galvanized nails/clou/screws to be used with all timber connections.
- T8. Continuous nailing must not be used for any timber connections.
- T9. All exposed CCA treated pine to have an application of penetrating sealer to reduce warping and twist of the timber due to varying moisture content in service.
- T10. All Stud walls to be 90x45 F7 Kiln Dried T2 Treated at 450 Cts and noggings to AS 1684.

COMPACTED FILL

- CF1. Only to be used with approval by Engineer & to be certified by a geotechnical Engineer.
- CF2. Clear organic material, topsoil and any uncontrolled existing fill under proposed slabs/footings.
- CF3. Filling shall be granular material compacted in not more than 200 mm layers to a minimum dry density ratio (AS 1289/E4.2 1982) of 95 percent standard maximum dry density.
- CF4. During clearing and excavation for slabs and footings cut out soft spots and fill as above.

INSPECTIONS BY ENGINEER

- 48 HOURS NOTICE IS REQUIRED BEFORE ANY SITE INSPECTION
1. Bearing strata of all footings prior to concrete pour by Geotechnical Engineer.
2. Any reinforcement prior to concrete pour.
3. Timber and Steel framing prior to cladding or lining.
4. Steel ladders after installation.
- CONTACT YOUR PCA (Principal Certifying Authority) AS TO REQUIREMENTS FOR MANDATORY CRITICAL STAGE INSPECTIONS IN ACCORDANCE WITH REVISED EP4A ACT REGULATIONS EFFECTIVE JULY 1, 2004.
6. Inspection by Geotechnical Engineer over 1.5m of vertical cut through Sandstone bed rock to permit identification of defects and remedial measures initiated.

A3

DOCUMENT CERTIFICATION

Date: JULY 11
Rick G. Wray
BE(Civil), CPEng, MIEAust., NPER.
(Director Northern Beaches Consulting Engineers)

NORTHERN BEACHES Consulting Engineers P/L.

A.C.N. 076 121 616 A.B.N. 24 076 121 616
Suite 207, 30 FISHER ROAD
DEE WHY N.S.W. 2099
Ph: (02) 9984 7000 Fax: (02) 9984 7444
e-mail: nb@nbconsulting.com.au
web page: www.nbconsulting.com.au

Architect:

ZEN SCAPES

Client:

MICHAEL ERROR

Project:

Lot 7 Wakehurst Parkway
SEAFORTH

Drawing Title:

GENERAL NOTES

Date:

JULY 11

Design:

RGW

Drawn:

LFC

Checked:

RFW

Job No:

110665

Drawing No:

S01

Rev:

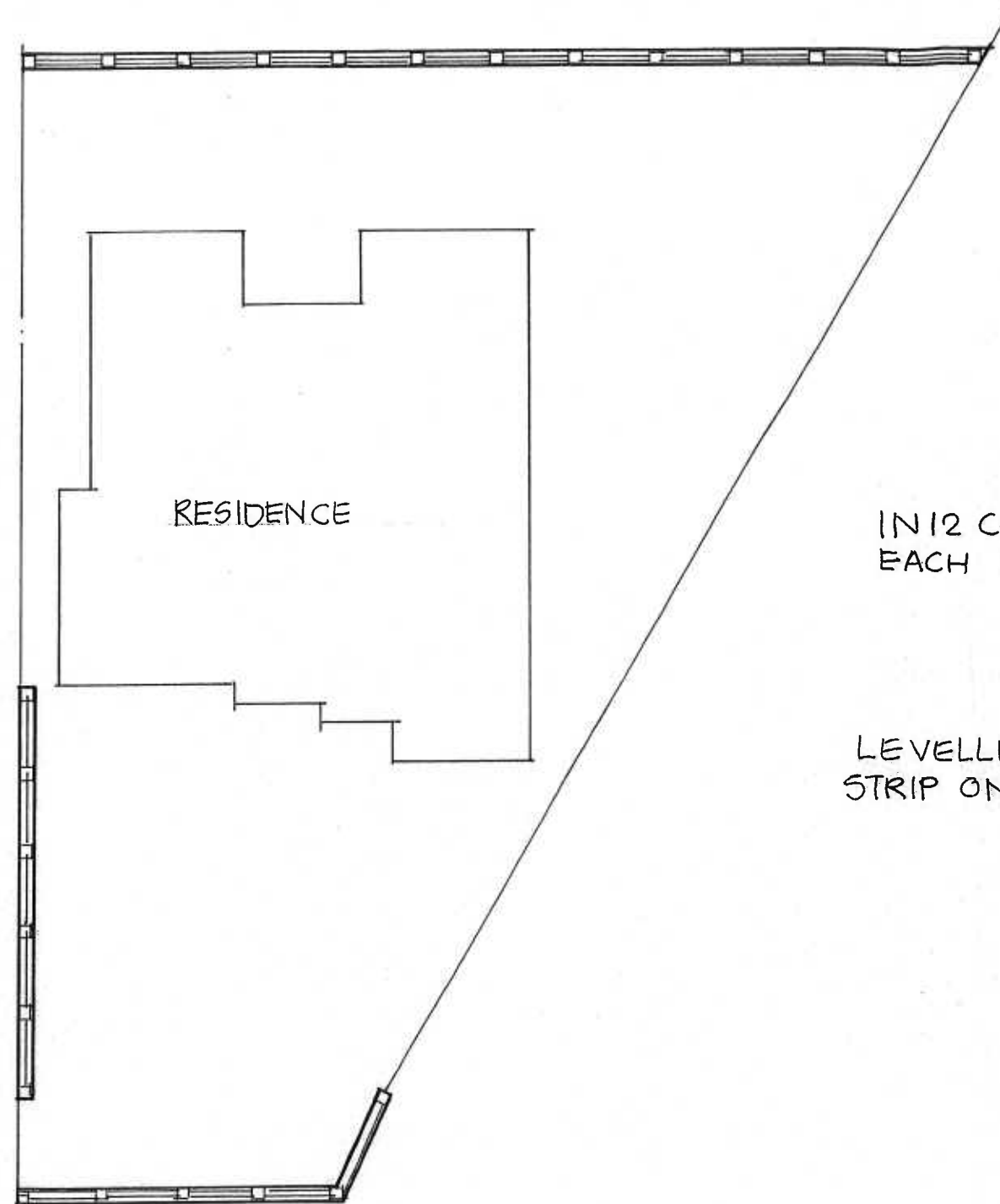
Date: Rev: Amendment:

By:

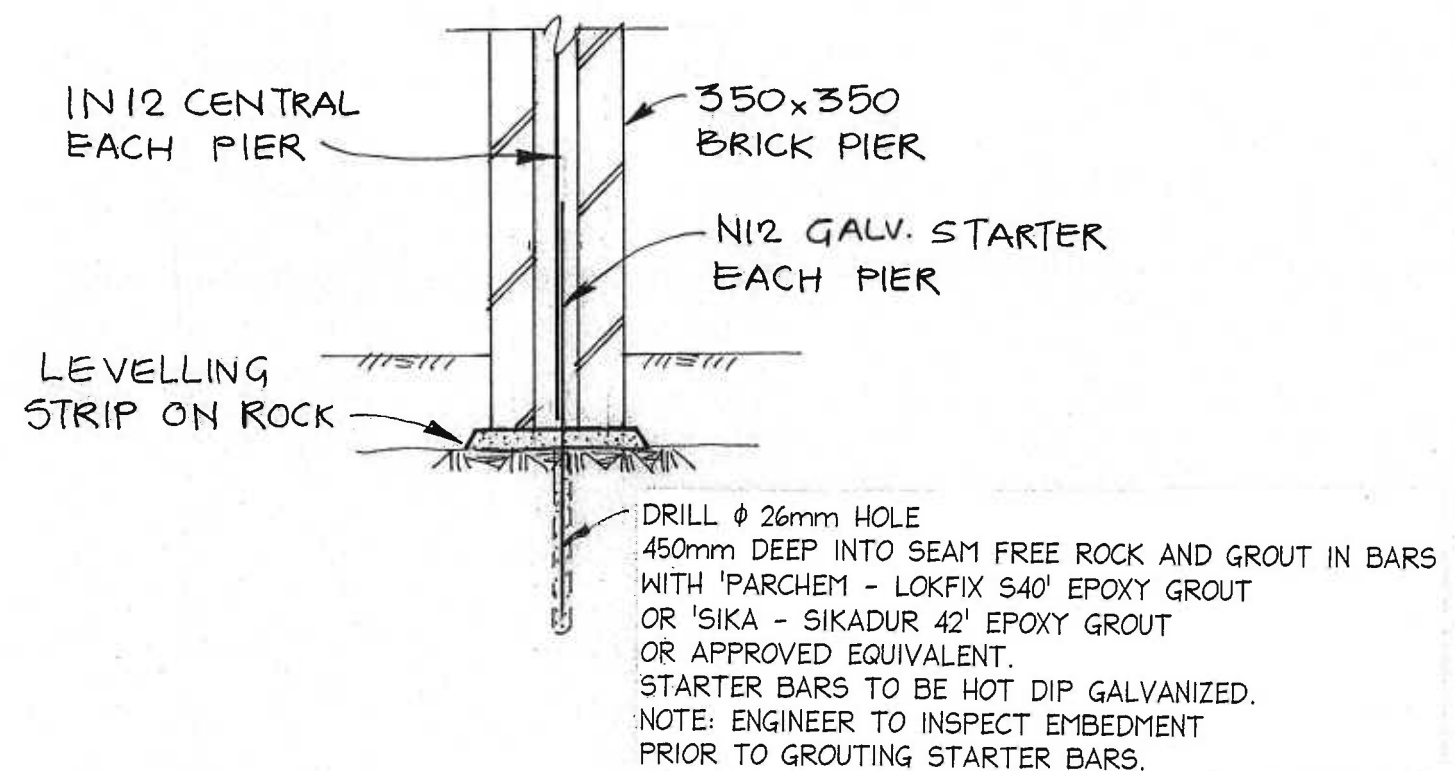
The copyright of this drawing remains with Northern Beaches Consulting Engineers P/L.

NOTES:

1. ALL DIMENSIONS TO BE VERIFIED ON SITE BY BUILDER BEFORE COMMENCING WITH WORK.
2. FOR GENERAL NOTES REFER TO DRAWING NUMBER: 501.



SITE PLAN
1:200



FENCE PIER DETAIL 1:20
(ROCK CLOSE TO SURFACE)



IF IN DOUBT ASK

NB Northern Beaches Consulting Engineers Pty Ltd.

DOCUMENT CERTIFICATION

Date: JULY 11
Rick G. Wray
BE(Civil), CPEng, MIEAust., NPER.
(Director Northern Beaches Consulting Engineers)

NORTHERN BEACHES Consulting Engineers P/L.

A.C.N. 076 121 616 A.B.N. 24 076 121 616
Suite 207, 30 FISHER ROAD
DEE WHY N.S.W. 2099
Ph: (02) 9984 7000 Fax: (02) 9984 7444
e-mail: nb@nbconsulting.com.au
web page: www.nbconsulting.com.au

Architect:

ZEN SCAPES

Client:

MICHAEL EROR

Project:

LOT 7 WAKEHURST P'WAY
SEAFORTH

Drawing Title:

SITE PLAN & DETAIL

Date:

JULY '11

Design:

RGW

Drawn:

LFC

Checked:

RW

Job No:

110665

Drawing No:

502

Rev:

A3

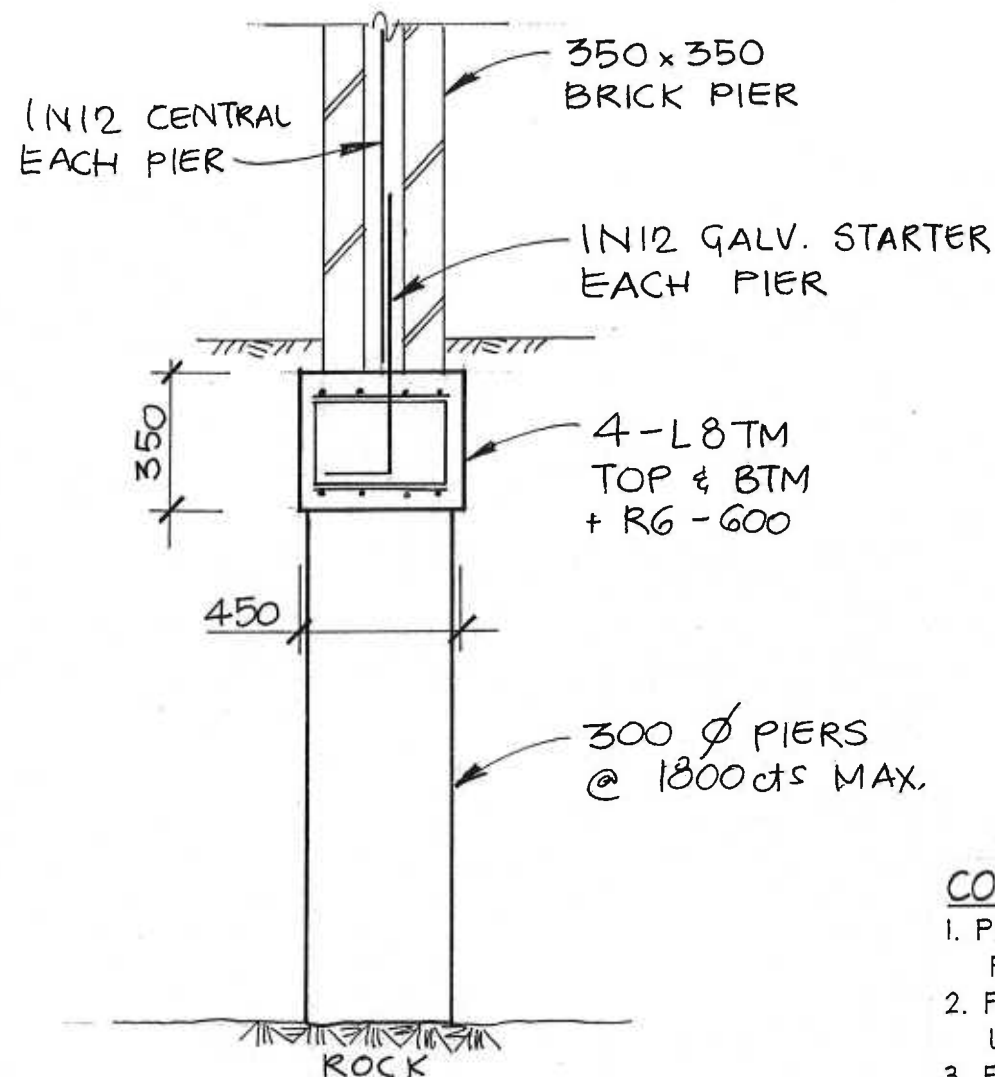
Date: Rev: Amendment:

By:

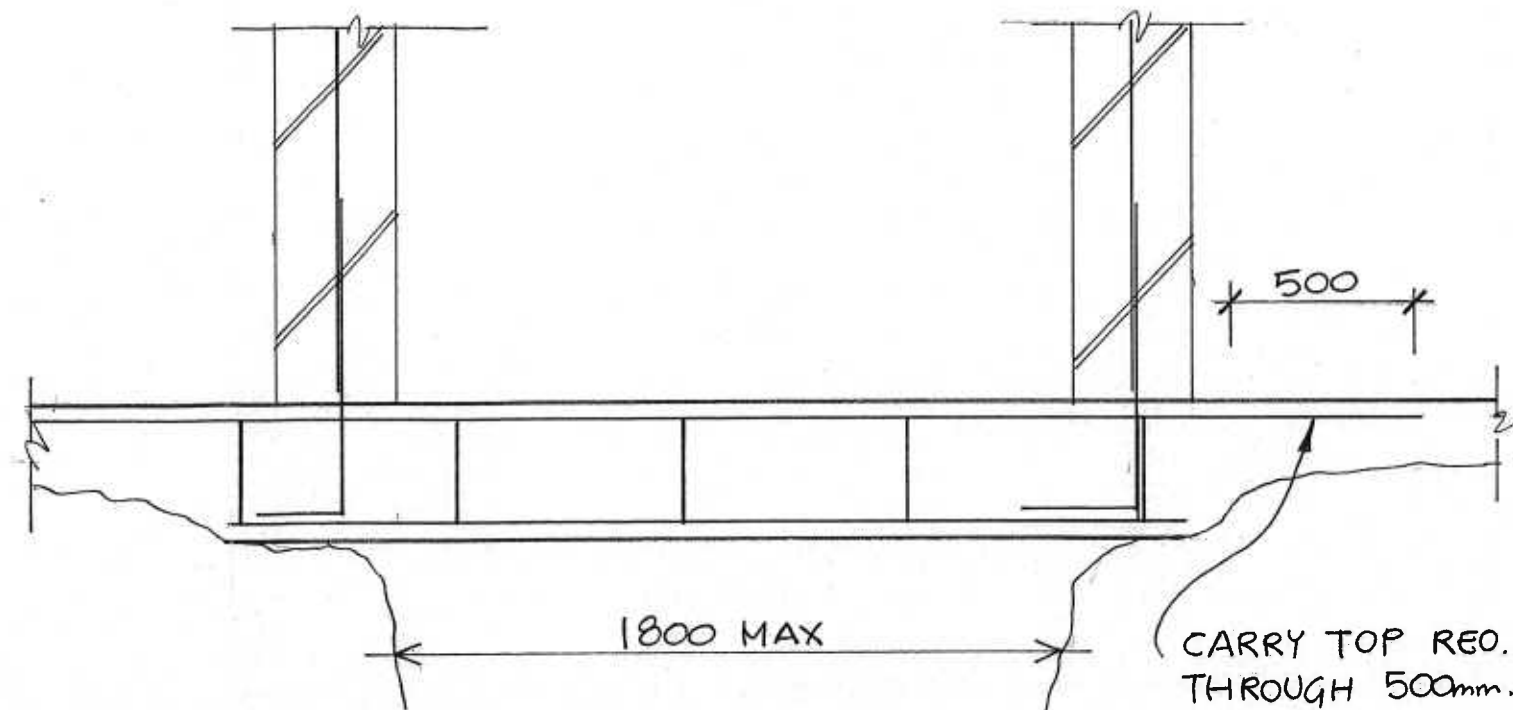
The copyright of this drawing remains with Northern Beaches Consulting Engineers P/L.

NOTES:

1. ALL DIMENSIONS TO BE VERIFIED ON SITE BY BUILDER BEFORE COMMENCING WITH WORK.
2. FOR GENERAL NOTES REFER TO DRAWING NUMBER: S01.



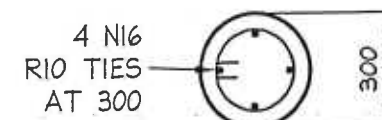
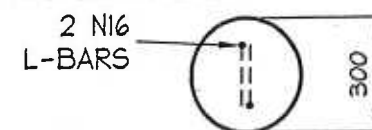
FENCE PIER DETAIL 1:20
(ROCK AT LOW LEVEL)



TYPICAL DETAIL ACROSS VOID IN ROCK
1:20

CONCRETE PIERS:

1. PIERS TO BE 300mm DIAMETER FOUNDED ON SOUND ROCK
2. FOR DEPTH LESS THAN 1200mm UN-REINFORCED.
3. FOR DEPTH GREATER THAN 1200mm AND LESS THAN 1800mm. 2 N16 L-BARS.
4. FOR DEPTH GREATER THAN 1800mm AND LESS THAN 3000mm. 4 N16, R10 TIES AT 300.
5. FOR DEPTH GREATER THAN 3000mm TO BE CONFIRMED BY ENGINEER



FOR LIGHT LOADS LEVEL SITE

TYPE 'FPI' FOOTING PIER SECTION

SCALE = 1 : 20



IF IN DOUBT ASK

Northern Beaches Consulting Engineers Pty Ltd.

DOCUMENT CERTIFICATION

Date: 20/11/11
Rick G. Wray
BE(Civil), CPEng, MIEAust., NPER.
(Director Northern Beaches Consulting Engineers)

NORTHERN BEACHES Consulting Engineers P/L.

A.C.N. 076 121 616 A.B.N. 24 076 121 616
Suite 207, 30 FISHER ROAD
DEE WHY N.S.W. 2099
Ph: (02) 9984 7000 Fax: (02) 9984 7444
e-mail: nb@nbconsulting.com.au
web page: www.nbconsulting.com.au

Architect: ZEN SCAPES

Client: MICHAEL EROR

Project: LOT 7 WAKEHURST P'WAY SEAFORTH

Drawing Title: PIER / FOOTING DETAILS

Date: JULY 11 Design: RGW Drawn: LFC Checked: RSW

Job No: 110665 Drawing No: S03 Rev:

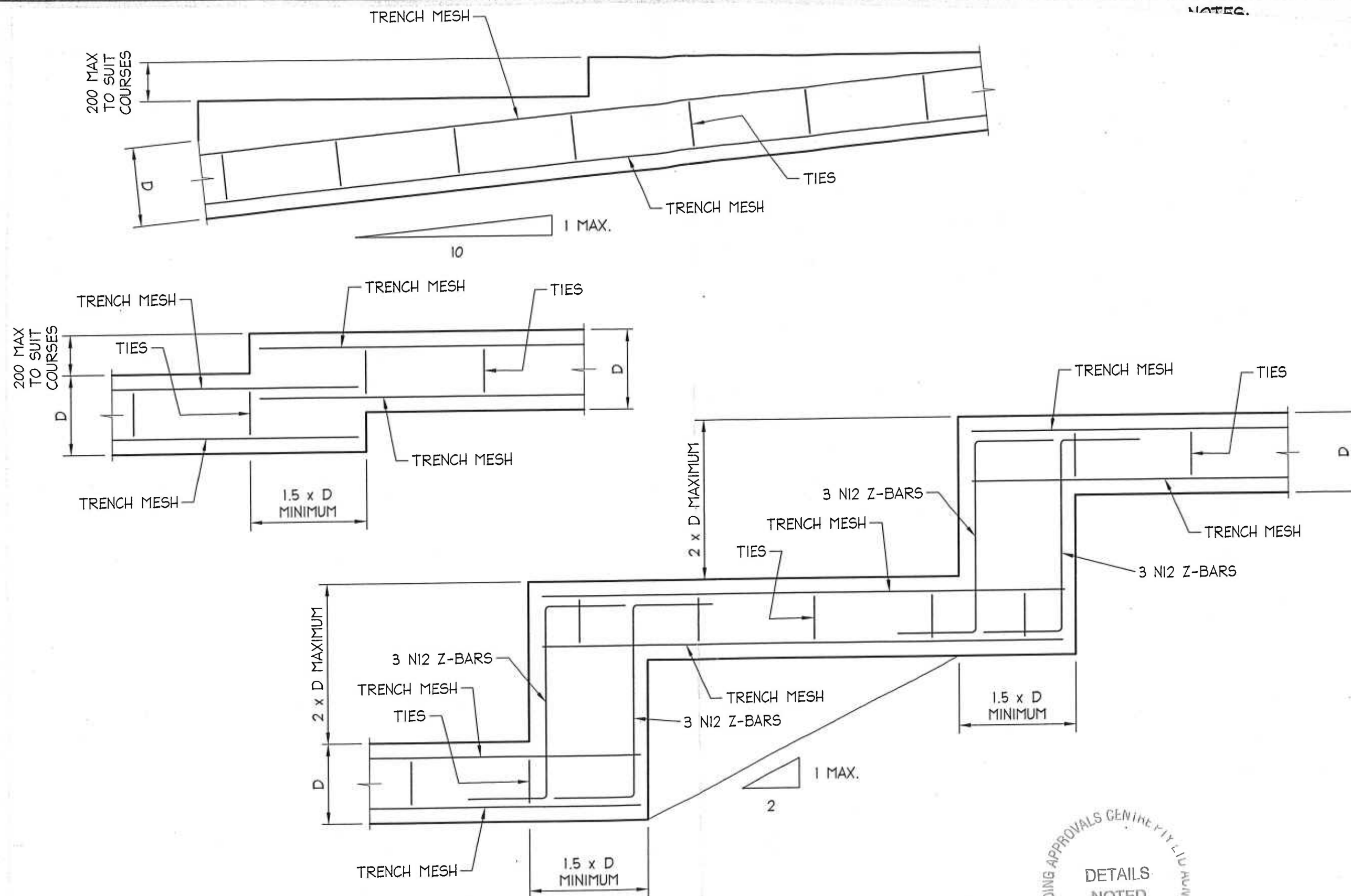
A3

Date: Rev: Amendment: By:

The copyright of this drawing remains with Northern Beaches Consulting Engineers P/L.

NOTES.

IFIED
RE
2



TYPICAL FOOTING STEP DETAILS

SCALE = 1 : 20



NB Northern Beaches Consulting Engineers Pty Ltd.

A3

DOCUMENT CERTIFICATION

Date: JULY 11
 Rick G. Wray
 BE(Civil), CPEng, MIEAust., NPER.
 (Director Northern Beaches Consulting Engineers)

NORTHERN BEACHES Consulting Engineers P/L

A.C.N. 076 121 616 A.B.N. 24 076 121 616
 Suite 207, 30 FISHER ROAD
 DEE WHY N.S.W. 2099
 Ph: (02) 9984 7000 Fax: (02) 9984 7444
 e-mail: nb@nbconsulting.com.au
 web page: www.nbconsulting.com.au

Architect:
 ZEN SCAPES

Client:
 MICHAEL EROR

Project:
 LOT 7 WAKEHURST P'WAY
 SEAFORTH

Drawing Title:
 FOOTING STEP DETAILS

Date:
 JULY 11

Design:
 RGW

Drawn:
 LFC

Checked:
 RGW

Job No:
 110665

Drawing No:
 504

Rev:

Date: Rev: Amendment: By:

The copyright of this drawing remains with Northern Beaches Consulting Engineers P/L.

WAKEHURST PARKWAY

EXISTING CONCRETE FOOTPATH

PROPOSED NATIVE TALL SHRUB PLANTING TO REAR PROPERTY BOUNDARY FOR SCREENING & PRIVACY (REFER TO PLANT SCHEDULE).

PROPOSED 1.8m HIGH MASONRY BOUNDARY FENCE - TYPE 1 ALONG FRONT BOUNDARY AS SHOWN (REFER TO DETAIL).

PROPOSED 1.2m WIDE CONCRETE ACCESS PATHWAY WITH GATE AND LETTERBOX

EXISTING NATIVE SHRUB TO COUNCIL VERGE TO BE REMOVED FOR FENCE CONSTRUCTION (REFER TO EXISTING TREE SCHEDULE).

EXISTING GRASSED COUNCIL VERGE

PROPOSED UNDERGROUND OSD / RAINWATER RE-USE TANKS, REFER TO STORMWATER MANAGEMENT PLAN.

PROPOSED 1.8m HIGH TIMBER PALING FENCE TO SIDE BOUNDARY AS SHOWN. (REFER TO DETAIL).

PROPOSED STEPPING STONES & DECORATIVE GRAVEL MULCH ACCESS TO SIDE OF RESIDENCE. PLANT WITH NATIVE GROUNDCOVER (REFER TO PLANT SCHEDULE & LANDSCAPE NOTES).

LOT 6 VACANT

PROPOSED 1.5m HIGH MASONRY & TIMBER FENCE AS SHOWN - TYPE 2 (REFER TO DETAIL).

RETAIN AND PROTECT EXISTING TREE ON ADJOINING PROPERTY. REMOVE OVERHANGING BRANCH (REFER TO EXISTING TREE SCHEDULE).

PROPOSED NATIVE FEATURE GARDEN BED FRAMING NEW DRIVEWAY WITH SHRUB & GROUNDCOVER PLANTINGS (REFER TO PLANT SCHEDULE).

PROPOSED SLIDING GATE TO DRIVEWAY ENTRY

APPROX. POSITION OF SEWER MAIN

ACCESS VIA ACACIA ROAD

RL 10.015 Invert level

RL 10.00

RL 10.13

RL 10.44

RL 11.33

RL 11.44

RL 11.54

RL 11.63

RL 11.77

RL 11.83

RL 11.94

RL 12.00

RL 12.11

RL 12.22

RL 12.33

RL 12.44

RL 12.55

RL 12.66

RL 12.77

RL 12.88

RL 12.99

RL 13.10

RL 13.21

RL 13.32

RL 13.43

RL 13.54

RL 13.65

RL 13.76

RL 13.87

RL 13.98

RL 14.09

RL 14.20

RL 14.31

RL 14.42

RL 14.53

RL 14.64

RL 14.75

RL 14.86

RL 14.97

RL 15.08

RL 15.19

RL 15.30

RL 15.41

RL 15.52

RL 15.63

RL 15.74

RL 15.85

RL 15.96

RL 16.07

RL 16.18

RL 16.29

RL 16.40

RL 16.51

RL 16.62

RL 16.73

RL 16.84

RL 16.95

RL 17.06

RL 17.17

RL 17.28

RL 17.39

RL 17.50

RL 17.61

RL 17.72

RL 17.83

RL 17.94

RL 18.05

RL 18.16

RL 18.27

RL 18.38

RL 18.49

RL 18.60

RL 18.71

RL 18.82

RL 18.93

RL 19.04

RL 19.15

RL 19.26

RL 19.37

RL 19.48

RL 19.59

RL 19.70

RL 19.81

RL 19.92

RL 20.03

RL 20.14

RL 20.25

RL 20.36

RL 20.47

RL 20.58

RL 20.69

RL 20.80

RL 20.91

RL 21.02

RL 21.13

RL 21.24

RL 21.35

RL 21.46

RL 21.57

RL 21.68

RL 21.79

RL 21.90

RL 22.01

RL 22.12

RL 22.23

RL 22.34

RL 22.45

RL 22.56

RL 22.67

RL 22.78

RL 22.89

RL 23.00

RL 23.11

RL 23.22

RL 23.33

RL 23.44

RL 23.55

RL 23.66

RL 23.77

RL 23.88

RL 23.99

RL 24.10

RL 24.21

RL 24.32

RL 24.43

RL 24.54

RL 24.65

RL 24.76

RL 24.87

RL 24.98

RL 25.09

RL 25.20

RL 25.31

RL 25.42

RL 25.53

RL 25.64

RL 25.75

RL 25.86

RL 25.97

RL 26.08

RL 26.19

RL 26.30

RL 26.41

RL 26.52

RL 26.63

RL 26.74

RL 26.85

RL 26.96

RL 27.07

RL 27.18

RL 27.29

RL 27.40

RL 27.51

RL 27.62

RL 27.73

RL 27.84

RL 27.95

RL 28.06

RL 28.17

RL 28.28

RL 28.39

RL 28.50

RL 28.61

RL 28.72

RL 28.83

RL 28.94

RL 29.05

RL 29.16

RL 29.27

RL 29.38

RL 29.49

RL 29.60

RL 29.71

RL 29.82

RL 29.93

RL 30.04

RL 30.15

RL 30.26

RL 30.37

RL 30.48

RL 30.59

RL 30.70

RL 30.81

RL 30.92

RL 31.03

RL 31.14

RL 31.25

RL 31.36

RL 31.47

RL 31.58

RL 31.69

RL 31.80

RL 31.91

RL 32.02

RL 32.13

RL 32.24

RL 32.35

RL 32.46

RL 32.57

RL 32.68

RL 32.79

RL 32.90

RL 33.01

RL 33.12

RL 33.23

RL 33.34

RL 33.45

RL 33.56

RL 33.67

RL 33.78

RL 33.89

RL 34.00

RL 34.11

RL 34.22

RL 34.33

RL 34.44

RL 34.55

RL 34.66

RL 34.77

RL 34.88

RL 34.99

RL 35.10

RL 35.21

RL 35.32

RL 35.43

RL 35.54

RL 35.65

RL 35.76

RL 35.87

RL 35.98

RL 36.09

RL 36.20

RL 36.31

RL 36.42

RL 36.53

RL 36.64

RL 36.75

RL 36.86

RL 36.97

RL 37.08

RL 37.19

RL 37.30

RL 37.41

RL 37.52

RL 37.63

RL 37.74

RL 37.85

RL 37.96

RL 38.07

RL 38.18

RL 38.29

RL 38.40

RL 38.51

RL 38.62

RL 38.73

RL 38.84

RL 38.95

RL 39.06

RL 39.17

RL 39.28

RL 39.39

RL 39.50

RL 39.61

RL 39.72

RL 39.83

RL 39.94

RL 40.05

RL 40.16

RL 40.27

RL 40.38

RL 40.49

RL 40.60

RL 40.71

RL 40.82

RL 40.93

RL 41.04

RL 41.15

RL 41.26

RL 41.37

RL 41.48

RL 41.59

RL 41.70

RL 41.81

RL 41.92

RL 42.03

RL 42.14

RL 42.25

</



Manly Council

This plan is sheet of sheet/s referred to in Council's
Notice of Determination letter dated in respect
Of Development Application No. 281/09 - 596/101
Signed:

This plan is to be read in conjunction with conditions contained within the
Notice of Determination that may change the form of the development or the
manner in which the development proceeds.
These plans are not for construction. Where demolition, site works or
building works are proposed a Construction Certificate is to be obtained prior
to commencement of work.
A copy of the approved Development Application Plans and Construction
Certificate must be kept on site for the duration of the works.

