

PRELIMINARY GEOTECHNICAL ASSESSMENT:

69 Evans Street, Freshwater

1.0	LANDSLIP RISK CLASS (Highlight indicates Landslip Risk Class of property)
<input type="checkbox"/>	A - Geotechnical Report not normally required
<input checked="" type="checkbox"/>	B - Geotechnical Engineer (Under Council Guidelines) to decide if Geotechnical Report is required
<input type="checkbox"/>	C - Geotechnical Report is required
<input type="checkbox"/>	D - Geotechnical Engineer (Under Council Guidelines) to decide if Geotechnical Report is required
<input type="checkbox"/>	E - Geotechnical Report required

2.0 Proposed Development

- 2.1 Update the existing garages and entry to the building on level 5 (ground level at Evans Street).
- 2.2 No excavations or fills are required.
- 2.3 Details of the proposed development are shown on 10 drawings prepared by Woodhouse & Danks Architects, Job number 20005, drawings numbered TD 01 to TD 10, dated 9/8/21.

3.0 Site Location

- 3.1 The site was inspected on the 20th September, 2021.
- 3.2 This residential property is on the low side of the road and has a SW aspect. It is located on the gentle to moderately graded middle reaches of a hillslope. Medium Strength Hawkesbury Sandstone bedrock outcrops on the downhill side of the property. Where sandstone is not exposed, it is expected to underlie the surface at relatively shallow depths. The natural surface of the block has been altered with an

excavation to create a level platform for the downhill side of the building. The proposed development will not alter the surface further for the proposed works.

3.3 The site shows no indications of historical movement in the natural surface that could have occurred since the property was developed. We are aware of no history of instability on the property.

4.0 Site Description

The natural slope falls across the site at an average angle of $\sim 8^\circ$. Three concrete driveways extend off the road frontage to the property. The SE driveway runs down the SE side of the property to a series of garages on level 4 of the building. The centre driveway runs to a series of garages and the entry foyer to the building on level 5. The fill for this driveway is supported on both sides by stable rendered masonry retaining walls reaching $\sim 2.5\text{m}$ high. The NW driveway runs down the NW side of the property to a series of garages on levels 2 and 3 of the building. The part ten-storey unit block is supported on concrete foundations with brick and concrete block infill walls. The supporting walls display no significant signs of movement and the supporting concrete columns stand vertical. An excavation has been made in the slope to create a level platform for the downhill side of the building. The $\sim 3.0\text{m}$ high cut has been taken entirely through competent Medium Strength Sandstone along the SE side of the property. The cut face displays no significant geological defects and is considered stable. The area surrounding the unit block and driveways is mostly paved with some lawn-covered areas. No significant signs of movement associated with slope instability were observed on the grounds. The adjoining neighbouring properties were observed to be in good order as seen from the road and the subject property.

5.0 Recommendations

The proposed development and site conditions were considered and applied to the Council Flow Chart.

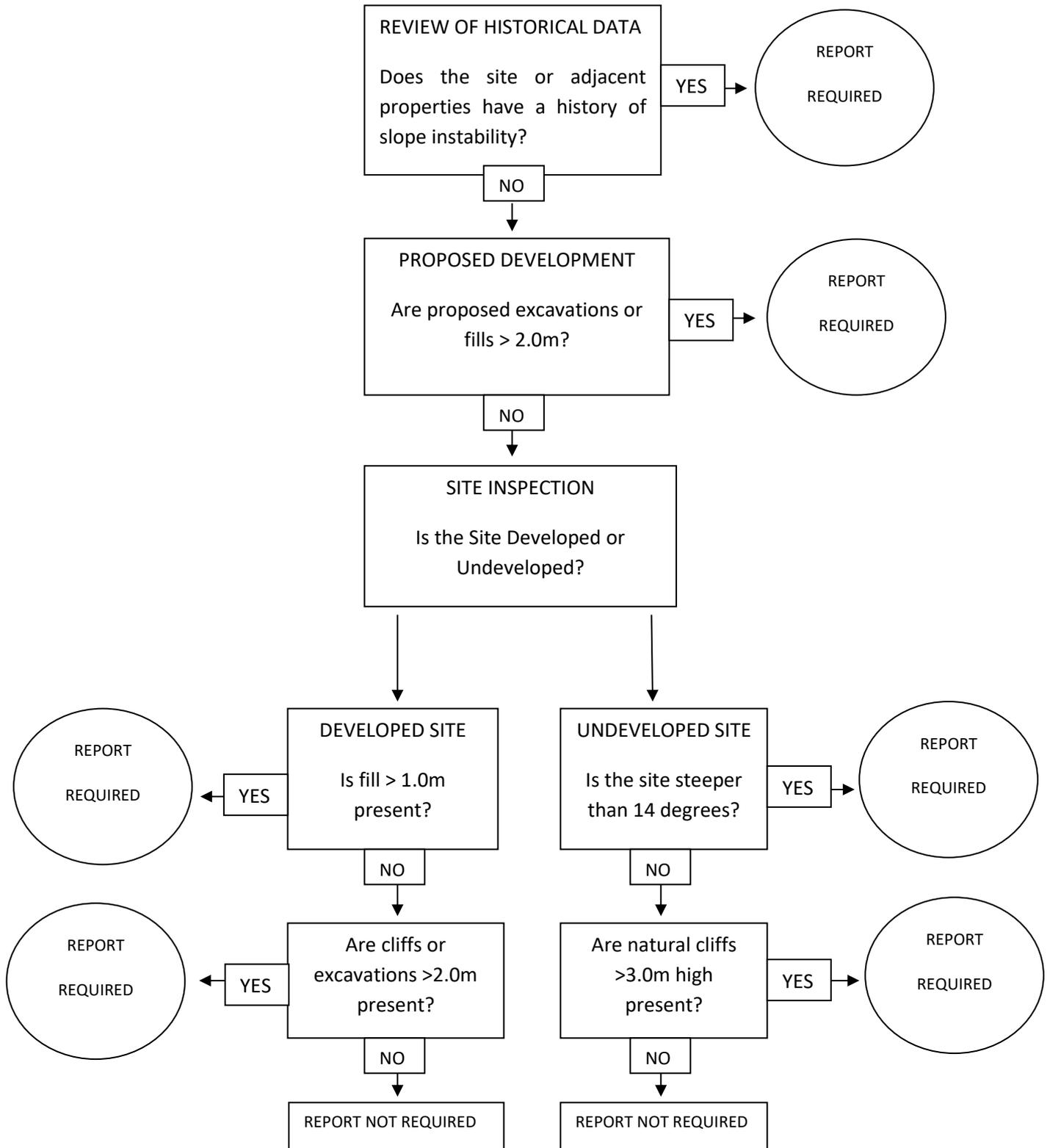
Provided good engineering and building practice are followed, no further Geotechnical assessment is recommended for the proposed development.

White Geotechnical Group Pty Ltd.



Ben White M.Sc. Geol.,
AusIMM., CP GEOL.
No. 222757
Engineering Geologist.

Preliminary Assessment Flow Chart – Northern Beaches Council (Warringah)



Information about your Preliminary Assessment

This Preliminary Assessment relies on visual observations of the surface features observed during the site inspection. Where reference is made to subsurface features (e.g., the depth to rock) these are interpretations based on the surface features present and previous experience in the area. No ground testing was conducted as part of this assessment and it is possible subsurface conditions will vary from those interpreted in the assessment.

In some cases, we will recommend no further geotechnical assessment is necessary despite the presence of existing fill or a rock face on the property that exceed the heights that would normally trigger a full geotechnical report, according to the Preliminary Assessment Flow Chart. Where this is the case, if it is an existing fill, it is either supported by a retaining wall that we consider stable, or is battered at a stable angle and situated in a suitable position on the slope. If it is a rock face that exceeds the flow chart limit height, the face has been deemed to be competent rock that is considered stable. These judgements are backed by the inspection of over 5000 properties on Geotechnical related matters.

The proposed excavation heights referred to in section 2.0 of this assessment are estimated by review of the plans we have been given for the job. Although we make every reasonable effort to provide accurate information excavation heights should be checked by the owner or person lodging the DA. If the excavation heights referred to in in section 2.0 of this assessment are incorrect, we are to be informed immediately and before this assessment is lodged with the DA.