

**GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER
FORM NO. 1 – To be submitted with Development Application**

Development Application for _____
Name of Applicant

Address of site 3 Waratah Road, Palm Beach

The following checklist covers the minimum requirements to be addressed in a Geotechnical Risk Declaration made by geotechnical engineer or engineering geologist or coastal engineer (where applicable) as part of a geotechnical report

I, Ben White on behalf of White Geotechnical Group Pty Ltd
(Insert Name) (Trading or Company Name)

on this the 17/8/23 certify that I am a geotechnical engineer or engineering geologist or coastal engineer as defined by the Geotechnical Risk Management Policy for Pittwater - 2009 and I am authorised by the above organisation/company to issue this document and to certify that the organisation/company has a current professional indemnity policy of at least \$10million.

I:

Please mark appropriate box

- have prepared the detailed Geotechnical Report referenced below in accordance with the Australia Geomechanics Society's Landslide Risk Management Guidelines (AGS 2007) and the Geotechnical Risk Management Policy for Pittwater - 2009
- am willing to technically verify that the detailed Geotechnical Report referenced below has been prepared in accordance with the Australian Geomechanics Society's Landslide Risk Management Guidelines (AGS 2007) and the Geotechnical Risk Management Policy for Pittwater - 2009
- have examined the site and the proposed development in detail and have carried out a risk assessment in accordance with Section 6.0 of the Geotechnical Risk Management Policy for Pittwater - 2009. I confirm that the results of the risk assessment for the proposed development are in compliance with the Geotechnical Risk Management Policy for Pittwater - 2009 and further detailed geotechnical reporting is not required for the subject site.
- have examined the site and the proposed development/alteration in detail and I am of the opinion that the Development Application only involves Minor Development/Alteration that does not require a Geotechnical Report or Risk Assessment and hence my Report is in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009 requirements.
- have examined the site and the proposed development/alteration is separate from and is not affected by a Geotechnical Hazard and does not require a Geotechnical Report or Risk Assessment and hence my Report is in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009 requirements.
- have provided the coastal process and coastal forces analysis for inclusion in the Geotechnical Report

Geotechnical Report Details:

Report Title: Geotechnical Report 3 Waratah Road, Palm Beach
Report Date: 17/8/23

Author: BEN WHITE

Author's Company/Organisation: WHITE GEOTECHNICAL GROUP PTY LTD

Documentation which relate to or are relied upon in report preparation:

Australian Geomechanics Society Landslide Risk Management March 2007.

White Geotechnical Group company archives.

I am aware that the above Geotechnical Report, prepared for the abovementioned site is to be submitted in support of a Development Application for this site and will be relied on by Pittwater Council as the basis for ensuring that the Geotechnical Risk Management aspects of the proposed development have been adequately addressed to achieve an "Acceptable Risk Management" level for the life of the structure, taken as at least 100 years unless otherwise stated and justified in the Report and that reasonable and practical measures have been identified to remove foreseeable risk.

Signature 
Name Ben White
Chartered Professional Status MScGEOLAusIMM CP GEOL
Membership No. 222757
Company White Geotechnical Group Pty Ltd

**GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER
FORM NO. 1(a) - Checklist of Requirements for Geotechnical Risk Management Report for
Development Application**

Development Application for _____	Name of Applicant
Address of site <u>3 Waratah Road, Palm Beach</u>	

The following checklist covers the minimum requirements to be addressed in a Geotechnical Risk Management Geotechnical Report. This checklist is to accompany the Geotechnical Report and its certification (Form No. 1).

Geotechnical Report Details:

Report Title: Geotechnical Report 3 Waratah Road, Palm Beach
Report Date: 17/8/23
Author: BEN WHITE
Author's Company/Organisation: WHITE GEOTECHNICAL GROUP PTY LTD

Please mark appropriate box

- Comprehensive site mapping conducted 5/6/23
(date)
- Mapping details presented on contoured site plan with geomorphic mapping to a minimum scale of 1:200 (as appropriate)
- Subsurface investigation required
 - No Justification _____
 - Yes Date conducted 5/6/23
- Geotechnical model developed and reported as an inferred subsurface type-section
- Geotechnical hazards identified
 - Above the site
 - On the site
 - Below the site
 - Beside the site
- Geotechnical hazards described and reported
- Risk assessment conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
 - Consequence analysis
 - Frequency analysis
- Risk calculation
- Risk assessment for property conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
- Risk assessment for loss of life conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
- Assessed risks have been compared to "Acceptable Risk Management" criteria as defined in the Geotechnical Risk Management Policy for Pittwater - 2009
- Opinion has been provided that the design can achieve the "Acceptable Risk Management" criteria provided that the specified conditions are achieved.
- Design Life Adopted:
 - 100 years
 - Other _____
specify
- Geotechnical Conditions to be applied to all four phases as described in the Geotechnical Risk Management Policy for Pittwater - 2009 have been specified
- Additional action to remove risk where reasonable and practical have been identified and included in the report.
- Risk assessment within Bushfire Asset Protection Zone.

I am aware that Pittwater Council will rely on the Geotechnical Report, to which this checklist applies, as the basis for ensuring that the geotechnical risk management aspects of the proposal have been adequately addressed to achieve an "Acceptable Risk Management" level for the life of the structure, taken as at least 100 years unless otherwise stated, and justified in the Report and that reasonable and practical measures have been identified to remove foreseeable risk.



 Signature

 Name Ben White

 Chartered Professional Status MScGEOLAusIMM CP GEOL

 Membership No. 222757

 Company White Geotechnical Group Pty Ltd

GEOTECHNICAL INVESTIGATION:

Alterations and Additions at 3 Waratah Road, Palm Beach

1. Proposed Development

- 2.1** Extend the existing garage and construct a studio over.
- 2.2** Extend the house to the SE.
- 2.3** Other external additions and alterations.
- 2.4** Details of the proposed development are shown on 11 drawings prepared by Two Form Architecture, drawings numbered 22 026 AR DA 00 to 09, and 22 026 AR DA 12, all Revision A, all dated AUG 2023.

2. Site Description

- 3.1** The site was inspected on the 5th June, 2023.
- 3.2** This residential property lies between Barrenjoey Road and Waratah Road. It is on the low side of Barrenjoey Road and the entire property has been levelled to a similar elevation as Waratah Road. The slope above the property continues at gradually increasing angles. The slope below the property continues at near-level angles to the waterfront.
- 3.3** At the road frontage to Waratah Road, a concrete driveway runs past the SW side of the house (Photo 1) to a stable garage on the SE side of the property (Photo 2) which is the site of the proposed works. Between the road frontage and the house is a tile path flanked by level lawns. A pool has been cut into the N corner of the property (Photo 3). The water level indicates no ground movement has occurred in the shell of the pool since its construction. The two-storey house is supported on rendered brick walls (Photo 4). No significant signs of movement were observed in the supporting walls of the house. A gently sloping lawn surrounded by gardens extend off the SE side of the house to the Barrenjoey Road frontage. The cut for this lawn is supported by a

stable ~1.1m high rendered masonry retaining wall that approximates the E boundary (Photo 5).

3. Geology

The Sydney 1:100 000 Geological Sheet indicates the site is underlain by Newport Formation of the Narrabeen Group, although the waterfront nearby shows medium to fine marine sand (Qhf) of the foredune and at a residential scale the map is not always accurate. Ground testing and observations on site indicate that the proposed works are underlain by medium to fine marine sand (Qhf).

4. Subsurface Investigation

One hand Auger Hole (AH) was put down to identify the soil materials. Three Dynamic Cone Penetrometer (DCP) tests were put down to determine the relative densities of the soil/sands through the profile. The locations of the tests are shown on the site plan attached. It should be noted that a level of caution should be applied when interpreting DCP test results. The test will not pass through hard buried objects so in some instances it can be difficult to determine whether refusal has occurred on an obstruction in the profile or on the natural rock surface. This is not expected to be an issue for the testing on this site. However, excavation and foundation budgets should always allow for the possibility that the interpreted ground conditions in this report vary from those encountered during excavations. See the appended "Important information about your report" for a more comprehensive explanation. The results are as follows:

GROUND TEST RESULTS ON NEXT PAGE

AUGER HOLE 1 (~RL2.6) – AH1 (Photo 6)

Depth (m)	Material Encountered
0.0 to 0.2	TOPSOIL , sandy soil, brown, Medium Dense to Dense, dry, medium to coarse grained, clay and organic material present.
0.2 to 0.5	CLAYEY SOIL , dark brown, Medium Dense, dry, medium grained.
0.5 to 0.75	SAND , brown, Medium Dense, damp, medium to coarse grained.
0.75 to 1.0	SAND , yellow-brown, Medium Dense, wet, medium to coarse grained.

End of hole @ 1.0m in Medium Dense Sand. Water table encountered at ~0.75m.

DCP TEST RESULTS – Dynamic Cone Penetrometer			
Equipment: 9kg hammer, 510mm drop, conical tip.		Standard: AS1289.6.3.2 - 1997	
Depth(m) Blows/0.3m	DCP 1 (~RL2.5)	DCP 2 (~RL2.6)	DCP 3 (~RL2.5)
0.0 to 0.3	6	8	8
0.3 to 0.6	4	7	4
0.6 to 0.9	3	6	8
0.9 to 1.2	7	8	15
1.2 to 1.5	22	14	13
1.5 to 1.8	27	20	16
1.8 to 2.1	34	32	16
2.1 to 2.4	45	#	15
2.4 to 2.7	#		20
2.7 to 3.0			26
3.0 to 3.3			34
3.3 to 3.6			#
	End of Test @ 2.4m	End of Test @ 2.1m	End of Test @ 3.3m

#refusal/end of test. F=DCP fell after being struck showing little resistance through all or part of the interval.

DCP Notes:

DCP1 – End of test @ 2.4m, DCP still very slowly going down, brown sand on wet tip.

DCP2 – End of test @ 2.1m, DCP still very slowly going down, brown sand on wet tip.

DCP3 – End of test @ 3.3m, DCP still very slowly going down, brown sand on wet tip.

5. Geological Observations/Interpretation

In the location of the proposed works, the site is underlain by sandy soil and sands that extend to the depth of the testing. To summarise the test results, Medium Dense to Dense sandy soil occupies the top ~0.2m of the profile, these overlie Medium Dense Sands that extend to the maximum depth of the testing at ~3.3m. Rock was not encountered to the extent of the tests at ~3.3m. See the Type Section attached for a diagrammatical representation of the expected ground materials.

6. Groundwater

The watertable was encountered in AH1 at a depth of ~0.75m below the current surface. This is expected to be below the base of the foundations for the proposed works. As such, the water table will not impact on the proposed development. However, it should be noted the watertable fluctuates with the tide and climatic changes.

7. Surface Water

No evidence of significant surface flows were observed on the property during the inspection. Normal sheet wash is expected to be quickly absorbed into the sandy soil where surfaces are unsealed.

8. Geotechnical Hazards and Risk Analysis

No geotechnical hazards were observed below and beside the property. The gently graded slope that rises across and above the property is a potential hazard (**Hazard One**).

Risk Analysis Summary

HAZARDS	Hazard One
TYPE	The gentle slope that rises across the site and continues above failing and impacting on the existing house and/or proposed works.
LIKELIHOOD	'Rare' (10^{-5})
CONSEQUENCES TO PROPERTY	'Minor (3%)
RISK TO PROPERTY	'Very Low' (5×10^{-7})
RISK TO LIFE	2.5×10^{-7} /annum
COMMENTS	This level of risk is 'ACCEPTABLE.'

(See Aust. Geomech. Jnl. Mar 2007 Vol. 42 No 1, for full explanation of terms)

9. Suitability of the Proposed Development for the Site

The proposed development is suitable for the site. No geotechnical hazards will be created by the completion of the proposed development provided it is carried out in accordance with the requirements of this report and good engineering and building practice.

10. Stormwater

Waratah Road is not guttered adjacent to the subject property. However, Northern Beaches Council mapping shows that there is an existing stormwater system running under Waratah Road. As such, it is recommended all stormwater or drainage runoff from the proposed development be piped to this stormwater system through any tanks that may be required by the regulating authorities.

11. Excavations

Apart from those for footings, no excavations are required.

12. Foundations

The proposed works can be supported on spread footings taken to a depth of no less than 0.4m into the underlying Medium Dense Sands of the natural profile. This is a suitable bearing material.

A maximum allowable bearing pressure of 100kPa can be assumed for footings supported on the undisturbed, Medium Dense Sands of the natural profile.

The footing excavation walls in sand are to be shored with timber to prevent collapse prior to the concrete pour. The base of the footing excavations should be compacted as the excavation will loosen the upper sands. This can be carried out with a hand-held plate compactor. Water may be used to assist in compaction in sand but footing materials should be kept damp but not saturated. As a guide to the level of compaction required a density index of >85% is to be achieved, correlating to a Very Dense Sand.

13. Geotechnical Review

The structural plans are to be checked and certified by the geotechnical engineer as being in accordance with the geotechnical recommendations. On completion, a Form 2B will be issued. This form is required for the Construction Certificate to proceed.

14. Inspection

The client and builder are to familiarise themselves with the following required inspection as well as council geotechnical policy. We cannot provide geotechnical certification for the regulating authorities or the owner if the following inspection has not been carried out during the construction process.

- All footings are to be inspected and approved by the geotechnical consultant while the excavation equipment and contractors are still onsite and before steel reinforcing is placed or concrete is poured.

White Geotechnical Group Pty Ltd.



Nathan Gardner
B.Sc. (Geol. & Geophys. & Env. Stud.)
Engineering Geologist and Environmental Scientist.

Reviewed By:



Ben White M.Sc. Geol.,
AusIMM., CP GEOL.
No. 222757
Engineering Geologist.



Photo 1



Photo 2



Photo 3



Photo 4



Photo 5



Photo 6: AH1 – Downhole is from bottom to top

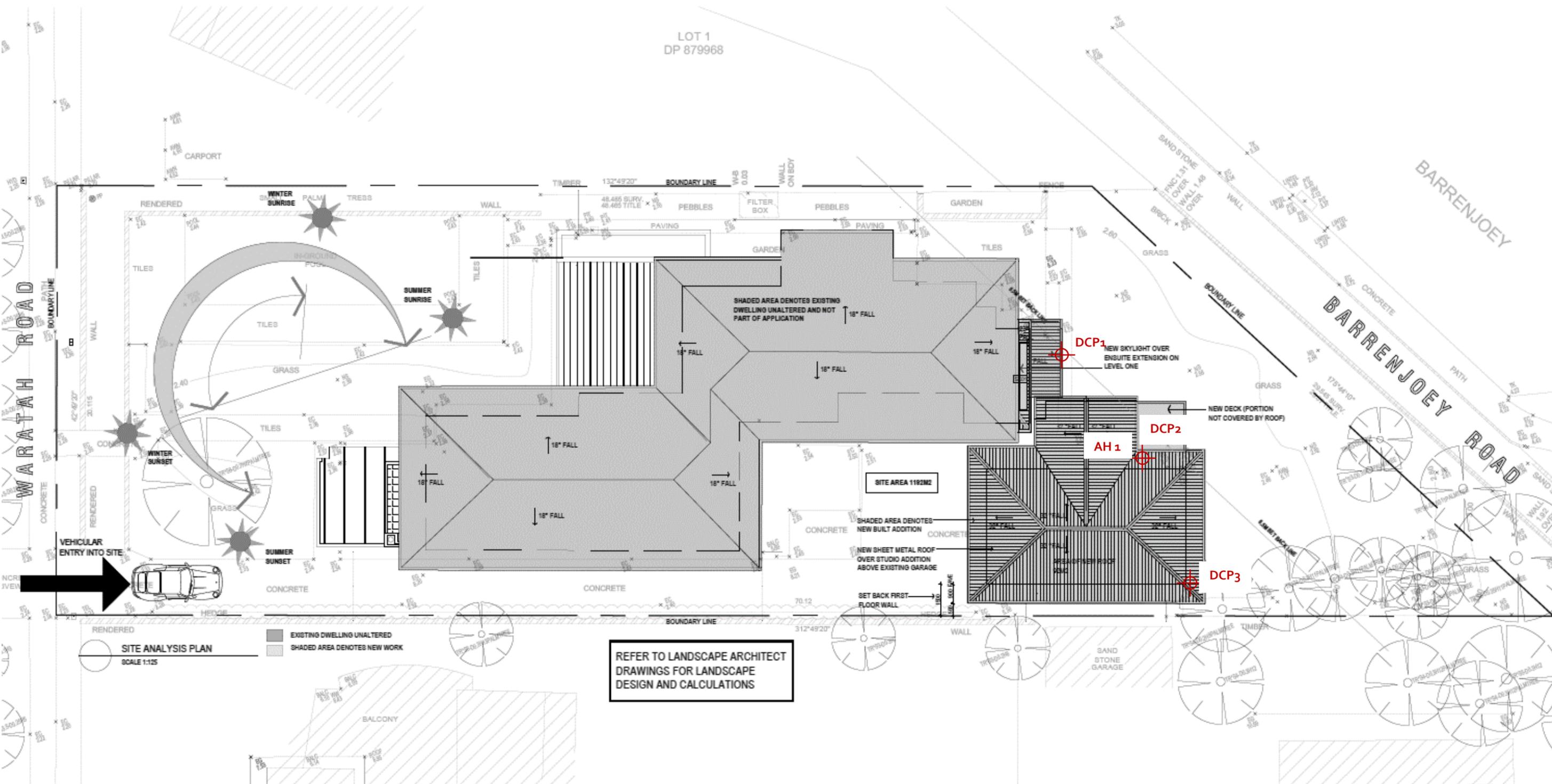
Important Information about Your Report

It should be noted that Geotechnical Reports are documents that build a picture of the subsurface conditions from the observation of surface features and testing carried out at specific points on the site. The spacing and location of the test points can be limited by the location of existing structures on the site or by budget and time constraints of the client. Additionally, the test themselves, although chosen for their suitability for the particular project, have their own limiting factors. The testing gives accurate information at the location of the test, within the confines of the test's capability. A geological interpretation or model is developed by joining these test points using all available data and drawing on previous experience of the geotechnical consultant. Even the most experienced practitioners cannot determine every possible feature or change that may lie below the earth. All of the subsurface features can only be known when they are revealed by excavation. As such, a Geotechnical report can be considered an interpretive document. It is based on factual data but also on opinion and judgement that comes with a level of uncertainty. This information is provided to help explain the nature and limitations of your report.

With this in mind, the following points are to be noted:

- If upon the commencement of the works the subsurface ground or ground water conditions prove different from those described in this report, it is advisable to contact White Geotechnical Group immediately, as problems relating to the ground works phase of construction are far easier and less costly to overcome if they are addressed early.
- If this report is used by other professionals during the design or construction process, any questions should be directed to White Geotechnical Group as only we understand the full methodology behind the report's conclusions.
- The report addresses issues relating to your specific design and site. If the proposed project design changes, aspects of the report may no longer apply. Contact White Geotechnical if this occurs.
- This report should not be applied to any other project other than that outlined in section 1.0.
- This report is to be read in full and should not have sections removed or included in other documents as this can result in misinterpretation of the data by others.
- It is common for the design and construction process to be adapted as it progresses (sometimes to suit the previous experience of the contractors involved). If alternative design and construction processes are required to those described in this report, contact White Geotechnical Group. We are familiar with a variety of techniques to reduce risk and can advise if your proposed methods are suitable for the site conditions.

SITE PLAN – showing test locations



REV.	DATE	DESCRIPTION
A	18.08.2023	ISSUED FOR DEVELOPMENT APPLICATION

CHECKED
KM

LEGEND

ABBREVIATIONS	DESCRIPTION
AC	ASBESTOS
AD	ALUMINIUM DOOR
AW	ALUMINIUM WINDOW
BR	BRICK
CB	CONCRETE BLOCK
CL	CLEAR FINISH
CM	CEMENT MORTAR
CP	CARPET
CS	CERAMIC TILE
CT	CONCRETE TILES
DC	DRY CONCRETE
DF	DRY FINISH
GL	GLASS
GR	GRASS
GS	GRAVEL
HT	HARDWOOD
IP	IRON PIPE
IS	INSULATION
LA	LANDSCAPE ARCHITECT
LD	LANDSCAPE DESIGN
LE	LEAD
LI	LIMEWASH
LP	LEAD PENCIL
LS	LEAD SURFACE
LT	LEAD TILES
LV	LEAD VENT
MA	MASONRY
MB	MASONRY BLOCK
MC	MASONRY CONCRETE
MD	MASONRY DRAIN
ME	MASONRY EXTERIOR
MF	MASONRY FINISH
MG	MASONRY GROUND
MH	MASONRY HATCH
MI	MASONRY INTERIOR
ML	MASONRY LIMEWASH
MM	MASONRY MORTAR
MO	MASONRY OIL
MP	MASONRY PAINT
MS	MASONRY SAND
MT	MASONRY TILE
MV	MASONRY VENT
NA	NATURAL
NC	NATURAL CONSTRUCTION
ND	NATURAL DRAIN
NE	NATURAL EXTERIOR
NF	NATURAL FINISH
NG	NATURAL GROUND
NH	NATURAL HATCH
NI	NATURAL INTERIOR
NL	NATURAL LIMEWASH
NM	NATURAL MORTAR
NO	NATURAL OIL
NP	NATURAL PAINT
NS	NATURAL SAND
NT	NATURAL TILE
NV	NATURAL VENT
OB	OBSCURE
OC	OBSCURE CONSTRUCTION
OD	OBSCURE DRAIN
OE	OBSCURE EXTERIOR
OF	OBSCURE FINISH
OG	OBSCURE GROUND
OH	OBSCURE HATCH
OI	OBSCURE INTERIOR
OL	OBSCURE LIMEWASH
OM	OBSCURE MORTAR
ON	OBSCURE OIL
OP	OBSCURE PAINT
OS	OBSCURE SAND
OT	OBSCURE TILE
OV	OBSCURE VENT
PA	PANORAMA
PC	PANORAMA CONSTRUCTION
PD	PANORAMA DRAIN
PE	PANORAMA EXTERIOR
PF	PANORAMA FINISH
PG	PANORAMA GROUND
PH	PANORAMA HATCH
PI	PANORAMA INTERIOR
PL	PANORAMA LIMEWASH
PM	PANORAMA MORTAR
PO	PANORAMA OIL
PP	PANORAMA PAINT
PS	PANORAMA SAND
PT	PANORAMA TILE
PV	PANORAMA VENT
RA	RAILING
RB	RAILING BLOCK
RC	RAILING CONCRETE
RD	RAILING DRAIN
RE	RAILING EXTERIOR
RF	RAILING FINISH
RG	RAILING GROUND
RH	RAILING HATCH
RI	RAILING INTERIOR
RL	RAILING LIMEWASH
RM	RAILING MORTAR
RO	RAILING OIL
RP	RAILING PAINT
RS	RAILING SAND
RT	RAILING TILE
RV	RAILING VENT
SA	SAND
SB	SAND BLOCK
SC	SAND CONCRETE
SD	SAND DRAIN
SE	SAND EXTERIOR
SF	SAND FINISH
SG	SAND GROUND
SH	SAND HATCH
SI	SAND INTERIOR
SL	SAND LIMEWASH
SM	SAND MORTAR
SO	SAND OIL
SP	SAND PAINT
SS	SAND SAND
ST	SAND TILE
SV	SAND VENT
TA	TIMBER
TB	TIMBER BLOCK
TC	TIMBER CONCRETE
TD	TIMBER DRAIN
TE	TIMBER EXTERIOR
TF	TIMBER FINISH
TG	TIMBER GROUND
TH	TIMBER HATCH
TI	TIMBER INTERIOR
TJ	TIMBER LIMEWASH
TK	TIMBER MORTAR
TL	TIMBER OIL
TM	TIMBER PAINT
TS	TIMBER SAND
TT	TIMBER TILE
TV	TIMBER VENT
UB	UNBUILT
UC	UNBUILT CONSTRUCTION
UD	UNBUILT DRAIN
UE	UNBUILT EXTERIOR
UF	UNBUILT FINISH
UG	UNBUILT GROUND
UH	UNBUILT HATCH
UI	UNBUILT INTERIOR
UL	UNBUILT LIMEWASH
UM	UNBUILT MORTAR
UO	UNBUILT OIL
UP	UNBUILT PAINT
US	UNBUILT SAND
UT	UNBUILT TILE
UV	UNBUILT VENT
VA	VERIFIED
VB	VERIFIED BLOCK
VC	VERIFIED CONCRETE
VD	VERIFIED DRAIN
VE	VERIFIED EXTERIOR
VF	VERIFIED FINISH
VG	VERIFIED GROUND
VH	VERIFIED HATCH
VI	VERIFIED INTERIOR
VJ	VERIFIED LIMEWASH
VK	VERIFIED MORTAR
VO	VERIFIED OIL
VP	VERIFIED PAINT
VS	VERIFIED SAND
VT	VERIFIED TILE
VV	VERIFIED VENT
WA	WATER
WB	WATER BLOCK
WC	WATER CONCRETE
WD	WATER DRAIN
WE	WATER EXTERIOR
WF	WATER FINISH
WG	WATER GROUND
WH	WATER HATCH
WI	WATER INTERIOR
WL	WATER LIMEWASH
WM	WATER MORTAR
WO	WATER OIL
WP	WATER PAINT
WS	WATER SAND
WT	WATER TILE
WV	WATER VENT
WY	WYLL
WZ	WYLL BLOCK
WX	WYLL CONCRETE
WY	WYLL DRAIN
WZ	WYLL EXTERIOR
XX	WYLL FINISH
XY	WYLL GROUND
YZ	WYLL HATCH
YA	WYLL INTERIOR
YB	WYLL LIMEWASH
YC	WYLL MORTAR
YD	WYLL OIL
YE	WYLL PAINT
YF	WYLL SAND
YG	WYLL TILE
YH	WYLL VENT
YI	WYLL WYLL
YJ	WYLL WYLL BLOCK
YK	WYLL WYLL CONCRETE
YL	WYLL WYLL DRAIN
YM	WYLL WYLL EXTERIOR
YN	WYLL WYLL FINISH
YO	WYLL WYLL GROUND
YP	WYLL WYLL HATCH
YQ	WYLL WYLL INTERIOR
YR	WYLL WYLL LIMEWASH
YS	WYLL WYLL MORTAR
YT	WYLL WYLL OIL
YU	WYLL WYLL PAINT
YV	WYLL WYLL SAND
YW	WYLL WYLL TILE
YX	WYLL WYLL VENT
YY	WYLL WYLL WYLL
YZ	WYLL WYLL WYLL BLOCK
ZA	ZINC
ZB	ZINC BLOCK
ZC	ZINC CONCRETE
ZD	ZINC DRAIN
ZE	ZINC EXTERIOR
ZF	ZINC FINISH
ZG	ZINC GROUND
ZH	ZINC HATCH
ZI	ZINC INTERIOR
ZJ	ZINC LIMEWASH
ZK	ZINC MORTAR
ZL	ZINC OIL
ZM	ZINC PAINT
ZN	ZINC SAND
ZO	ZINC TILE
ZP	ZINC VENT
ZQ	ZINC ZINC
ZR	ZINC ZINC BLOCK
ZS	ZINC ZINC CONCRETE
ZT	ZINC ZINC DRAIN
ZU	ZINC ZINC EXTERIOR
ZV	ZINC ZINC FINISH
ZW	ZINC ZINC GROUND
ZX	ZINC ZINC HATCH
ZY	ZINC ZINC INTERIOR
ZZ	ZINC ZINC LIMEWASH

REFER TO LANDSCAPE ARCHITECT DRAWINGS FOR LANDSCAPE DESIGN AND CALCULATIONS

two form
ARCHITECTURE • INTERIOR DESIGN

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Check and verify all dimensions on site and after any price and/or contract to be fulfilled before proceeding further. Do not make off the drawings. Drawings shall not be used for any other purpose until issued by the architect for such purpose. For registration of alterations and applications to appropriate agencies. © Copyright 1982-2023 P11-L12

CLIENT
STEPHEN AND SUSAN JONES

PROJECT
ALTERATIONS AND ADDITION TO EXISTING DWELLING
3 WARATAH ROAD, PALM BEACH LOT 15 DP 851513

DRAWING NAME
SITE ANALYSIS PLAN

DRAWING STAGE
DEVELOPMENT ASSESSMENT

DRAWING NUMBER
22 026 AR DA 01

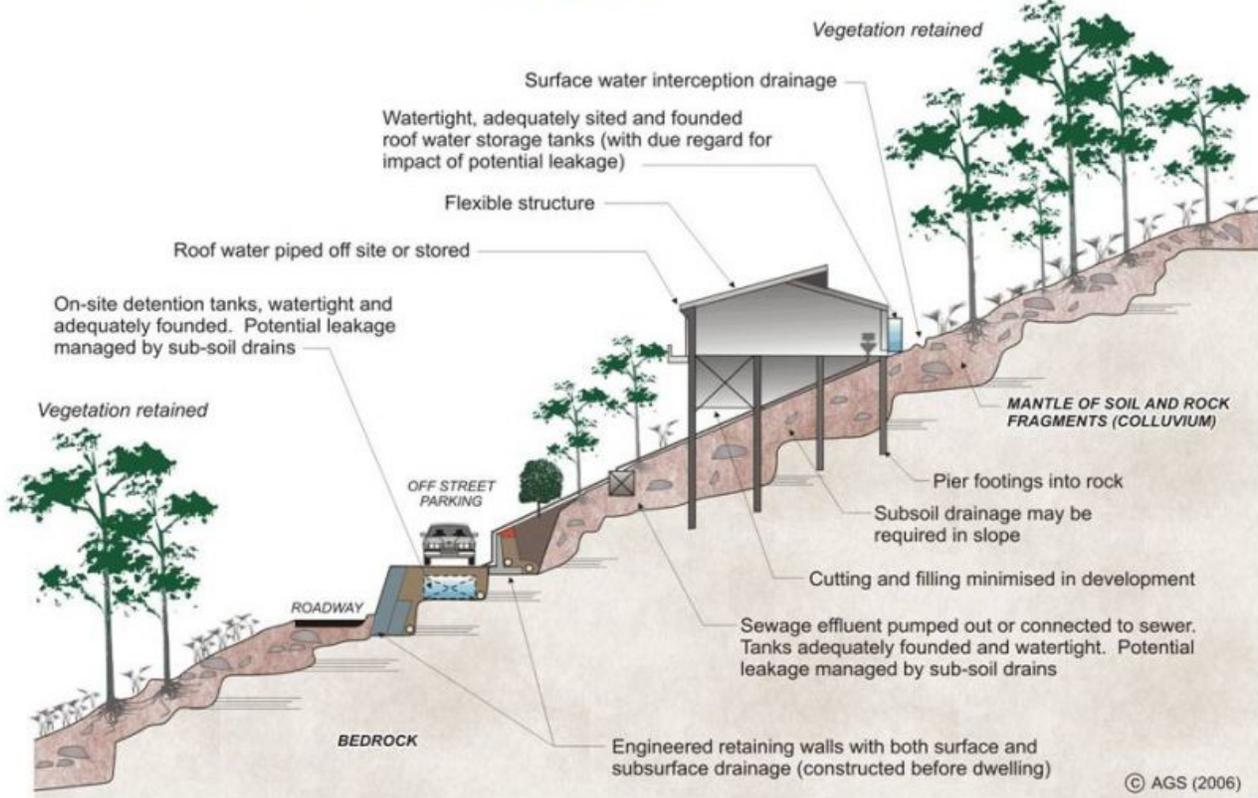


DATE
AUG 2023

SCALE
1:125 AT A2

REVISION
A

EXAMPLES OF **GOOD** HILLSIDE PRACTICE



EXAMPLES OF **POOR** HILLSIDE PRACTICE

