

SITE HYDROGEOLOGY REPORT

4 DELMAR PARADE & 812 PITTWATER ROAD, DEE WHY NSW

Prepared for:

LANDMARK GROUP AUSTRALIA PTY LTD

Reference: P3018_07

21 February 2024

Contents

1	INTE	RODUCTION	3
	1.1	Overview	3
	1.2	PROPOSED DEVELOPMENT	
	1.3	Proposed Dewatering Schedule	3
	1.4	Objectives	3
2	GEO	LOGICAL MODEL	4
	2.1	PUBLISHED GEOLOGICAL MAPPING	4
	2.2	Published Soil Landscapes	4
	2.3	SITE DESCRIPTION	
	2.4	Stratigraphic Model	6
	2.5	ACID SULFATE SOILS	9
3	HYD	ROGEOLOGICAL MODEL	9
	3.1	GROUNDWATER OBSERVATIONS	9
	3.2	Hydraulic Conductivity Testing	. 14
	3.3	LOCAL GROUNDWATER REGIME	
	3.4	WATER QUALITY OBSERVATIONS	15
4	STA	TEMENT OF LIMITATIONS	. 16
5	CLO	SURE	. 17

1 INTRODUCTION

1.1 Overview

Morrow Geotechnics Pty Ltd has carried out a Groundwater Study and prepared a Site Hydrogeology Report for the proposed development at 4 Delmar Parade & 812 Pittwater Road, Dee Why NSW (the site) also known as SP 32071 & SP 32072.

The following geotechnical reports have been prepared for the site:

- AssettGeoEnviro, Proposed Mixed-use Development, 4 Delmar Parade & 812 Pittwater Road, Dee Why, NSW; ref 6561-G1 and dated 25 November 2021 (AGE 2021);
- Reditus Consulting Pty Ltd, Preliminary Site Investigation, 4 Delmar Parade & 812 Pittwater Road, Dee Why; project number 21181, version 2 and dated 1 December 2021 (RC 2021);
- Geosyntec Consultants Pty Ltd, Detailed Site Investigation, 4 Delmar Parade & 812 Pittwater Road, Dee Why; project number 21325 and dated 4 May 2022 (GC 2022); and
- Reditus Consulting Pty Ltd, *Dewatering Management Plan, 4 Delmar Parade & 812 Pittwater Road, Dee Why;* report number 21181RP01, version 1 and dated 7 June 2022 (RC 2022).
- Morrow Geotechnics Pty Ltd, Geotechnical Investigation Report, 4 Delmar Parade & 812
 Pittwater Road, Dee Why NSW, referenced P3018_01 rev1, and dated 24 November 2023 (MG 2023).

The previous geotechnical reports present the results of a site investigation for the proposed development and geotechnical recommendations for design and construction.

1.2 Proposed Development

Architectural Drawings for the proposed development have been prepared by Rothe Lowman Property Pty Ltd, project number 221054. Based on the drawings provided, Morrow Geotechnics understands that the proposed development involves the construction of a multi-storey apartment building with up to 7 levels above ground and a three-level basement carpark. Morrow understands the development involves excavation to a maximum depth of approximately 13.95 metres below ground level (mBGL).

1.3 Proposed Dewatering Schedule

Given the relatively low permeability of the sandstone profile encountered at depth within the geotechnical investigation it is proposed to construct a secant pile cut-off wall socketed a minimum of 1000 mm into Class III Sandstone to minimise groundwater seepage. Dewatering through spear points and pumping is not proposed for the construction period, rather the minor groundwater seepage around the cut-off wall will be collected by sump pits within the basement.

The excavation program for the proposed basement is expected to take up to 6 months. Temporary construction dewatering is expected to occur for construction seepage inflows during this 6 month period.

1.4 Objectives

The objective of this Site Hydrogeology Report is to provide results on the presence of water at the site and comment on whether or not there is an aquifer present at the site in accordance with the definition of aquifer as laid out in the NSW DPI Office of Water *Aquifer Interference Policy*. Section 1.2 of the policy defines an aquifer as

"the term 'aquifer' is commonly understood to mean a groundwater system that is sufficiently permeable to allow water to move within it, and which can yield productive volumes of groundwater"

Further, this report provides analysis of the permeability of soils encountered within boreholes at the site and geotechnical advice and recommendations on the management of groundwater in the design and construction of the proposed development.

2 GEOLOGICAL MODEL

2.1 Published Geological Mapping

The Department of Mineral Resources Geological Map Sydney 1:100,000 Geological Series Sydney (DMR 1983) indicates the site to be underlain by Hawkesbury Sandstone, which comprises medium to coarse grained quartz sandstone, very minor shale and laminite lenses.

2.2 Published Soil Landscapes

The Soil Conservation Service of NSW Sydney 1:100,000 Soil Landscapes Series Sheet 9130 indicates that the residual landscape at the site is located on the boundary of the Gymea and Newport Landscapes.

The Gymea landscape type typically includes undulating to rolling rises and low hills on Hawkesbury Sandstone. Soils are generally shallow to moderately deep (0.3 - 1 m) yellow earths and earthy sands. These soils are noted to present localized steep slopes, high soil erosion hazard, rock outcrop and shallow highly permeable soil.

The Newport landscape type typically includes gently undulating plains of Holocene sands to rolling rises over other soils or bedrock. Soils are generally shallow (< 0.5 m) siliceous sands overlaying moderately deep buried sands (< 1.5m) yellow podzolic soil with sandy topsoil on crests and deep (> 2.0m) podzols in depressions earthy sands. These soils are noted present high soil erosion hazards, localized steep slopes, very low soil fertility and non-cohesive topsoil.

2.3 Site Description

At the time of the Geotechnical and Hydrogeological Investigations the site comprises an L-shaped block with a total area of approximately 7,800m². The site is bounded to the north by Delmar Parade, to the east by residential dwellings, to the south by Stony Range Regional Botanic Garden, to the west by Pittwater Road and to the north-west by a multi storey residential development. The site dips down towards the northwest with site RLs ranging from RL 37.17 mAHD in the southeast corner of the site to RL 27.85 mAHD in the north corner of the site adjacent to Delmar Parade.

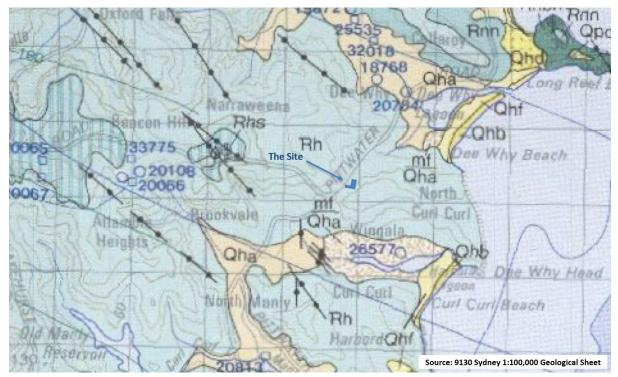


Figure 1: Regional Geology taken from Sydney 1:100,000 Geological Sheet

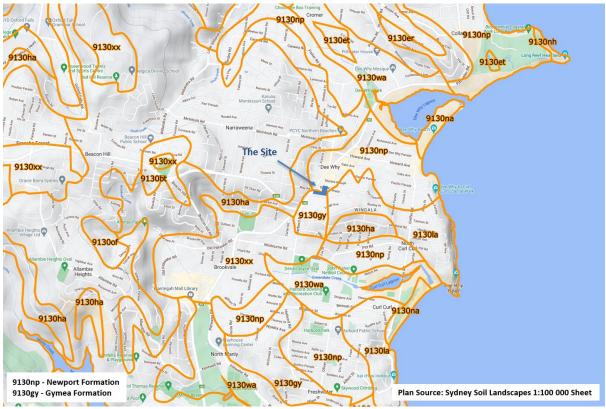


Figure 2: Soil Landscapes taken from Sydney 1:100,000 Soil Landscape Sheet

2.4 Stratigraphic Model

During the MG2023 investigation BH101, BH102, BH103 and BH105 were drilled by a track mounted drill rig using solid flight augers equipped with a tungsten-carbide bit (TC bit). The boreholes were extended beyond TC bit refusal by NMLC coring techniques to depths of between 12.99 m and 15.00 m below ground level (mBGL). BH104 was drilled using a tight access man portable drilling rig using solid flight augers equipped with a tungsten-carbide bit, it was extended beyond TC bit refusal using NMLC coring techniques to a depth of 22.22 mBGL.

Borehole locations are shown on Figure 3 below:



Figure 3: Borehole location plan

The stratigraphy at the site is characterized by topsoil and fill overlying alluvial sands, residual clay, sandstone and shale bedrock.

A summary of the subsurface conditions across the site, interpreted from the investigation results, is presented in **Table 1** and **Table 2**. Borehole Locations are shown on the plan attached to this report.

TABLE 1 SUMMARY OF INFERRED SUBSURFACE CONDITIONS

Unit	Material	Comments
1	Fill	Generally a concrete slab overlying gravely SAND, loose and moist.
2	Alluvial Soil	Alluvial SAND and Sandy CLAY, loose to dense and stiff to very stiff, low to medium plasticity, fine to medium grained with medium sized iron stone and quartz gravels.
3	Residual Soil	Residual sandy to gravelly CLAY, stiff to very stiff grading to hard, medium plasticity, fine to medium grained with medium sized iron stone and sandstone gravels and extremely weathered sandstone bands.
4	Class V and IV Sandstone	Extremely to highly weathered SANDSTONE, very low to low strength, medium grained. Defects within unit 4 are generally horizontally orientated bedding partings with infrequent joints inclined at 10 to 60°.
5	Class III Sandstone	Moderate to slightly weathered SANDSTONE, medium to high strength, medium grained. Defects within unit 5 are generally horizontally orientated bedding partings with infrequent joints inclined at 10 to 60°.
6	Class V Shale	SHALE, SILTSTONE and LAMINITE, extremely to moderately weathered, very low to low strength. Defects within Unit 6 are generally horizontally oriented bedding partings, joints inclined to 45° and clay seams.
7	Class III Shale	SHALE, moderately to slightly weathered, medium to high strength. Defects within Unit 7 are generally horizontally oriented bedding partings, joints inclined to 45°. Defect Spacing >200mm.

TABLE 2 ENCOUNTERED GEOTECHNICAL CONDITIONS

		Appr	ox. Depth Ra	ange of Unit ¹	mBGL (RL mA	MD)	
	Unit 1	Unit 2	Unit 3	Unit 4	Unit 5	Unit 6	Unit 7
	Fill	Alluvial Soil	Residual Soil	Class V/IV Sandstone	Class III Sandstone	Class V Shale	Class III Shale
BH1/CPT1*	0.0 to 1.0	1.0 to 3.5	3.5 to 6.0	6.0 to 8.93	8.93 to 13.9+	-	-
(AGE21)	(28.9 to 27.9)	(27.9 to 25.4)	(25.4 to 23.9)	(23.9 to 20.97)	(20.97 to 4.97+)	-	-
BH2*	0.0 to 1.6	-	1.6 to 3.7	3.7 to 5.63 & 6.6 to 8.4	8.4 to 13.95+	5.63 to 6.6	-
(AGE21)	(29.25 to 27.65)	-	(27.65 to 25.55)	(25.55 to 23.62 & 22.65 to 20.85)	(20.85 to 15.3+)	(23.62 to 22.65)	-
ВН3/СРТ4*	0.0 to 0.5	-	-	0.5 to 6.36 & 7.09 to 7.7	7.7 to 14.2+	-	6.36 to 7.09
(AGE21)	(30.0 to 29.5)	-	-	(29.5 to 23.64 & 22.91 to 22.3)	(22.3 to 15.8+)	-	(23.64 to 22.91)
BH4*	0.0 to 0.5	-	-	0.5 to 6.0 & 9.12 to 14.55+	6.0 to 7.46	7.46 to 9.12	-
(AGE21)	(31.92 to 31.42)	-	-	(31.42 to 25.92 & 22.8 to 17.37+)	(25.92 to 18.46)	(18.46 to 22.8)	-
BH5*	0.0 to 0.2	-	-	0.2 to 2.95	2.95 to 8.61 & 9.81 to 15.0+	8.61 to 9.81	-
(AGE21)	(32.66 to 32.44)	-	-	(32.44 to 29.71)	(29.71 to 24.05 & 22.85 to 17.66+)	(24.05 to 22.85)	-
ВН6/СРТ5*	0.0 to 0.2	0.2 to 3.2	-	3.2 to 9.15 & 10.1 to 11.06	11.06 to 14.0+	9.15 to 10.1	-
(AGE21)	(32.08 31.88)	(31.88 to 28.88)	-	(28.88 to 22.93 & 21.98 to 21.02)	(21.02 to 18.08+)	(22.93 to 21.98)	-
BH7/CPT2*	0.0 to 0.9	0.9 to 2.9	2.9 to 9.0	9.0 to 12.78	-	12.78 to 15.06+	-
(AGE21)	(31.8 to 29.9)	(29.9 to 28.9)	(28.9 to 22.8)	(22.8 to 19.02) -		(19.02 to 16.74+)	-
BH8/CPT3*	0.0 to 2.3	2.3 to 5.2	5.2 to 15.0	15.0 to 16.0+	-	-	-
(AGE21)	(30.51 to 28.21	(28.21 to 25.31)	(25.31 to 15.51)	(15.51 to 14.51+)		-	-
BH101	0.0 to 0.3	0.3 to 6.0	-	-	6.0 to 13.0+	-	-
PHIOI	(28.8 to 28.5)	(28.5 to 22.8)	-	-	(22.8 to 15.8+)	-	-
	0.0 to 1.3	1.3 to 2.21	2.21 to 2.8	2.8 to 4.8	4.8 to 8.06 & 10.0 to 12.99+	8.06 to 9.0	9.0 to 10.0
BH102	(32.1 to 30.8)	(30.8 to 29.89)	(29.89 to 29.3)	(29.3 to 27.3)	(29.3 to 24.04 & 22.1 to 19.11+)	(24.04 to 23.1)	(23.1 to 22.1)
	0.0 to 0.7	0.7 to 7.1	-	-	7.1 to 13.09	13.09 to 14.0	14.0 to 15.0+
BH103	(31.5 to 30.8)	(30.8 to 24.4)	-	-	(24.4 to 18.41)	(24.4 to 17.5)	(17.5 to 16.5+)
_	0.0 to 0.5	0.5 to 1.6+	-	-	-	-	-
BH104	(31.8 to 46.8)	(46.8 to 46.1+)	-	-	-	-	-
DU405	0.0 to 0.4	0.4 to 5.0	5.0 to 7.6	7.6 to 11.15	11.15 to 22.22+	-	-
BH105	(28.5 to 28.1)	(28.1 to 23.5)	(23.5 to 20.9)	(20.9 to 17.35)	(17.35 to 6.28+)	-	-

Notes:

1 Depths shown are based on material observed within test locations and will vary across the site\

2.5 Acid Sulfate Soils

According to the Warringah Local Environmental Plan 2011 mapping the site is not located within a known area of Acid Sulfate soils.

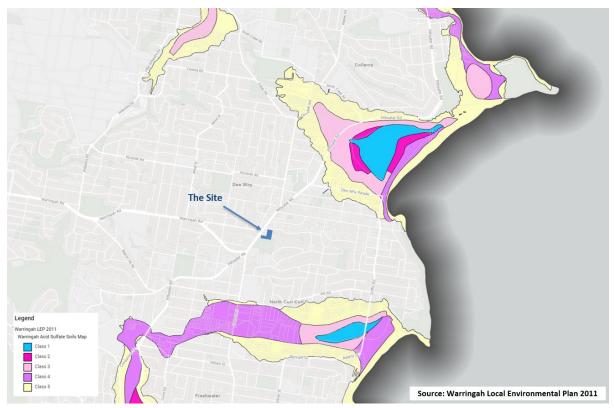


Figure 4: Acid Sulfate Soils Map with site location

The geological profile at the site comprises residual soil over shale and sandstone bedrock. The soils encountered are derived from weathering of the bedrock. No recent alluvial soils were encountered during the investigation. Acid Sulfate Soils are generally linked with alluvial and marine sediments of the Holocene era (<10,000 years ago). The soils encountered on site comprise the Wianamatta Group which ages to the Middle Triassic era (between 247.2 and 237 million years ago). The soils encountered at the site are not consistent in age or origin to produce Acid Sulfate Soils.

3 HYDROGEOLOGICAL MODEL

3.1 Groundwater Observations

Standpipe piezometer wells were installed within four boreholes drilled as part of the MG 2023 geotechnical investigations (BH101, BH102, BH103 and BH104). Monitoring well construction details are found in **Table 3** below. Before the installation of the piezometers, drill cuttings and water in the boreholes was flushed out. The monitoring wells were constructed using 50 mm diameter screw threaded PVC casing, sections of which were machine slotted. The annulus between the casing and boreholes was backfilled using 2 mm filter gravel pack to above the top of the screen. A bentonite plug with a minimum thickness of 0.5 m was then installed above the gravel pack, the remaining annulus was backfilled with drill cuttings. The wells were each finished with a cement plug and a gatic cover.

Groundwater levels within the three piezometer wells have been monitored by Morrow Geotechnics between 29 August 2023 and 25 October 2023.

TABLE 3 PIEZOMETER DETAILS

Piezometer	BH101	BH102	BH103	BH104		
Top of Piezometer						
approx.	28.8	32.1	31.5	31.8		
RL (mAHD)						
Piezometer Depth	13	12.99	15	20.91		
mBGL	15	12.99	15	20.91		
Bentonite Plug Depth mBGL	5.0 to 5.5	2.2 to 2.7	2.5 to 3.25	3.9 to 4.50		
Screen Depth mBGL	5.5 to 13.0	3.0 to 12.99	3.0 to 15.0	5.9 to 20.91		
Well Development Date	29/8/2023	29/8/2023	29/8/2023	1/11/2023		

TABLE 4 WATER LEVELS FROM MANUAL READINGS

Monitoring Date	Piezometer Groundwater Level (RL mAHD)						
	BH101	BH102	BH103	BH104			
25/10/2023	23.39	28.15	27.69	-			
8/11/2023	23.30	27.00	27.65	26.1			

Automatic dataloggers were installed within BH101, BH102, BH103 and BH104 in order to provide long term groundwater measurements. Dataloggers were installed within the piezometers on 29 August 2023, the loggers were set to measure groundwater levels at one hour intervals. Groundwater measurements were recorded between 29 August 2023 and 4 December 2023. Graphs of water level measurements taken by the automatic dataloggers are displayed in **Figure 5** to **Figure 9** below. Daily rainfall totals taken from Bureau of Meteorology Collaroy Long Reef Golf Club {station 66126} for the monitoring period are displayed alongside water level measurements.

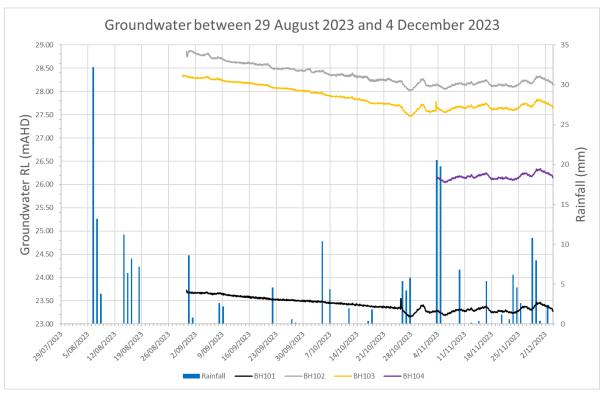


Figure 5: Data Logger Measurements and Daily Rainfall

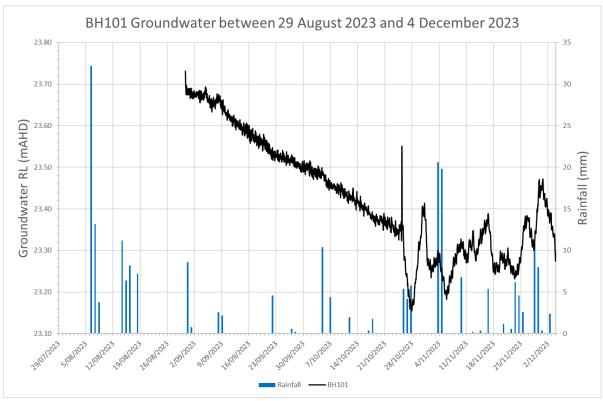


Figure 6: BH101 Data Logger Measurements and Daily Rainfall

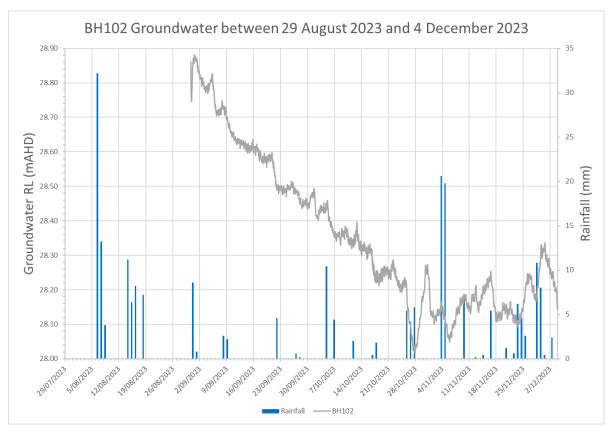


Figure 7: BH102 Data Logger Measurements and Daily Rainfall



Figure 8: BH103 Data Logger Measurements and Daily Rainfall

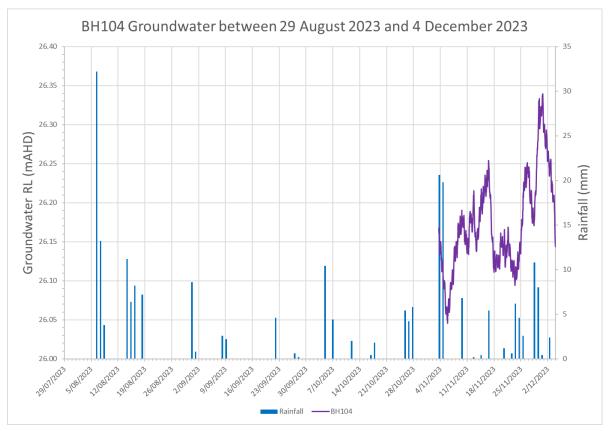


Figure 9: BH104 Data Logger Measurements and Daily Rainfall

A summary of the groundwater observations within the three wells is provided below:

TABLE 5 GROUNDWATER OBSERVATION SUMMARY

	Pi	D)		
Monitoring Date	BH101	BH102	BH103	BH104
Maximum Water Level	23.73	28.88	28.35	26.34
Minimum Water Level	23.15	28.02	27.46	26.04
Average Water Level	23.43	28.36	27.97	26.17
Median Water Level	23.41	28.30	27.99	26.16

Measured groundwater levels at the three piezometers are within the following rock strata:

BH101: Unit 2 – Alluvial Soil;

- BH102: Unit 4 Class V/IV Sandstone;
- BH103: Unit 2 Alluvial Soil; and
- BH104: Unit 5 Class III Sandstone

Groundwater depths observed within BH102 and BH103 were comparable, with average water level RLs of 28.50 and 28.03 mAHD respectively at an average depth of 3.9 mBGL for BH102 and BH103. The average water level within BH101 was measured lower at RL 23.51 mAHD. Groundwater observations within the four wells indicate a phreatic water table which is dipping generally in line with surface topography.

Groundwater within BH104 was observed at a deeper relative depth (i.e. 5.6 mBGL). BH104 water level was within the Class III Sandstone and is inferred to represent a water table which is isolated from the water table within the Alluvial Soil profile.

The monitoring within the boreholes indicated low responsiveness to rainfall events during the three-month monitoring period. This is inferred to be a result of the paved areas in the vicinity of the wells limiting surface water infiltration following rainfall.

On the basis of ongoing groundwater monitoring at the site it is recommended that a design groundwater level for the stable water table within the bedrock is taken at 1.0 m above the consistent water level observed within the boreholes, i.e (2.9 mBGL dipping with topography to the north-west of the site).

3.2 Hydraulic Conductivity Testing

Rising head permeability tests were carried out on BH101, BH102 and BH103 to give an indication of in-situ permeability of the material at the site. Permeability values were calculated on the basis of the testing in accordance with the formulas provided in British Standard BS5930 -1999 Code of Practice for Site Investigations, Section 21.4.6. Calculation sheets for permeability testing are provided as an attachment to this letter as **Appendix C**. Permeability tests were repeated three times to ensure reliability of results.

Permeability values which were adopted from the testing for the assessment of groundwater seepage volumes are shown in **Table 6** below.

TABLE 6 BOREHOLE PERMEABILITY VALUES

	Permeability Based or	ı In-situ Measurement				
	(m/s) (m/day)					
BH101	7.23 x 10 ⁻⁸	6.24 x 10 ⁻³				
BH102	4.57 x 10 ⁻⁸	3.94 x 10 ⁻³				
BH103	1.42 x 10 ⁻⁴	12.27				

Borehole permeability results varied across the test locations by approximately 4 orders of magnitude.

As indicated above, measured groundwater levels at the three piezometers are within the following rock strata:

- BH101: Unit 2 Alluvial Soil;
- BH102: Unit 4 Class V/IV Sandstone; and
- BH103: Unit 2 Alluvial Soil.

BH101 and BH102 results may be taken as indicative of south-eastern half of the site. The south-eastern portion of the site corresponds to soil landscape/geological mapping for the Gymea Landscape which is typical of residual soil slopes over Hawkesbury Sandstone. These soils contain a higher clay content and were observed to be of lower permeability.

BH103 results may be taken as indicative of the generalised permeability of Alluvial soils through the northwestern portion of the site, mapped as Newport Landscape soils. The alluvial material contained bands of gravelly sand, and permeabilities up to 1.4×10^{-4} m/s are consistent with the soil profile encountered in BH103.

The difference in measured permeabilities across the site footprint can be accounted for in the mapped geological conditions and in the borehole observations.

3.3 Local Groundwater Regime

Based on the water level monitoring results, it is inferred that there is a phreatic water table within alluvial soils and fractured rock at the site. It is recommended that a design groundwater level for the stable water table within the bedrock is taken at 1.0 m above the consistent water level observed within the boreholes, i.e (2.9 mBGL dipping with topography to the north-west of the site).

On the basis of permeability testing at the site it is recommended that the following design permeability values are used for modelling of the sandstone and residual soil strata:

- Alluvial Soils 1.4 x 10⁻⁴ m/s
- Bedrock 7.0 x 10⁻⁸ m/s

Generalising BH103 permeability to the entirety of the Alluvial profile may be taken as a suitably conservative, worst case model for sensitivity analysis.

3.4 Water Quality Observations

A sample of the water to be discharged was taken on 25 October 2023 and sent to a NATA accredited laboratory for testing against the Australia and New Zealand Environment Conservation Council ANZECC (2000) guidelines for 95% protection of marine ecosystems (in the absence of guidelines the criteria for fresh waters was used) and National Environment Protection (Assessment of Site Contamination) Measure (NEPM, 2013). Relevant water quality results are presented in **Table 7**, lab results are attached in **Appendix B**.

The groundwater samples were taken using an electric powered pump with a length of ¼ inch low-density polyethylene (LDPE) tubing. The groundwater sample was collected from the outflow tube and stored in containers proved by SGS laboratory. The containers were immediately placed in an esky with ice packs to maintain a cool temperature and delivered to SGS laboratory same day. Disposable nitrile gloves were used for sample collection to minimize potential contamination.

TABLE 7 WATER QUALITY RESULTS

Analytes	Measured Co	ncentration in V (μg/L)	Fresh Water Threshold	Marine Water Threshold			
	BH01	BH102	BH103	(μg/L)	(μg/L)		
рН	5.1	5.2	4.7				
Electrical Conductivity (µS/cm)	350	410	390				
Chloride (mg/L)	74	89	97				
Sulfate, SO4 (mg/L)	40	40	29				
Benzene	<0.5	<0.5	<0.5	950	500		
Toluene	2.2	<0.5	<0.5	-	-		
Ethylbenzene	<0.5	<0.5	<0.5	-			
O-Xylene	<0.5	<0.5	<0.5	350	-		
M/P-Xylene	<1	<1	<1	200	-		
Arsenic	<1	<1	<1	24	-		
Cadmium	<0.1	<0.1	<0.1	0.2	0.7		
Chromium	<1	2	<1	1	4.4		
Copper	3	<1	61	1.4	1.3		
Lead	<1	<1	<1	3.4	4.4		
Mercury (inorganic) (mg/L)	<0.0001	<0.0001	<0.0001	0.06	0.1		
Nickel	5	18	2	11	7		
Zinc	19	25	12	8	15		
Total PAH's (18)	<1	<1	1				
Naphthalene	0.2	0.3	0.4	16	50		
B(a)P	<0.1	<0.1	<0.1	-			

Concentrations of Chromium, Copper, Nickel and Zinc are above the ANZECC (2000) and NEPM (2013) freshwater guidelines for the water sample tested at the site.

4 STATEMENT OF LIMITATIONS

The advice and parameters presented in this Groundwater Management Plan are for assessment of the expected groundwater seepage based upon the proposed development and encountered site conditions at the investigation locations.

We draw your attention to the document "Important Information", which is attached to this letter. The statements presented in this document are intended to advise you of what your realistic expectations of this report should be. The document is not intended to reduce the level of responsibility accepted by Morrow Geotechnics, but rather to ensure that all parties who may rely on this report are aware of the responsibilities each assumes in so doing.

Should you have any queries regarding this report, please do not hesitate to contact the undersigned.

5 CLOSURE

Please do not hesitate to contact the undersigned should you have any questions.

For and on behalf of Morrow Geotechnics Pty Ltd,

Andrew Butel

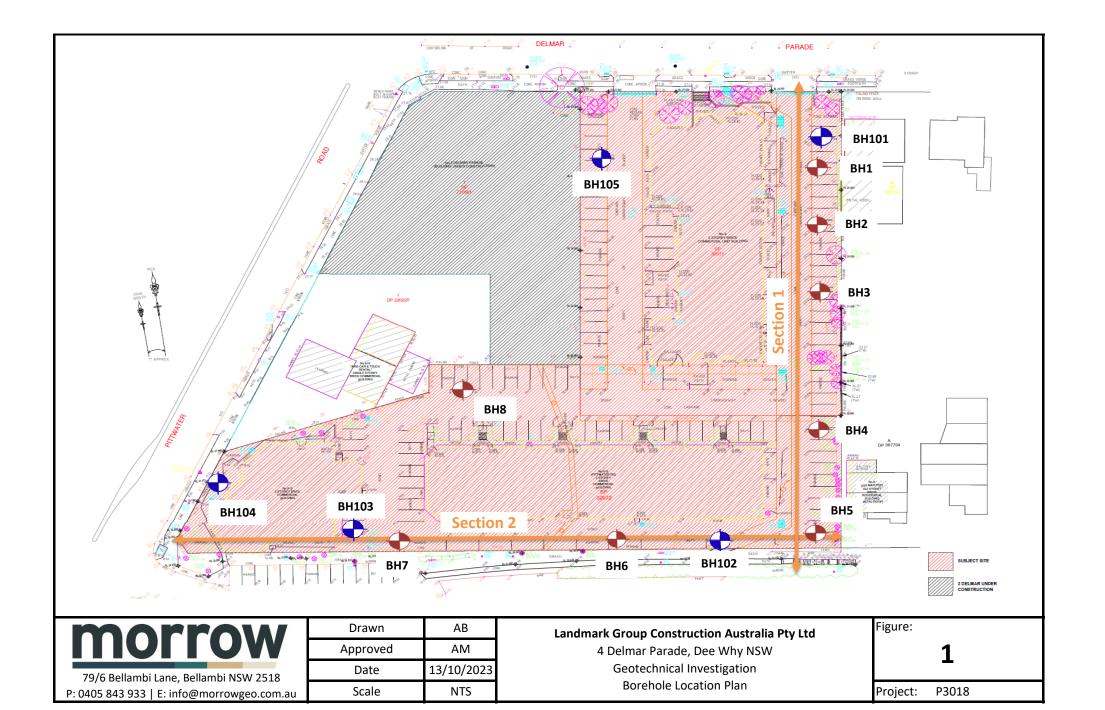
Hydrogeologist/Engineering Geologist

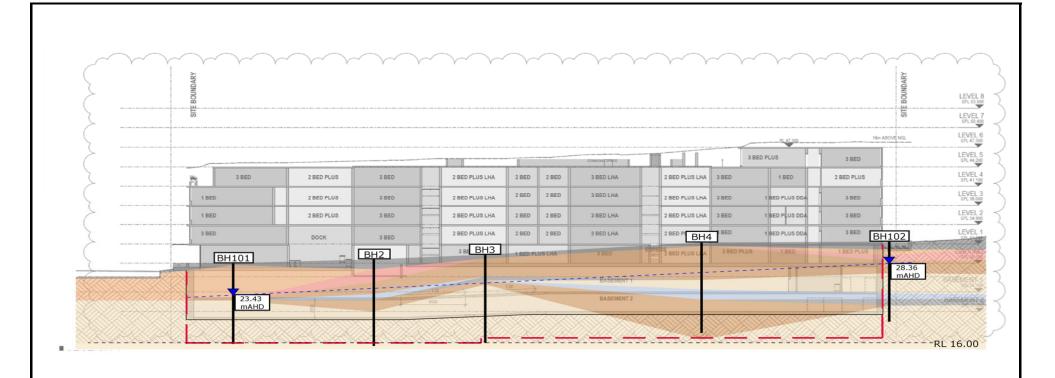
BSc (Geology), GradCertEngSc, MAIG

Alan Morrow

Principal Geotechnical Engineer

BE (Civil) BSc MIEAust CPEng NER





Unit 1 - Fill

Unit 2 - Alluvial

Unit 3 - Residual

Unit 4 - Class V/IV Sandstone

Unit 5 - Class III Sandstone

Unit 6 - Class V Shale

Unit 7 - Class III Shale

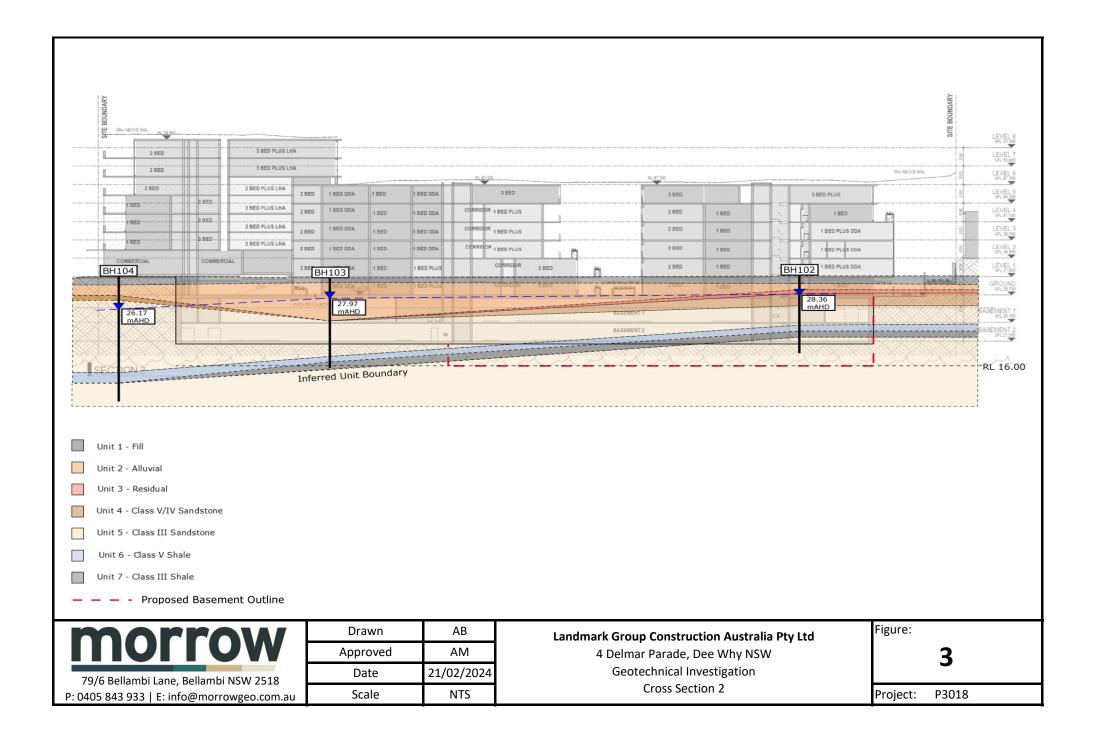
– – Proposed Basement Outline

mor		O	W
79/6 Bellambi Lane	e, Bella	mbi NS	W 2518
P: 0405 843 933 E: ir	nfo@m	norrowg	geo.com.au

Drawn	AB
Approved	AM
Date	21/02/2024
Scale	NTS

Landmark Group Construction Australia Pty Ltd
4 Delmar Parade, Dee Why NSW
Geotechnical Investigation
Cross Section 1

Figure:	2	
Project:	P3018	



BOREHOLE LOGS, EXPLANATORY NOTES AND SITE PHOTOS

Morrow Geotechnics

Bellambi, NSW Boring No.: BH101

Phone: 0405 843 933

Easting : 341051.1 Drill Supplier : MATRIX DRILLING Job Number : P3018 Sheet : 1 OF 2

Northing Elevation Total Dept	: 3410 : 6263 : 28.8 h : 13 m	534.7 (m)	Drill Supplier Driller Compai Logged By Date	ny	: MATRIX : MATRIX : Mark Pe	DRILL each			Client : Landmark Group Construction Australia Pty Ltd Project : Dee Why Location : 4 Delmar Parade, Dee Why NSW	Shee	τ :	1 OF 2
Drilling Method Water	Well Diagram	Testing	3	Soil Origin	Graphic Log	Classification Code	Depth (m)	Elevation (m)	Material	consistency/Density	Moisture	Observations
Datube			_	Residual Filhon-Soil		SC SW	0 <u>.15</u> - <u>0.3</u>		Concrete Fill Clayey to gravelly SAND (SC): low plasticity clay, loose, brown grey, fine to medium grained, medium to coarse sized gravel, moist, (low resistance). Residual Gravelly SAND (SW): loose, orange, fine to medium grained, medium to coarse sized gravel, trace low plasticity clay, moist, (low resistance, sandstone gravels).	L	М	
				ual		sc	- 1 - <u>1.4</u>	27.8	Pecidual Clavou to gravally SAND (SC), medium dense, law placticity day, law	MD		
		6, 8, 12, (N	= 20)	Residua			- 2	26.8	Residual Clayey to gravelly SAND (SC): medium dense, low plasticity clay, low plasticity, orange, fine to medium grained, medium sized gravel, moist, (low resistance, sandstone and quartz gravels).			
ADT	A A STATE OF THE S						- - -3	25.8				
		8, 12, 11, (N	= 23)				-	-				
		7, 6, 9, (N ·	= 15)				4 - -	- 24.8				
				Residual Residual		sc	<u>4.9</u> — 5	23.8	As above, but red orange.			
				Residual		CI	-	<u>-</u>	Residual Sandy CLAY (CI): very stiff, medium plasticity, grey red, medium grained sand, with fine sized gravel, w \approx pl, (low resistance, sandstone gravels).	VSt	w≈ PL	

Morrow Geotechnics

Bellambi, NSW Boring No.: BH101

Phone: 0405 843 933

Easting : 341051.1 Drill Supplier : MATRIX DRILLING Job Number : P3018 Sheet : 2 OF 2

Northing : 6263534.7 Driller Company : MATRIX DRILLING Client : Landmark Group Construction Australia Pty Ltd

Elev	hing ation	: 62635 : 28.8(r	m)	Driller Company Logged By	: MATRIX	each	LING		Client : Landmark Group Construction Australia Pty Ltd Project : Dee Why			
Iota	l Depti	13 m	Testing	Date	: 30/08/2	T		Τ	Location : 4 Delmar Parade, Dee Why NSW			
Drilling Method	Water	Well Diagram	SPT	Soil Origin	Graphic Log	Classification Code	Depth (m)	Elevation (m)	Material Description	Sonsistency/Density	Moisture	Observations
ADT			22, (N = 44	۲) §		SST		22.8	Extremelyweathered, rock Sandy CLAY (SST) : hard, low plasticity, light grey, medium grained sand, trace fine to medium sized gravel, w ≈ pl, (low to medium resistance).	Н		
							_	-	6.17m : Commenced NMLC Coring;			
							-	-				
							— 7	21.8				
							_	_				
							-	-				
							-	L				
							- 8	20.8				
							_	-				
							_	-				
							_9	19.8				
							-					
							-	-				
							-					
							— 10	18.8				
							_	-				
							_	-				
							<u> </u>	17.8				
							· · · · · · · · · · · · · · · · · · ·	F				
							-					
							-					

Morrow Geotechnics

Bellambi, NSW Boring No.: BH101

Phone: 0405 843 933

Easting : 34105.1 Drill Supplier : MATRIX DRILLING Job Number : P3018 Sheet : 2 OF 3
Northing : 6263534.7 Driller Company : MATRIX DRILLING Client : Landmark Group Construction Australia Pty Ltd

Northing Elevation Total De	n	: 6263534 : 28.8(m) : 13 m	.7		ler Company ged By	: MATF : Mark : 30/08		NG			Client : Landmark Group Construction Au Project : Dee Why Location : 4 Delmar Parade, Dee Why NSW	ustralia Pty Ltd		
Drilling Method	Water	Well Diagram	RQD% and TCR%	Testing (05) si	U.S. Estimated H.S. Strength	EHS Weathering	Depth (m)	- Elevation (m)	Graphic Log	Classification Code	Material Description	30 Defect Spacing 300 (mm)	Defect Depth	Defect Description type, inclination, planarity roughness, coating, thickness
			ROD = 26% TCR = 100%			sw	- - -6	22.8	1000	SST	rock SANDSTONE: slightly weathered, medium strength, grey, fine to medium grained, (massive, subvertical joints).		- - - 6	—6.24, J, defect bound: 6.20m to 6.28m, 80°, STP, CL, C
			RQD = 52% TCR = 100%	d: 0.34, a: 0.38			- - 7	21.8					- - 7 -	6.3.5, J, defect bounds 6.3.4m to 6.37m, 80°, STP, CL, C -6.8, J, defect bounds 6.65m to 6.93m, 80°, RO, STP, CL, OP -77.1, IR, CL, OP -7.34, P, clay coating, 6°, RO, CV, CT, OP 7.38, P, clay coating, PL, CT, OP 7.46, P, clay coating,
NMLC Coring			ROD = 43% TCR = 87%	d: 0.51, a:0.55			8	20.8					- - 8 -	1 7.46, P. clay coating 6°, RO, PL, CT, OP 7.64, J., defect bound 7.59m to 7.68m, 70°, IR, CL, C 7.87, P. 5°, RO, PL, C 7.87, P. 5°, RO, PL, C 8.19-8.39, XWS, infiling to the clay and extremely weathered sandstone 3°, IR, CT,
				d: 0.46, a: 0.76			- 9 -	19.8					- - - -	9,33, P, clay coating, 4°, PL, CT, OP 9,42, P, 5°, , STP, CL, OP 9,5-9,5-1, IS, infilled clay, 3°, RO, UN, CL, C 9,66, P, 4°, RO, PL, C 9,76, P, 3°, , PL, CL, C 9,89, P, 4°, IR, CL, C 9,99, P, 3°, NO, PL, CL, C

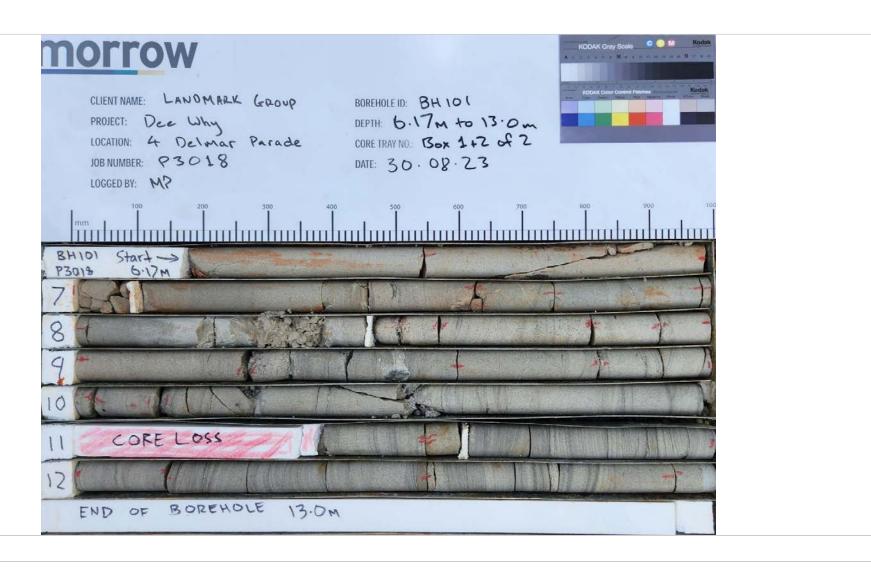
Morrow Geotechnics

Bellambi, NSW Boring No.: BH101

Phone: 0405 843 933

Easting : 341051.1 Drill Supplier : MATRIX DRILLING Job Number : P3018 Sheet : 3 OF 3

Northing Elevatio Total De	n	: 341051. : 6263534 : 28.8(m) : 13 m		Drill Log Date	Supplier er Company ged By						Client	: Landmark Group Construction Ai : Dee Why : 4 Delmar Parade, Dee Why NSW	ustralia Pty Ltd	Shee	et :3 OF 3
Drilling Method	Water	Well Diagram	RQD% and TCR%	Testing (20)sı	S Estimated S Strength	HS Weathering	Depth (m)	Elevation (m)	Graphic Log	Classification Code		Material Description	00 Defect Spacing 000 (mm)	Defect Depth	Defect Description type, inclination, planarity roughness, coating, thickness
NMLC			RQD = 85% TCR = 100%	d: 0.71, a: 0.43		SW SW	- 11 - 11 - 12	- 18.8 - 17.8 - 16.8 		SST	As above, but (gen	erally massive, minor carbonaceous laminations).	000	- 11 - 12 - 13	-10.08, P, 3*, UN, CL -10.17, P, 6*, IR, CL, OP 10.21, P, 3*, RO, IR, C 10.23, J, defect bounds 10.20m to 10.25m, F, RO, PL, CL, OP 10.25m, Sandy clay to 10.55m, infilled extremely weathere sandstone 85*, RO, PL, CI, OP 10.61, P, 3*, UN, CL 10.64, P, 4*, PL, CT, 11-11.38, CORELOS: Core Loss (notes). 11.39, P, clay coating 4*, PL, CT, OP 11.71, P, carbonaceo staining, 3*, RO, PL, CI, OP -12.99, P, 2*, RO, PL, CL, OP -12.99, P, 2*, RO, PL, CL, OP -12.56, P, 5*, UN, CL OP -12.75, P, carbonaceo staining, 5*, RO, PL, STN, OP -12.75, P, carbonaceo staining, 5*, RO, PL, STN, OP
							- 14 -	14.8 - -						- 14	







0405 843 933



Bellambi, NSW



info@morrowgeo.com.au

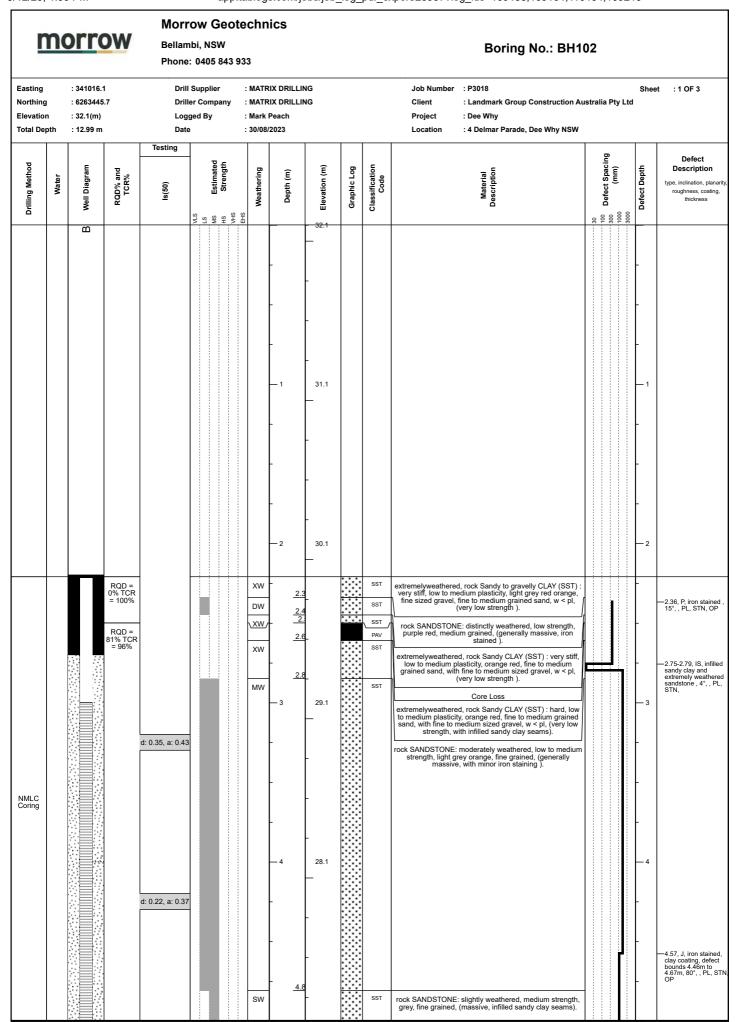
Photo description	BH101 - Tray 1 a	nd 2 of 2	
Client	Landmark Group	Construction Austra	lia Pty Ltd
Location	4 Delmar Parade	, Dee Why NSW	
Project name	Dee Why		
Project No	P3018	Scale	Not to Scale
BH No	BH101	BH Depth	6.17 to 13.00m

Morrow Geotechnics

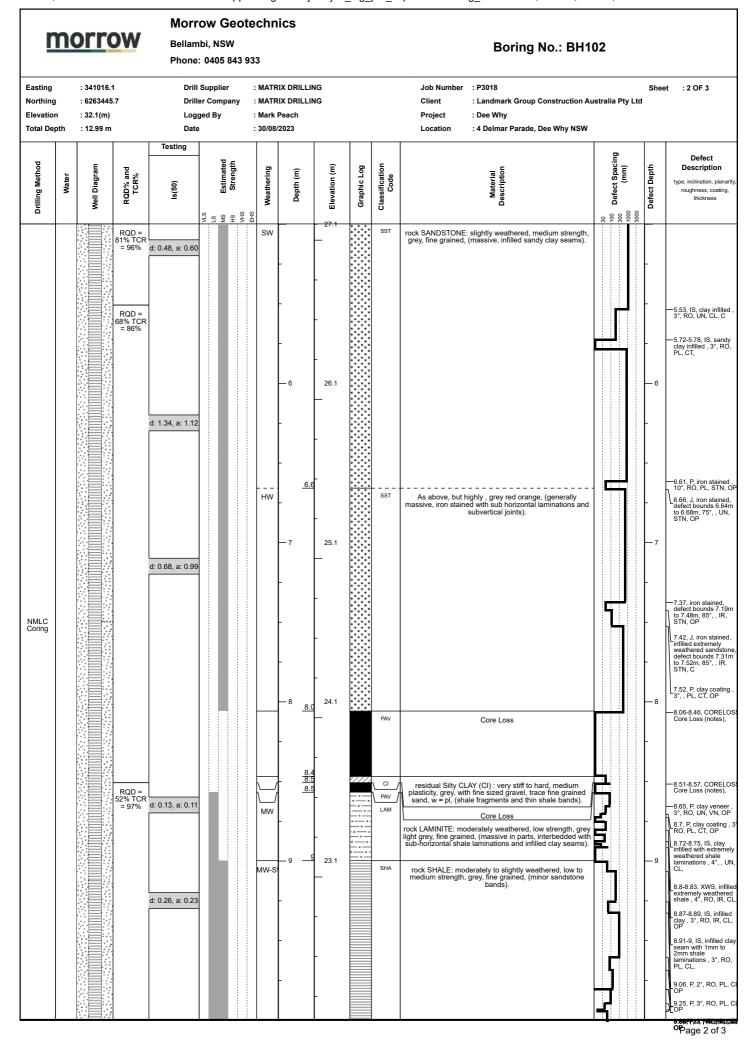
Bellambi, NSW Boring No.: BH102

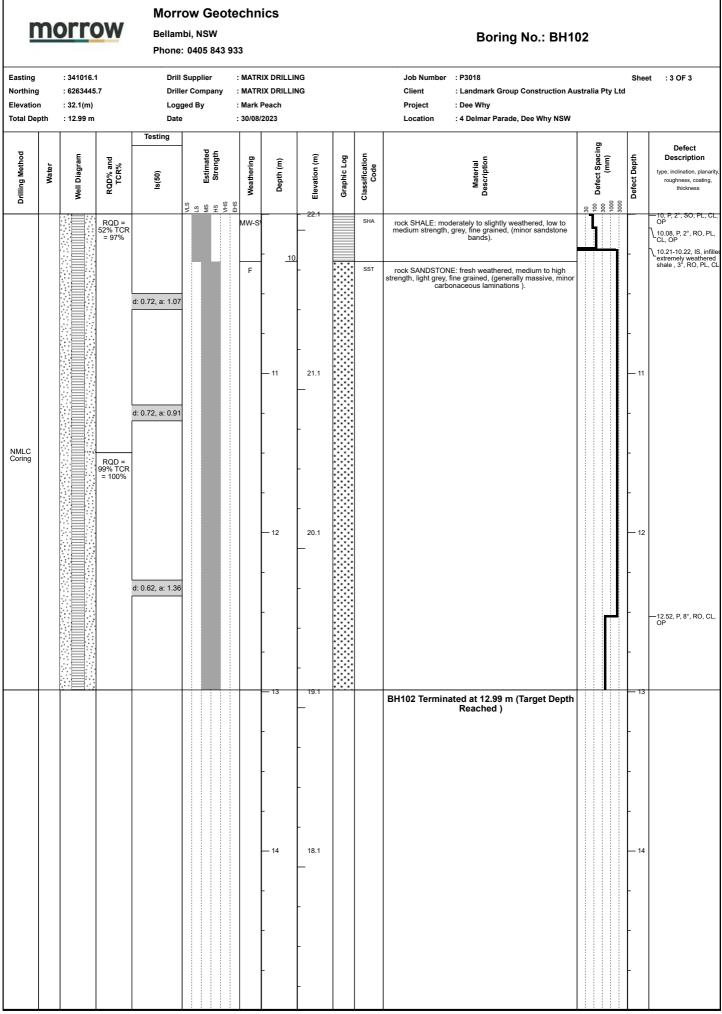
Phone: 0405 843 933

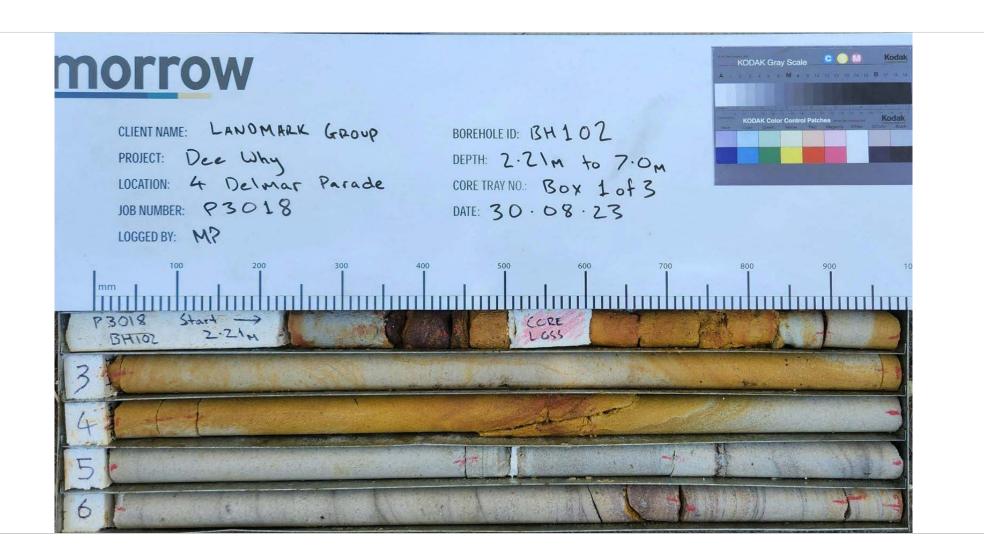
asting lorthing levation otal Depth		3445.7 (m)	,		Drill Sup Driller C Logged Date	ompany	: MATRIX DRILLING : MATRIX DRILLING : Mark Peach : 30/08/2023	Job Number Client Project Location	: P3018 : Landmark Group Cor : Dee Why : 4 Delmar Parade, Dee			Sheet : 1 OF 1 stralia Pty Ltd
Water	Testing	Soil Origin	Graphic Log	Classification Code	Depth (m)	Elevation (m)	Material	Description		Consistency/Density	Moisture	Observations
		Non-Soil		ССТ		- 32.1 -	Con	crete		Ŏ		
		Ē	488-5	SM		_	Fill Silty to gravelly SAND (SM): loose, gramedium sized gravel, trace low pla	ey dark grey, fine to r sticity clay, moist, (lo	medium grained, fine to w resistance).	L	М	
		Rock		SST	<u>0.9</u> — 1	31.1	Rock SANDSTONE: distinctly weathered, ve (sandstor	y low strength, grey e floater).	orange, medium grained,	VLS	D	
		Residual		SC	<u>- 1.3</u>	<u>-</u>	Residual Clayey to gravelly SAND (SC): loos grey, medium grained, medium to coarse six float	e to medium dense, zed gravel, moist, (lo lers).	low plasticity clay, orange w resistance, sandstone	L-MD	М	
					-2	30.1						
					- -3	29.1	2.21m : Commen	CEU NMLC COPI	ıry,			
					- - - -	28.1						
					-	-						



Page 1 of 3











0405 843 933

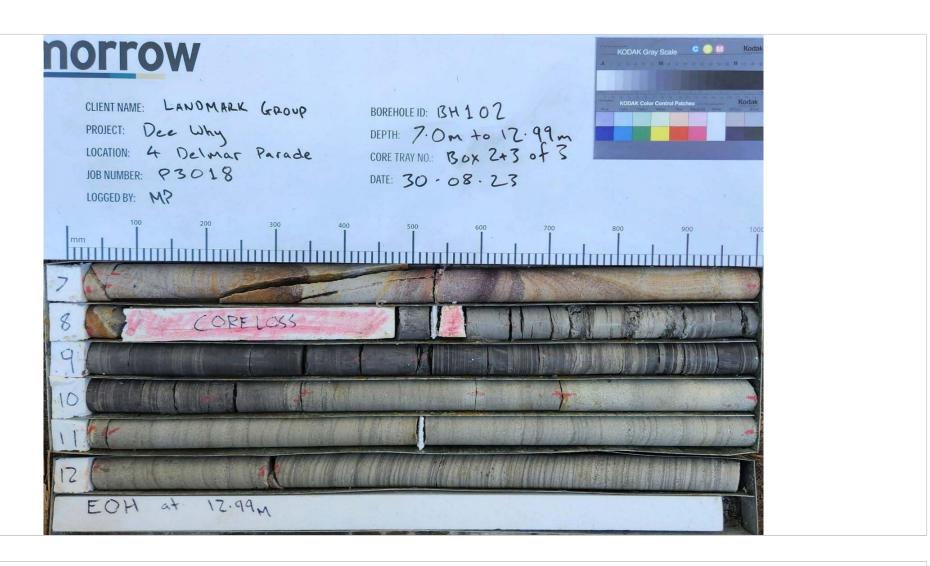


Bellambi, NSW



info@morrowgeo.com.au

Photo description	BH102 - Tray 1 (of 3	
Client	Landmark Group	Construction Australia	Pty Ltd
Location	4 Delmar Parade	e, Dee Why NSW	
Project name	Dee Why		
Project No	P3018	Scale	Not to Scale
BH No	BH102	BH Depth	2.21 to 7.00m







0405 843 933



Bellambi, NSW



info@morrowgeo.com.au

Photo description	BH102 - Tray 2 an	d 3 of 3	
Client	Landmark Group (Construction Australia	a Pty Ltd
Location	4 Delmar Parade,	Dee Why NSW	
Project name	Dee Why		
Project No	P3018	Scale	Not to Scale
BH No	BH102	BH Depth	7.00 to 12.99m

MORTOW Easting : 340947.7

Morrow Geotechnics

Bellambi, NSW Boring No.: BH103

Phone: 0405 843 933

Drill Supplier : MATRIX DRILLING Job Number : P3018 Sheet : 1 OF 2

1		: 340947.7 : 6263458.9 : 31.5(m)	3458.9 Driller Comp (m) Logged By n Date		: MATRIX : MATRIX : Andrew : 03/09/20	DRILI Butel			Job Number : P3018 Client : Landmark Group Construction Australia Pty Ltd Project : Dee Why Location : 4 Delmar Parade, Dee Why NSW	Shee	t :	1 OF 2
Drilling Method	Water	Well Diagram	Testing	Soil Origin	Graphic Log	Classification Code	Depth (m)	Elevation (m)	Material Description	Consistency/Density	Moisture	Observations
Diatube		D		Fill Non-Soil		SW	0 <u>.22</u> -	- 31.5 - -	Concrete Fill Gravelly SAND (SW): loose, orange grey, fine to medium grained, medium sized gravel, moist, (poorly graded).	L	М	
			4, 5, 4, (N = 9)	Alluvial		SW		- 30.5	Alluvial Gravelly SAND (SW) : loose, orange grey, fine to medium grained, medium sized gravel, moist, (poorly graded).			
							-	-				
			2, 4, 7, (N = 11)	Alluvial		CI	- -2 <u>-2</u>	- - 29.5	Alluvial Sandy CLAY (CI): stiff, medium plasticity, orange light grey, medium grained sand, with medium to coarse sized gravel, inorganic, w = pl, (quartz gravels).	St	w≈ PL	
ADT							-	_	gravels).			
			2, 6, 3, (N = 9)	Alluviai		CI	3 <u>3</u>	- 28.5	As above, but red orange.			
							-	-				
	>			Alluvial		SP	<u>4</u> 4	- 27.5 -	Alluvial Gravelly SAND (SP): medium dense to dense, grey orange, medium grained, medium sized gravel, wet, (quartz gravels).	MD-D	W	
			6, 7, 5, (N = 12)				-	-				

Morrow Geotechnics

Bellambi, NSW Boring No.: BH103

Phone: 0405 843 933

Easting : 340947.7 Drill Supplier : MATRIX DRILLING Job Number : P3018 Sheet : 2 OF 2
Northing : 6263458.9 Driller Company : MATRIX DRILLING Client : Landmark Group Construction Australia Pty Ltd

Elev			i(m) Logged By Date	uny	: MATRIX : Andrew : 03/09/20	Butel			Client : Landmark Group Construction Australia Pty Ltd Project : Dee Why Location : 4 Delmar Parade, Dee Why NSW			
Drilling Method	Water	Well Diagram SPT SPT		Soil Origin	Graphic Log	Classification Code	Depth (m)	Elevation (m)	Material Description	Consistency/Density	Moisture	Observations
ADT				Altuvial		SP	- 6	- 26.5 - - - 25.5	Alluvial Gravelly SAND (SP): medium dense to dense, grey orange, medium grained, medium sized gravel, wet, (quartz gravels).	MD-D	w	
150mm Washbore			4, 10, 11, (N = 21) 12, (N = 24), Hard Bounce	Rook		SST	- 7 <u>7.1</u>	- - - 24.5	Rock SANDSTONE: distinctly weathered, very low strength, very light grey, medium grained, (generally massive with minor carbaceous lamintions).	VLS	D	
							- 8 9	- 23.5 22.5 	7.83m : Commenced NMLC Coring;			

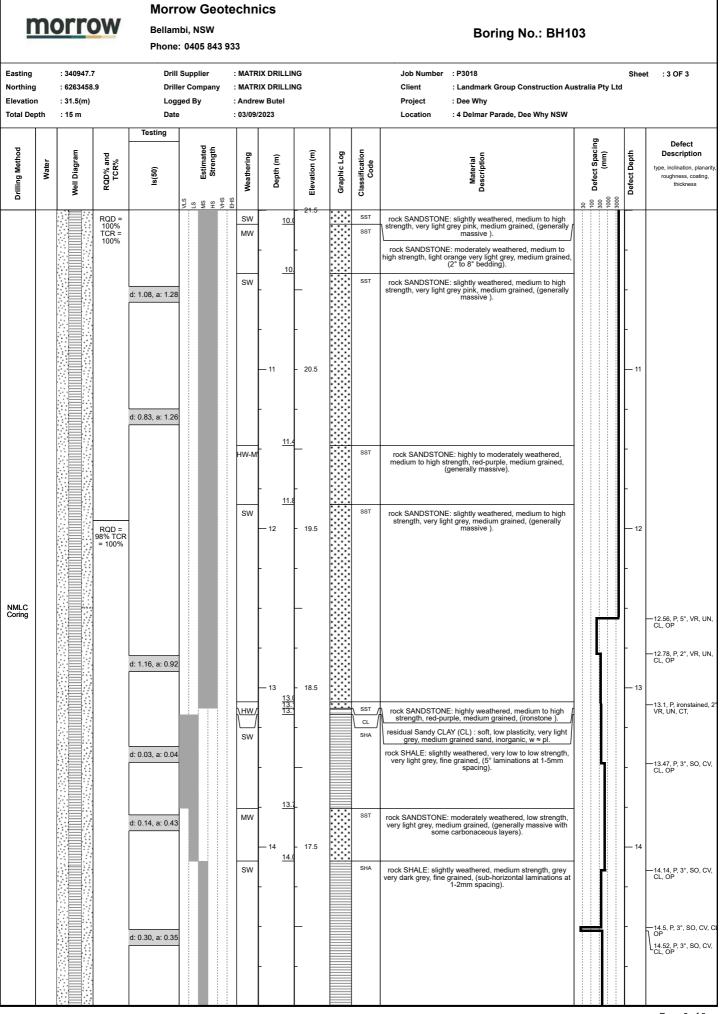
Morrow Geotechnics

Bellambi, NSW Boring No.: BH103

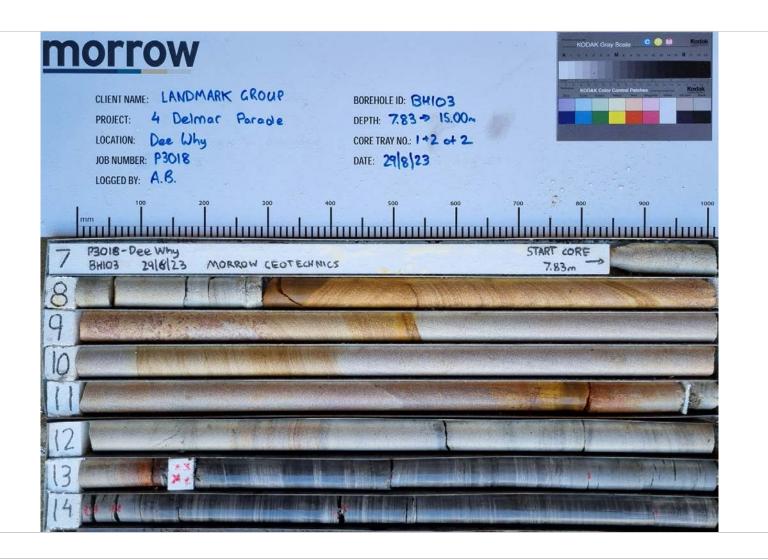
Phone: 0405 843 933

Easting : 340947.7 Drill Supplier : MATRIX DRILLING Job Number : P3018 Sheet : 2 OF 3

Northing Elevation Total Dep	n	: 340947. : 6263458 : 31.5(m) : 15 m		Drille Logg Date	er Company ged By	: MATF	RIX DRILLII RIX DRILLII ew Butel /2023				Client : Landmark Group Construction Au Project : Dee Why Location : 4 Delmar Parade, Dee Why NSW	ustralia Pty Lte	Shee	t : 2 OF 3
Drilling Method	Water	Well Diagram	RQD% and TCR%	Testing (09)sl	VLS LS MS Estimated HS Strength VMS	Weathering	Depth (m)	Elevation (m)	Graphic Log	Classification Code	Material Description	30 Defect Spacing 300 (mm)	Defect Depth	Defect Description type, inclination, planarity, roughness, coating, thickness
								- 25.5 - 24.5					- - - - -	
			RQD = 84% TCR = 100%	d: 0.70, a: 0.40		sw	—8 - <u>8.2</u>	- 23.5		SST	rock SANDSTONE: slightly weathered, medium strength, very light grey, medium grained, (generally massive with minor carbaceous lamintions).		— 8 -	—8.05, P, 3°, RO, UN, 0 OP —8.17, P, 6°, VR, UN, 0
NMLC Coring				d: 0.58, a: 0.48		MW	- 9 -	- 22.5		SST	rock SANDSTONE: moderately weathered, medium strength, orange red, medium grained, (20° to 40° bedding).		9	8.21, P, 1°, VR, UN, C OP 8.26, P, 1°, VR, UN, C OP 8.29, P, 1°, VR, UN, C OP 8.31-8.37, J, 40°, VR, UN, CL, OP
			RQD = 100% TCR = 100%			MW	- <u>9.2</u>			SST	rock SANDSTONE: moderately weathered, medium strength, red orange mottled, medium grained, (generally massive).	-	- 9	
						MW	- <u>9.</u> 5	_		SST	rock SANDSTONE: moderately weathered, medium strength, orange red, medium grained, (10° to 20° bedding).		-	
				d: 0.77, a: 1.11		sw	-	-		SST	rock SANDSTONE: slightly weathered, medium to high strength, very light grey pink, medium grained, (generally massive).		-	



Page 3 of 3







0405 843 933



Bellambi, NSW



info@morrowgeo.com.au

Photo description	BH103 - Tray 1 a	and 2 of 2								
Client	Landmark Group Construction Australia Pty Ltd									
Location	4 Delmar Parade, Dee Why NSW									
Project name	Dee Why									
Project No	P3018	Scale	Not to Scale							
BH No	BH103	7.83 to 15.00m								

UTM

Morrow Geotechnics

Bellambi, NSW Phone: 0405 843 933

Geotechnical Log - Borehole BH104

: 56H Drill Rig : Hand Auger Job Number : P3018

Easting (m) : 340914.1 Driller Supplier : MG Client : Landmark Group Construction Australia Pty Ltd

epth : 20.91 m BG	L	Da	ate	d By	: Rhiannon McKeon Location : 4 Delmar Parade, Dee W : 30/08/2023 Loc Comment :	ny NSW		
Water D Well Diagram	Soil Origin	Graphic Log	Classification Code	Depth (m)	Material Description	Elevation (m)	Consistency/Density	Moisture
Backfill	NGH-Soil		PAR SW	<u>.</u> - -		avel,	VL-L	M
NNE 50mm PVC Solid	Alluvial		sw	-	Alluvial SAND (SW): loose, grey, fine to medium grained, trace fine sized gravel, moist, low resistar	-	L-MD	
	Rock Alluvial			_ 3	Alluvial Sandy CLAY (CL-Cl): firm, low to medium plasticity, grey orange light grey, fine to medium grasand, with fine to medium sized gravel, w ≈ pl, low to medium resistance, ironstone and extremely weathered sandstone gravels.	28.8	F VSt-H	w≈ PL W< PL
	α <u>΄</u>			- - - 4	orange, medium grained sand, with fine sized gravel, w < pl, high resistance, with sandy clay band 3.3m: Commenced NMLC Coring;	27.8		PL
	-Backfill	Backfill Solid Payllow Payl	-Backfill Solid Finally Finall	-Backfill Somm PVC Solid Figure Solid Figu	Somm PVC Solid SW SW SW SW SW SW SST ST	Simple Solid Simple Solid Simple Solid Sol	Fill Gravelly SAND (SW) : very loose to tense, gray, fine to medium grained, fine to medium sized gravel, most, low resistance. Altuvial SAND (SW) : loose, grey, fine to medium grained, trace fine sized gravel, most, low resistance. Solid PVC Solid PVC Altuvial SAND (SW) : loose, grey, fine to medium grained, trace fine sized gravel, most, low resistance. Altuvial Clayery SAND (SC) : loose to medium dense, low plasticity day, prange light grey grey, medium grained. Trace medium sized gravel, most, low resistance. 28.8 Extremelyweathered, rock Sandy CLAY (CL-CI) : firm, low to medium plasticity, gray orange light grey, fine to medium grained sand, with fire to medium sized gravels, most, low resistance, with sized gravels, most, low resistance, and calcium grained sand, with fire to medium sized gravels, most, low resistance, with size or gravels, most, low to medium grained sand size of gravels, most, low to medium grained sand, with fire to medium grained sand, with fire to medium grained sand, with the sized gravel, w. cl. flight resistance, with aircly city Sandy CLAY (SL-CI) : firm, low to medium plasticity, gray orange light gray, fine to medium grained sand, with fire to medium grained sand, with the sized gravel, w. cl. flight resistance, with aircly city Sandy. 3.3 Extremelyweathered rock Sandy CLAY (SST) : way self to hard, low to medium grained sand, with the sized gravel, w. cl. flight resistance, with aircly city Sandy. 3.3 Sand Commenced MMLC Coring;	Parent Pa

Morrow Geotechnics

Bellambi, NSW Phone: 0405 843 933

Geotechnical Log - Borehole BH104

Drill Rig : Hand Auger Job Number : P3018

Easting (m) : 340914.1 Driller Supplier : MG Client : Landmark Group Construction Australia Pty Ltd

Northing		: 6263462.4 on : 31.8 (m)		ged By iewed By	: Mark Peach : Rhiannon Mck	(aon				Project : Dee Why Location : 4 Delmar Parade	Dee Why	NSW		
Total De		: 20.91 m BGL	Date		: 30/08/2023	ceon				Loc Comment :	, Dee why	NSW		
Drilling Method	Water	D Well Diagram	RQD% and TCR%	Testing	1.8 LS Festimated Strength HS HS	Weathering	Depth (m)	Graphic Log	Classification Code	Material Description	Elevation (m)	30 100Defect Spacing 1000 (mm)	Defect Depth	Defect Description type, inclination, planarity, roughness, coating, thickness
	ВЕТИЛ ВОТИТВОТИТЕВОТИТЕВОТИТЕВОТИТЕВОТИТЕВОТИТЕВОТИТЕВОТИТЕВОТИТЕВОТИТЕВОТИТЕВОТИТЕВОТИТЕВОТИТЕВОТИТЕВОТИТЕВОТ	-Backfill Solid					- - - - - - -				30.8			
NMLC Coring	0% Water Loss	-Bentonite	RQD = 92% TCR = 100%			HW F	- - <u>3.9</u> 4		SST	Rock SANDSTONE: highly weathered, low strength, light grey red orange, fine grained, massive, iron stained . Rock SANDSTONE: fresh weathered, medium strength, grey, fine grained, massive, minor carbonaceous laminations .	27.8		_ 4	—4.16, P, 5°, RO, PL, CL, OP
		-5mm Graded Sand	RQD = 99% TCR = 100%	d: 0.65, a: 0.53			-				-		-	-4.35, P, 3°, RO, PL, CL, OP -4.81, P, 5°, , PL, CL OP Page 1 of 5

Morrow Geotechnics

Bellambi, NSW Phone: 0405 843 933

Drill Rig

Geotechnical Log - Borehole BH104

: Hand Auger Job Number : P3018

Easting (m) : 340914.1 Driller Supplier : MG Client : Landmark Group Construction Australia Pty Ltd

Ground I		on : 31.8 (m)		iewed By	: Rhiannon Mc	Keon				Location : 4 Delmar Parade	, Dee Why	NSW		
Total Dep	oth	: 20.91 m BGL	Date		: 30/08/2023					Loc Comment :				
Drilling Method	Water	Well Diagram	RQD% and TCR%	Testing (05)sı	LS LS Estimated Strength HS HAS	Weathering	Depth (m)	Graphic Log	Classification Code	Material Description	Elevation (m)	30 100Defect Spacing 300 (mm) 1000	Defect Depth	Defect Description type, inclination, planarity, roughness, coating, thickness
		50mm PVC Solid	RQD = 99% TCR = 100%	d: 0.79, a: 0.83		F	-		SST	Rock SANDSTONE: fresh weathered, medium strength, grey, fine grained, massive, minor carbonaceous laminations.	26.8		-	—5.07, P. 5°, RO, PL, CL, OP
		-5mm Graded Sand	RQD = 100% TCR = 100%	d: 0.79, a: 1.32		F	-6 - 6	<u>s</u>	SST	Rock SANDSTONE: fresh weathered, medium to high strength, grey, fine grained, massive, minor carbonaceous laminations .	- 25.8 - - -		— 6 - -	
NMLC Coring	0% Water Loss		RQD = 100% TCR = 100%	d: 1.07, a: 1.25			—7 - -				24.8 - -		7 	
		50mm PVC Slotted	RQD = 100% TCR = 100%	d: 1.17, a: 0.78			- 8 - -				23.8	-	- 8	─8.14, P, 5°, , PL, CL OP
			TCR = 100%				- - 9 -				22.8		- 9 -	
							-				_		-	—9.71, P, 5°, , PL, CL OP

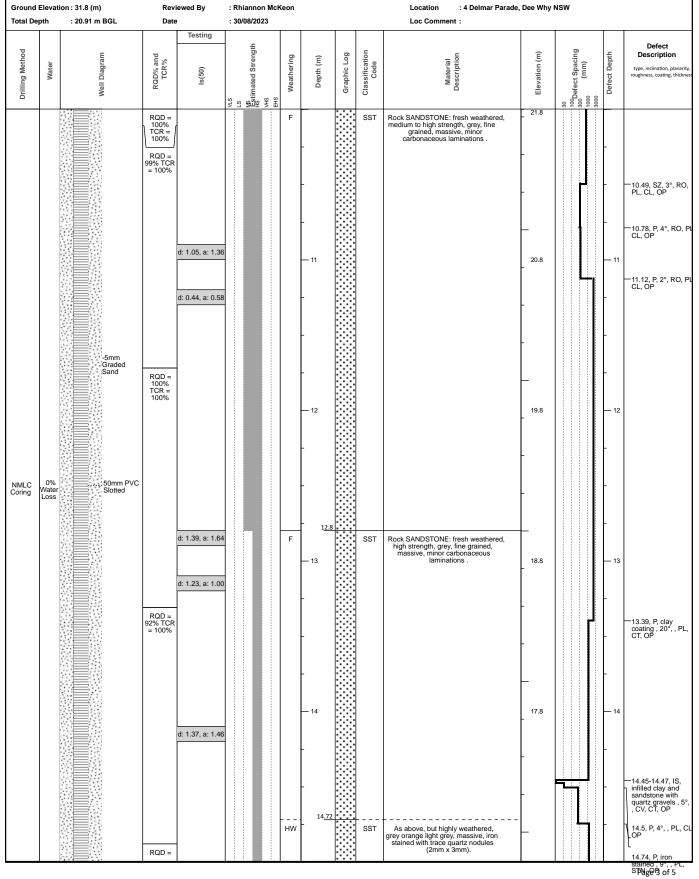
Morrow Geotechnics

Bellambi, NSW Phone: 0405 843 933

Geotechnical Log - Borehole BH104

Drill Rig : Hand Auger Job Number : P3018

Easting (m) : 340914.1 Driller Supplier : MG Client : Landmark Group Construction Australia Pty Ltd



Morrow Geotechnics

Bellambi, NSW Phone: 0405 843 933

Geotechnical Log - Borehole BH104

: 56H Drill Rig : Hand Auger Job Number : P3018

Easting (m) : 340914.1 Driller Supplier : MG Client : Landmark Group Construction Australia Pty Ltd

Northing		: 6263462.4		ged By	: Mark Peach	.,				Project : Dee Why				
Ground Total De		on : 31.8 (m) : 20.91 m BGL	Rev Date	iewed By	: Rhiannon Mc : 30/08/2023	Keon				Location : 4 Delmar Parad Loc Comment :	e, Dee Why	NSW		
Drilling Method	Water	Well Diagram	RQD% and TCR%	Testing (09)sı	LS Estimated Strength HS HHS	Weathering	Depth (m)	Graphic Log	Classification Code	Material Description	Elevation (m)	30 1900 1900 1000 1000 1000	Defect Depth	Defect Description type, inclination, planarity, roughness, coating, thickness
			RQD = 98% TCR = 100%	d: 1.23, a: 1.00		HW	-		SST	As above, but highly weathered, grey orange light grey, massive, iron stained with trace quartz nodules (2mm x 3mm).	16.8	00 00 00 00 00 00 00 00 00 00 00 00 00	-	
		-5mm	RQD = 80% TCR = 100%	d: 0.16, a: 0.15		F	15 <u>.95</u> — 16		SHA	Rock SHALE: fresh weathered, low strength, grey light grey, fine to medium grained, 1mm to 5mm laminations, with some massive sandstone bands.	15.8		— 16 -	—15.95-15.97, XWS, clay coating , 2°, RO, PL, CT, OP —16.16, P, 2°, RO, PL CL, OP
NMLC	0% Water	Graded Sand		d: 0.32, a: 0.43	-		- 17 				14.8		- 17 -	—16.93, P, 3°, , IR, Cl OP 17, J, defect bounds 16.95m to 17.05m, 80°, RO, PL, CL, OP
Coring	Loss	Jones	RQD = 99% TCR = 100%	d: 1.52, a: 2.65		HW	18		SST	Rock SANDSTONE: highly weathered, medium to high strength, red orange, fine to medium grained, generally massive, heavily iron stained, minor 1mm to 2mm quartz nodules.	13.8		- 18 -	17.47, P, 6°, RO, C' CL, OP 17.62, P, 3°, RO, UI CL, OP 17.72, P, iron stained , 6°, , IR, STN, OP 17.73, P, iron stained , 3°, , IR, STN, C
				d: 1.55, a: 2.65		SW	- 18 <u>.57</u> - 18 <u>.97</u> 19		SST	As above, but slightly weathered, high strength, light grey orange, massive. Rock SANDSTONE: fresh weathered, high strength, light grey, fine to medium grained, massive.	12.8		- 19	—18.72, P, 9°, , PL, CL, OP —19.03, P, 6°, , PL, CL, OP
			RQD = 100% TCR = 100%	-			-				-		-	



Bellambi, NSW Phone: 0405 843 933

Geotechnical Log - Borehole BH104

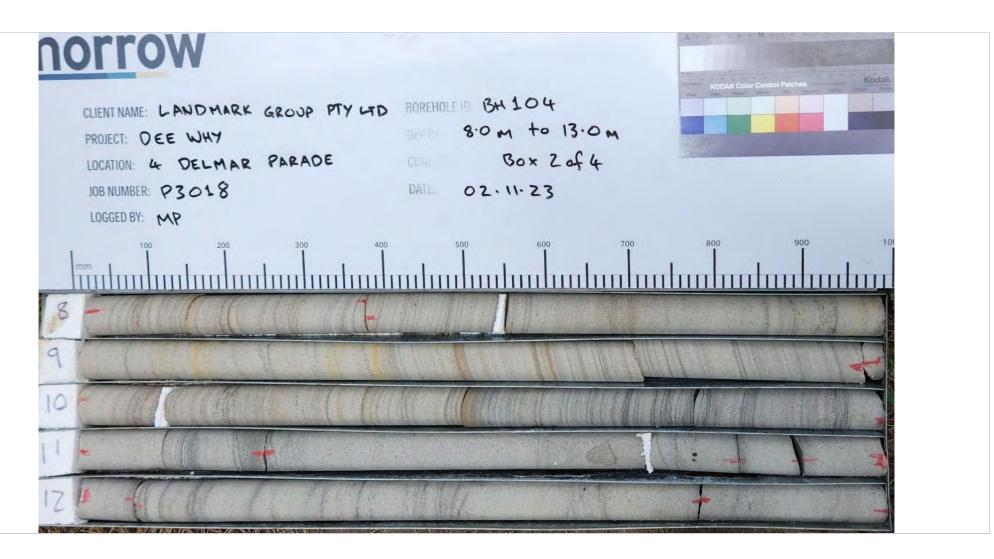
Drill Rig : Hand Auger Job Number : P3018

Easting (m) Driller Supplier : MG : Landmark Group Construction Australia Pty Ltd : 340914.1 Client

Northing Ground Total De	j (m) Elevati	: 6263462.4 on : 31.8 (m) : 20.91 m BGL		ged By ewed By	: Mark Peach : Rhiannon Mck : 30/08/2023	K eon				Project : Dee Why Location : 4 Delmar Parade Loc Comment :		NSW	_	
Drilling Method	Water	Well Diagram	RQD% and TCR%	Testing (09)s1	VLS LS FStimated Strength NHS HHS	Weathering	Depth (m)	Graphic Log	Classification Code	Material Description	Elevation (m)	30 100Defect Spacing 300 (mm)	Defect Depth	Defect Description type, inclination, planarity, roughness, coating, thickness
NMLC Coring	0% Water Loss	-5mm Graded Sand 50mm PVC Slotted	RQD = 100% TCR = 100%	d: 1.59, a: 2.61		F	-		SST	Rock SANDSTONE: fresh weathered, high strength, light grey, fine to medium grained, massive.			-	
							- 21			BH104 Terminated at 20.91m (Target Depth Reached)	10.8		— 21 -	
							— 22 -				9.8		— 22 - -	
							23 				8.8		- - 23 -	
							24 				7.8		- 24 - -	
							_						-	



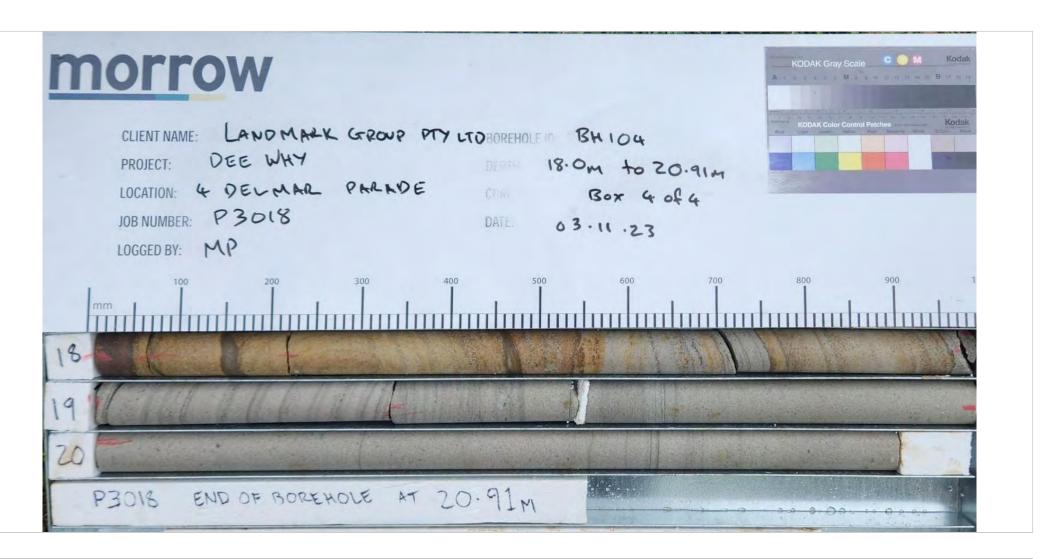
















Bellambi, NSW Phone: 0405 843 933

Geotechnical Log - Borehole BH105

UTM : 56H Drill Rig : Christie Rig Job Number : P3018

Easting (m) : 341010.2 Driller Supplier : Tracess Client : Landmark Group Construction Australia Pty Ltd

Nor Gro	ting (m thing (r und Ele I Depth	m) : 6263530 evation : 28.5 (m)	0.0	Logged By Reviewed By Date		: Mahmor : Mark Pe : 08/11/20		. ty Ltu		
Drilling Method	Water	DCP graph	Soil Origin	Graphic Log	Classification Code	Depth (m)	Material Description	Elevation (m)	Consistency/Density	Moisture
Diatube			Non-Soil		ССТ	0 <u>.15</u>	Concrete	28.5		
			Fill Alluvial		SC SC	0.4	Fill Clayey to gravelly SAND (SC): low plasticity clay, medium dense, brown, fine grained, fine sized gravel, moist. Alluvial Clayey SAND (SC): medium dense, low plasticity clay, brown, fine grained, trace fine		MD	М
			, mavia			- -1 -	Alluvial Clayey SAND (SC): medium dense, low plasticity clay, brown, fine grained, trace fine sized gravel, moist.	- 27.5		
ADT	GWNE		Alluvial		CI	- 2 	Alluvial Sandy CLAY (CI) : stiff to very stiff, medium plasticity, brown yellow, fine grained sand, w < pl.	- 26.5	St-VSt	w < PL
			Alluvial		CL	_3 _ - -	Alluvial Sandy CLAY (CL): very stiff, low plasticity, yellow brown orange brown yellow, fine grained sand, w < pl.	- 25.5 - -	VSt	
						-4		- 24.5		



Bellambi, NSW Phone: 0405 843 933

Geotechnical Log - Borehole BH105

Drill Rig : Christie Rig Job Number : P3018

Easting (m) : 341010.2 Driller Supplier : Tracess Client : Landmark Group Construction Australia Pty Ltd

Nor	ting (m thing (r		0.0	Driller Suppl Logged By Reviewed By		: Tracess : Mahmou : Mark Pe	Client : Landmark Group Construction Australia F Id Jangidaryan Project : Dee Why ach Location : 4 Delmar Parade, Dee Why NSW	ty Ltd		
1	al Depth			Date Date	•	: Wark Pe				
Drilling Method	Water	DCP graph	Soil Origin	Graphic Log	Classification Code	Depth (m)	Material Description	Elevation (m)	Consistency/Density	Moisture
	-		Residual		CL	- 6 6	Residual Sandy CLAY (CL) : very stiff, low plasticity, grey light grey orange, fine to medium grained sand, w ≈ pl. As above, but grey orange brown yellow, fine grained sand, trace fine sized gravel, w ≈ II.	- 22.5		w≈ PL w≈ LL
ADT						- - -7		- - - 21.5		
			Rock		SST	- <u>7.6</u> - 8	Extremelyweathered, rock Clayey SAND (SST): very dense, low plasticity clay, brown brown yellow, fine grained, trace fine sized gravel, wet.	- 20.5	VD	w
							8.72m : Commenced NMLC Coring;			
						9 -		- 19.5 - -		



Bellambi, NSW Phone: 0405 843 933

Geotechnical Log - Borehole BH105

TM : 56H Drill Rig : Christie Rig Job Number : P3018

Easting (m) : 341010.2 Driller Supplier : Tracess Client : Landmark Group Construction Australia Pty Ltd

Northing (m) : 6263530.0 Logged By : Mahmoud Jangidaryan Project : Dee Why

Ground	Elevation :	28.5 (m)		Revie	wed By		Mark Pea		uu. yu		Location	: 4 Deln	nar Parade, De	e Why	NSW
Total De	pth :	22.22 m B	GL	Date		:	08/11/202	3			Loc Commer	nt :			
Drilling Method	Water	RQD% and TCR%	Testing (09)sı	NS.	Estimated Strength HS WHS EHS	Weathering	Depth (m)	Graphic Log	Classification Code	Material Description		Elevation (m)	-30 -100Defect Spacing -300 (mm) -1000 (mm)	Defect Depth	Defect Description type, inclination, planarity, roughness, coating, thickness
							-					-		-	
							- 6					- 22.5 -		- -6	
							- - 7					- 21.5		-7	
							- 8					- - 20.5		- 8	
		RQD = 100% TCR = 100%	d: 0.17, a: 0.13			HW	-		SST	Rock SANDSTONE: highly very low strength, orange grained, generally massiv minor xw and dw sandstone iron stained.	weathered, a, fine e, wiith b bands,	- 10.5		-	
NMLC Coring	0% Water Loss		d: 0.05, a: 0.03 d: 0.06, a: 0.03				9 - -					- 19.5 - 		- - -	

Morrow Geotechnics

Bellambi, NSW Phone: 0405 843 933

Geotechnical Log - Borehole BH105

Drill Rig : Christie Rig Job Number : P3018

Easting (m) : 341010.2 Driller Supplier : Tracess Client : Landmark Group Construction Australia Pty Ltd

Northing (m) : 6263530.0 Logged By : Mahmoud Jangidaryan Project Ground Elevation: 28.5 (m) Reviewed By : Mark Peach Location : 4 Delmar Parade. Dee Why NSW Total Depth : 08/11/2023 Loc Comment : Date Testing Defect -30 100Defect Spacing -300 (mm) -3000 Classification Code Method Description RQD% and TCR% Elevation (m) Weathering Œ Graphic Log Water type, inclination, planarity, roughness, coating, thickness Is(50) Defect [18.5 Rock SANDSTONE: highly weathered, very low strength, orange, fine grained, generally massive, wiith minor xw and dw sandstone bands, iron stained. RQD = 100% TCR = 100% HW SST d: 0.07, a: 0.01 1<u>0.3</u> IW-N SST Rock SANDSTONE: highly to moderately weathered, low strength, grey light grey, fine to medium grained, medium grained. RQD = 90% TCR = 100% d: 0.20, a: 0.21 - 11 17.5 -11.06, P. RO, PL, CL, OP 11<u>.15</u> -11.14, P, RO, PL, CL, OP Rock SANDSTONE: slightly to fresh weathered, medium strength, light grey mottled yellow white, fine grained, 10-15 degree 2mm beddings at 10-20mm spacing. SW-F d: 1.01, a: 1.30 11.57, J, 30°, SO, PL, CL, OP 12 16.5 12.68, J, 20°, SO, PL, CL, OP -12.82. J. RO. PL. CL. OP d: 0.63, a: 1.17 13 15.5 13.32 Rock SANDSTONE: slightly to fresh weathered, medium to high strength, very light grey with grey, medium to coarse grained, generally massive, medium to coarse grained with sub rounded gravels. 13.32, P, 10°, RO, PL, CL, OP lsw-F SST RQD = 88% TCR = 100% - 14 14.5 d:0.81, a: 0.50 14.17, P, 20°, , CV, CL, OP _14.23, P, 30°, , PL, CL, OP 14.39, P, 20°, , PL, CL, OP Rock SANDSTONE: fresh weathered, medium to high strength, light grey, fine grained, generally massive, 10° laminations at 1-5mm spacing. F SST 14.53, P, 10°, SO, PL, CL, OP -14.62, P, 10°, SO, PL, CL, OP d: 1.30, a: 1.17



Bellambi, NSW Phone: 0405 843 933

Geotechnical Log - Borehole BH105

Drill Rig : Christie Rig Job Number : P3018

Easting (m) : 341010.2 Driller Supplier : Tracess Client : Landmark Group Construction Australia Pty Ltd

Northing (m) : 6263530.0 Logged By : Mahmoud Jangidaryan Project : Dee Why

Total Depth Double Method Mater Mater Total Depth Total Depth Total Depth	: 22.22 m E	Testing	Reviewed By Date	:	Mark Peac 08/11/2023			Loc Comme		nar Parade, Dee	iy l	
	Water RQD% and TCR%	Testing	ath.									
Dri		ls(50)	VLS LS Estimated Strength VHS HS	Weathering	Depth (m)	Graphic Log	Classification Code	Material Description	Elevation (m)	30 100Defect Spacing 300 (mm)	Defect Depth	Defect Description type, inclination, planarity, roughness, coating, thickness
	RQD = 88% TCR = 100%	d: 1.58, a: 1.52		F	- - - — 16		SST	Rock SANDSTONE: fresh weathered, medium to high strength, light grey, fine grained, generally massive, 10° laminations at 1-5mm spacing.	- 12.5		- - -	15.34, J, 75°, RO, PL, CL, OP 15.37, P, 10°, RO, PL, CL, OP
70 Wat Los	RQD = 91% TCR = 100%	d: 0.87, a: 0.90			- - - 17				- 11.5			16.3, P, 5°, SO, PL, CL, OP 16.59, P, 5°, SO, PL, CL, OP 16.73, J, 60°, RO, PL, CL, OP 16.77, P, RO, PL, CL, OP
NMLC Coring		d: 0.89, a: 1.08			- - - — 18				- 10.5		_ 18	——17.6, P, 5°, SO, PL, CL, OP ——17.7, P, 5°, RO, PL, CL, OP
		d: 0.92, a:1.07			- - 19				- 9.5		- - - 19	— 18.3, P, 5°, SO, PL, CL, OP — 18.37, P, 10°, RO, PL, CL, OP — 18.46, P, 10°, RO, PL, CL, OP — 18.57, P, 15°, RO, PL, CL, OP
	RQD = 71% TCR = 100%				-				-		-	——19.32, P, 5°, RO, PL, CL, OP ——19.66, J, 80°, RO, PL, CL, OP ——19.9, J, 75°, RO, PL, CL, OP



Bellambi, NSW Phone: 0405 843 933

Geotechnical Log - Borehole BH105

Drill Rig Job Number : P3018

Driller Supplier : Tracess Client : Landmark Group Construction Australia Pty Ltd

Easting (m) : 341010.2 Northing (m) : 6263530.0 Logged By : Mahmoud Jangidaryan Project : Dee Why

Ground Elevation : 28.5 (m) Reviewed By : Mark Peach Location : 4 Delmar Parade, Dee Why NSW

Total Dep		28.5 (m) 22.22 m B	GI	Reviewed By Date		: Mark Peac : 08/11/2023			Location Loc Comme		mar Parade, Dee	Why	NSW
			Testing			30,11,2023			Loc comme				
Drilling Method	Water	RQD% and TCR%	Is(50)	LS LS Estimated Strength MS MS	Weathering	Depth (m)	Graphic Log	Classification Code	Material Description	Elevation (m)	30 300 (mm) 1000 (mm)	Defect Depth	Defect Description type, inclination, planarity, roughness, coating, thickness
		RQD = 71% TCR = 100%			F			SST	Rock SANDSTONE: fresh weathered, medium to high strength, light grey, fine grained, generally massive, 10° laminations at 1-5mm spacing.	8.5			20.07, P, 5°, RO, PL, CL, OP
			d: 1.00, a: 0.98		 F	2 <u>0.3</u>		SST		ŀ	1	-	20.28, P, 5°, SO, PL, CL, OP
			u. 1.50, a. 0.30		r	-		551	As above, but high strength, generally massive with some carbonaceous layers.	_	L	-	20.45, P, 5°, RO, CV, CL, OP
NMLC Coring	70% Water Loss		d: 1.71, a: 2.02 d: 2.13, a: 2.27			— 21 -				- 7.5 -		— 21 -	21.08, J, 75°, RO, PL, CL, OP
						-				-		-	21.58, P, 5°, SO, PL, CL, OP 21.62, J, 85°, , PL, CL, OP
						— 22				- 6.5		— 22	21.84, P, 5°, RO, PL, CL, OP 21.88-22.22, J, 85°, RO, PL, CL, OP
						-	• • • •		BH105 Terminated at 22.22m (Target Depth Reached)	-			
						- 23				- - 5.5 -		- - 23	
										- 4.5		- 24 	













GENERAL

Information obtained from site investigations is recorded on log sheets. The "Cored Drill Hole Log" presents data from an operation where a core barrel has been used to recover material - commonly rock. The "Non-Core Drill Hole - Geological Log" presents data from an operation where coring has not been used and information is based on a combination of regular sampling and insitu testing. The material penetrated in non-core drilling is commonly soil but may include rock. The "Excavation - Geological Log" presents data and drawings from exposures of soil and rock resulting from excavation of pits, trenches, etc.

The heading of the log sheets contains information on Project Identification, Hole or Pit Identification, Location and Elevation. The main section of the logs contains information on methods and conditions, material substance description and structure presented as a series of columns in relation to depth below the ground surface which is plotted on the left side of the log sheet. The common depth scale is 8m per drill log sheet and about 3-5m for excavation logs sheets.

As far as is practicable the data contained on the log sheets is factual. Some interpretation is inevitable in the identification of material boundaries in areas of partial sampling, the location of areas of core loss, description and classification of material, estimation of strength and identification of drilling induced fractures. Material description and classifications are based on SAA Site Investigation Code AS 1726 - 1993 with some modifications as defined below.

These notes contain an explanation of the terms and abbreviations commonly used on the log sheets.

DRILLING

Drilling & Casing

ADV	Auger Drilling with V-Bit
ADT	Auger Drilling with TC Bit
WB	Wash-bore drilling
RR	Rock Roller
NMLC	NMLC core barrel
NQ	NQ core barrel
HMLC	HMLC core barrel
HQ	HQ core barrel

Drilling Fluid/Water

The drilling fluid used is identified and loss of return to the surface estimated as a percentage.

Drilling Penetration/Drill Depth

Core lifts are identified by a line and depth with core loss per run as a percentage. Ease of penetration in non-core drilling is abbreviated as follows:

VE	Very Easy
E	Easy
М	Medium
Н	High
VH	Very High

Groundwater Levels

Date of measurement is shown.

Standing water level measured in completed borehole

Level taken during or immediately after drilling

D	Disturbed
В	Bulk
U	Undisturbed
SPT	Standard Penetration Test
N	Result of SPT (sample taken)
PBT	Plate Bearing Test
PZ	Piezometer Installation
HP	Hand Penetrometer Test

EXCAVATION LOGS

Explanatory notes are provided at the bottom of drill log sheets. Information about the origin, geology and pedology may be entered in the "Structure and other Observations" column. The depth of the base of excavation (for the logged section) at the appropriate depth in the "Material Description" column. Refusal of excavation plant is noted should it occur. A sketch of the exposure may be added.

MATERIAL DESCRIPTION - SOIL

Classification Symbol - In accordance with the Unified Classification System (AS 1726-1993, Appendix A, Table A1)

Material Description - In accordance with AS 1726-1993, Appendix A2.3

Moisture Condition

D	Dry, looks and feels dry			
М	Moist, No free water on remoulding			
W	Wet, free water on remoulding			

Consistency - In accordance with AS 1726-1993, Appendix A2.5

VS	Very Soft	< 12.5 kPa		
S	Soft	12.5 – 25 kPa		
F	Firm	25 – 50 kPa		
St	Stiff	50 – 100 kPa		
VSt	Very Stiff	100 – 200 kPa		
Н	Hard	> 200 kPa		

Strength figures quoted are the approximate range of undrained shear strength for each class.

Density Index. (%) is estimated or is based on SPT results.

VL	Very Loose	< 15 %
L	Loose	15 – 35 %
MD	Medium Dense	35 – 65 %
D	Dense	65 – 85 %
VD	Very Dense	> 85 %

MATERIAL DESCRIPTION - ROCK

Material Description

Identification of rock type, composition and texture based on visual features in accordance with AS 1726-1993, Appendix A3.1-A3.3 and Tables A6a, A6b and A7.

Core Loss

Is shown at the bottom of the run unless otherwise indicated.

Bedding

	_		
Thinly Laminated	< 6 mm		
Laminated	6 - 20		
Very Thinly Bedded	20 - 60		
Thinly Bedded	60 - 200		
Medium Bedded	200 – 600		
Thickly Bedded	600 – 2000		
Very Thickly Bedded	> 2000		

Weathering - No distinction is made between weathering and alteration. Weathering classification assists in identification but does not imply engineering properties.

Fresh (F)	Rock substance unaffected by weathering		
Slightly Weathered	Rock substance partly stained or		
(SW)	discoloured. Colour and texture of fresh		
	rock recognisable.		
Moderately	Staining or discolouration extends		
Weathered (MW)	throughout rock substance. Fresh rock		
	colour not recognisable.		
Highly Weathered	Stained or discoloured throughout. Signs of		
(HW)	chemical or physical alteration. Rock texture		
	retained.		
Extremely	Rock texture evident but material has soil		
Weathered (EW)	properties and can be remoulded.		

Strength - The following terms are used to described rock strength:

Rock Strength	Abbreviation	Point Load Strength
Class		Index, Is(50)
		(MPa)
Extremely Low	EL	< 0.03
Very Low	VL	0.03 to 0.1
Low	L	0.1 to 0.3
Medium	М	0.3 to 1
High	Н	1 to 3
Very High	VH	3 to 10
Extremely High	EH	≥ 10

Strengths are estimated and where possible supported by Point Load Index Testing of representative samples. Test results are plotted on the graphical estimated strength by using:

Axial Point Load Test

Where the estimated strength log covers more than one range it indicates the rock strength varies between the limits shown.

MATERIALS STRUCTURE/FRACTURES

ROCK

Natural Fracture Spacing - A plot of average fracture spacing excluding defects known or suspected to be due to drilling, core boxing or testing. Closed or cemented joints, drilling breaks and handling breaks are not included in the Natural Fracture Spacing.

Visual Log - A diagrammatic plot of defects showing type, spacing and orientation in relation to core axis.

Defects	 Defects open in-situ or clay sealed		
	 Defects closed in-situ		
	 Breaks through rock substance		

Additional Data - Description of individual defects by type, orientation, in-filling, shape and roughness in accordance with AS 1726-1993, Appendix A Table A10, notes and Figure A2.

Orientation - angle relative to the plane normal to the core axis.

Type BP Joint Joint SM Seam FZ Fracture Zone SZ Shear Zone VN Vein FL Foliation CL Cleavage DL Drill Lift HB Handling Break DB Drilling Break Clay Clay Clay Clay KT Chlorite CA Calcite Fe Iron Oxide Qz Quartz MS Secondary Mineral MU Unidentified Mineral Shape PR Planar CU Curved UN Undulose ST Stepped IR Irregular DIS Discontinuous Rough VR Very Rough							
SM Seam FZ Fracture Zone SZ Shear Zone VN Vein FL Foliation CL Cleavage DL Drill Lift HB Handling Break DB Drilling Break Infilling CN Clean X Carbonaceous Clay Clay KT Chlorite CA Calcite Fe Iron Oxide Qz Quartz MS Secondary Mineral MU Unidentified Mineral Shape PR Planar CU Curved UN Undulose ST Stepped IR Irregular DIS Discontinuous Rougness POL Polished S Smooth RF Rough	Туре	BP	Bedding Parting				
FZ Shear Zone SZ Shear Zone VN Vein FL Foliation CL Cleavage DL Drill Lift HB Handling Break DB Drilling Break Infilling CN Clean X Carbonaceous Clay Clay KT Chlorite CA Calcite Fe Iron Oxide Qz Quartz MS Secondary Mineral MU Unidentified Mineral Shape PR Planar CU Curved UN Undulose ST Stepped IR Irregular DIS Discontinuous Rougness POL Polished S Smooth RF Rough		JΤ	Joint				
SZ Shear Zone VN Vein FL Foliation CL Cleavage DL Drill Lift HB Handling Break DB Drilling Break Infilling CN Clean X Carbonaceous Clay Clay KT Chlorite CA Calcite Fe Iron Oxide Qz Quartz MS Secondary Mineral MU Unidentified Mineral Shape PR Planar CU Curved UN Undulose ST Stepped IR Irregular DIS Discontinuous Rougness POL Polished S Smooth RF Rough		SM	Seam				
VN Vein FL Foliation CL Cleavage DL Drill Lift HB Handling Break DB Drilling Break Infilling CN Clean X Carbonaceous Clay Clay KT Chlorite CA Calcite Fe Iron Oxide Qz Quartz MS Secondary Mineral MU Unidentified Mineral Shape PR Planar CU Curved UN Undulose ST Stepped IR Irregular DIS Discontinuous Rougness POL Polished S Smooth RF Rough		FZ	Fracture Zone				
FL CL Cleavage DL Drill Lift HB Handling Break DB Drilling Break Infilling CN Clean X Carbonaceous Clay Clay KT Chlorite CA Calcite Fe Iron Oxide Qz Quartz MS Secondary Mineral MU Unidentified Mineral Shape PR Planar CU Curved UN Undulose ST Stepped IR Irregular DIS Discontinuous Rougness POL Polished S Smooth RF Rough		SZ	Shear Zone				
CL Cleavage DL Drill Lift HB Handling Break DB Drilling Break Infilling CN Clean X Carbonaceous Clay Clay KT Chlorite CA Calcite Fe Iron Oxide Qz Quartz MS Secondary Mineral MU Unidentified Mineral Shape PR Planar CU Curved UN Undulose ST Stepped IR Irregular DIS Discontinuous Rougness POL Polished S Smooth RF Rough		VN	Vein				
DL Drill Lift HB Handling Break DB Drilling Break Infilling CN Clean X Carbonaceous Clay Clay KT Chlorite CA Calcite Fe Iron Oxide Qz Quartz MS Secondary Mineral MU Unidentified Mineral Shape PR Planar CU Curved UN Undulose ST Stepped IR Irregular DIS Discontinuous Rougness POL Polished S Smooth RF Rough		FL	Foliation				
HB DB Drilling Break DB Drilling Break Infilling CN Clean X Carbonaceous Clay Clay KT Chlorite CA Calcite Fe Iron Oxide Qz Quartz MS Secondary Mineral MU Unidentified Mineral Shape PR Planar CU Curved UN Undulose ST Stepped IR Irregular DIS Discontinuous Rougness POL Polished S Smooth RF Rough		CL	Cleavage				
DB Drilling Break Infilling CN Clean X Carbonaceous Clay Clay KT Chlorite CA Calcite Fe Iron Oxide Qz Quartz MS Secondary Mineral MU Unidentified Mineral Shape PR Planar CU Curved UN Undulose ST Stepped IR Irregular DIS Discontinuous Rougness POL Polished S Smooth RF Rough		DL	Drill Lift				
Infilling CN Clean X Carbonaceous Clay Clay KT Chlorite CA Calcite Fe Iron Oxide Qz Quartz MS Secondary Mineral MU Unidentified Mineral Shape PR Planar CU Curved UN Undulose ST Stepped IR Irregular DIS Discontinuous Rougness POL Polished SL Slickensided S Smooth RF Rough		НВ	Handling Break				
X Carbonaceous Clay Clay KT Chlorite CA Calcite Fe Iron Oxide Qz Quartz MS Secondary Mineral MU Unidentified Mineral Shape PR Planar CU Curved UN Undulose ST Stepped IR Irregular DIS Discontinuous Rougness POL Polished SL Slickensided S Smooth RF Rough		DB	Drilling Break				
Clay KT CA CA Calcite Fe Iron Oxide Qz Quartz MS Secondary Mineral MU Unidentified Mineral Shape PR Planar CU Curved UN Undulose ST Stepped IR Irregular DIS Discontinuous Rougness POL Polished SL Slickensided S Smooth RF Rough	Infilling	CN	Clean				
KT Chlorite CA Calcite Fe Iron Oxide Qz Quartz MS Secondary Mineral MU Unidentified Mineral Shape PR Planar CU Curved UN Undulose ST Stepped IR Irregular DIS Discontinuous Rougness POL Polished SL Slickensided S Smooth RF Rough		x	Carbonaceous				
CA Calcite Fe Iron Oxide Qz Quartz MS Secondary Mineral MU Unidentified Mineral Shape PR Planar CU Curved UN Undulose ST Stepped IR Irregular DIS Discontinuous Rougness POL Polished SL Slickensided S Smooth RF Rough		Clay	Clay				
Fe		кт	· ·				
Qz Quartz MS Secondary Mineral MU Unidentified Mineral Shape PR Planar CU Curved UN Undulose ST Stepped IR Irregular DIS Discontinuous Rougness POL Polished SL Slickensided S Smooth RF Rough		CA	Calcite				
MS Secondary Mineral MU Unidentified Mineral Shape PR Planar CU Curved UN Undulose ST Stepped IR Irregular DIS Discontinuous Rougness POL Polished SL Slickensided S Smooth RF Rough		Fe	Iron Oxide				
MU Unidentified Mineral Shape PR Planar CU Curved UN Undulose ST Stepped IR Irregular DIS Discontinuous Rougness POL Polished SL Slickensided S Smooth RF Rough		Qz	Quartz				
Shape PR CU Curved UN Undulose ST Stepped IR Irregular DIS Discontinuous Rougness POL SL SL Slickensided S Smooth RF Rough		MS	Secondary Mineral				
CU Curved UN Undulose ST Stepped IR Irregular DIS Discontinuous Rougness POL Polished SL Slickensided S Smooth RF Rough		MU	Unidentified Mineral				
UN Undulose ST Stepped IR Irregular DIS Discontinuous Rougness POL Polished SL Slickensided S Smooth RF Rough	Shape	PR	Planar				
ST Stepped IR Irregular DIS Discontinuous Rougness POL Polished SL Slickensided S Smooth RF Rough		CU	Curved				
Rougness POL Polished SL Slickensided S Smooth RF Rough		UN	Undulose				
Rougness POL Polished SL Slickensided S Smooth RF Rough		ST	Stepped				
Rougness POL Polished SL Slickensided S Smooth RF Rough		IR	Irregular				
SL Slickensided S Smooth RF Rough		DIS	Discontinuous				
S Smooth Rough	Rougness	POL	Polished				
RF Rough		SL	Slickensided				
		S	Smooth				
VR Very Rough		RF	Rough				
Voly Nough		VR	Very Rough				

SOIL

Structures - Fissuring and other defects are described in accordance with AS 1726-1993, Appendix A2.6, using the terminology for rock defects.

Origin - Where practicable an assessment is provided of the probable origin of the soil, eg fill, topsoil, alluvium, colluvium, residual soil.

[°] Diametral Point Load Test

WATER QUALITY LABORATORY TESTING CERTIFICATES



ANALYTICAL REPORT





CLIENT DETAILS

LABORATORY DETAILS

Laboratory

Andrew Butel Contact

MORROW GEOTECHNICS PTY LTD Client

Address PO BOX 4069

NSW 2218

Huong Crawford Manager

SGS Alexandria Environmental

Address Unit 16, 33 Maddox St

Alexandria NSW 2015

Telephone (Not specified) Facsimile (Not specified)

Email andy@morrowgeo.com.au

P3018 - Dee Why Project

P3018 Order Number 3 Samples

+61 2 8594 0400 Telephone Facsimile +61 2 8594 0499

Email au.environmental.sydney@sgs.com

SGS Reference SE255664 R0 25/10/2023 Date Received 1/11/2023 Date Reported

COMMENTS

Accredited for compliance with ISO/IEC 17025 - Testing. NATA accredited laboratory 2562(4354).

SIGNATORIES

Dong LIANG

Metals/Inorganics Team Leader

Kamrul AHSAN Senior Chemist

Lv Kim HA

Organic Section Head

Skinly

уэлиуэли гивиц

Ying Ying ZHANG

Laboratory Technician



SE255664 R0

VOCs in Water [AN433] Tested: 30/10/2023

			BH101	BH102	BH103
			WATER	WATER	WATER
			25/10/2023	25/10/2023	25/10/2023
PARAMETER	UOM	LOR	SE255664.001	SE255664.002	SE255664.003
Benzene	μg/L	0.5	<0.5	<0.5	<0.5
Toluene	μg/L	0.5	2.2	<0.5	<0.5
Ethylbenzene	μg/L	0.5	<0.5	<0.5	<0.5
m/p-xylene	μg/L	1	<1	<1	<1
o-xylene	μg/L	0.5	<0.5	<0.5	<0.5
Total Xylenes	μg/L	1.5	<1.5	<1.5	<1.5
Total BTEX	μg/L	3	<3	<3	<3
Naphthalene (VOC)*	μg/L	0.5	<0.5	<0.5	<0.5

1/11/2023 Page 2 of 15



SE255664 R0

Volatile Petroleum Hydrocarbons in Water [AN433] Tested: 30/10/2023

			BH101	BH102	BH103
			WATER	WATER	WATER
			- 25/10/2023	- 25/10/2023	- 25/10/2023
PARAMETER	UOM	LOR	SE255664.001	SE255664.002	SE255664.003
TRH C6-C9	μg/L	40	<40	<40	<40
Benzene (F0)	μg/L	0.5	<0.5	<0.5	<0.5
TRH C6-C10	μg/L	50	<50	<50	<50
TRH C6-C10 minus BTEX (F1)	μg/L	50	<50	<50	<50

1/11/2023 Page 3 of 15



SE255664 R0

TRH (Total Recoverable Hydrocarbons) in Water [AN403] Tested: 30/10/2023

			BH101	BH102	BH103
			WATER	WATER	WATER
			- 25/10/2023	- 25/10/2023	- 25/10/2023
PARAMETER	UOM	LOR	SE255664.001	SE255664.002	SE255664.003
TRH C10-C14	μg/L	50	87	240	<50
TRH C15-C28	μg/L	200	280	330	<200
TRH C29-C36	μg/L	200	<200	<200	<200
TRH C37-C40	μg/L	200	<200	<200	<200
TRH >C10-C16	μg/L	60	110	300	<60
TRH >C10-C16 - Naphthalene (F2)	μg/L	60	110	300	<60
TRH >C16-C34 (F3)	μg/L	500	<500	<500	<500
TRH >C34-C40 (F4)	μg/L	500	<500	<500	<500
TRH C10-C40	μg/L	320	450	650	<320

1/11/2023 Page 4 of 15





PAH (Polynuclear Aromatic Hydrocarbons) in Water [AN420] Tested: 30/10/2023

			BH101	BH102	BH103
			WATER	WATER	WATER
			- 25/10/2023	- 25/10/2023	- 25/10/2023
PARAMETER	UOM	LOR	SE255664.001	SE255664.002	SE255664.003
Naphthalene	μg/L	0.1	0.2	0.3	0.4
2-methylnaphthalene	μg/L	0.1	<0.1	<0.1	0.4
1-methylnaphthalene	μg/L	0.1	<0.1	<0.1	0.3
Acenaphthylene	μg/L	0.1	<0.1	<0.1	<0.1
Acenaphthene	μg/L	0.1	<0.1	<0.1	<0.1
Fluorene	μg/L	0.1	<0.1	<0.1	<0.1
Phenanthrene	μg/L	0.1	<0.1	<0.1	0.1
Anthracene	μg/L	0.1	<0.1	<0.1	<0.1
Fluoranthene	μg/L	0.1	<0.1	<0.1	<0.1
Pyrene	μg/L	0.1	<0.1	<0.1	<0.1
Benzo(a)anthracene	μg/L	0.1	<0.1	<0.1	<0.1
Chrysene	μg/L	0.1	<0.1	<0.1	<0.1
Benzo(b&j)fluoranthene	μg/L	0.1	<0.1	<0.1	<0.1
Benzo(k)fluoranthene	μg/L	0.1	<0.1	<0.1	<0.1
Benzo(a)pyrene	μg/L	0.1	<0.1	<0.1	<0.1
Indeno(1,2,3-cd)pyrene	μg/L	0.1	<0.1	<0.1	<0.1
Dibenzo(ah)anthracene	μg/L	0.1	<0.1	<0.1	<0.1
Benzo(ghi)perylene	μg/L	0.1	<0.1	<0.1	<0.1
Total PAH (18)	μg/L	1	<1	<1	1

1/11/2023 Page 5 of 15



SE255664 R0

Anions by Ion Chromatography in Water [AN245] Tested: 27/10/2023

			BH101	BH102	BH103
			WATER	WATER	WATER
			25/10/2023	25/10/2023	25/10/2023
PARAMETER	UOM	LOR	SE255664.001	SE255664.002	SE255664.003
Chloride	mg/L	1	74	89	97
Sulfate, SO4	mg/L	1	40	40	29

1/11/2023 Page 6 of 15



SE255664 R0

Alkalinity [AN135] Tested: 27/10/2023

			BH101	BH102	BH103
			WATER	WATER	WATER
			25/10/2023	25/10/2023	25/10/2023
PARAMETER	UOM	LOR	SE255664.001	SE255664.002	SE255664.003
Bicarbonate Alkalinity as CaCO3	mg/L	5	14	16	6
Carbonate Alkalinity as CaCO3	mg/L	1	<1	<1	<1
Hydroxide Alkalinity as CaCO3	mg/L	5	<5	<5	<5
Phenolphthalein Alkalinity as CaCO3*	mg/L	5	<5	<5	<5
Total Alkalinity as CaCO3	mg/L	5	14	16	6

1/11/2023 Page 7 of 15



SE255664 R0

Acidity and Free CO2 [AN140] Tested: 27/10/2023

			BH101	BH102	BH103
			WATER	WATER	WATER
			25/10/2023	25/10/2023	25/10/2023
PARAMETER	UOM	LOR	SE255664.001	SE255664.002	SE255664.003
Acidity to pH 8.3	mg CaCO3/L	5	110	130	110

1/11/2023 Page 8 of 15



SE255664 R0

pH in water [AN101] Tested: 25/10/2023

			BH101	BH102	BH103
			WATER	WATER	WATER
					-
			25/10/2023	25/10/2023	25/10/2023
PARAMETER	UOM	LOR	SE255664.001	SE255664.002	SE255664.003
pH**	No unit	-	5.1	5.2	4.7

1/11/2023 Page 9 of 15



SE255664 R0

Conductivity and TDS by Calculation - Water [AN106] Tested: 25/10/2023

			BH101	BH102	BH103
			WATER	WATER	WATER
			25/10/2023	25/10/2023	25/10/2023
PARAMETER	UOM	LOR	SE255664.001	SE255664.002	SE255664.003
Conductivity @ 25 C	μS/cm	2	350	410	390
Total Dissolved Solids (by calculation)	mg/L	2	210	240	240

1/11/2023 Page 10 of 15



SE255664 R0

Metals in Water (Dissolved) by ICPOES [AN320] Tested: 27/10/2023

			BH101	BH102	BH103
			WATER	WATER	WATER
			- 25/10/2023	- 25/10/2023	- 25/10/2023
PARAMETER	UOM	LOR	SE255664.001	SE255664.002	SE255664.003
Calcium, Ca	mg/L	0.1	1.5	4.5	2.4
Magnesium, Mg	mg/L	0.1	6.1	9.5	9.4
Sodium, Na	mg/L	0.1	43	64	49
Potassium, K	mg/L	0.2	1.1	1.1	0.8

1/11/2023 Page 11 of 15



SE255664 R0

Trace Metals (Dissolved) in Water by ICPMS [AN318] Tested: 26/10/2023

			BH101	BH102	BH103
			WATER	WATER	WATER
			- 25/10/2023	- 25/10/2023	- 25/10/2023
PARAMETER	UOM	LOR	SE255664.001	SE255664.002	SE255664.003
Arsenic	μg/L	1	<1	<1	<1
Cadmium	μg/L	0.1	<0.1	<0.1	<0.1
Chromium	μg/L	1	<1	2	<1
Copper	μg/L	1	3	<1	61
Lead	μg/L	1	<1	<1	<1
Nickel	μg/L	1	5	18	2
Zinc	μg/L	5	19	25	12

1/11/2023 Page 12 of 15



SE255664 R0

Mercury (dissolved) in Water [AN311(Perth)/AN312] Tested: 26/10/2023

			BH101	BH102	BH103
			WATER	WATER	WATER
					-
			25/10/2023	25/10/2023	25/10/2023
PARAMETER	UOM	LOR	SE255664.001	SE255664.002	SE255664.003
Mercury	mg/L	0.0001	<0.0001	<0.0001	<0.0001

1/11/2023 Page 13 of 15



on their retention time and

METHOD SUMMARY



AN101

AN106

AN140

AN245

AN318

AN320

AN403

AN403

AN403

AN420

METHOD _

ΔN020 Unpreserved water sample is filtered through a 0.45µm membrane filter and acidified with nitric acid similar to

APHA3030B

pH in Soil Sludge Sediment and Water: pH is measured electrometrically using a combination electrode (glass plus reference electrode) and is calibrated against 3 buffers purchased commercially. For soils, an extract with

μS/cm @ 25°C. For soils, an extract with water is made at a ratio of 1:5 and the EC determined and reported on the extract, or calculated back to the as-received sample. Total Dissolved Salts can be estimated from conductivity

water is made at a ratio of 1:5 and the pH determined and reported on the extract. Reference APHA 4500-H+.

Conductivity and TDS by Calculation: Conductivity is measured by meter with temperature compensation and is calibrated against a standard solution of potassium chloride. Conductivity is generally reported as µmhos/cm or

> using a conversion factor, which for natural waters, is in the range 0.55 to 0.75. SGS use 0.6. Reference APHA 2510 B.

AN106 Salinity may be calculated in terms of NaCl from the sample conductivity. This assumes all soluble salts present,

measured by the conductivity, are present as NaCl.

AN135 Alkalinity (and forms of) by Titration: The sample is titrated with standard acid to pH 8.3 (P titre) and pH 4.5 (T titre) and permanent and/or total alkalinity calculated. The results are expressed as equivalents of calcium carbonate or

recalculated as bicarbonate, carbonate and hydroxide. Reference APHA 2320. Internal Reference AN135

Acidity by Titration: The water sample is titrated with sodium hydroxide to designated pH end point. In a sample pH 8.3 at 25°C corresponds to containing only carbon dioxide, bicarbonates and carbonates, titration to

stoichiometric neutralisation of carbonic acid to bicarbonate. Method reference APHA 2310 B.

Anions by Ion Chromatography: A water sample is injected into an eluent stream that passes through the ion chromatographic system where the anions of interest ie Br, Cl, NO2, NO3 and SO4 are separated on their relative affinities for the active sites on the column packing material. Changes to the conductivity and the UV-visible

absorbance of the eluent enable identification and quantitation of the anions based peak height or area. APHA 4110 B

AN311(Perth)/AN312 Mercury by Cold Vapour AAS in Waters: Mercury ions are reduced by stannous chloride reagent in acidic solution to elemental mercury. This mercury vapour is purged by nitrogen into a cold cell in an atomic absorption spectrometer or mercury analyser. Quantification is made by comparing absorbances to those of the calibration

standards. Reference APHA 3112/3500.

Determination of elements at trace level in waters by ICP-MS technique,, referenced to USEPA 6020B and USEPA

200.8 (5.4).

AN320 Metals by ICP-OES: Samples are preserved with 10% nitric acid for a wide range of metals and some non-metals. This solution is measured by Inductively Coupled Plasma. Solutions are aspirated into an argon plasma at 8000-10000K and emit characteristic energy or light as a result of electron transitions through unique energy

levels. The emitted light is focused onto a diffraction grating where it is separated into components .

Photomultipliers or CCDs are used to measure the light intensity at specific wavelengths. This intensity is directly proportional to concentration. Corrections are required to compensate for spectral overlap between elements .

Reference APHA 3120 B.

Total Recoverable Hydrocarbons: Determination of Hydrocarbons by gas chromatography after a solvent

extraction. Detection is by flame ionisation detector (FID) that produces an electronic signal in proportion to the combustible matter passing through it. Total Recoverable Hydrocarbons (TRH) are routinely reported as four alkane groupings based on the carbon chain length of the compounds: C6-C9, C10-C14, C15-C28 and C29-C36 and in recognition of the NEPM 1999 (2013), >C10-C16 (F2), >C16-C34 (F3) and >C34-C40 (F4). Where F2 is

corrected for Naphthalene, the VOC data for Naphthalene is used.

Additionally, the volatile C6-C9/C6-C10 fractions may be determined by a purge and trap technique and GC/MS because of the potential for volatiles loss. Total Recoveerable Hydrocarbons - Silica (TRH-Silica) follows the same

method of analysis after silica gel cleanup of the solvent extract. Aliphatic/Aromatic Speciation follows the same method of analysis after fractionation of the solvent extract over silica with differential polarity of the eluent

solvents.

The GC/FID method is not well suited to the analysis of refined high boiling point materials (ie lubricating oils or greases) but is particularly suited for measuring diesel, kerosene and petrol if care to control volatility is taken. This method will detect naturally occurring hydrocarbons, lipids, animal fats, phenols and PAHs if they are present at sufficient levels, dependent on the use of specific cleanup/fractionation techniques. Reference USEPA 3510B,

8015B.

(SVOCs) including OC, OP, PCB, Herbicides, PAH, Phthalates and Speciated Phenols (etc) in soils, sediments and waters are determined by GCMS/ECD technique following appropriate solvent extraction process (Based on

USEPA 3500C and 8270D). Total PAH calculated from individual analyte detections at or above the limit of reporting.

VOCs and C6-C9 Hydrocarbons by GC-MS P&T: VOC's are volatile organic compounds. The sample is presented to a gas chromatograph via a purge and trap (P&T) concentrator and autosampler and is detected with a Mass Spectrometer (MSD). Solid samples are initially extracted with methanol whilst liquid samples are processed

directly. References: USEPA 5030B, 8020A, 8260.

1/11/2023

AN433

Page 14 of 15



METHOD SUMMARY

SE255664 R0

Calculation

Free and Total Carbon Dioxide may be calculated using alkalinity forms only when the samples TDS is <500mg/L. If TDS is >500mg/L free or total carbon dioxide cannot be reported . APHA4500CO2 D.

FOOTNOTES

* NATA accreditation does not cover the performance of this service.

** Indicative data, theoretical holding time exceeded.

*** Indicates that both * and ** apply.

Not analysed.NVL Not validated.

IS Insufficient sample for analysis.

LNR Sample listed, but not received.

UOM Unit of Measure.
LOR Limit of Reporting.
↑↓ Raised/lowered Limit of

Reporting.

Unless it is reported that sampling has been performed by SGS, the samples have been analysed as received. Solid samples expressed on a dry weight basis.

Where "Total" analyte groups are reported (for example, Total PAHs, Total OC Pesticides) the total will be calculated as the sum of the individual analytes, with those analytes that are reported as <LOR being assumed to be zero. The summed (Total) limit of reporting is calculated by summing the individual analyte LORs and dividing by two. For example, where 16 individual analytes are being summed and each has an LOR of 0.1 mg/kg, the "Totals" LOR will be 1.6 / 2 (0.8 mg/kg). Where only 2 analytes are being summed, the "Total" LOR will be the sum of those two LORs.

Some totals may not appear to add up because the total is rounded after adding up the raw values.

If reported, measurement uncertainty follow the ± sign after the analytical result and is expressed as the expanded uncertainty calculated using a coverage factor of 2, providing a level of confidence of approximately 95%, unless stated otherwise in the comments section of this report.

Results reported for samples tested under test methods with codes starting with ARS-SOP, radionuclide or gross radioactivity concentrations are expressed in becquerel (Bq) per unit of mass or volume or per wipe as stated on the report. Becquerel is the SI unit for activity and equals one nuclear transformation per second.

Note that in terms of units of radioactivity:

- a. 1 Bq is equivalent to 27 pCi
- b. 37 MBq is equivalent to 1 mCi

For results reported for samples tested under test methods with codes starting with ARS-SOP, less than (<) values indicate the detection limit for each radionuclide or parameter for the measurement system used. The respective detection limits have been calculated in accordance with ISO 11929.

The QC and MU criteria are subject to internal review according to the SGS QAQC plan and may be provided on request or alternatively can be found here; www.sgs.com.au/en-gb/environment-health-and-safety.

This document is issued by the Company under its General Conditions of Service accessible at www.sgs.com/en/Terms-and-Conditions.aspx.

Attention is drawn to the limitation of liability, indemnification and jurisdiction issues defined therein.

Any holder of this document is advised that information contained hereon reflects the Company's findings at the time of its intervention only and within the limits of Client's instructions, if any. The Company's sole responsibility is to its Client only. Any unauthorized alteration, forgery or falsification of the content or appearance of this document is unlawful and offenders may be prosecuted to the fullest extent of the law.

This report must not be reproduced, except in full.

1/11/2023 Page 15 of 15

RISING HEAD PERMEABILITY SHEETS

Rising Head Permeability Results



79/6 Bellambi Lane, Bellambi NSW 2518 P: 0405 843 933 | E: info@morrowgeo.com.au

Project Number P3018

Address 4 Delmar Parade, Dee Why
Borehole BH101

Monitoring Date 25/10/2023

5.41 Static water level (mBGL) Internal Diameter (D) (m) 0.07 12.95 Length of Standpipe below Ground Level (m) 0.00 Height of Water above Ground Level (m) 0.00 Length of Standpipe above Ground Level (m) 5.41 Water level at start of test (mBGL) 6.18 Top of Response Zone (mBGL) Bottom of Response Zone (mBGL) 12.95 Time (t1) (s) 38.401
Time (t2) (s) 4641.4

Initial Head (H1) at (t1)

Final Head (H2) at (t2)

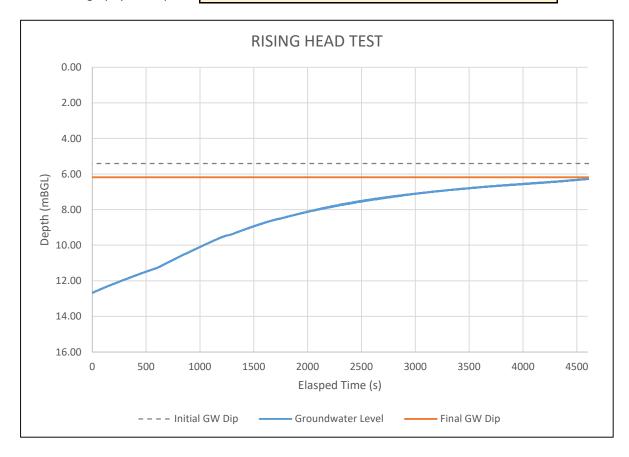
Length of Response Zone (L)

Cross Sectional Area (A)

0.0038

Stratigraphy Description:

Fill and Alluvial Sand over Sandstone Bedrock



Intake Factor (F) Case D
$$\frac{2pL}{\log_e [(L/D) + \ddot{O}(1 + (L/D)^2)]} = \frac{42.54}{5.26} = 8.08$$

Permeability (k) A $\log_e \frac{H1}{H2} = 7.23E-08 \text{ m/s}$

Rising Head Test Method 1 (after Hvorslev)

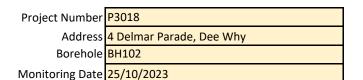
Formulae for borehole permeability tests(21.4.6) BS5930: 1981

Calculation by: AB Checked by: AM Date: 27/10/2023

Rising Head Permeability Results



79/6 Bellambi Lane, Bellambi NSW 2518 P: 0405 843 933 | E: info@morrowgeo.com.au

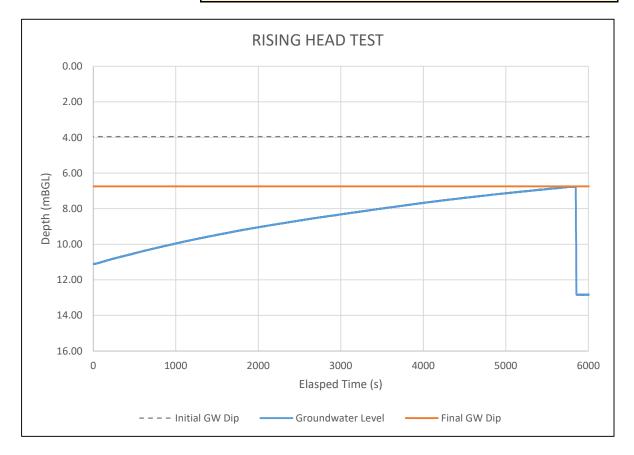


Length of Standpipe below Ground Level (m) Height of Water above Ground Level (m)	2.72
Height of Water above Ground Level (m)	2.72
()	
Length of Standpipe above Ground Level (m)	.00
	.00
Water level at start of test (mBGL)	.95
Top of Response Zone (mBGL)	5.74
Bottom of Response Zone (mBGL) 1	

Time (t1) (s)	51
Time (t2) (s)	5485
Initial Head (H1) at (t1)	11.06
Final Head (H2) at (t2)	6.90
Length of Response Zone (L)	5.98
Cross Sectional Area (A)	0.0038

Stratigraphy Description:

Fill, Alluvial and Residual Soil over Sandstone/Shale bedrock



Intake Factor (F) Case D
$$\frac{2pL}{\log_{e} [(L/D) + \ddot{O}(1 + (L/D)^{2})]} = \frac{37.57}{5.14} = 7.31$$
Permeability (k) A
$$\log_{e} \frac{H1}{F \text{ (t2-t1)}} = 4.57E-08 \text{ m/s}$$

Rising Head Test Method 1 (after Hvorslev)

Formulae for borehole permeability tests(21.4.6) BS5930: 1981

Calculation by: AB Checked by: AM Date: 27/10/2023

Rising Head Permeability Results



79/6 Bellambi Lane, Bellambi NSW 2518 P: 0405 843 933 | E: info@morrowgeo.com.au

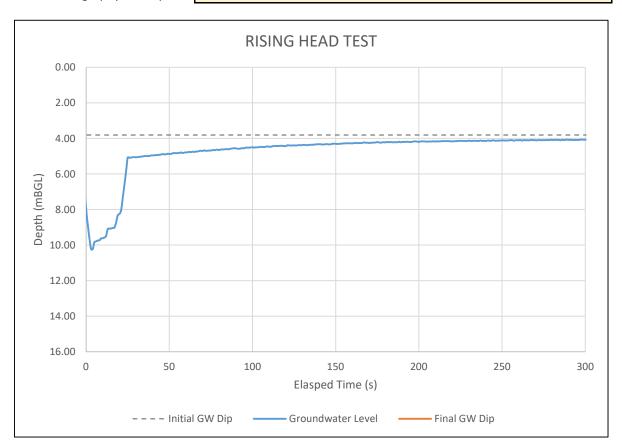
Project Number	P3018
Address	4 Delmar Parade, Dee Why
Borehole	BH103
Monitoring Date	25/10/2023

Static water level (mBGL)	3.81
Internal Diameter (D) (m)	0.07
Length of Standpipe below Ground Level (m)	0.00
Height of Water above Ground Level (m)	0.00
Length of Standpipe above Ground Level (m)	0.00
Water level at start of test (mBGL)	3.81
Top of Response Zone (mBGL)	3.81
Bottom of Response Zone (mBGL)	5.06

Time (t1) (s)	20
Time (t2) (s)	26
Initial Head (H1) at (t1)	8.26
Final Head (H2) at (t2)	5.08
Length of Response Zone (L)	1.25

Cross Sectional Area (A) 0.0038

Stratigraphy Description: Fill and Alluvial Soil over Sandstone and Shale bedrock



Intake Factor (F) Case D
$$\frac{2pL}{\log_{e} [(L/D) + \ddot{O}(1 + (L/D)^{2})]} = \frac{7.85}{3.58} = 2.20$$

Permeability (k) A $\log_{e} \frac{H1}{H2} = 1.42E-04 \text{ m/s}$

Rising Head Test Method 1 (after Hvorslev)

Formulae for borehole permeability tests(21.4.6) BS5930: 1981

Calculation by: AB Checked by: AM Date: 27/10/2023

IMPORTANT INFORMATION



This Document has been provided by Morrow Geotechnics Pty Ltd subject to the following limitations:

This Document has been prepared for the particular purpose outlined in Morrow Geotechnics' proposal and no responsibility is accepted for the use of this Document, in whole or in part, in other contexts or for any other purpose.

The scope and the period of Morrow Geotechnics' Services are as described in Morrow Geotechnics' proposal, and are subject to restrictions and limitations. Morrow Geotechnics did not perform a complete assessment of all possible conditions or circumstances that may exist at the site referenced in the Document. The scope of services may have been limited by such factors as time, budget, site access or other site conditions. If a service is not expressly indicated, do not assume it has been provided. If a matter is not addressed, do not assume that any determination has been made by Morrow Geotechnics in regards to it. Any advice given within this document is limited to geotechnical considerations only. Other constraints particular to the project, including but not limited to architectural, environment, heritage and planning matters may apply and should be assessed independently of this advice.

Conditions may exist which were undetectable given the limited nature of the enquiry Morrow Geotechnics was retained to undertake with respect to the site. Variations in conditions may occur between investigatory locations, and there may be special conditions pertaining to the site which have not been revealed by the investigation and which have not therefore been taken into account in the Document. Accordingly, additional studies and actions may be required. No geotechnical investigation can provide a full understanding of all possible subsurface details and anomalies at a site.

In addition, it is recognised that the passage of time affects the information and assessment provided in this Document. Morrow Geotechnics' opinions are based upon information that existed at the time of the production of the Document. It is understood that the Services provided allowed Morrow Geotechnics to form no more than an opinion of the actual conditions of the site at the time the site was visited and cannot be used to assess the effect of any subsequent changes in the quality of the site, or its surroundings, or any laws or regulations.

Any assessments made in this Document are based on the conditions indicated from published sources and the investigation described. No warranty is included, either express or implied, that the actual conditions will conform exactly to the assessments contained in this Document.

Where data supplied by the client or other external sources, including previous site investigation data, have been used, it has been assumed that the information is correct unless otherwise stated. No responsibility is accepted by Morrow Geotechnics for incomplete or inaccurate data supplied by others.

Where ground conditions encountered at the site differ significantly from those anticipated in the report, either due to natural variability of subsurface conditions or construction activities, it is a condition of the report that Morrow Geotechnics be notified of any variations and be provided with an opportunity to review the recommendations of this report.

This Document is provided for sole use by the Client and is confidential to it and its professional advisers. No responsibility whatsoever for the contents of this Document will be accepted to any person other than the Client. Any use which a third party makes of this Document, or any reliance on or decisions to be made based on it, is the responsibility of such third parties. Morrow Geotechnics accepts no responsibility for damages, if any, suffered by any third party as a result of decisions made or actions based on this Document.