PROPOSED SUBDIVISION No.12-14 GLADYS AVENUE, FRENCHS FOREST STORMWATER MANAGEMENT CONCEPT PLAN



LOCATION PLAN

	DRAWING REGISTER	
DRAWING NO.	TITLE	REVISION
DA-SW100	COVERSHEET	6
DA-SW200	STORMWATER MANAGEMENT CONCEPT PLAN - GROUND FLOOR	6
DA-SW201	WSUD CATCHMENT PLAN & DETAILS	6
DA-SW300	STORMWATER DETAILS SHEET	6
DA-SW500	HGL ANALYSIS & EASEMENT PIT CONNECTION DETAIL	6
DA-SW501	DRAINS MODEL DATA & RESULTS - 1	6
DA-SW502	DRAINS MODEL DATA & RESULTS - 2	6
DA-SW600	EROSION AND SEDIMENT CONTROL PLAN & DETAILS	6

GENERAL NOTES

- THESE PLANS SHALL BE READ IN CONJUNCTION WITH OTHER RELEVANT CONSULTANTS' PLANS, SPECIFICATIONS, CONDITIONS OF DEVELOPMENT CONSENT AND CONSTRUCTION CERTIFICATE REQUIREMENTS. WHERE DISCREPANCIES ARE FOUND JCO CONSULTANTS MUST BE CONTACTED IMMEDIATELY FOR VERIFICATION
- WHERE THESE PLANS ARE NOTED FOR DEVELOPMENT APPLICATION PURPOSES ONLY, THEY SHALL NOT BE USED FOR OBTAINING
- SUBSOIL DRAINAGE SHALL BE DESIGNED AND DETAILED BY THE STRUCTURAL ENGINEER. SUBSOIL DRAINAGE SHALL NOT BE CONNECTED INTO THE STORMWATER SYSTEM IDENTIFIED ON THESE PLANS UNLESS

STORMWATER CONSTRUCTION NOTES

- ALL WORK SHALL BE CARRIED OUT IN ACCORDANCE WITH AS/NZS 3500 (CURRENT EDITION) AND THE REQUIREMENTS OF THE LOCAL COUNCIL'S POLICIES AND CODES
- THE MINIMUM SIZES OF THE STORMWATER DRAINS SHALL NOT BE LESS THAN DN90 FOR CLASS 1 BUILDINGS AND DN100 FOR OTHER CLASSES OF BUILDING OR AS REQUIRED BY THE REGULATORY
- THE MINIMUM GRADIENT OF STORMWATER DRAINS SHALL BE 1%, UNLESS NOTED OTHERWISE
- 4. COUNCIL'S TREE PRESERVATION ORDER IS TO BE STRICTLY ADHERED TO, NO TREES SHALL BE REMOVED UNTIL PERMIT IS
- PUBLIC UTILITY SERVICES ARE TO BE ADJUSTED AS NECESSARY AT THE CLIENT'S EXPENSE
- 6. ALL PITS TO BE BENCHED AND STREAMLINED. PROVIDE STEP IRONS FOR ALL PITS OVER 1.2m DEEP
- MAKE SMOOTH JUNCTION WITH ALL EXISTING WORK
- VEHICULAR ACCESS AND ALL SERVICES TO BE MAINTAINED AT ALL TIMES TO ADJOINING PROPERTIES AFFECTED BY CONSTRUCTION
- SERVICES SHOWN ON THESE PLANS HAVE BEEN LOCATED FROM INFORMATION SUPPLIED BY THE RELEVANT AUTHORITIES AND FIELD INVESTIGATIONS AND ARE NOT GUARANTEED COMPLETE NOR CORRECT. IT IS THE CLIENT & CONTRACTOR'S RESPONSIBILITY TO LOCATE ALL PRIOR TO CONSTRUCTION
- 10. ANY VARIATION TO THE WORKS AS SHOWN ON THE APPROVED DRAWINGS ARE TO BE CONFIRMED BY JCO CONSULTANTS PRIOR TO

RAINWATER RE-USE SYSTEM NOTES

- RAINWATER SUPPLY PLUMBING TO BE CONNECTED TO OUTLETS WHERE REQUIRED BY BASIX CERTIFICATE (BY OTHERS)
- TOWN WATER CONNECTION TO RAINWATER TANK TO BE TO THE SATISFACTION OF THE REGULATORY AUTHORITY. THIS MAY REQUIRE PROVISION OF: PERMANENT AIR GAP
- BACKFLOW PREVENTION DEVICE
- NO DIRECT CONNECTION BETWEEN TOWN WATER SUPPLY AND THE RAIN WATER SUPPLY
- AN APPROVED STOP VALVE AND/OR PRESSURE LIMITING VALVE AT THE RAINWATER TANK PROVIDE APPROPRIATE FLOAT VALVES AND/OR SOLENOID
- TO CONTROL TOWN WATER SUPPLY INLET TO TANK IN ORDER TO
- ACHIEVE THE TOP-UP INDICATED ON THE TYPICAL DETAIL
- ALL PLUMBING WORKS ARE TO BE CARRIED OUT BY LICENSED PLUMBERS IN ACCORDANCE WITH AS/NZS3500.1 NATIONAL PLUMBING AND DRAINAGE CODE PRESSURE PUMP ELECTRICAL CONNECTION TO BE CARRIED OUT BY A LICENSED
- WATER INLETS ARE NOT TO BE CONNECTED PIPE MATERIALS FOR RAINWATER SUPPLY PLUMBING ARE TO BE APPROVED MATERIALS TO AS/NZS3500 PART 1 SECTION 2 AND TO BE CLEARLY AND PERMANENTLY IDENTIFIED AS 'RAINWATER'
- BE ACHIEVED FOR BELOW GROUND PIPES USING IDENTIFICATION TAPE (MADE IN ACCORDANCE WITH AS2648) OR FOR ABOVE GROUND PIPES BY USING ADHESIVE PIPE MARKERS (MADE IN ACCORDANCE WITH AS1345)
- 12. EVERY RAINWATER SUPPLY OUTLET POINT AND THE RAINWATER TANK ARE TO BE LABELED 'RAINWATER' ON A METALLIC SIGN IN ACCORDANCE WITH AS1319
- 13. ALL INLETS AND OUTLETS TO THE RAINWATER TANK ARE TO HAVE SUITABLE MEASURES PROVIDED TO PREVENT MOSQUITO AND VERMIN ENTRY

DIAL BEFORE YOU DIG



IMPORTANT: THE CONTRACTOR IS TO MAINTAIN A CURRENT SET OF "DIAL BEFORE YOU DIG" DRAWINGS ON SITE AT ALL TIMES.

PIT SIZES AND DESIGN:

DEPTH (mm)	MINIMUM PIT SIZE (mm)
UP TO 450mm	450 x 450
450mm TO to 600mm	600 x 600
600mm TO 900mm	600 x 900
900mm TO 1500mm	900 x 900 (WITH STEP IRONS)
1500mm TO 2000mm	1200 x 1200 (WITH STEP IRONS)

ALL PIPES SHOULD BE CUT FLUSH WITH THE WALL OF THE PIT.

PITS GREATER THAN 600mm DEEP SHALL HAVE A MINIMUM ACCESS OPENING OF 600 x 600mm

THE GRATED COVERS OF PITS LARGER THAN 600 x 600mm ARE TO BE HINGED TO PREVENT THE GRATE FROM FALLING INTO THE PIT.

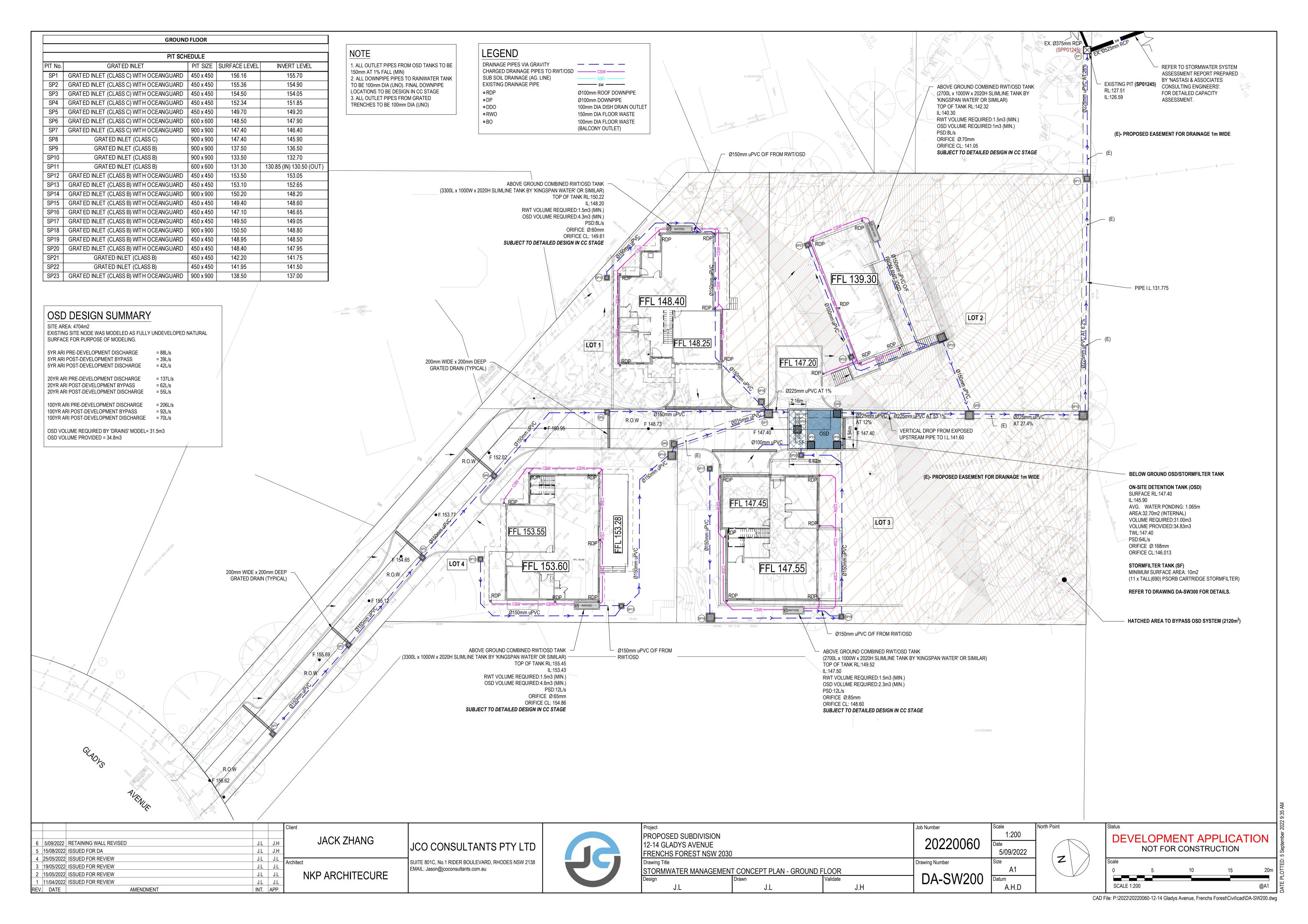
THE BASE OF THE DRAINAGE PITS SHOULD BE AT THE SAME LEVEL AS THE INVERT OF THE OUTLET PIPE. RAINWATER SHOULD NOT BE PERMITTED TO POND WITHIN THE STORMWATER SYSTEM

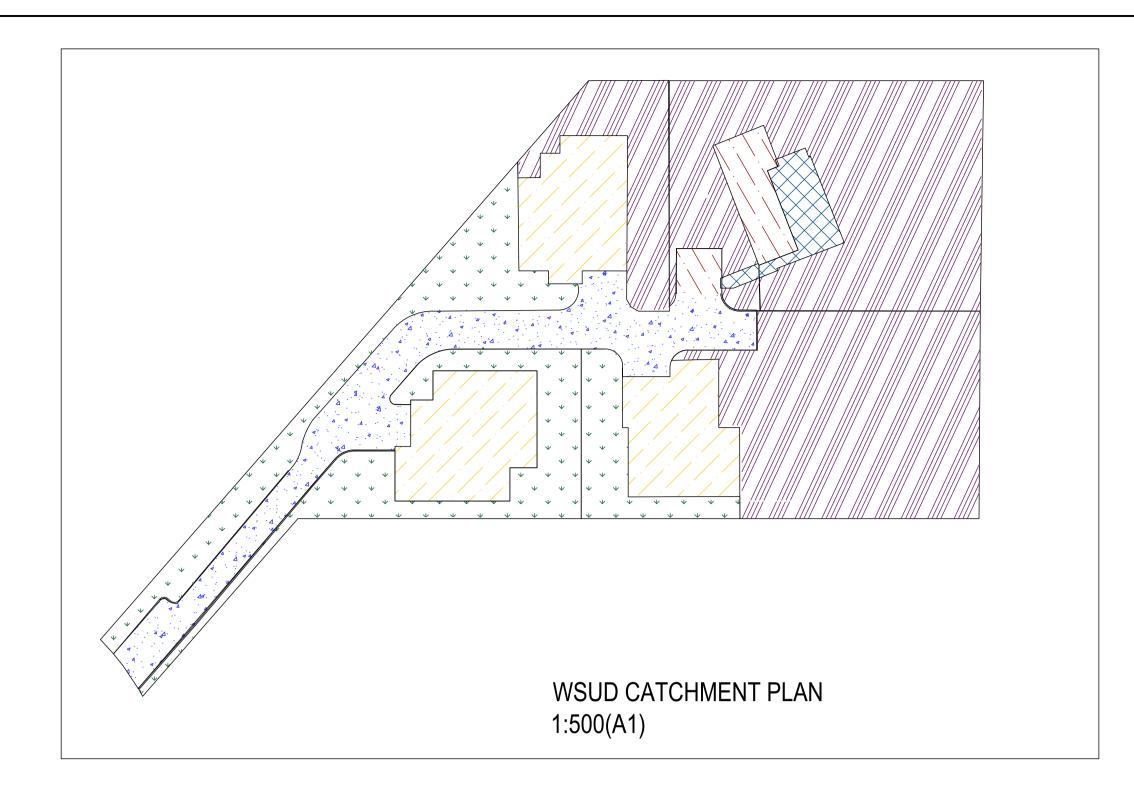
- CONTINUOUS TRENCH DRAINS ARE TO BE OF WIDTH NOT LESS THAN 150mm AND DEPTH NOT LESS THAN 100mm. THE BARS OF THE GRATING ARE TO BE PARALLEL TO THE DIRECTION OF SURFACE FLOW.
- PITS BETWEEN 1.2m AND 6m ARE TO HAVE STEP IRONS IN ACCORDANCE WITH AS1657. FOR PITS GREATER THAN 6m OTHER MEANS OF ACCESS MUST BE PROVIDED.
- PVC PITS WILL ONLY BE PERMITTED IF THEY ARE NOT A GREATER SIZE THAN 450 x 450mm (MAXIMUM DEPTH 450mm) AND ARE HEAVY DUTY
- IN-SITU PITS ARE TO BE CONSTRUCTED ON A CONCRETE BED OF AT
- LEAST 150mm THICK. THE WALLS ARE TO BE DESIGNED TO MEET THE MINIMUM REQUIREMENTS OF CLAUSE 7.5.5.1 OF AS3500.3-2018. PITS DEEPER THAN 1.8m SHALL BE CONSTRUCTED WITH REINFORCED
- GRATES:
- GRATES ARE TO BE GALVANISED STEEL GRID TYPE. GRATES ARE TO BE OF HEAVY-DUTY TYPE IN AREAS WHERE THEY MAY BE SUBJECT TO VEHICLE LOADING.

				Client	
				14 01/ 71 14 14 0	
6	5/09/2022 RETAINING WALL REVISED	J.L	J.H	JACK ZHANG	JCO CONSULTANTS PTY LTI
5	15/08/2022 ISSUED FOR DA	J.L	J.H		JCO CONSULTANTS PTT LTI
4	25/05/2022 ISSUED FOR REVIEW	J.L	J.L	Architect	CHITE 901C No.1 DIDER DOLLLEVADD, DHODEC NCW 21
3	19/05/2022 ISSUED FOR REVIEW	J.L	J.L		SUITE 801C, No.1 RIDER BOULEVARD, RHODES NSW 21 EMAIL: Jason@jcoconsultants.com.au
2	15/05/2022 ISSUED FOR REVIEW	J.L	J.L	NKP ARCHITECURE	EMAIL. Jason@jcoconsultants.com.au
1	11/04/2022 ISSUED FOR REVIEW	J.L	J.L	Title 7 (KOTITEOOTE	
REV.	DATE AMENDMENT	INT.	APP.		



									2 9:35
Project			Job Number	Scale	North Point	Status			202
PROPOSED SUBDIVISION 12-14 GLADYS AVENUE FRENCHS FOREST NSW 2030			20220060	NTS Date 5/09/2022		DEV	ELOPMENT NOT FOR CON	APPLICATIC ISTRUCTION	S September
Drawing Title			Drawing Number	Size	I \	Scale			
COVERSHEET			D A O O A / 4 O O	A1		0 0.0	0.04	0.06 0.08	0.1m
Design	Drawn	Validate	DA-SW100	Datum		224544			
J.L	J.L	J.H		A.H.D		SCALE 1:1			@A1 BA







STORMWATER TREATMENT SUMMARY

SITE AREA = 4706m²

WE MODELLED WITH FOLLOWING PARAMETERS:

- MUSIC VERSION 6.3.0
- RAINFALL STATION 066037 SYDNEY AIRPORT, 6 MINUTE TIME STEP FROM 1979 TO 1988
- SYDNEY CATCHMENT MANAGEMENT AUTHORITY (CMA) UTILIZING MODIFIED % IMPERVIOUS AREA, RAINFALL THRESHOLD, SOIL PROPERTIES & POLLUTANT CONCENTRATION
- NO DRAINAGE ROUTING BETWEEN NODES.

WE HAVE MODELLED THE SYSTEMS TO MEET CURRENT NORTHERN BEACHES COUNCIL WATER

- MANAGEMENT FOR DEVELOPMENT POLICY. THESE ARE: 85% TOTAL SUSPENDED SOLIDS REDUCTION
- 65% TOTAL PHOSPHORUS REDUCTION
- 45% TOTAL NITROGEN REDUCTION
- 90% GROSS POLLUTANT REDUCTION

THE SYSTEM HAS BEEN MODELLED TO MEET THE NORTHERN BEACHES COUNCIL DCP TARGET

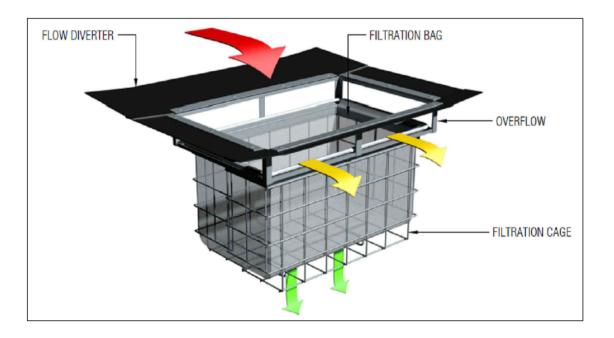
- 85% TOTAL SUSPENDED SOLIDS REDUCTION
- 70% TOTAL PHOSPHORUS REDUCTION
- 50% TOTAL NITROGEN REDUCTION 100% GROSS POLLUTANTS REDUCTION

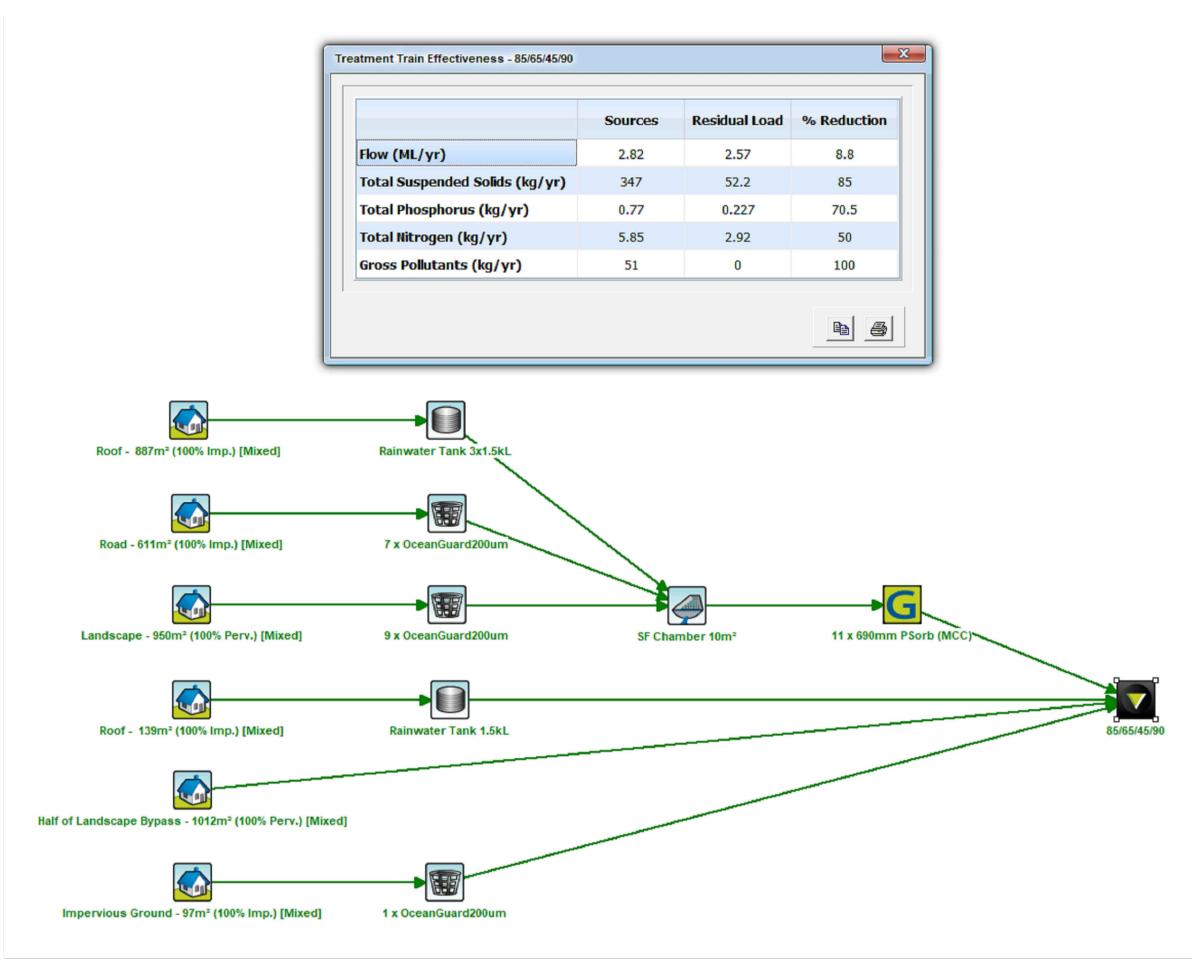
AMENDMENT

TREATMENT DEVICES:

REV. DATE

- 1. 4 x 1,500L OF RAINWATER TANK CONNECTED TO ALL TOILETS AND AT LEAST 1 OUTDOOR TAB FOR IRRIGATION
- 2. 17 x OCEANGUARDS WITH 200um MESH BAGS (OG-200).
- 3. 11 x TALL(690) PSORB CARTRIDGE STORMFILTER SYSTEM WITHIN A 10m² STORMFILTER





'MUSIC' MODELLING RESULT

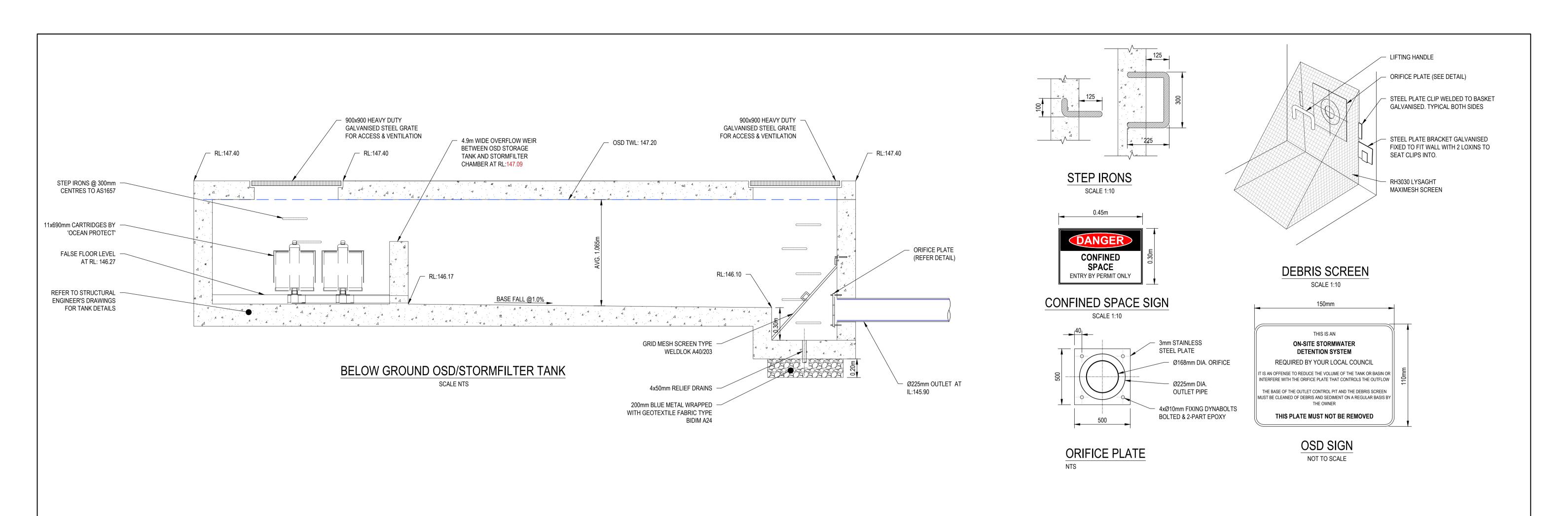
	ст	ORMFILTE	ER DESI	GN TARI I	=			GENERAL NOTES
THE SIZE 4.5 x 2.1m STORI REGION SPECIFIC INTERNAL SPECIFIED STRUCTURE(S) P INTERNAL ASSEMBLY BY OC	MFILTER TREATME FLOW CONTROLS ER CIVIL ENGINEE	ENT CAPACITY . · THE STAND R WILL BE SHO	VARIES BY N ARD CONFIG DWN ON SUE	IUMBER OF FIL	TER CARTRIDG	CONFIGURATION	ON OF THE	I. INLET AND OUTLET PIPING SHALL BE SPECIFIED BY SITE CIVIL ENGINEER (SEE PLANS) AND PROVIDED BY CONTRACTOR. STORMFILTER IS PROVIDED WITH OPENINGS AT INLET AND OUTLET LOCATIONS. 2. IF THE PEAK FLOW RATE, AS DETERMINED BY THE SITE CIVIL ENGINEER, EXCEEDS THE PEAK HYDRAULIC CAPACITY OF THE PRODUCT, AN UPSTREAM BYPASS STRUCTUR IS REQUIRED. PLEASE CONTACT OCEANPROTECT FOR OPTIONS. 3. THE FILTER CARTRIDGE(S) ARE SIPHON-ACTUATED AND SELF-CLEANING. THE STANDARD DETAIL DRAWING SHOWS THE MAXIMUM NUMBER OF CARTRIDGES. THE ACTUAL NUMBER SHALL BE SPECIFIED BY THE SITE CIVIL ENGINEER ON SITE PLANS OR IN DATA TABLE BELOW. CONCRETE STRUCTURE TO BE
ARTRIDGE HEIGHT		690			60	1 2	10	PROVIDED BY OTHERS. 4. SEE STORMFILTER DESIGN TABLE FOR REQUIRED HYDRAULIC DROP. FOR SHALLOW, LOW DROP OF
STEM HYDRAULIC DROP (H	- REO'D MIN)	930			700		50	SPECIAL DESIGN CONSTRAINTS, CONTACT OCEANPROTECT FOR DESIGN OPTIONS. 5. ALL WATER QUALITY PRODUCTS REQUIRE PERIODIC MAINTENANCE AS OUTLINED IN THE O&M GUIDELINES. PROVIDE MINIMUM CLEARANCE FOR
REATMENT BY MEDIA SURFACE		1.4	0.7	1.4	0.7	1.4	0.7	MAINTENANCE ACCESS, 6. STRUCTURE AND ACCESS COVERS DESIGNED BY OTHERS, ACCESS COVERS TO BE A MINIMUM
ARTRIDGE FLOW RATE (L/s)		1.42	0.71	0.95	0.47	0.63	0.32	900X900 ABOVE CARTRIDGES. 7. THE STRUCTURE THICKNESSES SHOWN ARE FOR REPRESENTATIONAL PURPOSES AND VARY REGIONALLY. 8. ANY BACKFILL DEPTH, SUB-BASE, AND OR ANTI-FLOTATION PROVISIONS ARE SITE-SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE SPECIFIED BY SITE CIVIL ENGINEER. 9. CARTRIDGE HEIGHT AND ASSOCIATED DESIGN PARAMETERS PER STORMFILTER DESIGN TABLE. 10. STORMFILTER BY OCEANPROTECT: SYDNEY (AU) PHONE: 1300 354 722
DIANID		1 144	TABLIA DIT DI A	NI DIMENOIONO				www.oceanprotect.com.au
PLAN ID		MAX		N DIMENSIONS				
S			450mm x					
M			600mm x					
L			900mm x					
XL			1200mm x	1200mm				
DEPTH ID	BAG [DEPTH		OVERALL DEPTH 270				
2 3 S M L XL	30	DEPTI 2		450 700				STORMFILTER CARTRIDGE FILTRATION UNIT FALSE FLOOR
THE MINIMUM CLEARANCE DEPENDS OF CLEARANCE FOR ANY PIT WITHOUT AN RECOMMENDED CLEARANCE SHOULD OF CLEARANCE SHOULD OF THE BAG FOR TARGETING GROSS POLLUTARY DRAWINGS NOT TO SCALE.	I INLET PIPE (ONLY USED F BE GREATER OR EQUAL T RATION BAG TYPES:- 200 N	FOR SURFACE FLOW O THE PIPE OBVERT) CAN BE AS LOW SO AS NOT TO IN	AS 50mm. FOR OTHI HIBIT HYDRAULIC CA	APACITY.			STORMFILTER CARTRIDGE DETAIL NTS

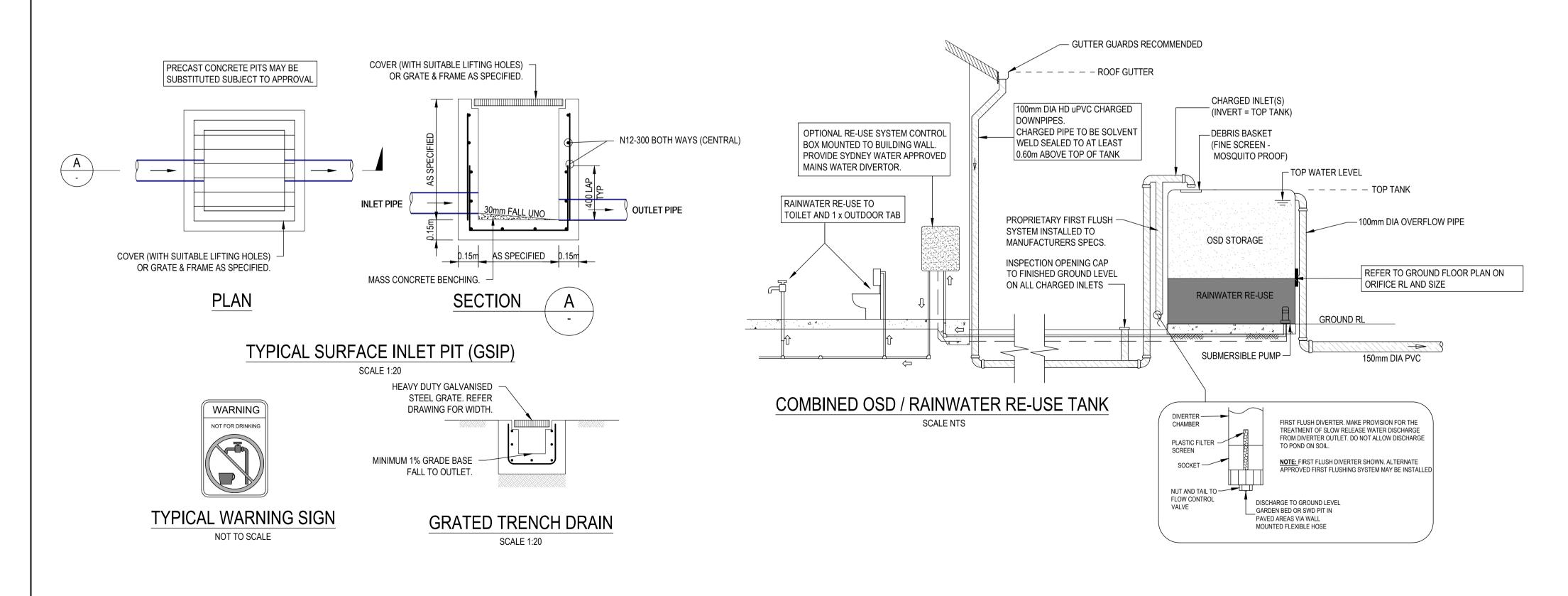
				Client	
L					
L	6 5/09/2022 RETAINING WALL REVISED	J.L	J.H	JACK ZHANG	JCO CONSULTANTS PTY LTD
L	5 15/08/2022 ISSUED FOR DA	J.L	J.H		1300 CONSULTANTS FIT LIL
	4 25/05/2022 ISSUED FOR REVIEW	J.L	J.L	Architect	SUITE 801C, No.1 RIDER BOULEVARD, RHODES NSW 2138
	3 19/05/2022 ISSUED FOR REVIEW	J.L	J.L		EMAIL: Jason@jcoconsultants.com.au
L	2 15/05/2022 ISSUED FOR REVIEW	J.L	J.L	NKP ARCHITECURE	Livin II. Subonic Joseph Culturitio. Softman
	1 11/04/2022 ISSUED FOR REVIEW	J.L	J.L		

INT. APP.

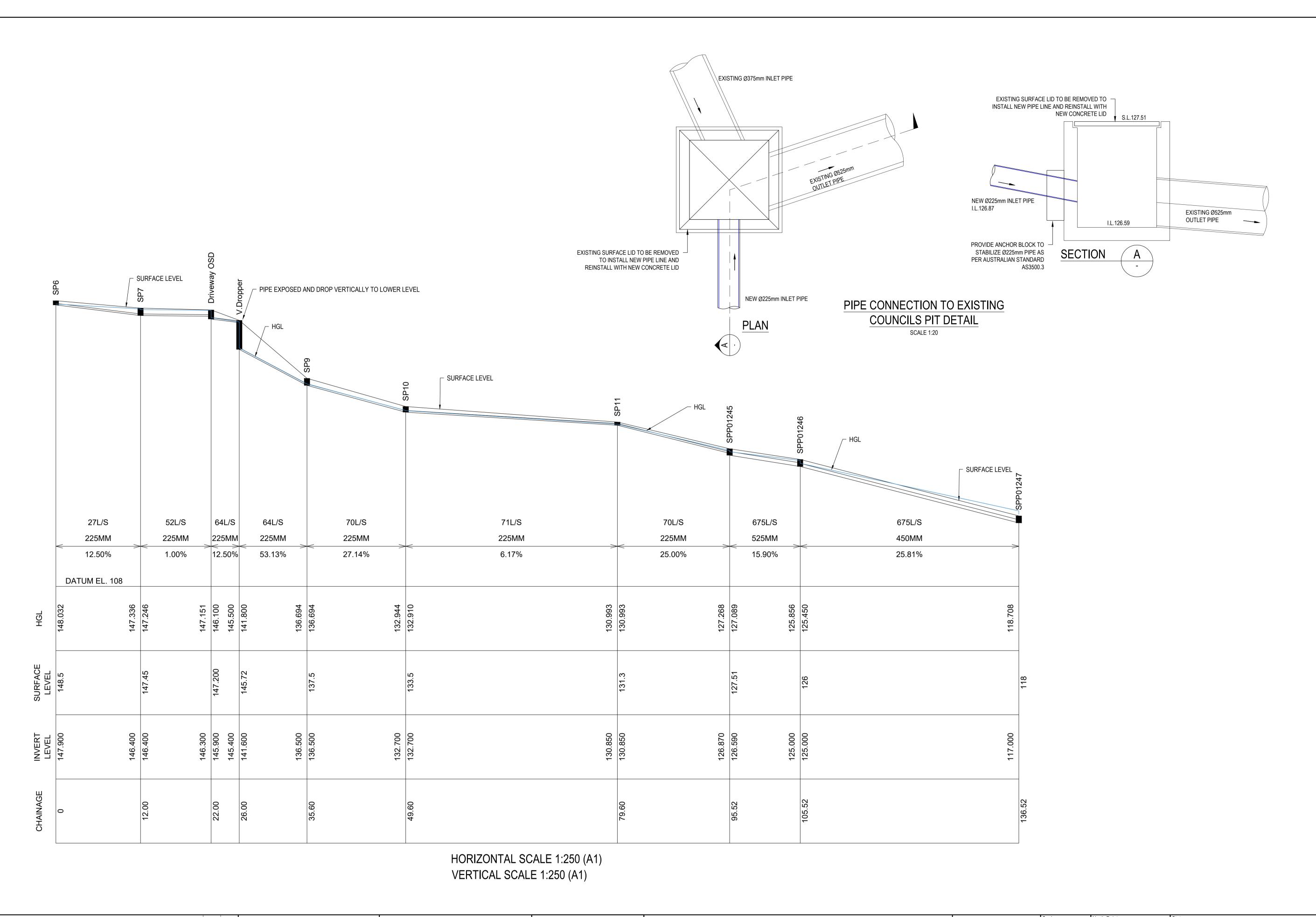


						2 9:35 /
roject	Job Number	Scale	North Point	Status		202
ROPOSED SUBDIVISION	20220060	1:500		DEVELOPM	ENT APPLICATION	TION B
2-14 GLADYS AVENUE RENCHS FOREST NSW 2030	20220000	Date 5/09/2022		NOT FOR	R CONSTRUCTION	5 Sept
rawing Title	Drawing Number	Size	1 (Z / /)	Scale		
VSUD CATCHMENT PLAN & DETAILS	D 4 014/00/	A1		0 10	20 30 40	50m
esign Validate	DA-SW201	Datum				
J.L J.H		A.H.D		SCALE 1:500		@A1 DATE





6 5/09/2022 RETAINING WALL REVISED 5 15/08/2022 ISSUED FOR DA 4 25/05/2022 ISSUED FOR REVIEW 3 19/05/2022 ISSUED FOR REVIEW 2 15/05/2022 ISSUED FOR REVIEW 1 11/04/2022 ISSUED FOR REVIEW REV. DATE AMENDMENT	J.L J.L Architect	JCO CONSULTANTS PTY LTD SUITE 801C, No.1 RIDER BOULEVARD, RHODES NSW 2138 EMAIL: Jason@jcoconsultants.com.au		Project PROPOSED SUBDIVISION 12-14 GLADYS AVENUE FRENCHS FOREST NSW 2030 Drawing Title STORMWATER DETAILS SHEET Design Drawn Validate J.L J.H	Job Number 20220060 Drawing Number DA-SW300	Scale AS NOTED Date 5/09/2022 Size A1 Datum A.H.D	North Point	DEVELOPMENT APPLICATION NOT FOR CONSTRUCTION Scale
---	-------------------	---	--	--	---	--	-------------	---



	_ Client		Project			Job Number	Scale	North Point	Status	203
6 F/00/2022 DETAINING WALL DEV/ISED	JACK ZHANG		PROPOSED SUBDIVISION			20220060	AS NOTED	1	DEVELOPMENT APPLICATION	mber
6 5/09/2022 RETAINING WALL REVISED J.L J.H 5 15/08/2022 ISSUED FOR DA J.L J.H	07101121171110	JCO CONSULTANTS PTY LTD	12-14 GLADYS AVENUE FRENCHS FOREST NSW 2030			20220060	5/09/2022		NOT FOR CONSTRUCTION	Septe
4 25/05/2022 ISSUED FOR REVIEW J.L J.L	Arabitaat	SUITE 801C, No.1 RIDER BOULEVARD, RHODES NSW 2138	Drawing Title	'		Drawing Number	Size	+	Scale	D: 5
3 19/05/2022 ISSUED FOR REVIEW J.L J.L		EMAIL: Jason@jcoconsultants.com.au	Drawing file	DIT COMMECTION DETAIL		Drawing Number	Λ1		Occident	ıΕ
2 15/05/2022 ISSUED FOR REVIEW J.L J.L	NKP ARCHITECURE	Livi viz. Gadon @ jococi i Ganarito.com.au	HGL ANALYSIS & EASEMENT	PIT CONNECTION DETAIL	T	\dashv DA CMEOO	Λ1	_		0
1 11/04/2022 ISSUED FOR REVIEW J.L J.L	1 WIN / WOITH LOOK		Design	Drawn	Validate	↑ DA-SW500	Datum			ПР
REV. DATE AMENDMENT INT. APP.			J.L	J.L	J.H		A.H.D			M
								CA	D File: P:\2022\20220060-12-14 Gladys Avenue, Frenchs Forest\Civil\cad\DA-SW500.dwg	j

NOTE

THE STORMWATER PLANS IS TO BE READ IN CONJUNCTION WITH THE STORMWATER SYSTEM ASSESSMENT REPORT PREPARED BY 'NASTASI & ASSOCIATES CONSULTING ENGINEERS'.

ACCORDING TO THE DOWNSTREAM PIPE CAPACITY CALCULATION FROM STORMWATER SYSTEM ASSESSMENT REPORT
'THE ESTIMATED MAXIMUM DISCHARGE RATE FROM 12-14 GLADYS AVE TO PIT SPP01245 SHALL BE 85 L/S FOR 100-YEAR ARI EVENT.'

THE POST DEVELOPMENT TOTAL SITE DISCHARGE TOWARDS THE EXISTING SPP01245 IS ONLY <u>70L/s</u> WHICH IS SIGNIFICANTLY LESS THAN THE PERMITTED DISCHARGED FLOW RATE 85L/s. HENCE, THE EXISTING DOWNSTREAM PIPE SYSTEM WILL HAVE ADDITIONAL CAPACITY TO CATER FOR FUTURE DEVELOPMENT WITHIN THE SUBJECT SITE, SUCH AS A GRANNY FLAT.

OSD DESIGN SUMMARY

SITE AREA: 4704m2

EXISTING SITE NODE WAS MODELED AS FULLY UNDEVELOPED NATURAL SURFACE FOR PURPOSE OF MODELING.

POST DEVELOPMENT ROOF AREA TO ABOVE GROUND OSD STORAGE, THEN DISCHARGE TO DRIVEWAY COMMON OSD TANK.

POST DEVELOPMENT BYPASS AREA = 0.21Ha (90% IMPERVIOUS AREA)

5YR ARI PRE-DEVELOPMENT DISCHARGE = 88L/s 5YR ARI POST-DEVELOPMENT BYPASS = 39L/s

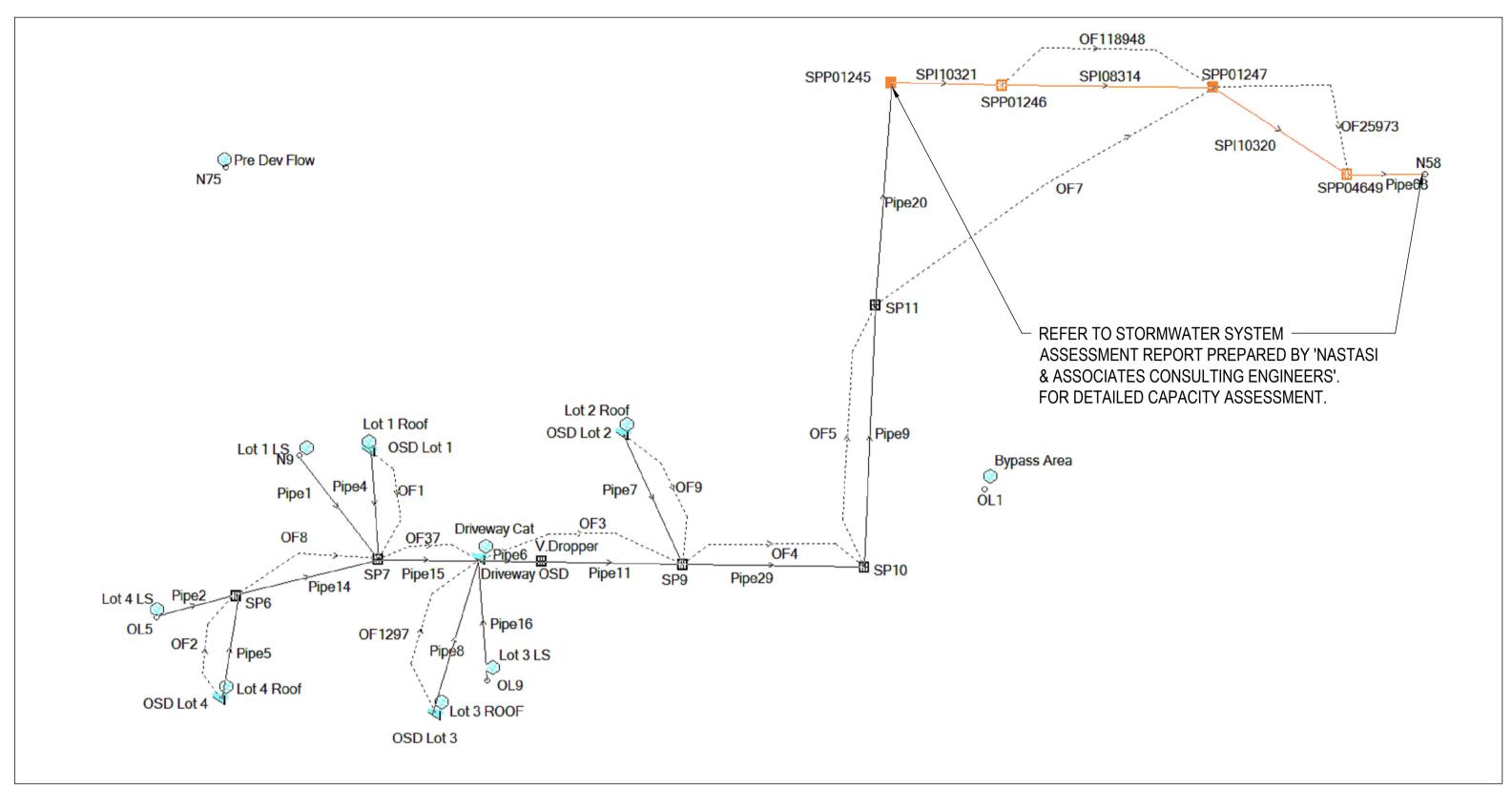
5YR ARI POST-DEVELOPMENT DISCHARGE = 42L/s

20YR ARI PRE DEVELOPMENT DISCHARGE = 137L/s 20YR ARI POST-DEVELOPMENT BYPASS = 62L/s

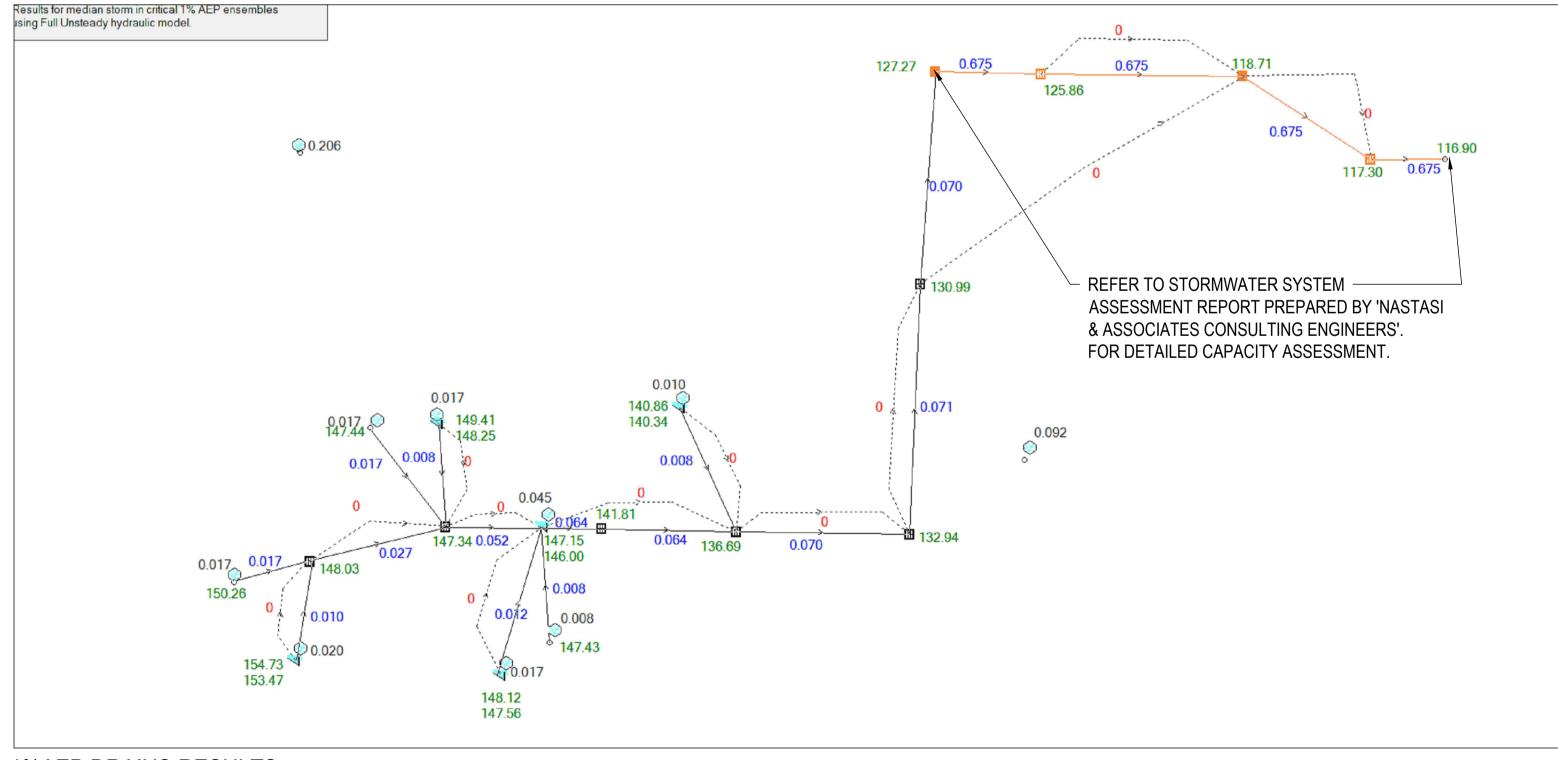
20YR ARI POST-DEVELOPMENT DISCHARGE = 55L/s

100YR ARI PRE DEVELOPMENT DISCHARGE 100YR ARI POST-DEVELOPMENT BYPASS

100YR ARI POST-DEVELOPMENT BYPASS – 92L/s 100YR ARI POST-DEVELOPMENT DISCHARGE = 70L/s



1%AEP DRAINS LAYOUT



1%AEP DRAINS RESULTS

					Client	
					1401/7114110	
6	5/09/2022	RETAINING WALL REVISED	J.L	J.H	JACK ZHANG	JCO CONSULTANTS PTY LTD
5	15/08/2022	ISSUED FOR DA	J.L	J.H		DCO CONSULTANTS PIT LID
4	25/05/2022	ISSUED FOR REVIEW	J.L	J.L	Architect	SUITE 801C, No.1 RIDER BOULEVARD, RHODES NSW 2138
3	19/05/2022	ISSUED FOR REVIEW	J.L	J.L		EMAIL: Jason@jcoconsultants.com.au
2	15/05/2022	ISSUED FOR REVIEW	J.L	J.L	NKP ARCHITECURE	
1	11/04/2022	ISSUED FOR REVIEW	J.L	J.L	1414 741401111200112	
REV	DATE	AMENDMENT	INT.	APP.		



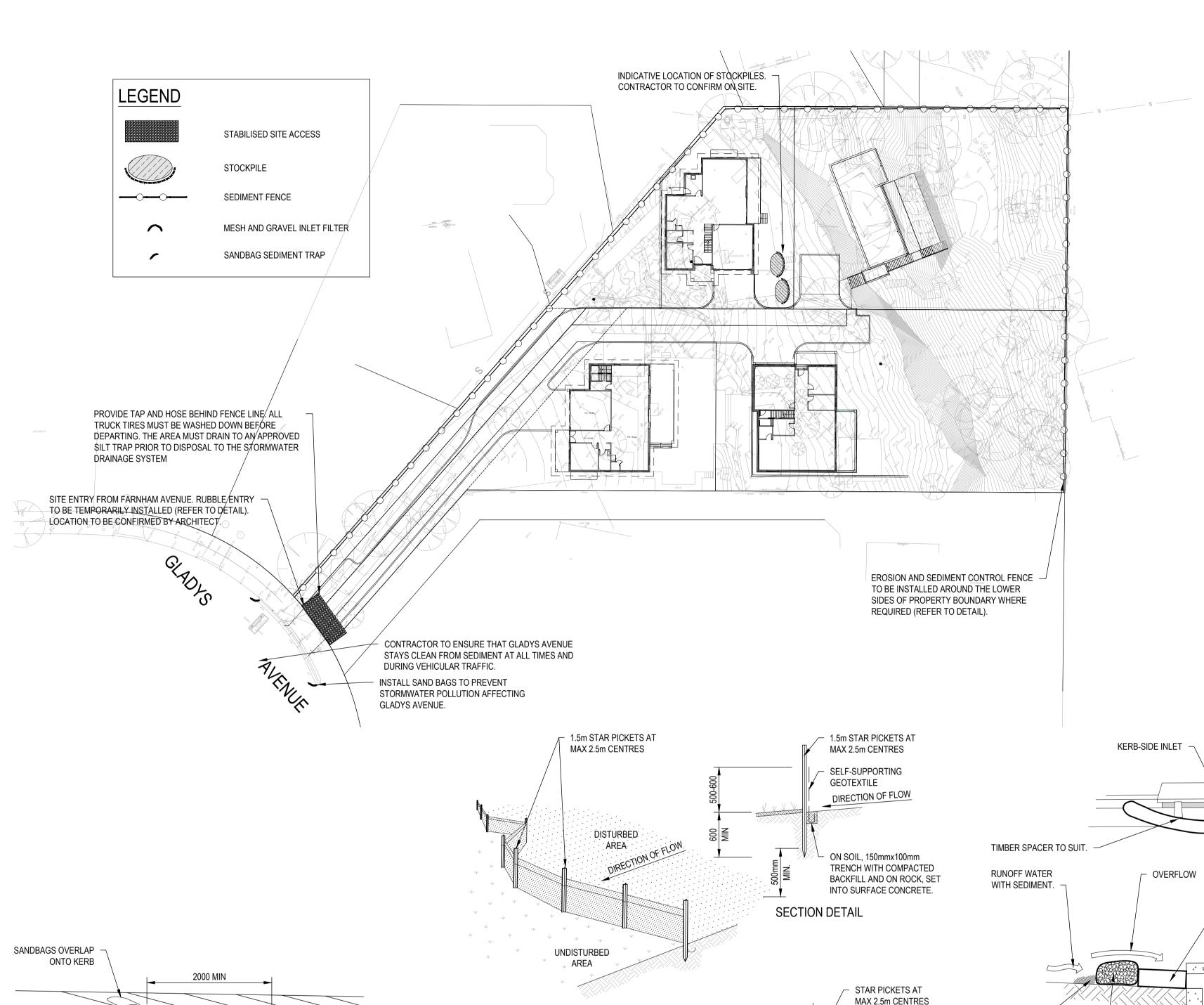
pject	Job Number	Scale	North Point	Status
ROPOSED SUBDIVISION		NTS		DEVELOPMENT APPLICATION
2-14 GLADYS AVENUE	20220060	Date	1	
RENCHS FOREST NSW 2030		5/09/2022		NOT FOR CONSTRUCTION
awing Title	Drawing Number	Size		Scale
RAINS MODEL DATA & RESULTS - 1	D A OVA/504	A1		
sign Validate	□ DA-SW501	Datum	1	
J.L J.H		A.H.D		

Fig. Control	/ NODE DETAILS ne Type	Family	Version 15 Size	Ponding Volume	Pressure Change				Blocking x Factor	y Bolt-down			Inflow			Inflow is Minor Sal		
Column	OnGrada	NIM/S Dite	CSID 900-900		Coeff. Ku											(m)		
Second S	roppe OnGrade	Downpipe	Downpipe		1.5	145.72			0 761.431	-177.319 No	84502735	1 x Ku	No			7.00		
Column		NWS Pits	ORIGINAL CHRONICALINE					0										
Company	OnGrade	NWS Pits	GSIP 600x600		0.2	2 131.3		C	0.5 836.084	-119.961 No	66	1 x Ku	No	New		No		
March Marc								0.605				1						
The content	1247 OnGrade	Junction Pit or Manhole (sealed)	Junction Pit or Manhole		1	1 118		C	0 911.715	-71.121 Yes	179	1 x Ku	No	Existing		Yes		
The content		NSW Dept. of Housing RM7 Inlet, 3% crossfall, 2% grade	RM7		0.3			0						Existing		No		
Second S	OnGrade	NWS Pits	GSIP 600x600		1.1			0	0.5 693.055	-184.894 No				New		No		
The content						150		0										
Company	Node							C	860.504	-161.172	18076853	3	No					
Control Cont		The state of the s						0.601								No		
Company	460 OnGrade	Junction Pit or Manhole (sealed)			1	1 118		C	0 909.343	-28.841 Yes								
TATE OF THE PARTY		NSW Dept. of Housing RM7 Inlet, 3% crossfall, 2% grade	RM7		1.4			0						Existing		Yes		
Second	Node																	
State Stat	Node					150.5			675.031	-189.739	92071704	ı	No					
THE COLOR STATE OF THE COLOR STA		Surf. Area	Not Used	Outlet Type	K	Dia(mm)	Centre RL	Pit Family	y Pit Type x	y HED	Crest RL	Crest Len	٤id					
1			4	Orifice		60	148	3.25	723.056	-152.567 No			13	3				
Mary	way 145	9.0		Orifice		168	146.0	013	747.634	-176.845 No			92068988	8				
14 15 16 17 18 18 18 18 18 18 18																		
Mary	147	.2		_														
Column C			2	Orifice		70	140	0.35	779.71	-148.985 No			16	б				
1	ot 3	.5	4	Orifice		85	147	7.55	737.63	-211.639 No			24	4				
Part			4	Orifice		65	153	3.48	689.373	-208.344 No			26	6				
The content of the			4				155											
Mary		Total	Paved	Grace	Supp	Paved	Grass	Supp	Paved Gress	Sunn Payed	Grass	Supp	Paved	Grace	Sunn	lag Time Cuttor	Gutter	utter Ra
Column		Area		Area	Area	Time	Time	Time	Length Length	Length Slope(%)	Slope					or Factor Length	Slope Fl	owFactc M
March Marc	Roof OSD Lot 1	(na) 0.023	8 10		0 ((min)	(min)		(m) (m)	(m) %	%	%				(m) 0	%	
Secretary (1986) 1986	vay Driveway OSD	0.06	1 10	00				10 2								0		
1875 1976 19																0		
Mary	oof OSD Lot 4	0.027	6 10	00	0 () 5		10 2	2							0		
March Marc) 5										0		
Second S	Ar OL1	0.2	1	.0 90	00 () 5		13 2	2							0		
Part																0		
Mary		5.030		10				12 2										
State 1987 1988 1989		То	Length	U/S IL	D/S IL	Slope	Туре	Dia	I.D. Rough	Pipe Is No. Pipes	Chg From	At Chg	Chg	RI	Chg	RL etc		
Second Column			(m)	(m)	(m)	(%)		(mm)	(mm)									
Private Color Private Colo	SP7	Driveway OSD			4 146.3	3 1	· · · · · · · · · · · · · · · · · · ·			NewFixed	1 SP7	0)					
1975 1976	Driveway OSD	V.Dropper	_	4 145.9	9 145.4	1 12.5	Concrete, not under roads, 0.5% minimum slope	225	5 225 0.013	NewFixed	1 Driveway OSD	0)					
1925 1926	SP9	SP10								NewFixed	1 SP9	0)					
	SP10	SP11		30 132.	7 130.85	6.17	uPVC, not under roads, 1% minimum slope	225	5 242 0.012	NewFixed	1 SP10							
1																		
3 1988	SPP01246	SPP01247	3	31 12	25 117	7 25.81	Concrete, not under roads, 0.5% minimum slope	450	450 0.013	Existing	1 SPP01246		-					
Second 199																		
Second S	OSD Lot 2	SP9	2	25 140.3	3 137.2	2 12.4	Concrete, not under roads, 0.5% minimum slope	150	150 0.013	NewFixed	1 OSD Lot 2							
195 197																		
29 (2017) POR	SP6	SP7	1	147.9	9 146.4	1 12.5	Concrete, not under roads, 0.5% minimum slope	225	5 225 0.013	NewFixed	1 SP6		1					
Bingeline State												0)					
Second Ministry	98 Pit90368	Pit90460	3	31 12	25 117	7 25.81	Concrete, not under roads, 0.5% minimum slope	450	450 0.013	Existing	1 Pit90368	C						
George Control Contr				_						0		0)					
Tree Section Property Pro	OL9	Driveway OSD	3	30 146.3	8 146.5	5 1	uPVC, not under roads, 1% minimum slope		105 0.012	NewFixed	1 OL9	0)					
Female Part	OL5	SP6	2	25 150.2	2 147.9	9.2	uPVC, not under roads, 1% minimum slope	150	154 0.012	New	1 OL5	С)					
The content of the property		То	Travel	Spill	Crest	Weir	Cross	Safe Dep	t SafeDeptl Safe	Bed D/S Area		id	U/S IL	D/S IL	Length (m	n)		
Copy 1 10 10 10 10 10 10 10			Time	Level	Length			Major Sto	Minor Sto DxV	Slope Contributing					,			
Secretary Secr			0.	.1 150.2		1.7		0.3	0.15	11.08	0							
599 Spin S					4 0.9	9 1.7					0					1 1		
99114 \$9902367	SP9	SP10	0.	.1			Swale with 1:4 sideslopes	0.45	0.3 1	25.68	0	1160626	137.5	5 133.5	18.11	L		
18 18 18 18 18 18 18 18											0							
SS 10.74 SP9	48 SPP01246	SPP01247	0.	.1			4 m wide pathway	0.3	0.15 0.4	25.81	0	61130854	126	6 118	31	L		
OSD 1043 DrivewyGD Drive					32 0.9	9 1.7					0							
996 SP7 0.1	OSD Lot 3	Driveway OSD	0.	.1 149.5	0.9	1.7	4 m wide pathway	0.3	0.15 0.4	30.2	0	2316612	149.52	2 147.4	22	2		
\$ PRESSURAS \$ PRES					O.1	1.7					0							
Safe Cover (m) Safe Cover (m) Cover			0.	.1			4 m wide pathway	0.3	0.15 0.4	25.81	0	61130851	126	6 118	31	ı		
Type	D F1130400	F1(30333	0.	.3			4111 wide patriway	0.3	0.15 0.4	1	U	549/3840	, 118	0 117.8	38			
Concrete, not under roads, 195 minimum slope 150																		
Concrete, not under roads, 5.5% minimum slope 225 0.45 0.74 Concrete, not under roads, 5.5% minimum slope 225 0.45 0.74 Concrete, not under roads, 5.5% minimum slope 225 0.45 0.74 0.74 0.75 0.76 0.77 0.75 0.75 0.75 0.75 0.75 0.75 0.75					5													
Concrete, not under roads, D5K minimum slope 225	Concrete, not under roads, 0.5% minimum slope	22	5 0.4	0.6	34													
Concrete, not under roads, DSK minimum slope 225 0.45 0.54 0.54 0.54 0.54 0.55 0.5					17													
uPVC, not under roads, 1,5% minimum slope 24 0.3 0.2 1 Concrete, not under roads, 0,5% minimum slope 55 0.45 0.51 4 Concrete, not under roads, 0,5% minimum slope 55 0.45 0.43 0 Concrete, not under roads, 0,5% minimum slope 55 0.45 0.43 UPVC, not under roads, 1,5% minimum slope 55 0.45 0.33 UPVC, not under roads, 1,5% minimum slope 150 0.3 0.35 UPVC, not under roads, 1,5% minimum slope 150 0.3 0.35 UPVC, not under roads, 1,5% minimum slope 150 0.3 0.35 UPVC, not under roads, 1,5% minimum slope 150 0.3 0.35 UPVC, not under roads, 1,5% minimum slope 150 0.3 0.35 UPVC, not under roads, 2,5% minimum slope 150 0.3 0.35 Concrete, not under roads, 2,5% minimum slope 25 0.45 0.47 Concrete, not under roads, 2,5% minimum slope 35 0.45 0.35 Concrete, not under roads, 2,5% minimum slope 45 0.45 0.45 0.45 <t< td=""><td>Concrete, not under roads, 0.5% minimum slope</td><td>22</td><td>5 0.4</td><td>15 0.54</td><td>64</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>	Concrete, not under roads, 0.5% minimum slope	22	5 0.4	15 0.54	64													
1. Concrete, not under roads, 0.5% minimum slope 450 0.45 0.51 0. Concrete, not under roads, 0.5% minimum slope 55 0.45 0.43 0. Concrete, not under roads, 0.5% minimum slope 55 0.45 0.43 0. Concrete, not under roads, 0.5% minimum slope 55 0.45 0.43 0. Concrete, not under roads, 0.5% minimum slope 55 0.45 0.53 0.85 0.87 0.89 0.89 0.89 0.89 0.89 0.89 0.89 0.89																		
2 Concrete, not under roads, 0.5% minimum slope 525 545 50.45 50.35 50.45 50.35 50.45 50.3	Concrete, not under roads, 0.5% minimum slope	52	5 0.4	0.3	5													
Concrete, not under roads, 0.5% minimum slope 150 0.3 0.35 0.9VC, not under roads, 1.5% minimum slope 150 0.3 0.35 0.35 0.45 0.45 0.45 0.35 0.45 0.35 0.45 0.35 0.45 0.35 0.45 0.35 0.45 0.35 0.45 0.35 0.45 0.36 0.36 0.37 0.37 0.38 0.38 0.38 0.38 0.38 0.38 0.38 0.38																		
uPVC, not under roads, 1% minimum slope 150 0.3 0.35 uPVC, not under roads, 1% minimum slope 150 0.3 0.35 uPVC, not under roads, 1% minimum slope 150 0.3 0.35 Concrete, not under roads, 0.5% minimum slope 225 0.45 0.34 Concrete, not under roads, 0.5% minimum slope 150 0.45 0.87 Concrete, not under roads, 0.5% minimum slope 150 0.45 0.87 Concrete, not under roads, 0.5% minimum slope 50 0.45 0.81 Concrete, not under roads, 0.5% minimum slope 450 0.45 0.51 Concrete, not under roads, 0.5% minimum slope 525 0.45 0.53 Concrete, not under roads, 0.5% minimum slope 525 0.45 0.53 Concrete, not under roads, 0.5% minimum slope 525 0.45 0.53 UPVC, not under roads, 1% minimum slope 105 0.3 0.59 UPVC, not under roads, 1% minimum slope 104 0.3 0.14 UPVC, not under roads, 1% minimum slope 105 0.3 0.14 UPVC, not under roads, 1% minimum slope 105 0.3 0.14	Concrete, not under roads, 0.5% minimum slope	52	5 0.4	0.5	i3													
uPVC, not under roads, 1% minimum slope 150 0.3 0.35 0.34 0.35 0.35 0.35 0.35 0.35 0.35 0.35 0.35 0.35 0.35 0.35 0.34 0.35 0.34 0.35 0.34 0.35		15	0.															
Concrete, not under roads, 0.5% minimum slope Concrete, not under roads, 0.5% minimum slope 150 0.45 0.87 0.07 0.08 0.08 0.08 0.08 0.08 0.08																		
2 Concrete, not under roads, 0.5% minimum slope 3 Concrete, not under roads, 0.5% minimum slope 4 Concrete, not under roads, 0.5% minimum slope 5 Concrete, not under roads, 0.5% minimum slope 5 Concrete, not under roads, 0.5% minimum slope 6 Concrete, not under roads, 0.5% minimum slope 6 UPVC, not under roads, 1% minimum slope 6 UPVC, not under roads, 1% minimum slope 7 UPVC, not under roads, 1% minimum slope 8 UPVC, not under roads, 1% minimum slope 9 UPVC, not under roads, 1% minimum	Concrete, not under roads, 0.5% minimum slope	22	5 0.4	0.34	34													
Solution Concrete, not under roads, 0.5% minimum slope 450 0.45 0.51 0.45 0.43 0.50 0.45 0.43 0.50 0.45 0.43 0.50 0.45																		
6 Concrete, not under roads, 0.5% minimum slope	8 Concrete, not under roads, 0.5% minimum slope	45	0 0.4	15 0.5	51													
uPVC, not under roads, 1% minimum slope uPVC, not under roads, 1% minimum slope 105 0.3 0.59 0.14 0.3 0.14 0.3 0.14 0.3 0.14 0.3 0.14																		
odel has no pipes with non-return valves	uPVC, not under roads, 1% minimum slope	10	5 0.	.3 0.59	9													
	urvc, not under roads, 1% minimum slope	15	4 0.	.5 0.14	.4													
	del has no pipes with non-return valves																	
	act has no pipes with hon-return valves		l				DRAINS DATA			l	I	1						

PIT / NODE DET	TAILS			Version 8				
Name	Max HGL	Max Pond	Max Surface	Max Pond	Min	Overflow	Constraint	
		HGL	Flow Arriving	Volume	Freeboard	(cu.m/s)		
00			(cu.m/s)		(m)	_		
P7	147.34		0		0.11		None	
/.Dropper P9	141.81 136.69		0		3.91 0.81		None None	
P10	132.94		0		0.56		None	
P11	130.99		0		0.30		None	
PP01245	127.27		0		0.24		None	
PP01246	125.86		0		0.14		None	
PP01247	118.71		0		0		Outlet System	
PP04649	117.3		0		0.5		None	
158	116.9		0					
P6	148.03		0		0.47	0	None	
19	147.44		0.019					
Pit90275	127.23		0		0.28		None	
it90368	125.69		0		0.31	0	None	
it90460	118.77		0		0		Outlet System	
it90553	117.64		0		0.16		None	
1207083	116.89		0					
)L9	147.43		0.008					
)L5	150.26		0.019					
LID CATCUMATI	NIT DETAIL C							
UB-CATCHMEI		Dayod	Crossed	Dayad	Crassad	Cumm	Due to Storm	
lame	Max Flow Q	Paved May O	Grassed Max Q	Paved Tc	Grassed Tc	Supp. Tc	Due to Storm	
	(cu.m/s)	Max Q (cu.m/s)	(cu.m/s)	(min)	(min)	(min)		
ot 1 Roof	(cu.m/s) 0.017		(cu.m/s)		(min) 10		1% AEP, 5 min burst, Storm 1	
ot 1 Roof Oriveway Cat	0.017		0		10		1% AEP, 5 min burst, Storm 1 1% AEP, 5 min burst, Storm 1	
ot 2 Roof	0.043		0		10		1% AEP, 5 min burst, Storm 1	
ot 2 ROOF	0.017		0		10		1% AEP, 5 min burst, Storm 1	
ot 4 Roof	0.017		0		10		1% AEP, 5 min burst, Storm 1	
Lot 1 LS	0.02	0.02	0.017		12		1% AEP, 15 min burst, Storm 1	
re Dev Flow	0.017		0.017		13		1% AEP, 15 min burst, Storm 8	
Sypass Area	0.206				13		1% AEP, 15 min burst, Storm 8	
ot 3 LS	0.092		0.008		13		1% AEP, 15 min burst, Storm 8	
ot 4 LS	0.008				12		1% AEP, 15 min burst, Storm 8	
	/		5.527		12	_		
PIPE DETAILS								
lame	Max Q	Max V	Max U/S	Max D/S	Due to Storm			
	(cu.m/s)	(m/s)	HGL (m)	HGL (m)				
ipe4	0.008	0.47	149.267	147.336	1% AEP, 20 min burst, Storm 2			
ipe15	0.052	1.31	147.246	147.151	1% AEP, 15 min burst, Storm 8			
ipe6	0.064	3.78	146.1	145.5	1% AEP, 20 min burst, Storm 6			
ipe11	0.064	1.77	141.8	136.694	1% AEP, 25 min burst, Storm 3			
ipe29	0.07	1.92	136.694	132.944	1% AEP, 20 min burst, Storm 9			
ipe9	0.071	2.52	132.91	130.993	1% AEP, 20 min burst, Storm 6			
ipe20	0.07	2.48	130.993	127.268	1% AEP, 20 min burst, Storm 9			
SPI10321	0.675	3.18	127.089	125.856	1% AEP, 20 min burst, Storm 9			
SPI08314	0.675	4.24	125.45	118.708	1% AEP, 20 min burst, Storm 9			
SPI10320	0.675	3.12	118.229	117.298	1% AEP, 20 min burst, Storm 9			
ipe68	0.675	3.16	117.149	116.903	1% AEP, 20 min burst, Storm 9			
Pipe7	0.008	2.3	140.57	137.236	1% AEP, 10 min burst, Storm 7			
Pipe8	0.012	0.65	148.048	147.151	1% AEP, 10 min burst, Storm 7			
ipe5	0.01	3.05	154.261	148.036	1% AEP, 20 min burst, Storm 4			
ipe14	0.027		148.032		1% AEP, 15 min burst, Storm 8			
Pipe1	0.017		147.44		1% AEP, 15 min burst, Storm 8			
156602	0.601		127.073		1% AEP, 10 min burst, Storm 8			
156698	0.601		125.45		1% AEP, 2 hour burst, Storm 8			
156792	0.601	2.78	118.38		1% AEP, 5 min burst, Storm 1	_		
156886	0.601		117.085		1% AEP, 1.5 hour burst, Storm			
ipe16	0.008		147.435		1% AEP, 15 min burst, Storm 8			
ipe2	0.017	2.57	150.26	148.032	1% AEP, 15 min burst, Storm 8			
NIA AIRIE								
CHANNEL DETA		NA			Due to Chama			
lame	Max Q	Max V			Due to Storm			
	(cu.m/s)	(m/s)						
VERFLOW RO	UTE DETAILS							
lame		Max Q D/S	Safe Q	Max D	Max DxV	Max Width	Max V	Due to Storm
F1	0				0		(
)F37	0		1.497		0		(
)F3	0				0		(
)F4	0				0			
)F5	0				0			
)F7	0	0	1.302	0	0	0	()
F118948	0	0	1.283	0	0	0	()
)F25973	0		1.479		0		(
F9	0				0		(
)F1297	0		1.26		0		(
F2	0				0			
F8	0				0		(
F118945	0				0			
F107558	0	0	1.479	0	0	0	(j
	CIN E							
ETENTION BA			Me:: O	N/ C	May C			
ame	Max WL	MaxVol	Max Q	Max Q	Max Q			
CD I -: f	4.44	1912	Total	Low Level				
SD Lot 1	149.41				0			
riveway OSD	147.15		0.064		0			
SD Lot 2	140.86		0.008		0			
SD Lot 3	148.12				0			
SD Lot 4	154.73	5.2	0.01	0.01	0			
			11 on 11/8/202	2 using vers	ion 2022.012			
o water upwe	elling from a		CDDO4245					
		Ibm at CD7	SPP012/16					
eeboard was								

		Clie	ent		Project			Job Number	Scale	North Point
	F/00/0000 DETAINING MALL DEV/OED		JACK ZHANG		PROPOSED SUBDIVISION			20220060	NTS	
	5/09/2022 RETAINING WALL REVISED	J.L J.H	0/10/11/11/10	JCO CONSULTANTS PTY LTD	12-14 GLADYS AVENUE			20220060	Date	
	15/08/2022 ISSUED FOR DA	J.L J.H			FRENCHS FOREST NSW 2030				5/09/2022	
4	25/05/2022 ISSUED FOR REVIEW	J.L J.L Arc	chitect	SUITE 801C, No.1 RIDER BOULEVARD, RHODES NSW 2138	Drawing Title			Drawing Number	Size	7
3	19/05/2022 ISSUED FOR REVIEW	J.L J.L		EMAIL: Jason@jcoconsultants.com.au	_	TO 0		Brawing Nambor	۸1	
2	15/05/2022 ISSUED FOR REVIEW	J.L J.L	NKP ARCHITECURE	Living Control (Control Control Contro	DRAINS MODEL DATA & RESULT	18-2		- DA OMEGO	Λ1	
	11/04/2022 ISSUED FOR REVIEW	J.L J.L	INITIALIONAL		Design	awn	Validate	│ DA-SW502	Datum	
RE\	. DATE AMENDMENT	INT. APP.			J.L	J.L	J.H		A.H.D	

DEVELOPMENT APPLICATION
NOT FOR CONSTRUCTION



MAX 2.5m CENTRES (UNLESS STATED OTHERWISE ON SWMP/ESCP)

SANDBAG SEDIMENT TRAP

THREE LAYERS OF SANDBAGS

WITH ENDS OVERLAPPED

RUNOFF

GAP BETWEEN BAGS

ACT AS SPILLWAY

CONSTRUCTION NOTES

1. CONSTRUCT SEDIMENT FENCES AS CLOSE AS POSSIBLE TO BEING PARALLEL TO THE CONTOURS OF THE SITE, BUT WITH SMALL RETURNS AS SHOWN IN THE DRAWING TO LIMIT THE CATCHMENT AREA OF ANY ONE SECTION. THE CATCHMENT AREA SHOULD BE SMALL ENOUGH TO LIMIT WATER FLOW IF CONCENTRATED AT ONE POINT TO 50 LITRES PER SECOND IN THE DESIGN STORM EVENT, USUALLY THE 10-YEAR EVENT.

PLAN

- 2. CUT A 150mm DEEP TRENCH ALONG THE UPSLOPE LINE OF THE FENCE FOR THE BOTTOM OF THE FABRIC TO BE
- 3. DRIVE 1.5 METRE LONG STAR PICKETS INTO GROUND AT 2.5 METRE INTERVALS (MAX) AT THE DOWNSLOPE EDGE OF THE TRENCH. ENSURE ANY STAR PICKETS ARE FITTED WITH SAFETY CAPS.
- 4. FIX SELF-SUPPORTING GEOTEXTILE TO THE UPSLOPE SIDE OF THE POSTS ENSURING IT GOES TO THE BASE OF THE TRENCH. FIX THE GEOTEXTILE WITH WIRE TIES OR AS RECOMMENDED BY THE MANUFACTURER. ONLY USE GEOTEXTILE SPECIFICALLY PRODUCED FOR SEDIMENT FENCING. THE USE OF SHADE CLOTH FOR THIS PURPOSE IS NOT SATISFACTORY.
- 5. JOIN SECTIONS OF FABRIC AT A SUPPORT POST WITH A 150mm OVERLAP.
- 6. BACKFILL THE TRENCH OVER THE BASE OF THE FABRIC AND COMPACT IT THOROUGHLY OVER THE GEOTEXTILE.

GRAVEL-FILLED WIRE MESH OR GEOTEXTILE 'SAUSAGE' TIMBER SPACER TO SUIT **FILTERED** SEDIMENT WATER GRAVEL-FILLED WIRE MESH OR GEOTEXTILE 'SAUSAGE'

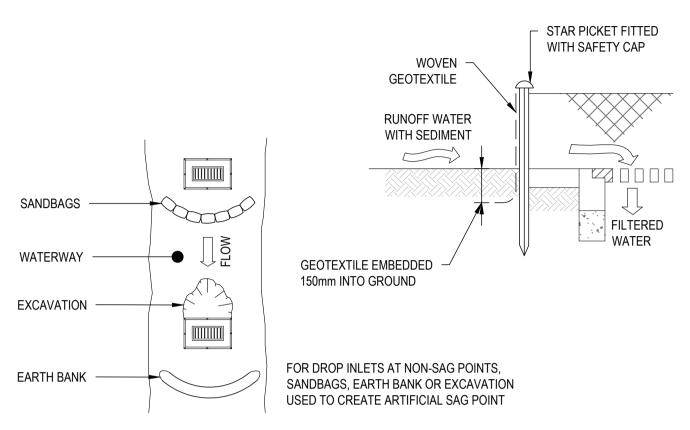
NOTE: THIS PRACTICE ONLY TO BE USED WHERE SPECIFIED IN APPROVED SWMP/ESCP.

CONSTRUCTION NOTES

- INSTALL FILTERS TO KERB INLETS ONLY AT SAG POINTS.
- 2. FABRICATE A SLEEVE MADE FROM GEOTEXTILE OR WIRE MESH LONGER THAN THE LENGTH OF THE INLET PIT AND
- FILL IT WITH 25mm TO 50mm GRAVEL. 3. FORM AN ELLIPTICAL CROSS-SECTION ABOUT 150mm HIGH x 400mm WIDE.
- 4. PLACE THE FILTER AT THE OPENING LEAVING AT LEAST A 100mm SPACE BETWEEN IT AND THE KERB INLET.
- MAINTAIN THE OPENING WITH SPACER BLOCKS.
- 5. FORM A SEAL WITH THE KERB TO PREVENT SEDIMENT BYPASSING THE FILTER.
- 6. SANDBAGS FILLED WITH GRAVEL CAN SUBSTITUTE FOR THE MESH OR GEOTEXTILE PROVIDING THEY ARE PLACED SO THAT THEY FIRMLY ABUT EACH OTHER AND SEDIMENT-LADEN WATERS CANNOT PASS BETWEEN.

MESH AND GRAVEL INLET FILTER (SD 6-11)

1 METRE MAX. - DROP INLET WITH GRATE **PICKETS** WIRE OR STEEL MESH (14 GAUGE x 150mm OPENINGS) WHERE GEOTEXTILE IS NOT SELF-SUPPORTING **WOVEN GEOTEXTILE**

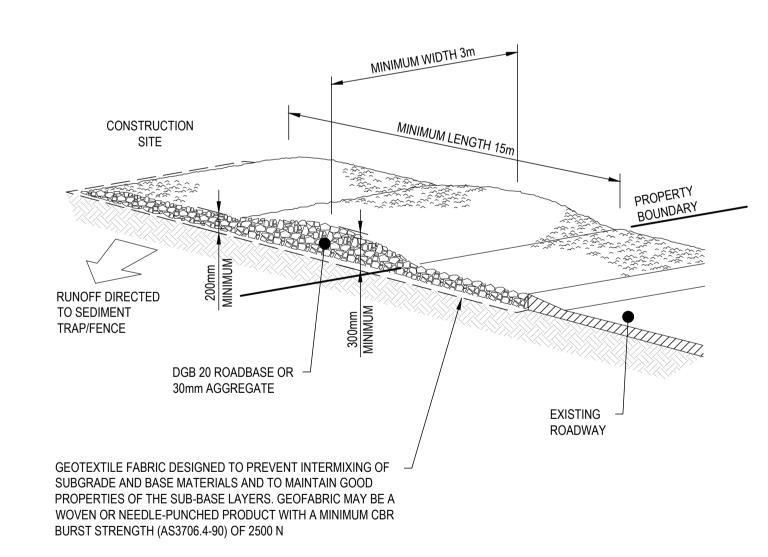


CONSTRUCTION NOTES

1. FABRICATE A SEDIMENT BARRIER MADE FROM GEOTEXTILE OR STRAW BALES.

- 2. FOLLOW STANDARD DRAWING 6-7 AND STANDARD DRAWING 6-8 FOR INSTALLATION PROCEDURES FOR THE STRAW BALES OR GEOFABRIC. REDUCE THE PICKET SPACING TO 1 METRE CENTRES.
- 3. IN WATERWAYS, ARTIFICIAL SAG POINTS CAN BE CREATED WITH SANDBAGS OR EARTH BANKS AS SHOWN IN
- 4. DO NOT COVER THE INLET WITH GEOTEXTILE UNLESS THE DESIGN IS ADEQUATE TO ALLOW FOR ALL WATERS TO BYPASS IT.

GEOTEXTILE INLET FILTER (SD 6-12)



CONSTRUCTION NOTES

- 1. STRIP THE TOPSOIL, LEVEL THE SITE AND COMPACT THE SUBGRADE.
- 2. COVER THE AREA WITH NEEDLE-PUNCHED GEOTEXTILE.
- 3. CONSTRUCT A 200mm THICK PAD OVER THE GEOTEXTILE USING ROAD BASE OR 30mm AGGREGATE. 4. ENSURE THE STRUCTURE IS AT LEAST 15 METRES LONG OR TO BUILDING ALIGNMENT AND AT LEAST 3 METRES WIDE.
- 5. WHERE A SEDIMENT FENCE JOINS ONTO THE STABILISED ACCESS, CONSTRUCT A HUMP IN THE STABILISED ACCESS

TO DIVERT WATER TO THE SEDIMENT FENCE.

STABILISED SITE ACCESS (SD 6-14)

		SEDIMENT FENCE (SD 6-8)]
6 5/09/2022 RETAINING WALL REVISED 5 15/08/2022 ISSUED FOR DA 4 25/05/2022 ISSUED FOR REVIEW J.L J.L 3 19/05/2022 ISSUED FOR REVIEW J.L J.L 2 15/05/2022 ISSUED FOR REVIEW J.L J.L 1 11/04/2022 ISSUED FOR REVIEW J.L J.L REV. DATE AMENDMENT INT. APP.	JACK ZHANG Architect NKP ARCHITECURE	JCO CONSULTANTS PTY LTD SUITE 801C, No.1 RIDER BOULEVARD, RHODES NSW 2138 EMAIL: Jason@jcoconsultants.com.au	Project PROPOSED SUBDIVISION 12-14 GLADYS AVENUE FRENCHS FOREST NSW 2030 Drawing Title EROSION AND SEDIMENT CONTROL PLAN & DETAILS Design J.L J.L	Validate J.H	Job Number 20220060 Drawing Number DA-SW600	AS NOTED Date 5/09/2022 Size A1	North Point	ENT APPLICATION CONSTRUCTION 20 30 40m @A1