139 George Street, Avalon

Geotechnical Comments for Section 4.55

We have reviewed the existing geotechnical report, the original plans, and the 10 amended plans by Blue Sky Building Designs, Job number AVA.2021012, drawings numbered CC101. CC105.2, CC106.2, CC107.2, and CC.109 to CC114, dated 14.5.25.

The changes are as follows:

- Remove the proposed pool.
- Remove the proposed lift.
- Remove the proposed excavation in the subfloor space for store room and gym.
- Various other minor internal and external alterations and additions.

The changes to the plans are considered minor from a geotechnical perspective. The removal of the excavation for the subfloor space reduces the overall risk of the project and removes the excavation support requirements. The changes do not alter the remaining recommendations or risk assessment in the original report carried out by this firm numbered J4120 and dated the 23rd March, 2022 and the Section 4.55 Letter, dated 15th June, 2023.

White Geotechnical Group Pty Ltd.

Tyler Jay Johns
BEng (Civil)(Hons),
Geotechnical Engineer.

Reviewed By:

Nathan Gardner B.Sc. (Geol. & Geophys. & Env. Stud.)
AIG., RPGeo Geotechnical & Engineering.
No. 10307
Engineering Geologist & Environmental Scientist.

J4120A.
15th June, 2023

139 George Street, Avalon

Geotechnical Comments in Regards to Updated Plans

We have reviewed the existing geotechnical report, the original plans, and the 19 amended plans by Blue Sky Building Designs, Job number AVA.2021012, drawings numbered A100 to A117, and A119, dated 29.5.23.

The changes are as follows:

- Reduce extent of pool.
- Remove proposed boat parking space.
- Change shape of deck.
- Various other minor internal and external alterations and additions.

The changes are considered minor from a geotechnical perspective and do not alter the recommendations or the risk assessment in the original report carried out by this firm numbered J4120 and dated the 23rd March, 2022.

White Geotechnical Group Pty Ltd.

Tyler Jay Johns
BEng (Civil)(Hons),
Geotechnical Engineer.

Reviewed By:

Ben White M.Sc. Geol.,
AusIMM., CP GEOL.
No. 222757
Engineering Geologist.

GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER
FORM NO. 1 – To be submitted with Development Application

Development Application for _____
Name of Applicant

Address of site 139 George Street, Avalon

The following checklist covers the minimum requirements to be addressed in a Geotechnical Risk ***Declaration made by geotechnical engineer or engineering geologist or coastal engineer (where applicable) as part of a geotechnical report***

I, Ben White *on behalf of* White Geotechnical Group Pty Ltd
(Insert Name) (Trading or Company Name)

on this the 23/2/22 certify that I am a geotechnical engineer or engineering geologist or coastal engineer as defined by the Geotechnical Risk Management Policy for Pittwater - 2009 and I am authorised by the above organisation/company to issue this document and to certify that the organisation/company has a current professional indemnity policy of at least $10million.

I:
Please mark appropriate box

☒ have prepared the detailed Geotechnical Report referenced below in accordance with the Australia Geomechanics Society's Landslide Risk Management Guidelines (AGS 2007) and the Geotechnical Risk Management Policy for Pittwater - 2009

☒ am willing to technically verify that the detailed Geotechnical Report referenced below has been prepared in accordance with the Australian Geomechanics Society's Landslide Risk Management Guidelines (AGS 2007) and the Geotechnical Risk Management Policy for Pittwater - 2009

☐ have examined the site and the proposed development in detail and have carried out a risk assessment in accordance with Section 6.0 of the Geotechnical Risk Management Policy for Pittwater - 2009. I confirm that the results of the risk assessment for the proposed development are in compliance with the Geotechnical Risk Management Policy for Pittwater - 2009 and further detailed geotechnical reporting is not required for the subject site.

☐ have examined the site and the proposed development/alteration in detail and I am of the opinion that the Development Application only involves Minor Development/Alteration that does not require a Geotechnical Report or Risk Assessment and hence my Report is in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009 requirements.

☐ have examined the site and the proposed development/alteration is separate from and is not affected by a Geotechnical Hazard and does not require a Geotechnical Report or Risk Assessment and hence my Report is in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009 requirements.

☐ have provided the coastal process and coastal forces analysis for inclusion in the Geotechnical Report

Geotechnical Report Details:

Report Title: Geotechnical Report **139 George Street, Avalon**
Report Date: 23/2/22

Author: BEN WHITE

Author's Company/Organisation: **WHITE GEOTECHNICAL GROUP PTY LTD**

Documentation which relate to or are relied upon in report preparation:

Australian Geomechanics Society Landslide Risk Management March 2007.

White Geotechnical Group company archives.

I am aware that the above Geotechnical Report, prepared for the abovementioned site is to be submitted in support of a Development Application for this site and will be relied on by Pittwater Council as the basis for ensuring that the Geotechnical Risk Management aspects of the proposed development have been adequately addressed to achieve an "Acceptable Risk Management" level for the life of the structure, taken as at least 100 years unless otherwise stated and justified in the Report and that reasonable and practical measures have been identified to remove foreseeable risk.

Signature _____

Name Ben White

Chartered Professional Status MScGEOLAusIMM CP GEOL

Membership No. 222757

Company White Geotechnical Group Pty Ltd

GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER
FORM NO. 1(a) - Checklist of Requirements for Geotechnical Risk Management Report for Development Application

Development Application for _____
Name of Applicant

Address of site 139 George Street, Avalon

The following checklist covers the minimum requirements to be addressed in a Geotechnical Risk Management Geotechnical Report. This checklist is to accompany the Geotechnical Report and its certification (Form No. 1).

Geotechnical Report Details:

Report Title: Geotechnical Report **139 George Street, Avalon**

Report Date: 23/3/22

Author: BEN WHITE

Author's Company/Organisation: WHITE GEOTECHNICAL GROUP PTY LTD

Please mark appropriate box

☒ Comprehensive site mapping conducted **14/3/22**
(date)

☒ Mapping details presented on contoured site plan with geomorphic mapping to a minimum scale of 1:200 (as appropriate)

☒ Subsurface investigation required
- ☐ No Justification _____
- ☒ Yes Date conducted **14/3/22**

☒ Geotechnical model developed and reported as an inferred subsurface type-section

☒ Geotechnical hazards identified
- ☒ Above the site
- ☒ On the site
- ☐ Below the site
- ☐ Beside the site

☒ Geotechnical hazards described and reported

☒ Risk assessment conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
- ☒ Consequence analysis
- ☒ Frequency analysis

☒ Risk calculation

☒ Risk assessment for property conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009

☒ Risk assessment for loss of life conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009

☒ Assessed risks have been compared to "Acceptable Risk Management" criteria as defined in the Geotechnical Risk Management Policy for Pittwater - 2009

☒ Opinion has been provided that the design can achieve the "Acceptable Risk Management" criteria provided that the specified conditions are achieved.

☒ Design Life Adopted:
- ☒ 100 years
- ☐ Other _____
specify

☒ Geotechnical Conditions to be applied to all four phases as described in the Geotechnical Risk Management Policy for Pittwater - 2009 have been specified

☒ Additional action to remove risk where reasonable and practical have been identified and included in the report.

☐ Risk assessment within Bushfire Asset Protection Zone.

I am aware that Pittwater Council will rely on the Geotechnical Report, to which this checklist applies, as the basis for ensuring that the geotechnical risk management aspects of the proposal have been adequately addressed to achieve an "Acceptable Risk Management" level for the life of the structure, taken as at least 100 years unless otherwise stated, and justified in the Report and that reasonable and practical measures have been identified to remove foreseeable risk.

Signature _____

Name Ben White

Chartered Professional Status MScGEOLAusIMM CP GEOL

Membership No. 222757

Company White Geotechnical Group Pty Ltd

GEOTECHNICAL INVESTIGATION:

Alterations and Additions at **139 George Street, Avalon**

1. Proposed Development

1.1 Level the existing subfloor to create space for a workshop by excavating to a maximum depth of ~1.6m.

1.2 Construct a deck and two new balconies off the downhill side of the house.

1.3 construct an extension to the downhill side of the house.

1.4 Install a lift in the location of the existing staircase.

1.5 Install a pool on the downhill side of the property.

1.6 Various other internal and external alterations and additions.

1.7 Details of the proposed development are shown on 22 drawings prepared by Blue Sky Building Designs, project number AVA.2021012, drawings numbered A100 to A120, and A122, dated 18.2.2022.

2. Site Description

2.1 The site was inspected on the 14th March, 2022.

2.2 This residential property is accessed by a Right of Carriageway (ROW) off the high side of George Street and has a N aspect. It is located on the moderately graded lower reaches of a hillslope. The natural slope rises across the property at an average angle of ~12°. The slope above the property continues at similar angles, and the slope below the property eases to the waterfront.

2.3 At the road frontage, a ROW runs up the slope to a parking area, carport, and garage underneath the downhill side of the house (Photo 1). A stable ~1.3m high

timber log retaining wall supports a fill for the ROW (Photo 2). A second, stable ~1.3m high timber log retaining wall supports a fill for the driveway (Photo 3). The part three-storey brick house is supported on brick walls. The supporting brick walls show no significant signs of movement. A stable ~0.7m high masonry retaining wall supports a cut for a carport attached to the E side of the house (Photo 4). A series of ~1.4m high timber retaining walls terrace the slope on the E side of the property (Photo 5 & 6). One of the supporting soldier posts has separated and the wall has begun to bulge in the middle. See **Section 16** for advice regarding this wall. A moderately sloping lawn extends from the uphill side of the house to the upper common boundary (Photo 7). Several Sandstone boulders can be seen embedded in stable positions in this location (Photo 8).

3. Geology

The Sydney 1:100 000 Geological sheet indicates the site is underlain by the Newport Formation of the Narrabeen Group. This is described as interbedded laminite, shale and quartz to lithic quartz sandstone.

4. Subsurface Investigation

One hand Auger Hole (AH) was put down to identify soil materials. Four Dynamic Cone Penetrometer (DCP) tests were put down to determine the relative density of the overlying soil and the depth to weathered rock. The locations of the tests are shown on the site plan attached. It should be noted that a level of caution should be applied when interpreting DCP test results. The test will not pass through hard buried objects so in some instances it can be difficult to determine whether refusal has occurred on an obstruction in the profile or on the natural rock surface. This is not expected to be an issue for the testing on this site. However, excavation and foundation budgets should always allow for the possibility that the interpreted ground conditions in this report vary from those encountered during excavations. See the appended “Important information about your report” for a more comprehensive explanation. The results are as follows:

AUGER HOLE 1 (~RL15.9) **–** AH1 (Photo 9)

Depth (m)	Material Encountered
0.0 to 0.2	**TOPSOIL**, dark brown clayey soil, medium grained, loose, fine trace of organic matter, dry.
0.2 to 0.6	**CLAY**, brown, fine grained, firm to stiff, dry.
0.6 to 0.8	**CLAY**, orange, yellow, fine grained, stiff, dry.

End of test @ 0.8m. No water table encountered.

DCP TEST RESULTS – Dynamic Cone Penetrometer

Equipment: 9kg hammer, 510mm drop, conical tip. Standard: AS1289.6.3.2 - 1997

Depth(m) Blows/0.3m	DCP 1 (~RL16.2)	DCP 2 (~RL15.9)	DCP 3 (~RL9.5)	DCP 4 (~RL7.8)
0.0 to 0.3	3	2	3	3
0.3 to 0.6	3	2	7	7
0.6 to 0.9	8	12	10	10
0.9 to 1.2	27	24	18	10
1.2 to 1.5	36	31	24	12
1.5 to 1.8	#	#	31	18
1.8 to 2.1			#	32
2.1 to 2.4				#
	Refusal on Rock @ 1.4m	End of Test @ 1.5m	End of Test @ 1.8m	End of Test @ 2.1m

#refusal/end of test. F=DCP fell after being struck showing little resistance through all or part of the interval.

DCP Notes:

DCP1 – Refusal on Rock @ 1.4m, DCP thudding, orange clay on dry tip.
DCP2 – End of test @ 1.5m, DCP still going down slowly, orange clay on dry tip.
DCP3 – End of test @ 1.8m, DCP still going down slowly, orange clay on dry tip.
DCP4 – End of test @ 2.1m, DCP still going down slowly, orange clay on dry tip.

5. Geological Observations/Interpretation

The slope materials are colluvial at the near surface and residual at depth. In the test locations, the ground materials consist of shallow soils over clays. The clay merges into the underlying weathered rock at depths of between ~1.2m to ~1.8m below the current surface. The weathered zone is interpreted to be Extremely Low Strength Shale. Sandstone boulders were observed embedded in the slope above the house. It is interpreted that DCP 1 refused on an underlying boulder similar to the ones at the surface. See Type Section attached for a diagrammatical representation of the expected ground materials.

6. Groundwater

Normal ground water seepage is expected to move over the buried surface of the rock and through the cracks. Due to the slope and elevation of the block, the water table is expected to be many metres below the base of the proposed works.

7. Surface Water

No evidence of surface flows were observed on the property during the inspection. It is expected that normal sheet wash will move onto the site from above the property during heavy down pours.

8. Geotechnical Hazards and Risk Analysis

No geotechnical hazards were observed below or beside the property. The moderately graded slope that rises across the property and continues above is a potential hazard (**Hazard One**). The bulging timber retaining wall on the E side of the property is a potential hazard (**Hazard Two**). The proposed excavation is a potential hazard until retaining walls are in place (**Hazard Three**). The proposed excavation undercutting the footings for the house is a potential hazard (**Hazard Four**).

Risk Analysis Summary

HAZARDS	Hazard One	Hazard Two
TYPE	The moderate slope that rises across the property and continues above failing and impacting on the proposed works.	Further movement of the timber retaining wall on the E side of the property that causes damage or failure (Photos 5 & 6).
LIKELIHOOD	'Unlikely' (10^{-4})	'Likely' (10^{-2})
CONSEQUENCES TO PROPERTY	'Minor' (5%)	'Minor' (10%)
RISK TO PROPERTY	'Low' (2×10^{-5})	'Moderate' (5×10^{-4})
RISK TO LIFE	5.5×10^{-7}/annum	1.3×10^{-5}/annum
COMMENTS	This level of risk is 'ACCEPTABLE'.	This level of risk to life and property is 'TOLERABLE'. To move the risk to 'ACCEPTABLE' levels, the recommendations in **Section 16** are to be followed.

HAZARDS	Hazard Three	Hazard Four
TYPE	The excavation (Up to a maximum depth of ~1.6m) collapsing onto the work site before retaining walls are in place.	The proposed excavation undercutting the footings of the house causing failure.
LIKELIHOOD	'Possible' (10^{-3})	'Possible' (10^{-3})
CONSEQUENCES TO PROPERTY	'Medium' (15%)	'Medium' (35%)
RISK TO PROPERTY	'Moderate' (2×10^{-4})	'Moderate' (2×10^{-4})
RISK TO LIFE	8.3×10^{-6}/annum	5.3×10^{-5}/annum
COMMENTS	This level of risk to life and property is 'UNACCEPTABLE'. To move risk to 'ACCEPTABLE' levels, the recommendations in **Section 13 and 14** are to be followed.	This level of risk to life and property is 'UNACCEPTABLE'. To move risk to 'ACCEPTABLE' levels, the recommendations in **Section 13** are to be followed.

(See Aust. Geomech. Jnl. Mar 2007 Vol. 42 No 1, for full explanation of terms)

9. Suitability of the Proposed Development for the Site

The proposed development is suitable for the site. No geotechnical hazards will be created by the completion of the proposed development provided it is carried out in accordance with the requirements of this report and good engineering and building practice.

10. Stormwater

The fall is to George Street. Roof water from the development is to be piped to the street drainage system through any tanks that may be required by the regulating authorities.

11. Excavations

An excavation to a maximum depth of ~1.6m is required to level the existing subfloor to create space for a workshop. The excavations are expected to be through shallow soil over clay with Extremely Low Strength Shale expected at depths of between ~1.2m and ~1.8m. It is envisaged that excavations through soil, clay, and Extremely Low Strength Shale can be carried out with an excavator and bucket.

12. Vibrations

No excessive vibrations will be generated by excavation through soil, clay, and Extremely Low Strength Shale. Any vibrations generated by a domestic machine and bucket up to 16 ton carrying out excavation works will be below the threshold limit for infrastructure or building damage.

13. Excavation Support Advice

The excavation to construct a workshop will reach a maximum depth of ~1.6m. Allowing for 0.5m of back wall drainage, the setbacks are as follows:

- Flush with the existing walls of the subject house.
- ~0.9m from the uphill common boundary.

As such, both the existing walls of the house and the uphill common boundary will lie within the zone of influence of the proposed excavation. In this instance, the zone of influence is the area above a theoretical 45° line through clay and shale from the base of the excavation towards the surrounding structures and boundaries. This line reduces to 30° through the fill and soil.

Where the supporting walls of the subject house fall within the zone of influence of the excavation, exploration pits along the walls will need to be put down by the builder to determine the foundation depth and material. These are to be inspected by the geotechnical consultant.

If the foundations are confirmed to extend below the zone of influence of the proposed excavation, the excavation may commence. If they are not, the walls will need to be underpinned to below the zone of influence of the cut prior to the excavation commencing. See the site plan attached for the minimum extent of the required exploration pits/underpinning.

Underpinning is to follow the underpinning sequence 'hit one miss two'. Under no circumstances is the bulk excavation to be taken to the edges of the walls and then underpinned. Underpins are to be constructed from drives that should be proportioned according to footing type and size. Allowances are to be made for drainage through the underpinning to prevent a build-up of hydrostatic pressure. Underpins that are not designed as retaining walls are to be supported by retaining walls. The void between the retaining walls and the underpinning is to be filled with free-draining material such as gravel.

It should be noted that floating sandstone boulders were observed embedded in the soil profile on the site. If these are exposed in the excavation face or nearby, they can cause instability in unsupported cut batters. Should any boulders be observed during the excavation process the Geotechnical Consultant is to be contacted to assess the stability implications and provide shoring advice if necessary.

During the excavation process for the house, the geotechnical consultant is to inspect the early stages of the excavation, while the machine/excavation equipment is on site, to ensure the ground materials are as expected and no additional temporary support is required.

Upslope runoff is to be diverted from the cut faces by sandbag mounds or other diversion works. The materials and labour to construct the retaining walls are to be organised so on completion of the excavations they can be constructed as soon as possible. The excavation is to be carried out during a dry period. No excavations are to commence if heavy or prolonged rainfall is forecast.

All excavation spoil is to be removed from site following the current Environmental Protection Agency (EPA) waste classification guidelines.

14. Retaining Walls

For cantilever or singly-propped retaining walls, it is suggested the design be based on a triangular pressure distribution of lateral pressures using the parameters shown in Table 1.

Table 1 – Likely Earth Pressures for Retaining Walls

Unit	Earth Pressure Coefficients		
	Unit weight (kN/m^3)	'Active' K_a	'At Rest' K_0
Soil, and Residual Clays	20	0.35	0.45
Extremely Low Strength Shale	22	0.3	0.25

For rock classes refer to Pells et al "Design Loadings for Foundations on Shale and Sandstone in the Sydney Region". Australian Geomechanics Journal 1978.

It is to be noted that the earth pressures in Table 1 assume a level surface above the structure, do not account for any surcharge loads, such as those from the boulders observed in the

profile, and assume retaining walls are fully drained. Rock strength and relevant earth pressure coefficients are to be confirmed on site by the geotechnical consultant.

All retaining walls are to have sufficient back-wall drainage and be backfilled immediately behind the structure with free-draining material (such as gravel). This material is to be wrapped in a non-woven Geotextile fabric (i.e., Bidim A34 or similar), to prevent the drainage from becoming clogged with silt and clay. If no back-wall drainage is installed in retaining walls, the likely hydrostatic pressures are to be accounted for in the structural design.

15. Foundations

The proposed workshop and lift can be supported on a thickened edge / raft slab with piers taken to Extremely Low Strength Shale where necessary. This ground material is expected to be exposed across the uphill side of the excavation for the workshop and exposed at the surface in the area of the proposed lift as the ground level has already been lowered in this location. Where it is not exposed, and where this material drops away with the slope, piers will be required to maintain a uniform bearing material across the structure. This ground material is expected at depths of between 1.2m to 1.5m below the current surface in the area of the proposed works.

The proposed pool and any additional footings required for the proposed deck, balconies, and extensions can be supported on piers taken to the underlying Extremely Low Strength Shale. It is expected at depths of between 1.5m to 1.8m below the current surface in the area of the proposed works.

A maximum allowable bearing pressure of 600kPa can be assumed for footings on Extremely Low Strength Shale. It should be noted that this material is a soft rock and a rock auger will cut through it so the builders should not be looking for refusal to end the footings.

As the bearing capacity of clay and shale reduces when it is wet, we recommend the footings be dug, inspected, and poured in quick succession (ideally the same day if possible). If the

footings get wet, they will have to be drained and the soft layer of wet clay or shale on the footing surface will have to be removed before concrete is poured.

If a rapid turnaround from footing excavation to the concrete pour is not possible, a sealing layer of concrete may be added to the footing surface after it has been cleaned.

NOTE: If the contractor is unsure of the footing material required, it is more cost-effective to get the geotechnical consultant on site at the start of the footing excavation to advise on footing depth and material. This mostly prevents unnecessary over-excavation in clay-like shaly-rock but can be valuable in all types of geology.

16. Site Maintenance / Recommendations

The timber retaining wall on the E side of the property is in the slow process of collapse (Photos 5 & 6). We recommend consideration be made to repairing/replacing the retaining wall during the proposed works. Alternatively, the retaining wall can to be inspected by the owners on an annual basis or after heavy prolonged rainfall, whichever occurs first, keeping a photographic record of the inspections. We can carry out these inspections upon request. Should any new movement be observed, the retaining walls are to be remediated or rebuilt to current engineering standards.

17. Geotechnical Review

The structural plans are to be checked and certified by the geotechnical engineer as being in accordance with the geotechnical recommendations. On completion, a Form 2B will be issued. This form is required for the Construction Certificate to proceed.

18. Inspections

The client and builder are to familiarise themselves with the following required inspections as well as council geotechnical policy. We cannot provide geotechnical certification for the owners and Occupation Certificate if the following inspections have not been carried out during the construction process.

- The exploration pits to determine the foundation material along the supporting walls of the subject house are to be inspected by the geotechnical consultant to determine if underpinning is necessary. This is to occur before the bulk excavation for the workshop commences.
- Should any floating boulders be observed or become exposed in the close proximity to the proposed excavation, the Geotechnical Consultant is to be contacted to assess the stability implications and provide shoring advice if necessary.
- The geotechnical consultant is to inspect the early stages of the excavation progress while the machine/excavation equipment is on site, to ensure the ground materials are as expected and no additional temporary support is required.
- All footings are to be inspected and approved by the geotechnical consultant while the excavation equipment and contractors are still onsite and before steel reinforcing is placed or concrete is poured.

White Geotechnical Group Pty Ltd.

Ben White M.Sc. Geol.,
AusIMM., CP GEOL.
No. 222757
Engineering Geologist.

Photo 1

Photo 2

Photo 3

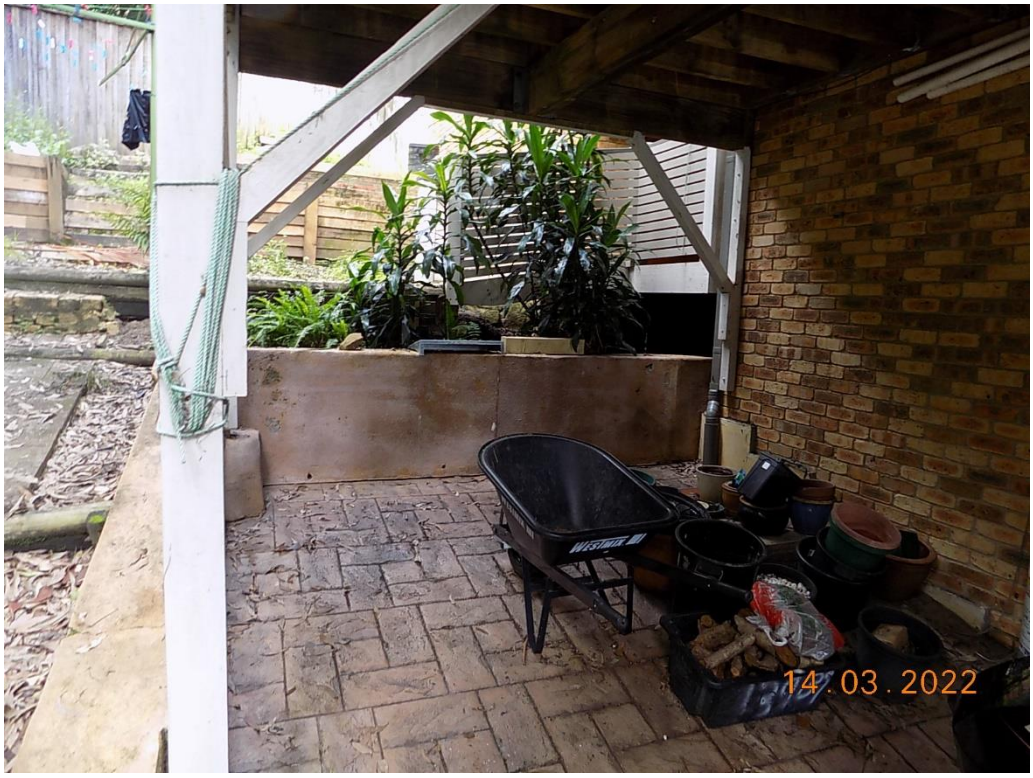


Photo 4

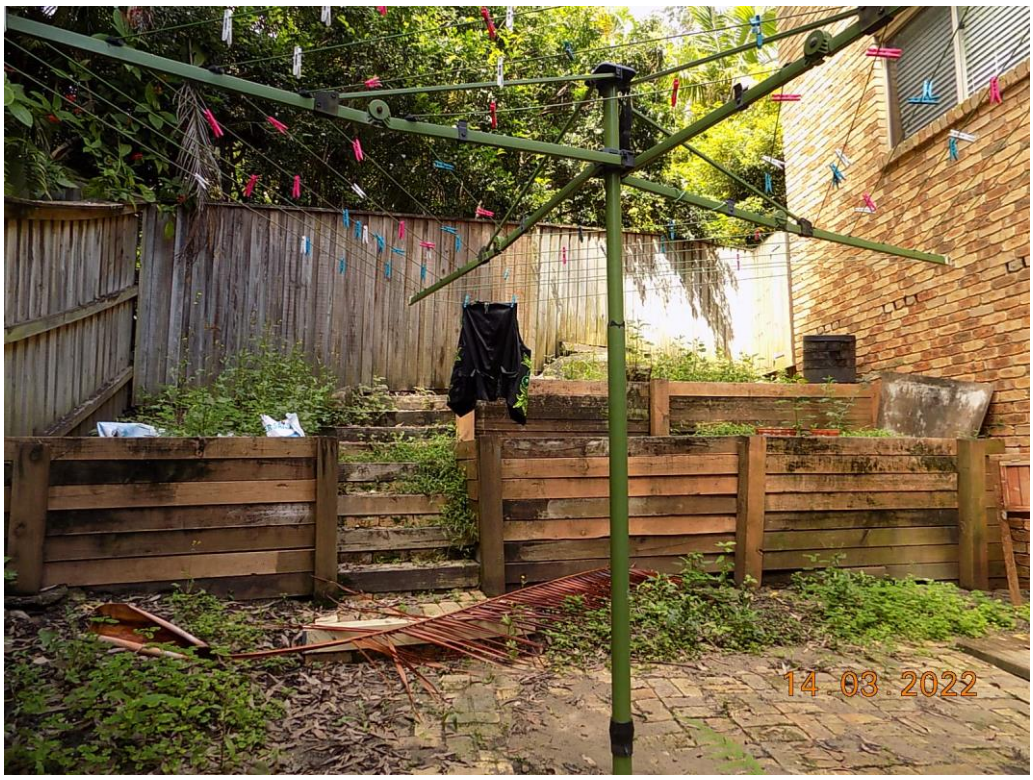


Photo 5

Photo 6

Photo 7

Photo 8

Photo 9 (Top to Bottom)

Important Information about Your Report

It should be noted that Geotechnical Reports are documents that build a picture of the subsurface conditions from the observation of surface features and testing carried out at specific points on the site. The spacing and location of the test points can be limited by the location of existing structures on the site or by budget and time constraints of the client. Additionally, the test themselves, although chosen for their suitability for the particular project, have their own limiting factors. The testing gives accurate information at the location of the test, within the confines of the test's capability. A geological interpretation or model is developed by joining these test points using all available data and drawing on previous experience of the geotechnical consultant. Even the most experienced practitioners cannot determine every possible feature or change that may lie below the earth. All of the subsurface features can only be known when they are revealed by excavation. As such, a Geotechnical report can be considered an interpretive document. It is based on factual data but also on opinion and judgement that comes with a level of uncertainty. This information is provided to help explain the nature and limitations of your report.

With this in mind, the following points are to be noted:

- If upon the commencement of the works the subsurface ground or ground water conditions prove different from those described in this report, it is advisable to contact White Geotechnical Group immediately, as problems relating to the ground works phase of construction are far easier and less costly to overcome if they are addressed early.

- If this report is used by other professionals during the design or construction process, any questions should be directed to White Geotechnical Group as only we understand the full methodology behind the report's conclusions.

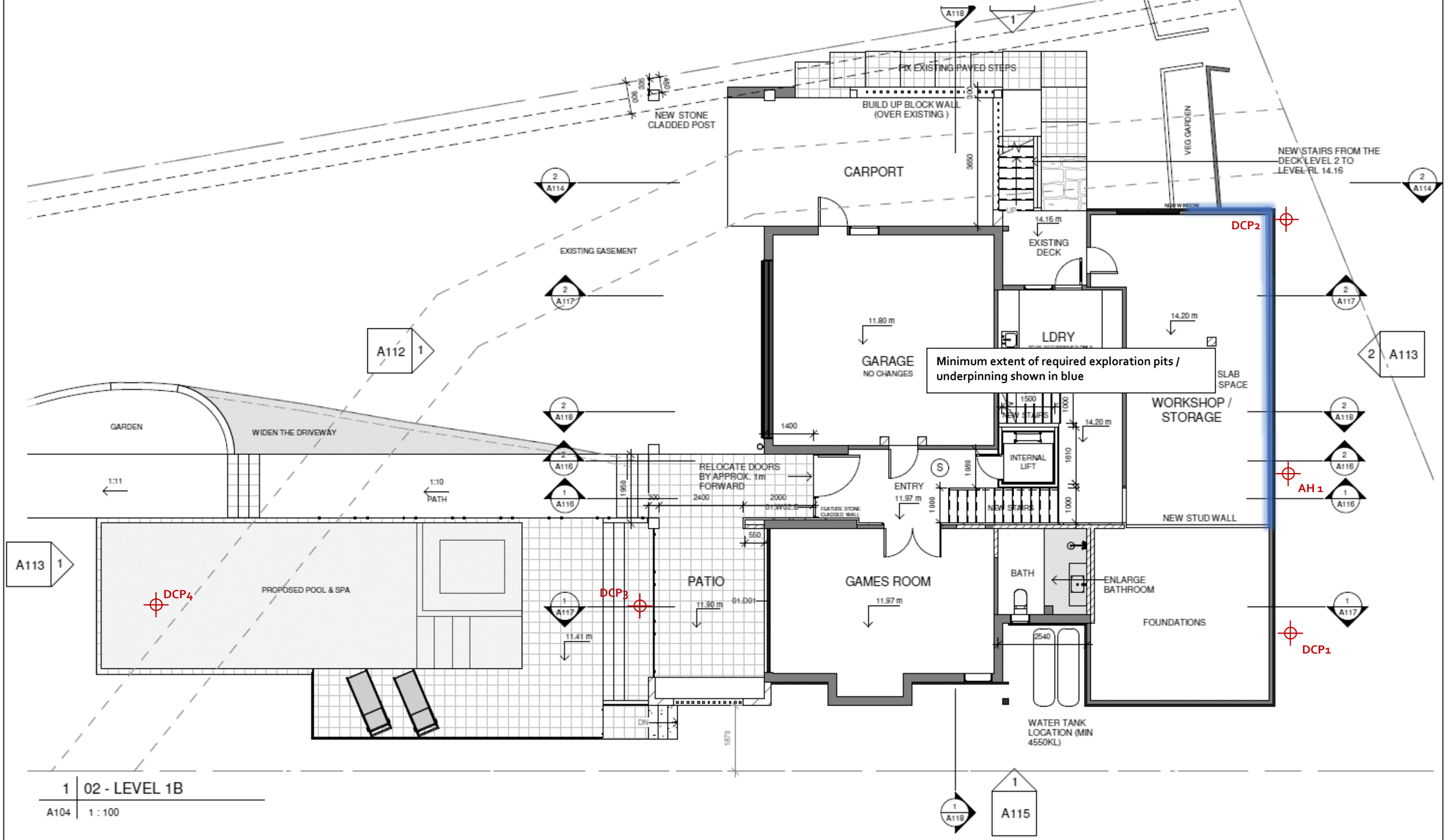
- The report addresses issues relating to your specific design and site. If the proposed project design changes, aspects of the report may no longer apply. Contact White Geotechnical if this occurs.

- This report should not be applied to any other project other than that outlined in section 1.0.

- This report is to be read in full and should not have sections removed or included in other documents as this can result in misinterpretation of the data by others.

- It is common for the design and construction process to be adapted as it progresses (sometimes to suit the previous experience of the contractors involved). If alternative design and construction processes are required to those described in this report, contact White Geotechnical Group. We are familiar with a variety of techniques to reduce risk and can advise if your proposed methods are suitable for the site conditions.

SITE PLAN – showing test locations and minimum extent of exploration pits / underpinning



1 | 02 - LEVEL 1B

A104 | 1 : 100

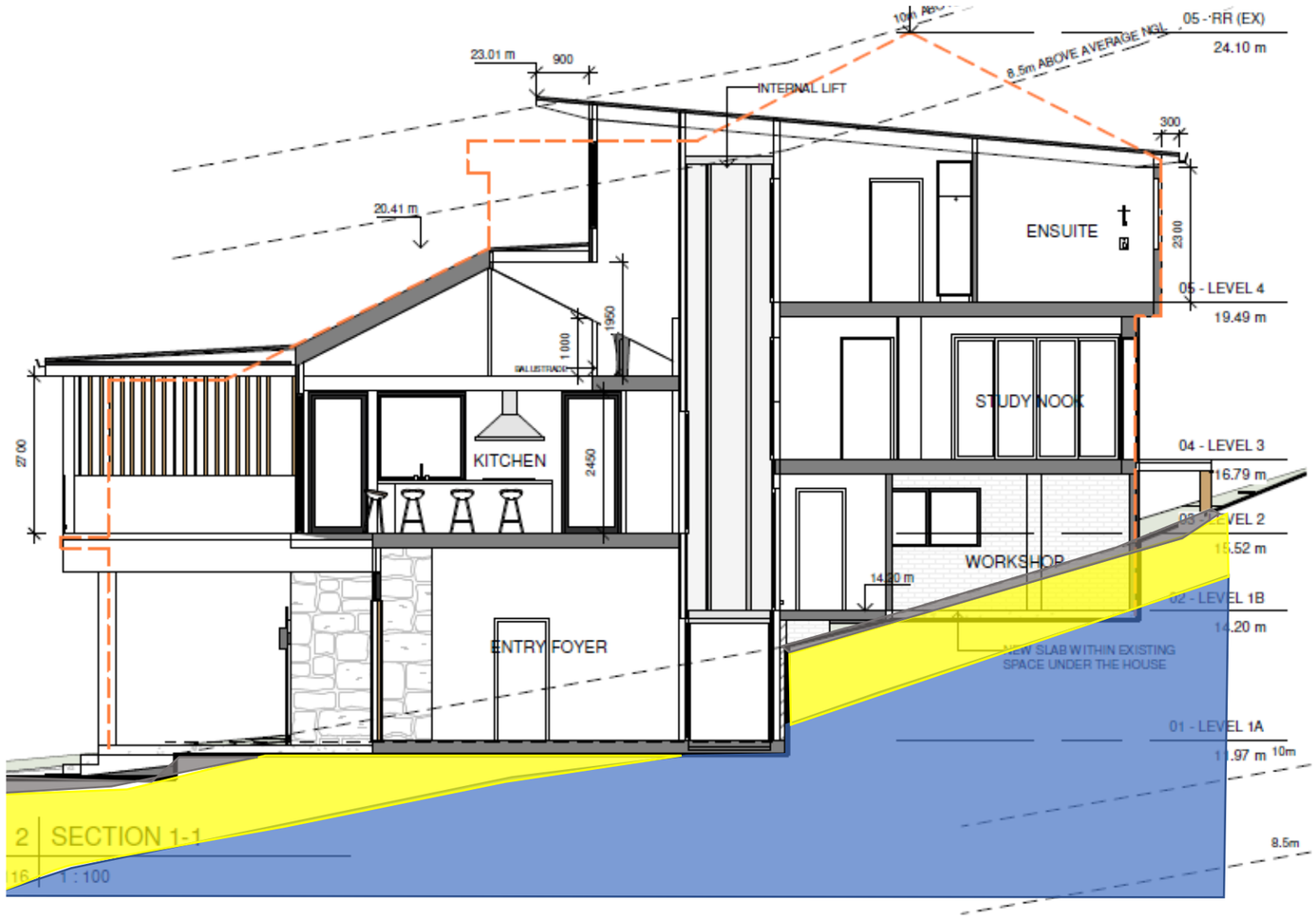
Do not scale from plans. All dimensions and levels shown on plan are subject to confirmation on site.

ISSUE	DATE	DESCRIPTION	DRWN	CHKD
-	16.07.2021	EXISTING	KM	-
2B	24.01.2022	CONCEPT	KM	
1	18.02.2022	DA ISSUE	KM	

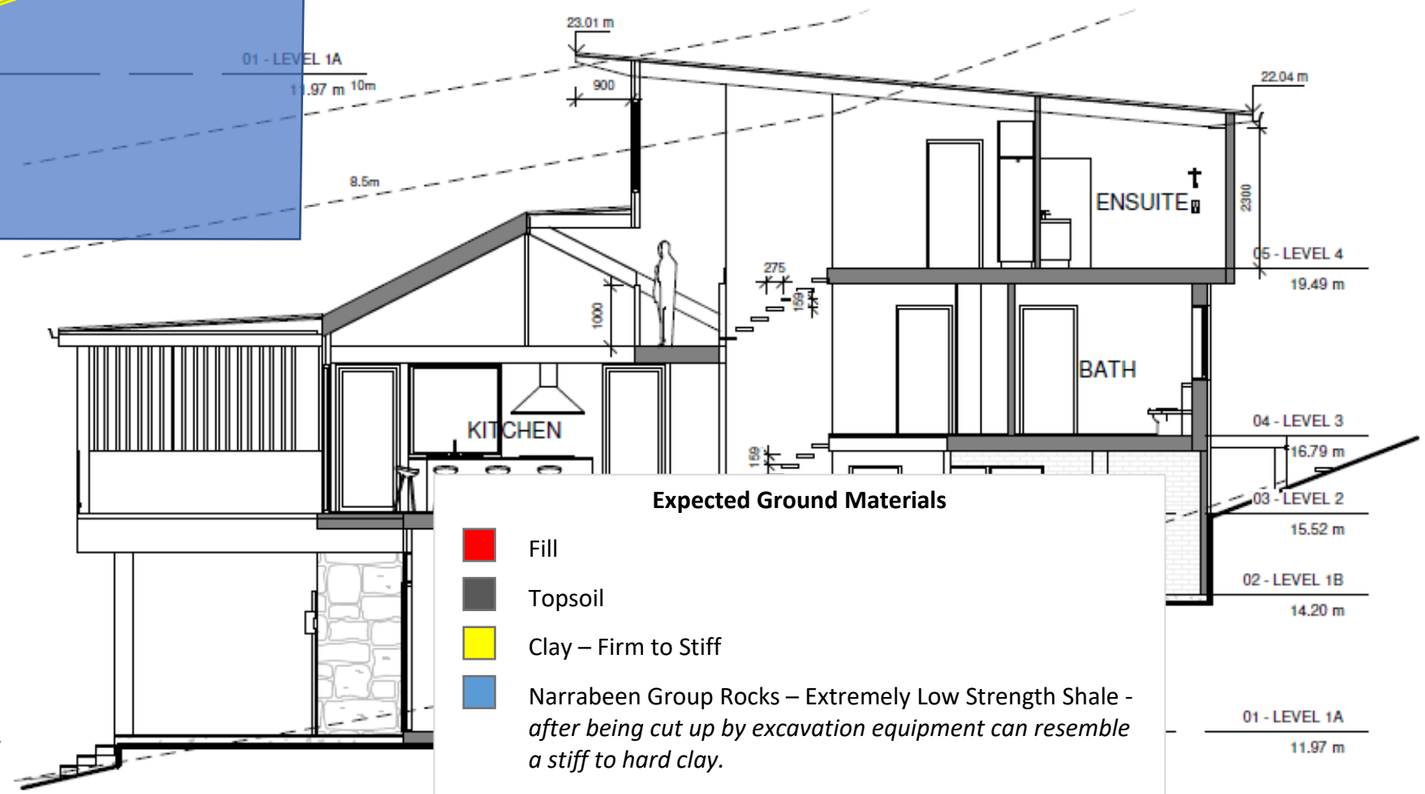
BLUE SKY BUILDING DESIGNS
E: info@bsbd.com.au
www.blueskybuildingdesigns.com.au

PROJECT TITLE:	ALTERATION & ADDITION
PROJECT NO.:	AVA.2021012
AT:	139 GEORGE ST, AVALON 2107
FOR:	DARREN KERSHAW
SHEET TITLE:	LEVEL 1 - PROPOSED
SHEET NO:	A104
SCALE A3:	1 : 100

TYPE SECTION – Diagrammatical Interpretation of expected Ground Materials



2 | SECTION 1-1
16 | 1 : 100



1 | SECTION 2-2
A116 | 1 : 100

Expected Ground Materials

- Fill
- Topsoil
- Clay – Firm to Stiff
- Narrabeen Group Rocks – Extremely Low Strength Shale - *after being cut up by excavation equipment can resemble a stiff to hard clay.*

Do not scale from plans. All dimensions and levels shown on plan are subject to confirmation on site.

ISSUE	DATE	DESCRIPTION	DRWN	CHKD
-	16.07.2021	EXISTING	KM	-
2B	24.01.2022	CONCEPT	KM	
1	18.02.2022	DA ISSUE	KM	

BLUE SKY BUILDING DESIGNS
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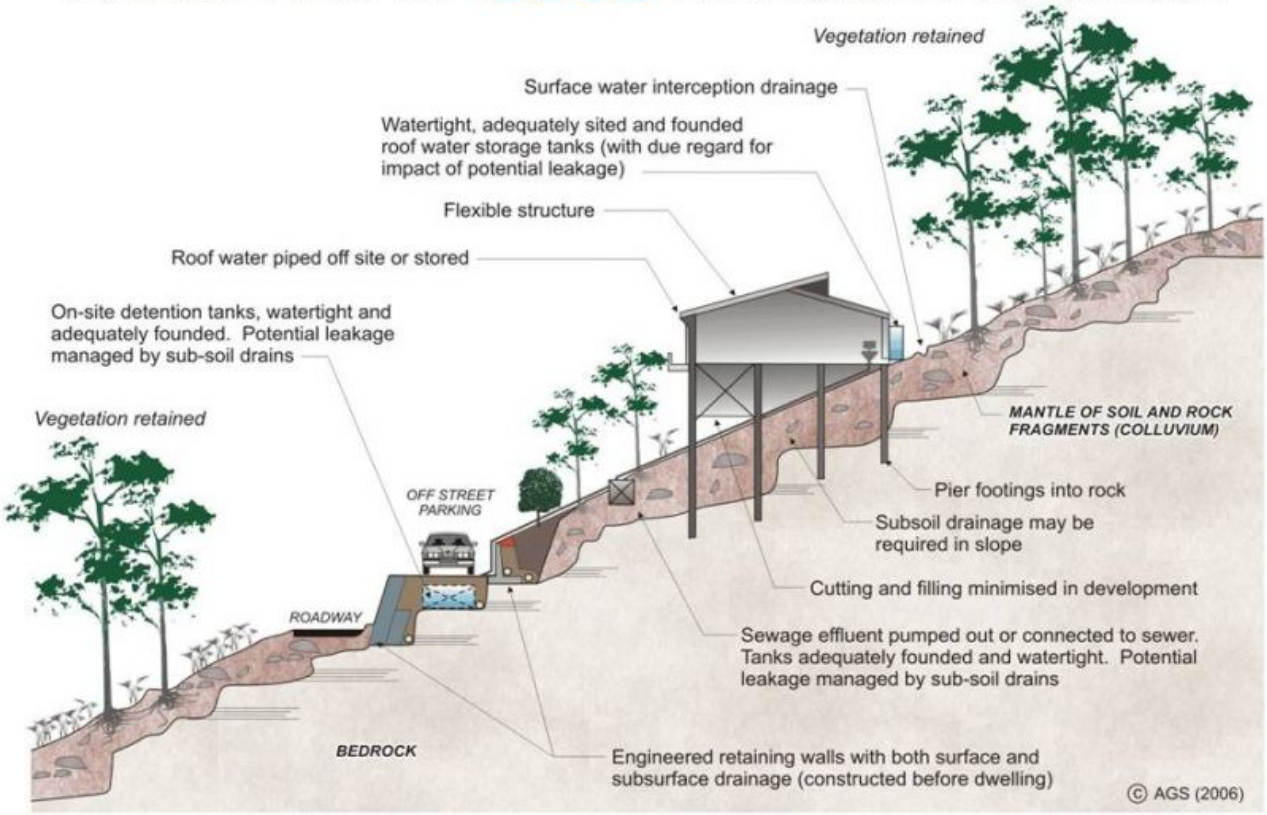
PROJECT TITLE:	ALTERATION & ADDITION
PROJECT NO.:	AVA.2021012
AT:	139 GEORGE ST, AVALON 2107
FOR:	DARREN KERSHAW
SHEET TITLE:	SECTIONS
SHEET NO:	A116
SCALE A3:	1 : 100

EXAMPLES OF **GOOD** HILLSIDE PRACTICE



EXAMPLES OF **POOR** HILLSIDE PRACTICE