SENIOR LIVING DEVELOPMENT PROJECT:

CONCEPT STORMWATER MANAGEMENT PLAN PLANSET:

CLIENT: **BELROSE RB1 PTY LTD**



LOCALITY PLAN NOT TO SCALE

LGA: NORTHERN BEACHES COUNCIL

171 FOREST WAY, BELROSE, NSW 2085 LOT 9 DP737255

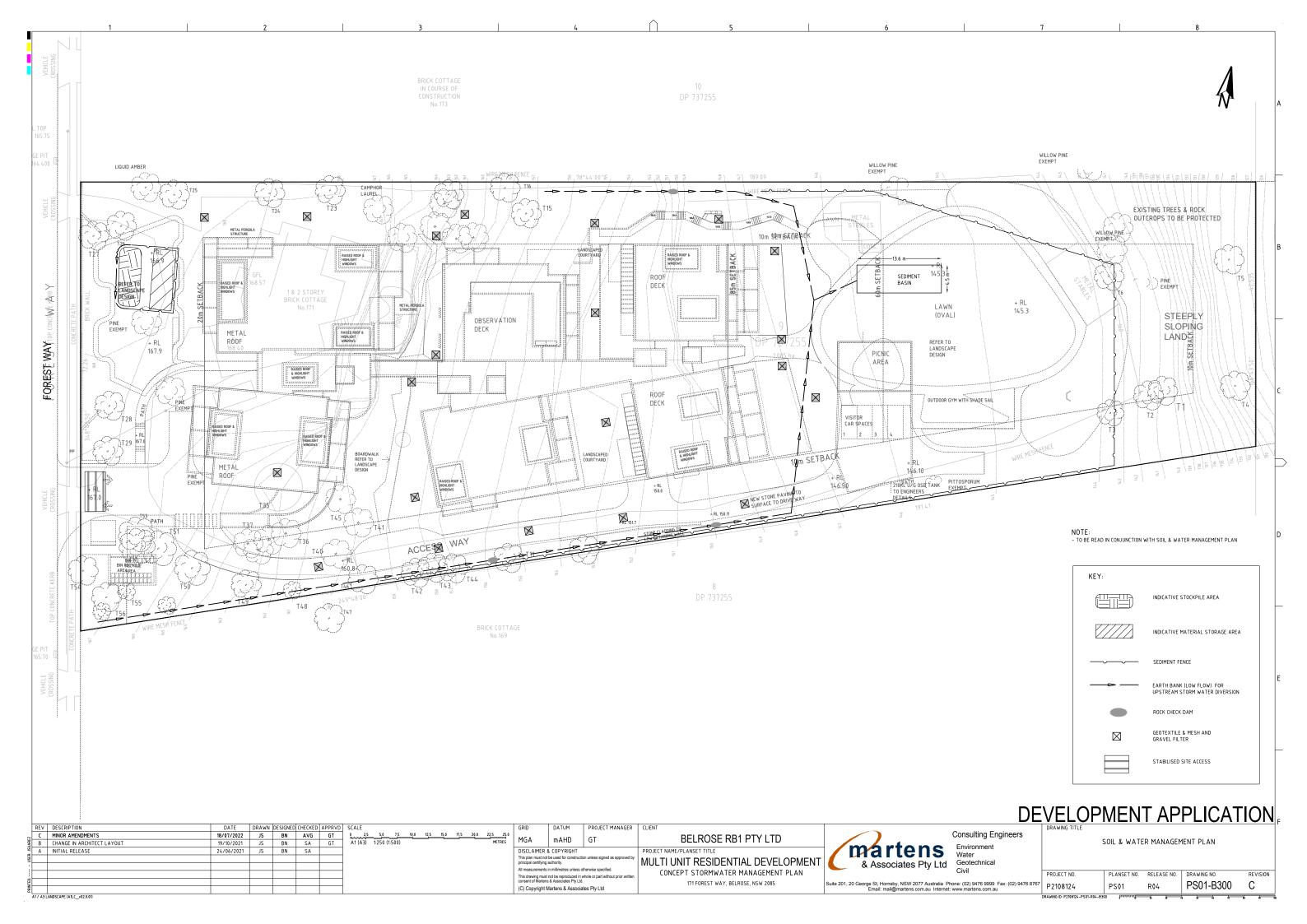
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	8E V	DESCRIPTION	DATE	DRAWN	DESIGNED	CHECKE	DAPPRVI	SCALE	GRID	DATUM	PROJECT MANAGER	CLIENT		
13	C	MINOR AMENDMENTS	18/07/2022	JS	BN	AVG	GT				GT	BELROSE RB1 PTY LTD		Consulting Engine
LAR	В	CHANGE IN ARCHITECT LAYOUT	19/10/2021	JS	BN	SA	GT				u,	BEERGGERBITTTETB		Environment
S	А	INITIAL RELEASE	24/06/2021	JS	BN	SA			DISCLAIMER &			PROJECT NAME/PLANSET TITLE	martens	Motor
									This plan must not be used for construction unless signed as approved by principal certificing authority.		n unless signed as approved by	MULTI UNIT RESIDENTIAL DEVELOPMENT		
<u> </u>									All measurements in	a millimetres unless of	honvice coecified		& Associates Pty Ltc	
											hole or part without prior written	CONCEPT STORMWATER MANAGEMENT PLAN		CIVII
Ë									consent of Martens	& Associates Pty Ltd.		171 FOREST WAY, BELROSE, NSW 2085	Suite 201, 20 George St, Hornsby, NSW 2077 Australia	Phone: (02) 9476 9999 Fax: (
No.									(C) Copyright M	artens & Associat	es Pty Ltd		Email: mail@martens.com.au Intern	
A1	/ A3 L	LANDSCAPE (A1LC_v02.0.01)												

DRAWI	DRAWING LIST										
DWG NO. REV DWG TITLE											
GENERAL											
PS01-A000 C COVER SHEET											
CONSTRUCTION MANAGEMENT WORKS											
PS01-B300	C	SOIL & WATER MANAGEMENT PLAN									
PS01-B310	В	SOIL & WATER MANAGEMENT DETAILS									
PS01-B315	В	SEDIMENT BASIN CALCULATIONS									
DRAINAG	e woi	RKS									
PS01-E100	C	DRAINAGE PLAN									
PS01-E200	В	DRAINAGE DETAILS									
PS01-E600 C ON SITE DETENTION CATCHMENT PLANS, MODELS & RESUL											
PS01-E700 C WATER QUALITY CATCHMENT PLANS, MODEL & RESULTS											

DRAWING ID: P2108124-PS01-R04-A000

GENERAL NOTES
 THIS PLAN IS FOR DEVELOPMENT APPLICATION PURPOSE AND NOT FOR CONSTRUCTION DESIGN TO BE REVIEWED AND UPDATED FOR CONSTRUCTION CERTIFICATE.
 ALL WORK TO BE CARRIED OUT IN ACCORDANCE WITH, AND THESE NOTES ARE TO BE READ IN CONJUNCTION WITH THE RELEVANT AUSTRALIAN STANDARDS, COUNCIL SPECIFICATIONS, AND ALL PROJECT CONSULTANTS PLANS AND REPORTS.
 INTERNAL SURVEY INFORMATION SHOWN BASED ON SURVEY INFORMATION PROVIDED BY S.J. DIXON SURVEYORS.
 EXTERNAL SITE BOUNDARY BASED ON INFORMATION PROVIDED BY S.J. DIXON SURVEYORS.

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nical						
	PROJECT NO.	PLANSET NO.	RELEASE NO.	DRAWING NO.	REVISION	
9999 Fax: (02) 9476 8767 .com.au	P2108124	PS01	R04	PS01-A000	C	

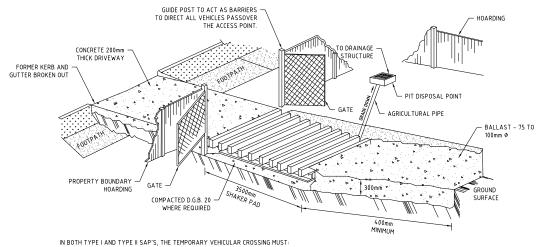


STABILISED ACCESS POINT

TYPE II SAP

THE TYPE II SAP DESIGN IS MORE DEFINED IN THAT IT REQUIRES AN AREA OF BALLAST WITHIN THE SITE COMBINED WITH A SHAKER PAD: ADJACENT THE SHAKER PAD AND IN THE PUBLIC WAY IS A TEMPORARY (CONCRETE) VEHICULAR CROSSING. (SEE DIAGRAM)

STABILISED ACCESS POINT - TYPE 2



- CONNECT TO AN EXISTING GUTTER LAYBACK (WHERE THE KERB AND GUTTER EXIST). IF A GUTTER LAYBACK DOES NOT EXIST THEN THE CONNECTION MUST BE MADE TO THE GUTTER BY REMOVING THE AD ICENT KERB SECTION ONLY
- CONNECT TO A DISH CROSSING (WHERE KERB AND GUTTER DOES NOT EXIST). IF A DISH CROSSING DOES NOT EXIST, THEN IT MUST BE CONSTRUCTED IN ACCORDANCE WITH DETAILS CONTAINED IN COUNCIL'S ISSUED FOOTPATH CROSSING LEVELS
- IT SHOULD BE NOTED THAT THESE TYPES OF SAPS ARE CONSIDERED TO BE APPLICABLE FOR THE MAJORITY OF ACTIVITIES HOWEVER SOME SITES MAY REQUIRE SPECIAL CONSIDERATION.

SHAKER PAD (CATTLE GRID)

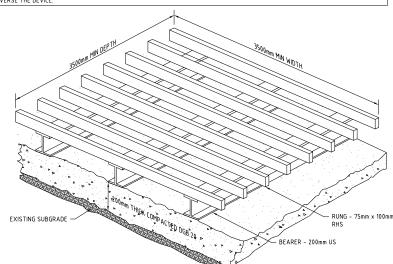
A CORRECTLY DESIGNED AND INSTALLED SHAKER PAD WILL ASSIST IN PREVENTING SEDIMENT TRANSFERE FROM A SITE. ANY STABILISED ACCESS POINT (SAP) CAN BE DESIGNED WITH A SHAKER PAD (COMPULSOPRY IN TYPE II SAP'S)

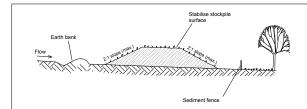
SHAKER PADS CAN BE DESIGNED AND CONSTRUCTED TO ENABLE RE-USE ON FUTURE PROJECTS.

THE SHAKER PAD:

- MUST BE DESIGNED AND CERTIFIED BY A PRACTICING STRUCTURAL ENGINEER. THE CERTIFIED DESIGN SHOULD BE SUBMITTED WITH THE RELEVENT APPLICATION.
- CAN BE CONSTRUCTED FROM ANY SUITABLE MATERIAL. MUST BE LOCATED ON A SUITABLY PREPARED AND COMPACTED SUB-GRADE/BASE MATERIAL
- MUST BE SITUATED SUCH THAT THE RUNGS OF THE SHAKER PAD ARE LEVEL WITH THE ADJOINING NATURAL SURFACE
- MUST BE A MINIMUM OF 3.5m IN LENGTH.
- MUST BE A MINIMUN OF 3.5m IN WIDTH.
- MUST HAVE CLEAR SPACING BETWEEN RUNGS OF 200 250mm
- RUNGS MUST HAVE A MAXIMUM WIDTH (BEARING AREA) OF 75mm. MUST HAVE A MINIMUM CLEAR DEPTH OF 300mm IE FORM THE ROP OF THE RUNG TO THE FINISHED SUB-GRADE/BASE LEVEL.

THE SHAKER PAD MUST BE PROVIDED WITH SUITABLE BARRIERS AT THE SIDES TO ENSURE THAT ALL TYERS OF VEHICLES LEAVING THE SITE TRAVERSE THE DEVICE





Construction Notes

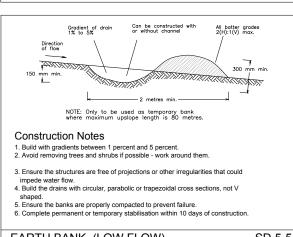
STOCKPILES

1. Place stockpiles more than 2 (preferably 5) metres from existing vegetation, concentrated Where there is sufficient area, topsoil stockpiles shall be less than 2 metres in height.
 Where there is sufficient area, topsoil stockpiles shall be less than 2 metres in height.

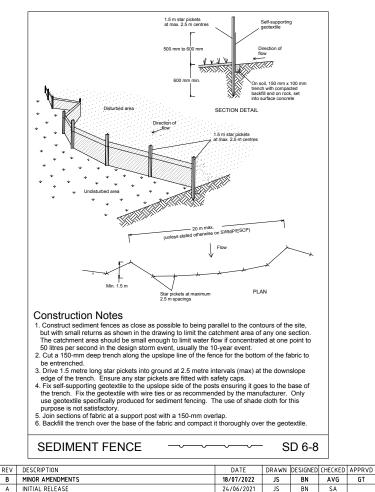
- 4. Where they are to be in place for more than 10 days, stabilise following the approved ESCP or SWMP to reduce the C-factor to less than 0.10.
- 5. Construct earth banks (Standard Drawing 5-5) on the upslope side to divert water around stockpiles and sediment fences (Standard Drawing 6-8) 1 to 2 metres downslope.

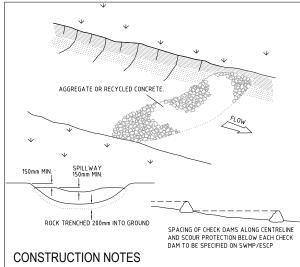
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SD 4-1



GRID DATUM PRO IECT MANAGER CLIEN **Consulting Engineers** BELROSE RB1 PTY LTD MGΔ GT mAHD Environment martens DISCLAIMER & COPYRIGHT PROJECT NAME/PLANSET TITL Water This plan must not be used for construction unless signed as principal certifying authority. MULTI UNIT RESIDENTIAL DEVELOPMENT Geotechnica & Associates Pty Ltd nents in millimetres unless otherwise specified CONCEPT STORMWATER MANAGEMENT PLAN This drawing must not be reproduced in whole or part without prior writte consent of Martens & Associates Pty Ltd. 171 FOREST WAY, BELROSE, NSW 2085 uite 201, 20 George St, Hornsby, NSW 2077 Australia Phone: (02) 9476 9999 Fax: (02) 9476 8767 Email: mail@martens.com.au Internet: www.martens.com.au (C) Copyright Martens & Associates Pty Ltd





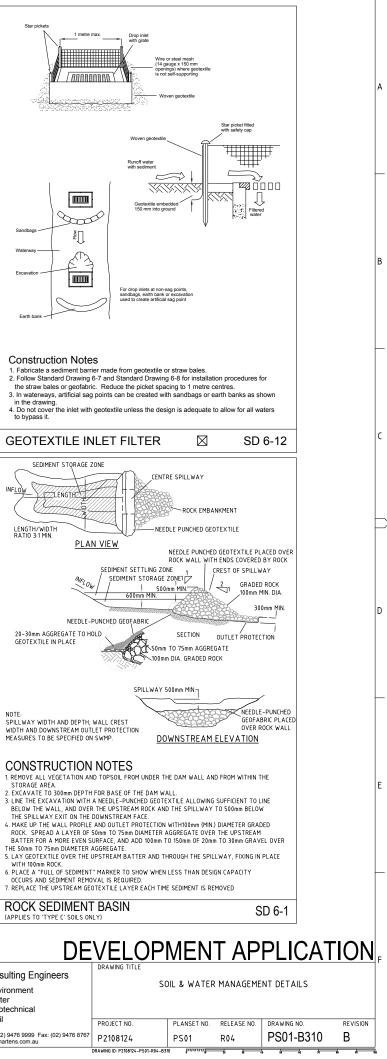
1. CHECK DAMS CAN BE BUILT WITH VARIOUS MATERIALS, INCLUDING ROCKS, LOGS, SANDBAGS AND STRAW BALES. THE MAINTENANCE PROGRAM SHOULD ENSURE THEIR INTEGRITY IS RETAINED, ESPECIALLY WHERE CONSTRUCTED WITH STRAW BALES. IN THE CASE OF BALES, THIS MIGHT REQUIRE THEIR REPLACEMENT EACH TWO TO FOUR MONTHS. 2. TRENCH THE CHECK DAM 200mm INTO THE GROUND ACROSS ITS WHOLE WIDTH. WHERE ROCK IS

USED, FILL THE TRENCHES TO AT LEAST 100mm ABOVE THE GROUND SURFACE TO REDUCE THE RISK OF UNDERCUTTING. B NORMALLY THEIR MAXIMUM HEIGHT SHOULD NOT EXCEED 600mm ABOVE THE GULLY ELOOR. THE 3. NORMALLT, THEIR MAXIMUM HELION I SHOULD NOT EXCEED BOUMM ABOVE THE GOLLT FLOOR. I CENTRE SHOULD ACT AS A SPILLWAY, BEING AT LEAST 150mm LOWER THAN THE OUTER EDGES. 4. SPACE THE DAMS SO THE TOE OF THE UPSTREAM DAM IS LEVEL WITH THE SPILLWAY OF THE NEXT DOWNSTREAM DAM.

SD 5-4

ROCK CHECK DAM

A1 / A3 LANDSCAPE (A1LC_v02.0.01



SWMP Commentary, Detailed Calculations

Note: These "Detailed Calculation" spreadsheets relate only to high erosion hazard lands as identified in figure 4.6 or where the designer chooses to use the RUSLE to size sediment basins. The "Standard Calculation" spreadsheets should be used on low erosion hazard lands as identified by figure 4.6 and where the designer chooses not to run the RUSLE in calculations.

1. Site Data Sheet

Site Name: P2008124

Site Location: 171 Forest Way, Belrose NSW 2085

Precinct: N/A

Description of Site: Somersby (so): Sydney Soil Landscape

Site area	Site	Remarks	
Sile alea	CAT 1		Remarks
Total catchment area (ha)	1.09		
Disturbed catchment area (ha)	0.77		

Soil analysis		
% sand (faction 0.02 to 2.00 mm	70	Soil texture should be assessed through
% silt (fraction 0.002 to 0.02 mm)	10	mechanical dispersion only. Dispersing
% day (fraction finer than 0.002 mm)	20	agents (e.g. Calgon) should not be used
Dispersion percentage	15.0	E.g. enter 10 for dispersion of 10%
% of whole soil dispersible	3.75	See Section 6.3.3(e)
Soil Texture Group	C	See Section 6.3.3(c), (d) and (e)

Rainfall data

Design rainfall depth (days)	7	See Sections 6.3.4 (d) and (e)
Design rainfall depth (percentile)	80	See Sections 6.3.4 (f) and (g)
x-day, y-percentile rainfall event	36.8	See Section 6.3.4 (h)
Rainfall intensity: 2-year, 6-hour storm	10.6	See IFD chart for the site

Rainfall erosivity (R-factor)	2460	Automatic calculation from above data
Soil erodibility (K-factor)	0.027	
Slope length (m)	100	
Slope gradient (%)	13.5	RUSLE data can be obtained from
Length/gradient (LS-factor)	5.1475	Appendixes A, B and C
Erosion control practice (P-factor)	1.3	
Ground cover (C-factor)		

Calculations

Soil loss (t/ha/yr)	444	
Soil Loss Class	4	See Section 4.4.2(b)
Soilloss (m ³ /ha/yr)	342	2 Decide
Sediment basin storage volume, m ³	45	See Sections 6.3.4(i) and 6.3.5 (e)

1

P208124JS03V01 RUSLE

SWMP Commentary, Detailed Calculations

2. Storm Flow Calculations

Peak flow is given by the Rational Formula:

Qy = 0.00278 x C₁₀ x F_Y x I_{y, to} x A

where:	Qy	is peak flow rate (m ³ /sec) of average recurrence interval (ARI) of "Y" years
	C ₁₀	is the runoff coefficient (dimensionless) for ARI of 10 years. Rural runoff coefficients are given in Volume 2, figure 5 of Pilgrim (1998), while urban runoff coefficients are given in Volume 1, Book VIII, figure 1.13 of Pilgrim (1998) and construction runoff coefficients are given in Appendix F
	Fγ	is a frequency factor for "Y" years. Rural values are given in Volume 1,

- Book IV, Table 1.1 of Pilgrim (1998) while urban coefficients are given in Volume 1, Book VIII, Table 1.6 of Pilgrim (1998) 's the catchment area in hectares (ha)
- $I_{y,{\bf tc}}$ is the average rainfall intensity (mm/hr) for an ARI of "Y" years
- and a design duration of "tc" (minutes or hours)

Time of concentration (t_c) = $0.76 \text{ x} (\text{A}/100)^{0.38}$ hrs (Volume 1, Book IV of Pilgrim, 1998)

Note: For urban catchments the time of concentration should be determined by more precise calculations or reduced by a factor of 50 per cent.

Peak flow calculations, 1

	A	tc	Rainfall intensity, I, mm/hr								
Site CAT 1	(ha)	(enim)	1 _{yr.to}	5 _{yr,to}	10 _{yr.to}	20 _{yr,to}	50 _{yr.to}	100 _{yr.ts}	C 10		
CAT 1	1.09	8	88.75	143.5	160.5	183.5	213	235.5	0.9		
							2				
2	8							2	2		
								1			
	05										

Peak flow calculations, 2

	Frequency								
ARI (yrs)	factor (Fy)	CAT 1 (m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	5 (m ³ /s)	(m3/s)	Comment	
1 yr,tc	0.8	0.194							
5 yr,tc	0.95	0.372							
10 yr,tc	1	0.438							
20 yr,tc	1.05	0.525			_				
50 yr,tc	1.15	0.668					9	6	
100 yr te	12	0.771				-			

P20

P208124JS03V01 RUSLE

	REV	DESCRIPTION	DATE	DRAW	N DESIG	GNED CH	ECKED	APPRVD	SCALE	GRID	DATUM	PROJECT MANAGER	CLIENT				
	В	MINOR AMENDMENTS	18/07/2022	JS	B	N /	AVG	GT		MGA	mAHD	GT	BELROSE RB1 PTY LTD			Consulting Eng	
101	A	INITIAL RELEASE	24/06/2021	JS	B	N	SA			MUA	MAHD		DELRUSE RDIFITLID			Environment	
<u>i</u>										DISCLAIMER 8	COPYRIGHT		PROJECT NAME/PLANSET TITLE		martens	Water	
										This plan must not be used for construction unless signed as approved by principal certifying authority.		on unless signed as approved by	MULTI UNIT RESIDENTIAL DEVELOPMENT				
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	-				_				-	All measurements	n millimetres unless o	therwise specified.	CONCEPT STORMWATER MANAGEMENT PLAN			Civil	
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5										consent of Martens	& Associates Pty Ltd.		171 FOREST WAY, BELROSE, NSW 2085	Suite 201, 20	George St, Hornsby, NSW 2077 Australia Ph	one: (02) 9476 9999 F	
										(C) Copyright Martens & Associates Pty Ltd		ites Pty Ltd			Email: mail@martens.com.au Internet: www.martens.com.au		
	A1 / A3	LANDSCAPE (A1LC v02.0.01)		•						•			•	•			

SWMP Commentary, Detailed Calculations

3. Volume of Sediment Basins: Type C Soils

Settling Zone Volume The settling zone volume for Type C soils is calculated to provide capacity to allow the design particle (e.g. 0.02 mm in diameter) to settle in the peak flow expected from the design storm (e.g. 0.25-year AR). The volume of the basin's settling zone (Y) can be determined as a function of the basin's surface area and depth to allow for particles to settle. Peak flow/discharge for the 0.25-year, ARI storm is given by the Rational Formula:

where:

Easin surface area (A) = area factor x $Q_{to,0.25}$ m2



Sediment Storage Zone V
In the detailed calculation on Soil
percent of the settling zone capa-
calculated by the RUSLE (Sectio

Place an "	X" in the	box below to show the sediment storage zo
		100% of settling zone capacity,
	Х	2 months soil loss calculated by RUSLE

Total Ba	asin Volu	ime	-
Site	Q _{to, 0.25} (m ³ /s)	Area factor	10000
CAT 1	0.097	635	l
-			┝
	1		Г

08124JS03V01	RUSLE		

Basin volume = settling zone volume + sediment storage volume

Q to 0.25 = 0.5 x [0.00278 x C10 x Fy x I 1yr, tc x A] (m3/sec)

Q to.0.25 = flow rate (m³/sec) for the 0.25 ARI storm event C₁₀ = runoff coefficient (dimensionless for ARI of 10 years)

Fy = frequency factor for 1 year ARI storm I _{tytte} = arequency factor for 1 year Ard storm I _{tytte} = average rainfall intensity (mm/hr) for the 1-year ARI storm A = area of catchment in hectares (ha)

Particle settling velocities under ideal conditions (Section 6.3.5(e))
Particle Size Area Factor

щe	Meanación	
8	170	Т
8	635	1
	4100	

Volume of settling zone = basin surface area x depth (Section 6.3.5(e)(ii))

olume

In the detailed calculation on Soil Loss Classes 1 to 4 lands, the sediment storage zone can be taken as 100 percent of the settling zone capacity. Alternately designers can design the zone to store the 2-month soil loss as calculated by the RUSLE (Section 6.3.5(e)(iv)). However, on Soil Loss Classes 5, 6 and 7 lands, the zone must contain the 2-month soil loss as calculated by the RUSLE (Section 6.3.5(e)(v)).

o show the sediment storage zone design parameters used here:

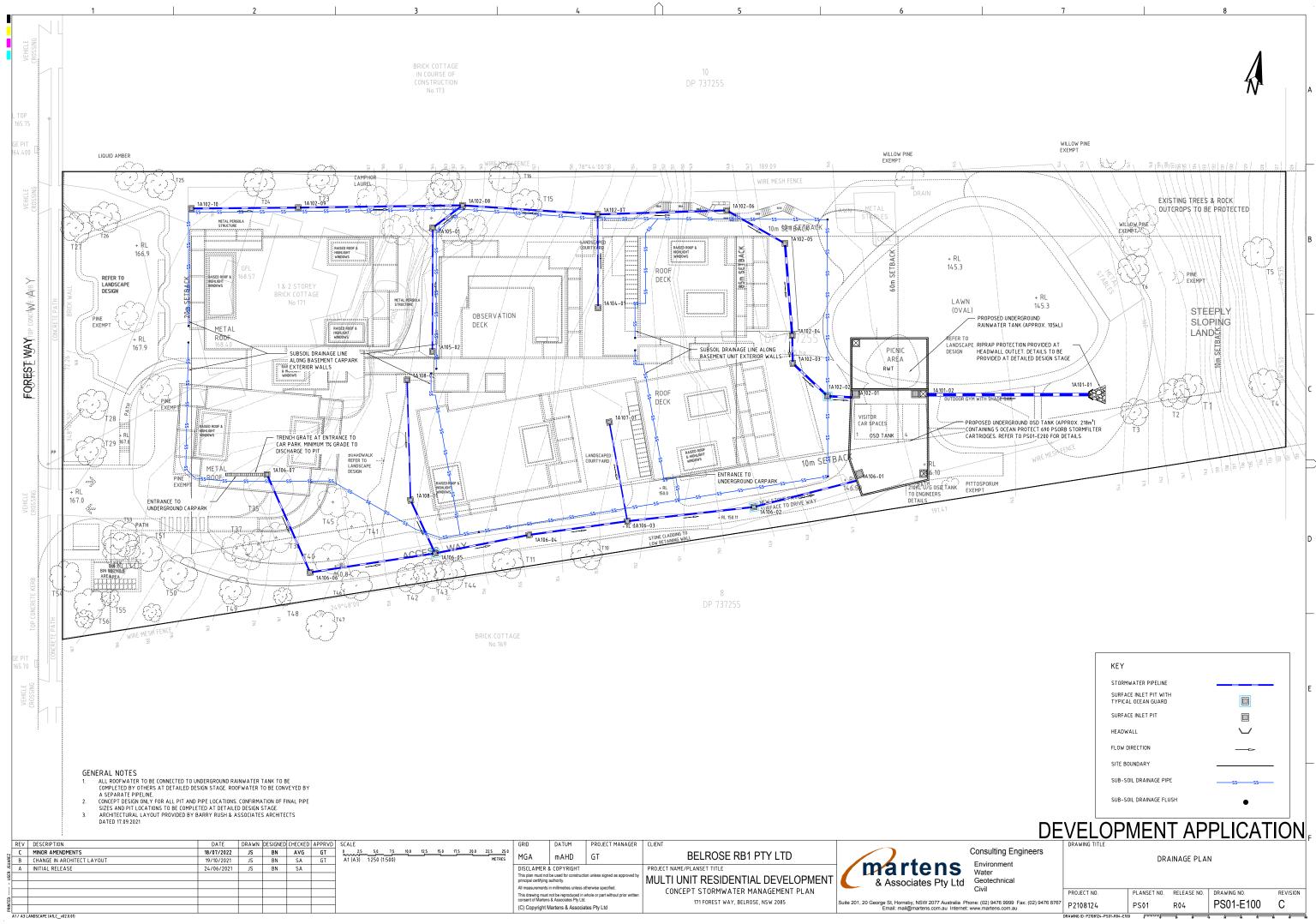
Basin surface	Depth of	Settling zone	Sediment storage	Total basin	Basin shape				
area (m²)	settling zone (m)	volume (m ⁸)	volume (m ³)	volume (m ⁸)	L:W Ratio	Length (m)	Width (m)		
61	0.6	37	45	82	3	13.6	4.5		

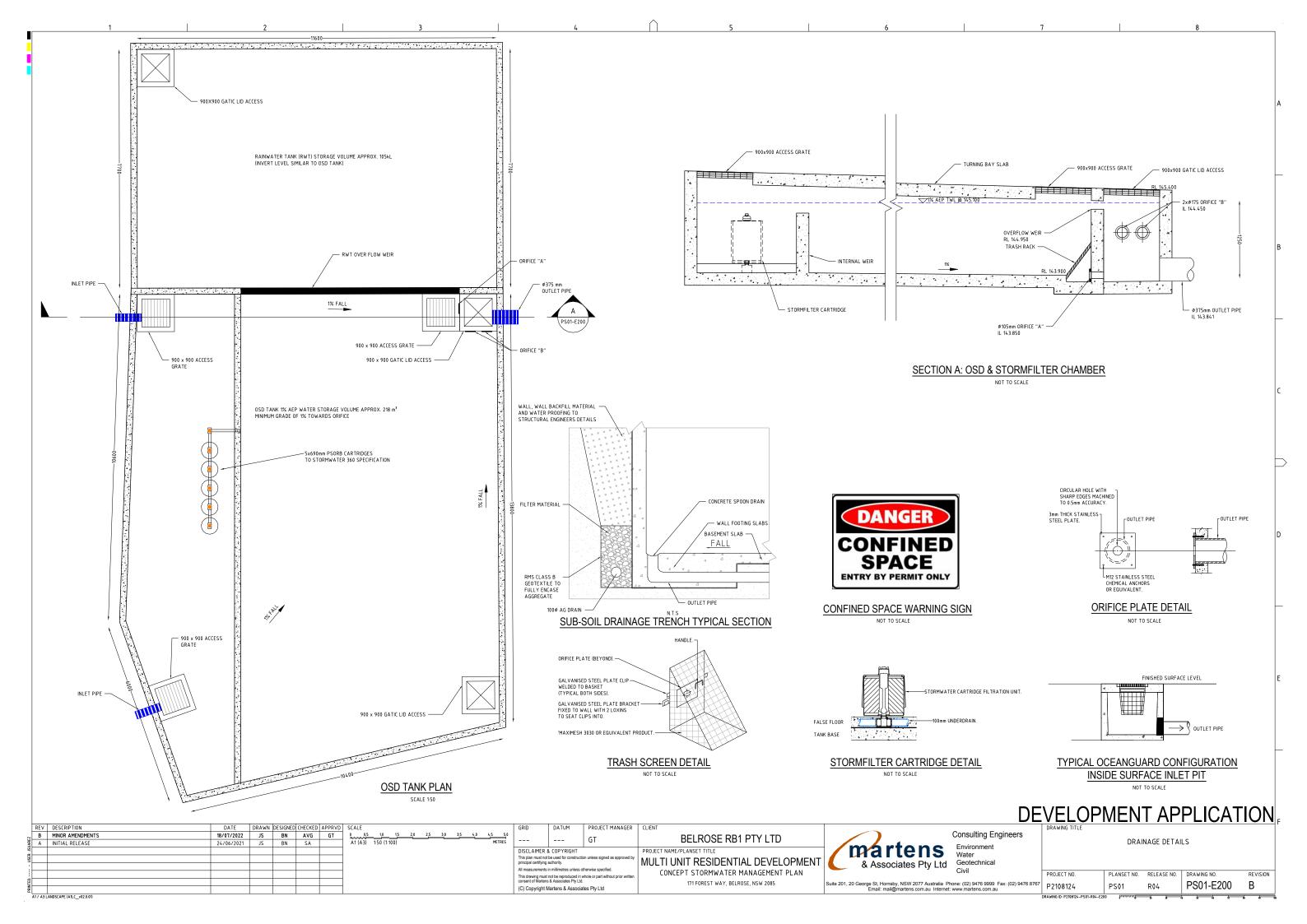
DEVELOPMENT APPLICATION

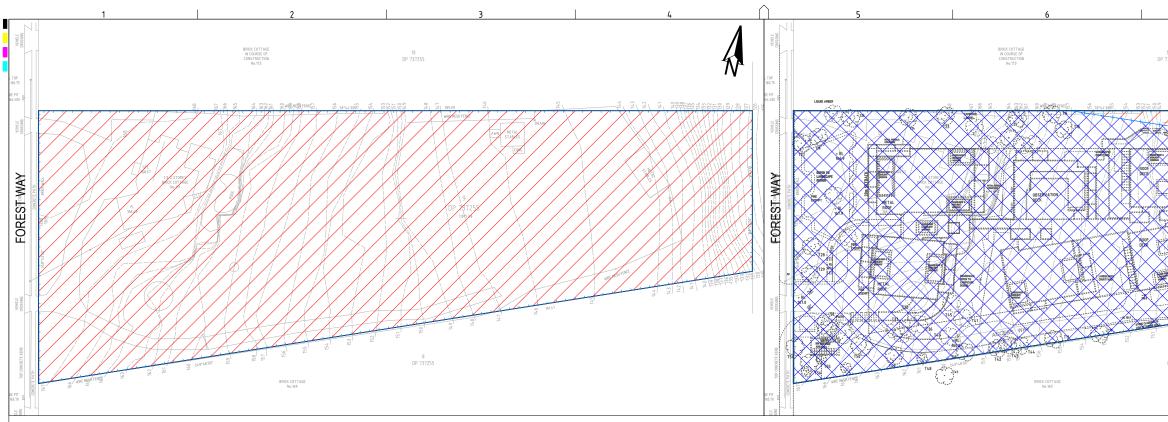
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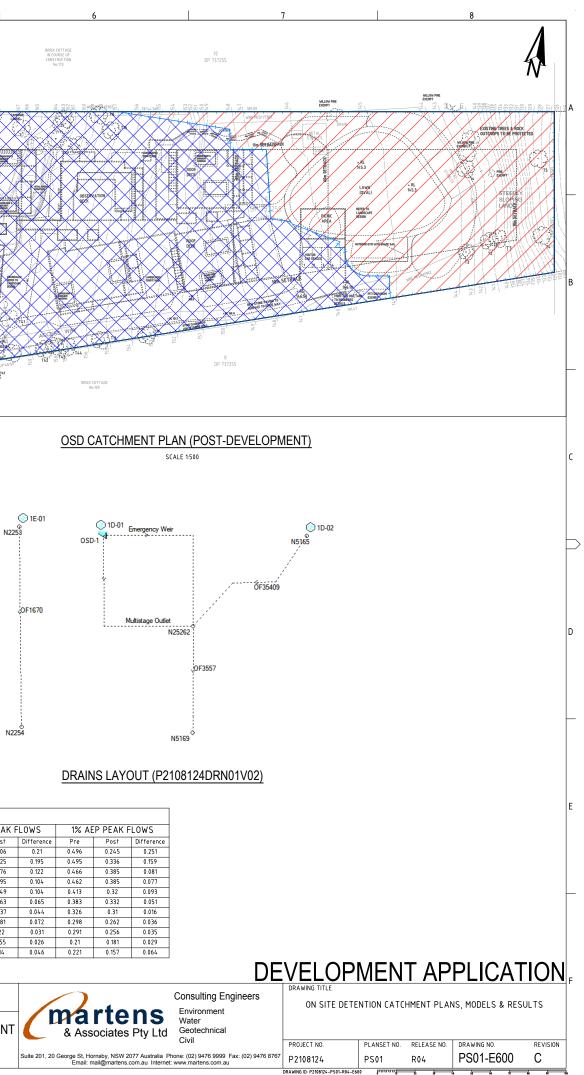
SEDIMENT BASIN CALCULATIONS

	PROJECT NO.	PLANSET NO.	RELEASE NO.	DRAWING NO.	REVISION
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	DRAWING ID: P2108124-PS01-R04-B31	։ հորորե		* * *	4 4 4









					DRA	INS OSE) Mode	ELLING	RESULT	S (P21	08124D	RN01V0	2)					
	20% A	EP PEAK	FLOWS	0.2 E	Y PEAK F	LOWS	10% A	EP PEAK	FLOWS	5% A	EP PEAK	LOWS	2% A	EP PEAK F	LOWS	1% AI	EP PEAK	FLOWS
Duration	Pre	Post	Difference	Pre	Post	Difference	Pre	Post	Difference	Pre	Post	Difference	Pre	Post	Difference	Pre	Post	Difference
10 min	0.167	0.083	0.084	0.175	0.085	0.09	0.241	0.241	0.107	0.314	0.15	0.164	0.416	0.206	0.21	0.496	0.245	0.251
15 min	0.202	0.087	0.115	0.208	0.09	0.118	0.269	0.269	0.141	0.334	0.181	0.153	0.42	0.225	0.195	0.495	0.336	0.159
20 min	0.189	0.1	0.089	0.196	0.107	0.089	0.266	0.266	0.148	0.328	0.183	0.145	0.398	0.276	0.122	0.466	0.385	0.081
25 min	0.19	0.102	0.088	0.196	0.107	0.089	0.249	0.249	0.136	0.311	0.169	0.142	0.399	0.295	0.104	0.462	0.385	0.077
30 min	0.176	0.105	0.071	0.185	0.111	0.074	0.225	0.225	0.143	0.282	0.174	0.108	0.353	0.249	0.104	0.413	0.32	0.093
45 min	0.194	0.116	0.078	0.203	0.121	0.082	0.18	0.18	0.142	0.234	0.171	0.063	0.328	0.263	0.065	0.383	0.332	0.051
1 hour	0.155	0.116	0.039	0.16	0.119	0.041	0.251	0.251	0.149	0.3	0.184	0.116	0.281	0.237	0.044	0.326	0.31	0.016
1.5 hou	0.119	0.096	0.023	0.127	0.099	0.028	0.155	0.155	0.124	0.2	0.155	0.045	0.253	0.181	0.072	0.298	0.262	0.036
2 hour	0.148	0.107	0.041	0.153	0.109	0.044	0.169	0.169	0.116	0.21	0.141	0.069	0.251	0.22	0.031	0.291	0.256	0.035
3 hour	0.088	0.074	0.014	0.091	0.077	0.014	0.127	0.127	0.098	0.153	0.127	0.026	0.181	0.155	0.026	0.21	0.181	0.029
4.5 hou	0.073	0.072	0.001	0.075	0.075	0	0.094	0.094	0.088	0.114	0.103	0.011	0.186	0.14	0.046	0.221	0.157	0.064

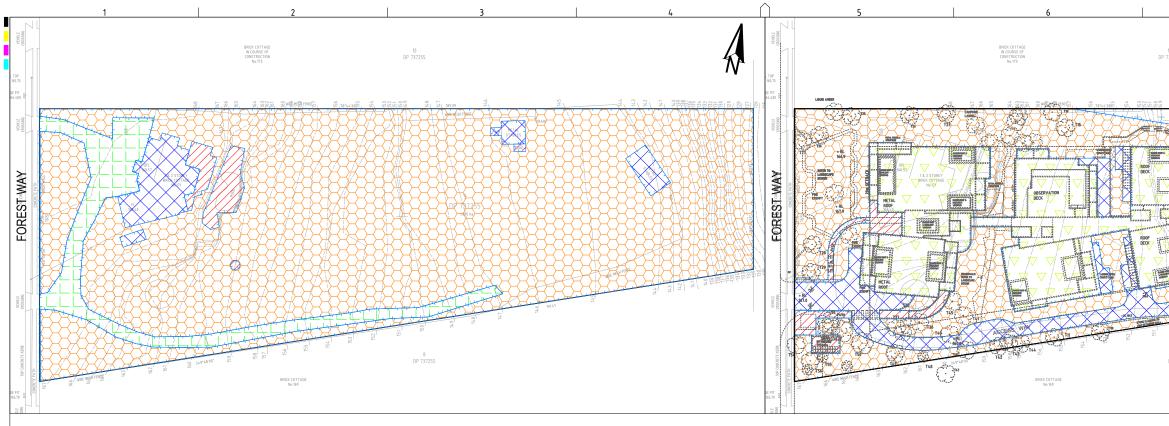
R	V DESCRIPTION	DATE	DRAW	N DESIGNE	D CHECKED	APPRVD	SCALE	GRID	DATUM	PROJECT MANAGER	CLIENT		
	MINOR AMENDMENTS	18/07/2022	JS	BN	AVG	GT		MGA	mAHD	GT	BELROSE RB1 PTY LTD		Consulting
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ISL	INITIAL RELEASE	24/06/2021	JS	BN	SA			DISCLAIMER	& COPYRIGHT		PROJECT NAME/PLANSET TITLE	martens	Motor
USEF										tion unless signed as approved by	MULTI UNIT RESIDENTIAL DEVELOPMENT		Geotechn
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										whole or part without prior written	CONCEPT STORMWATER MANAGEMENT PLAN		CIVII
Ë									s & Associates Pty Ltd		171 FOREST WAY, BELROSE, NSW 2085	Suite 201, 20 George St, Hornsby, NSW 2077 Australia F	Phone: (02) 9476 9
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A1.	A3 LANDSCAPE (A1LC_v02.0.01)											1	

OSD CATCHMENT PLAN (PRE-DEVELOPMENT)

SCALE 1:500

PRE I	PRE DEVELOPMENT CATCHMENT DETAILS								
	(P2008021DRN01V02)								
KEY	KEY DRAINS NODE AREA (ha) % PAVED								
	1.00E-01	1.09	14%						
	TOTAL AREA	1.09	= 100% OF TOTAL AREA						
	TOTAL IMPERVIOUS AREA 0.15 = 14% OF TOTAL AREA								
	TOTAL PERVIOUS AREA	0.93	= 86% OF TOTAL AREA						

POST	POST DEVELOPMENT CATCHMENT DETAILS								
	(P2008021DRN01V02)								
KEY	DRAINS NODE	AREA (ha)	% PAVED						
	1D-01	0.75	47%						
	1D-02	0.33	36%						
	TOTAL AREA	1.09	= 100% OF TOTAL AREA						
TOTAL IMPERVIOUS AREA 0.47 = 43% OF TOTAL AREA									
	TOTAL PERVIOUS AREA	0.61	= 57% OF TOTAL AREA						



MUSIC MODELLING CATCHMENT PLAN (PRE-DEVELOPMENT) SCALE 1:500

T01

RUNOFF NT 55.8%

SCALE 1:500

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TITLE ESIDENTIAL DEVELOPMENT RMWATER MANAGEMENT PLAN	& Associates Pty Ltd	Water Geotechnical Civil
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Pre-D

MUSIC MODELLING RESULTS FOR WATER QUANTITY (P2108124MUS02V02)								
PARAMETER	PRE-DEVELOPMENT	POST-DEVELOPMENT	VOLUMETRIC RU COEFFICIEN					
FLOW (ML/yr)	5.51	5.87	1.07					
NO. STORM EVENTS/YEAR	187	176	-					

MUSIC MODELLING RESULTS FOR WATER QUALITY PRE VS POST DEVELOPMENT (P2108124MUS01V02) PRE-DEVELOPMENT | POST-DEVELOPMENT | PARAMETER % CHANGE

FARAPILIER	FRE-DEVELOFFILINT	FUST-DEVELOFMENT	76 CHANUL
TOTAL SUSPENDED SOLIDS (kg/yr)	468	89.6	-80.9%
TOTAL PHOSPHORUS (kg/yr)	1.29	0.48	-62.8%
TOTAL NITROGEN (kg/yr)	8.99	6.41	-28.7%

MUSIC MODELLING RESULTS FOR WATER QUALITY TARGETS										
(P2108124MUS01V02)										
PARAMETER SOURCES RESIDUAL LOAD % REDUCTION										
TAL SUSPENDED SOLIDS (kg/yr)	603	89.6	85.1%							
TOTAL PHOSPHORUS (kg/yr)	1.67	0.48	71.3%							
TOTAL NITROGEN (kg/yr)	14.5	6.41	55.8%							

PRE DEVELOPMENT MUSIC CATCHMENT DETAILS (P2008124MUS01V02) KEY MUSIC NODE AREA (ha) % EFFECTVE IMPERVIOUS AREA										
	1.00E-02	0.09	100%							
	1.00E-03	0.02	100%							
	1.00E-04	0.05	100%							
~~~~~~	TOTAL AREA	1.09	= 100% OF TOTAL AREA							
	TOTAL IMPERVIOUS AREA	0.16	= 14% OF TOTAL AREA							
	TOTAL PERVIOUS AREA	0.93	= 86% OF TOTAL AREA							

POST DEVELOPMENT MUSIC CATCHMENT DETAILS (P2008124MUS01V02)										
KEY	MUSIC NODE	% EFFECTVE IMPERVIOUS AREA								
	1D-01	0.33	0%							
	1D-02	0.06	100%							
	1D-03	0.03	100%							
	1D-04	0.04	100%							
	1D-05	0.27	100%							
+ + + + + + + + + + + + + + + + + + +	1D-06	0.33	0%							
	1D-07	0.02	100%							
	TOTAL AREA	1.09	= 100% OF TOTAL AREA							
	TOTAL IMPERVIOUS AREA	0.42	= 39% OF TOTAL AREA							
	TOTAL PERVIOUS AREA	0.66	= 61% OF TOTAL AREA							

RE	V	DESCRIPTION	DATE	DRAWN	DESIGNED	CHECKED	APPRVD	SCALE	GRID	DATUM	PROJECT MANAGER	CLIENT	
N (		MINOR AMENDMENTS	18/07/2022	JS	BN	AVG	GT	0 5 10 15 20 25 30 35 40 45 50	MGA	mAHD	GT	BELROSE RB1 PTY LTD	Co
BAR B	3	CHANGE IN ARCHITECT LAYOUT	19/10/2021	JS	BN	SA	GT	A1 (A3) 1:500 (1:1,000) METRES	MUA				
S A	1	INITIAL RELEASE	24/06/2021	JS	BN	SA			DISCLAIMER	& COPYRIGHT		PROJECT NAME/PLANSET TITLE	martens 🖩
USEF									This plan must not principal certifying		ion unless signed as approved by	MULTI UNIT RESIDENTIAL DEVELOPMENT	
										in millimetres unless o			& Associates Pty Ltd
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A1 / A	A 1 / A 3 LANDSCAPE (ALC_v02.0.1)												

