



ASCENT

GEOTECHNICAL CONSULTING

Geotechnical Assessment

Project: New Residence
189 Riverview Road, Avalon Beach NSW.


Prepared for:
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Geotechnical Assessment

For New Dwelling at

189 Riverview Road, Avalon Beach NSW

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Limitations

This report has been prepared for Jamie Durie, in accordance with Ascent Geotechnical Consulting's (Ascent) Fee Proposal dated 1 September 2020.

The report is provided for the exclusive use of the property owner and their nominated agents for the specific development and purpose as described in the report. This report must not be used for purposes other than those outlined in the report or applied to any other projects.

The information contained within this report is considered accurate at the time of issue with regard to the current conditions onsite as identified by Ascent and the documentation provided by others.

The report should be read in its entirety and should not be separated from its attachments or supporting notes. It should not have sections removed or included in other documents without the express approval of Ascent.

Contents

1	Overview.....	3
1.1	Background.....	3
1.2	Proposed Development.....	3
1.3	Relevant Instruments	3
2	Site Description.....	4
2.1	Summary	4
2.2	Geology and Geological Interpretation	5
2.3	Fieldwork.....	6
3	Geotechnical Assessment	7
3.1	Site Classification.....	7
3.2	Groundwater	7
3.3	Surface Water.....	7
3.4	Slope Instability	8
3.6	Recommendations	11
4	References.....	17
5	Appendices	
	Appendix A: General notes	
	CSIRO Publishing, 2012. ‘Foundation Maintenance and Footing Performance: A Homeowners Guide’, Sheet BTF-18.	
	Australian GeoGuide LR8, 2007. ‘Examples of Good/Bad Hillside Construction Practice’.	
	Australian Geomechanics, 2007. ‘Practice Note Guidelines for Landslide Management’, Appendix C: Qualitative Terminology.	
	Appendix B: Site Plan/Ground Test Locations & Geological Cross Section	
	Appendix C: Engineering logs	
	Appendix D: Northern Beaches Council – Pittwater Geotechnical Forms 1 & 1A	

1 Overview

1.1 Background

This report presents the findings of a limited geotechnical assessment carried out at 189 Riverview Road, Avalon Beach NSW (the “Site”), by Ascent Geotechnical Consulting (Ascent). This assessment has been prepared to meet Northern Beaches Council lodgement requirements for Development Application (DA), as well as informing detailed structural design and construction methodology.

1.2 Proposed Development

Details of the proposed development are outlined in a series of architectural plans prepared by Durie Design, Job DD, drawing numbers 001: 050, 100-102, 120-121, 200-206, 210-212, 240-241, 250-271, 300-303, 310-311, 320-321, revision K, dated 15 November 2021; and architectural plans for proposed inclined lift prepared by Durie Design, drawing number 001: 050, revision A, undated.

The proposed works comprise the following:

- Demolition of existing residence, and soft and hard landscaping
- Construction of new terraced multi-level dwelling, incorporating internal lift, courtyards, green roof, terraces, decks, above ground swimming pool, and inclined lift.
- Construction of new vehicle crossover, driveway, garage, and workshop.

The proposed development will take place on a 1016.0m² (by Title) residential block being Lot C Sec in DP 381427 (929.5m²), and Lot Sec LIC 567410 (86.5m²).

1.3 Relevant Instruments

This geotechnical assessment has been prepared in accordance with the following relevant guidelines and standards:

- Northern Beaches Council – Pittwater Local Environment Plan (LEP) 2014 and Pittwater Development Control Plan (DCP) 2014
- Appendix 5 (to Pittwater P21) Geotechnical Risk Management Policy for Pittwater – 2009
- Australian Geomechanics Society’s Landslide Risk Management Guidelines AGS2007
- Australian Standard 1726–2017 Geotechnical Site Investigations
- Australian Standard 2870–2011 Residential Slabs and Footings
- Australian Standard 1289.6.3.2–1997 Methods of Testing Soils for Engineering Purposes
- Australian Standard 3798–2007 Guidelines on Earthworks for Commercial and Residential Developments.

2 Site Description

2.1 Summary

A summary of site conditions identified at the time of our Assessment is provided in Table 1.

Table 1: Summary of site conditions

Parameter	Description
Site visit	Ben Morgan, Geotechnical Engineer – 15/09/20 & 17/09/20
Site address	189 Riverview Road, Avalon Beach NSW – Lot C DP 381427 & Lot Sec LIC 567410
Site area m ² (approx.)	1016.0m ² (by Title) Lot C Sec in DP 381427 (929.5m ²), and Lot Sec LIC 567410 (86.5m ²)
Existing development	Part two-story brick and timber clad residence with a fibro roof
Slope aspect	West
Average gradient	~27 degrees, sections of the slope drop at significantly higher gradients.
Vegetation	Well-maintained garden beds, small to medium shrubs and palms, and medium to large palms and trees.
Retaining structures	Stable, recently constructed timber, metal mesh and loose rock walls across the western portion of the slope. Stable mortared and loose sandstone rock, block and flagging walls.
Neighbouring environment	Residentially developed to the north and south. Riverview Road to the east. Pittwater to the west.



Image 1: Site location – 189 Riverview Road, Avalon Beach NSW (© SIX Maps NSW Gov)

2.2 Geology and Geological Interpretation

The Sydney 1:100,000 Geological Sheet 9130 (NSW Dept. Mineral Resources, 1983) indicates that the site is underlain by the rocks of the Newport Formation of the upper Narrabeen Group (Rnn). The Newport Formation is typically comprised of interbedded laminite, shale and quartz to lithic quartz sandstones. The Newport Formation sandstone is difficult to distinguish from the overlying Hawkesbury Sandstone. The Hawkesbury Sandstones form capping units in this area found at the higher topographic locations, with the Newport Formation Geology forming the dominant geology of the flanking slopes.

Based on available geological mapping, and visual assessment of the site, it is likely that this site is underlain predominately by upper Newport Formation geology, with abundant upper Newport Formation/Hawkesbury Sandstone floaters and detached joint blocks, entrained in the upper profile. These floaters have been transported downslope over long periods of time, as the steep flanking slopes of the Newport Formation erode and undermine the capping sandstones.

A sandstone escarpment extends approximately north-south across the middle of the block. The escarpment rises approximately 4–6m. The escarpment is undercut along the southern boundary, forming a cave. Honeycomb weathering was identified in the roof of the cave. Significant sub-horizontal jointing was identified in the lower portion of the cantilever arm of the cave roof.

A smaller cave was identified at the northern boundary of the site. This cave is approximately 2.0m deep and 1.5m in height. Small sections of the cave roof show evidence of sub horizontal jointing. The cave is situated at the base of a large joint block, which is at least partially detached from the escarpment at its eastern end. An immature fig tree is situated on top of the joint block, its roots can be seen extending into the joint. While the joint block is currently considered stable, it was difficult to determine the extent of the joint block's attachment to the escarpment and its stability from the subject site. Further investigation, with consent to access from the northern neighbouring property, is recommended to establish its stability and to comment on requirements for any potential stabilisation works.

The soil profile consists of shallow uncontrolled fill and silty/sandy topsoil (O & A Horizons), silty clay (B Horizon), and weathered sandstone and shale bedrock (C Horizon). Based on our observations and the results of testing onsite, we would expect competent weathered bedrock to be found within 1000–2000mm from current surface levels across the site, where not already exposed at the surface.

NOTE: The local geology is comprised predominantly of shale, siltstone and sandstone. Sandstone floaters or large detached joint blocks are often found in the soil profile. The shale and sandstone bedrock are often found in benched terraces, subsequently ground conditions on site may alter significantly across short distances. This variability should be anticipated and accounted for in the design and construction of any new foundations.

2.3 Fieldwork

A site investigation was undertaken on the 17 September 2020, which included a geotechnically focused visual assessment of the property and its surrounds, geotechnical mapping, photographic record and limited subsurface investigation.

Eight (8) Dynamic Cone Penetrometer (DCP) tests were carried out to determine the relative density of the subgrade, and the depth to weathered rock (if encountered). These tests were conducted to the Australian Standard for ground testing: AS 1289.6.3.2–1997. Possible locations of testing were constrained by existing structures, sandstone floaters, hard surfaces and the presence of utilities. The location of these tests is shown on the site plan provided and summary of the test results is presented in Table 2, with full details in the engineering logs presented in the appendix section of this report.

Table 2: Summary DCP test results

TEST	DCP 1	DCP 2	DCP 3	DCP 4
SUMMARY	Refusal @ 1.95m Bouncing on bedrock. Orange/ brown fine-grained impact dust on wet tip.	Refusal @ 1.5m Bouncing on bedrock. Wet rods to 1.3m. Orange brown fine-grained impact dust on wet tip.	Refusal @ 1.65m Bouncing on bedrock. Damp rods to 0.9m. White impact dust on wet tip.	Refusal @ 0.50m Bouncing on floater. Clean and dry tip
TEST	DCP 5	DCP 6	DCP 7	DCP 8
SUMMARY	Refusal @ 1.55m Bouncing on bedrock. White impact dust on dry tip.	Refusal @ 0.50m Bouncing on bedrock. Orange/ white silt on damp tip.	Refusal @ 1.65m Bouncing on bedrock. Orange yellow silt on dry tip.	Refusal @ 2.3m Bouncing on bedrock. Orange/ white silt on dry tip.

Excavation Cut Logging

Assessment of the soil materials was possible due to existing exposed cut batters; Hand Auger boreholes were deleted from the site work in favour of additional DCP tests.

NOTE: The nature of the stratigraphy and geology of this particular slope make preliminary subsurface investigations using traditional hand methods such as DCP and Hand Auger very difficult. The equipment chosen to undertake ground investigations provides the most cost-effective method for understanding the subsurface conditions. Our interpretation of the subsurface conditions is limited to the results of testing undertaken and the known geology in the area. While every care is taken to accurately identify the subsurface conditions onsite, variation between the interpreted model presented herein, and the actual conditions onsite may occur. Should actual ground conditions vary from those anticipated, we would recommend the geotechnical engineer be informed as soon as possible to advise if modifications to our recommendations are required.

3 Geotechnical Assessment

3.1 Site Classification

Due to the presence of shallow uncontrolled fill, the steep natural gradient, and existing and recently removed trees, the Site is classified as “P” in accordance with AS 2870–2011.

Footings taken to and socketed into the underlying weathered bedrock may be designed in accordance with an “A” Classification.

3.2 Groundwater

Normal groundwater seepage is expected to move downslope through the soil profile along the interface with underlying bedrock, or any impervious horizons in the profile such as clays.

Due to the position of the block relative to the slope and the underlying geology, no significant standing water table is expected to influence the portion of the site to be developed.

The western boundary of the block corresponds to the mean high-water mark for Pittwater. Ground water at the extreme western portion of the block is expected to be influenced by tidal fluctuations.

3.3 Surface Water

Overland or surface flows entering the site from the adjoining areas were not identified at the time of our inspection, however normal overland runoff could enter the site from above during heavy or extended rainfall.

3.4 Slope Instability

A landslide hazard assessment of the existing slope has been undertaken in accordance with Australian Geomechanics Society's 'Landslide Risk Management', published in March 2007.

- No evidence of significant soil creep, or tension cracks were identified across the site or on adjacent properties, as viewed from the subject site, at the time of our inspection.
- The sub-vertical sandstone face that extends north-south through the centre of the block is comprised on massive sandstone and is considered stable in its condition at the time of inspection.
- The cave situated at the southern boundary is considered stable at the time of our assessment, though it should be noted that predicting the possible failure of sandstone outcrops of this kind can be difficult. It is considered likely that should a failure occur; it would likely result from the failure of the sub-horizontal joint in the lower portion of the cave roof cantilever arm.
- A large (~40 tonne), potentially unstable sandstone block is situated on the boundary between the subject site and 191 Riverview Road. The block had been formed by at least partially open joints on the eastern and northern sides. A 2.5 – 3.0m wide and 1.5 – 2.0m undercut or cave is situated at the base of the block. Our initial advice was that additional investigation would be required to determine the potential instability of the block, and any supporting works that may be required. This work has subsequently been carried out by JK Geotechnics (Ref:33589BNM Site report 1, 20/11/2020).
- Minor movement of the recently constructed timber walls can be attributed to inadequate spacing and embedment of supporting posts, rather than slope instability.
- Based on reference to the plan entitled "Geotechnical Hazard Mapping" (Ref. P21DCP-BC-MDCP2002, dated 2007) prepared by GHD LONGMAC on behalf of Pittwater Council, the site is mapped as a **Geotechnical Hazard H1** zone.

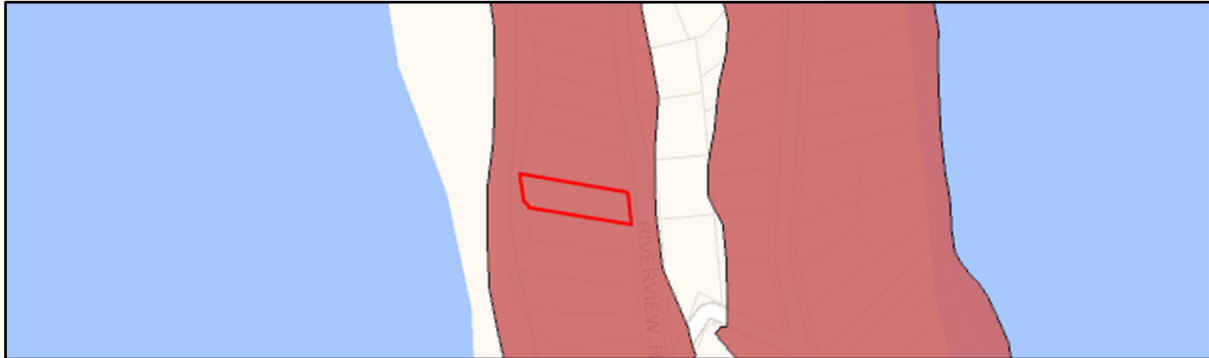
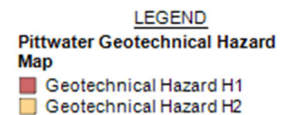


Image 2. PLEP Geotechnical Hazard Map
– 189 Riverview Road, Avalon Beach NSW (NBC Maps)



3.5 Geotechnical Hazards and Risk Analysis

The following four potential landslide hazards have been identified with reference to the existing site conditions and the proposed works:

- **Hazard One** – Insatiability of existing sandstone stack rock and timber/loose stone/metal mesh retention systems
- **Hazard Two** – Instability of slope materials
- **Hazard Three** – Instability of rock outcrops and overhangs
- **Hazard Four** – Instability of temporary excavation batter during construction.

Table 3 below provides a summary of our qualitative assessment of the hazards identified and the consequences to property and life should the identified hazard occur. The terms and methods used in this assessment are outlined in Appendix A of this report and are based on ‘Guidelines for Landslide Risk Management’ (Australian Geomechanics Society, 2007), and as adopted by NBC Pittwater Risk Management Policy PLEP.

Table 3. Landslide Risk Assessment

HAZARD	HAZARD ONE	HAZARD TWO
TYPE	Failure or collapse of existing sandstone stack-rock, and timber/loose rock/metal mesh walls	Landslip of existing soil materials
LIKELIHOOD	‘Possible’ (10^{-3})	‘Unlikely’ (10^{-4})
CONSEQUENCES TO PROPERTY	‘Insignificant’ (0.5%)	‘Insignificant’ (0.5%)
RISK TO PROPERTY	‘Very Low’	‘Very Low’
RISK TO LIFE	2×10^{-9} /annum	1.5×10^{-9} /annum
COMMENTS	The level of risk is ‘Acceptable’ subject to adherence to the recommendations outlined in Section 3.6.	The level of risk is ‘Acceptable’ subject to adherence to the recommendations outlined in Section 3.6.

HAZARD	HAZARD THREE	HAZARD FOUR
TYPE	Collapse or failure of rock outcrops/overhangs/blocks	Failure of temporary excavation batter during construction
LIKELIHOOD	‘Possible’ (10^{-3})	‘Possible’ (10^{-3})
CONSEQUENCES TO PROPERTY	‘Medium’ (20%)	‘insignificant’ (5%)
RISK TO PROPERTY	‘Moderate’	‘Very Low’
RISK TO LIFE	5×10^{-6} /annum	3.5×10^{-8} /annum
COMMENTS	The risk level is currently assessed as ‘UNACCEPTABLE’, To move this level of risk to an acceptable level, the works outlined in the JK Geotechnics site report should be carried out, and certified by JKG.	The level of risk is ‘Acceptable’ subject to adherence to the recommendations outlined in Section 3.6.

3.6 Recommendations

The proposed development is considered to be suitable for the site. No significant geotechnical hazards will result from the completion of the proposed development ***provided the recommendations presented in Table 3 are adhered to during design and construction.***

It should be noted that these recommendations are based on our site assessment, limited ground testing, and our experience of similar slopes. More intensive ground testing, including mechanical drilling, and laboratory testing of soils and rock samples may be required to provide more accurate recommendation for structural design of foundation and retention systems.

Table 4: Geotechnical Recommendations

Recommendation	Description
Dilapidation Reporting	We would recommend that detailed dilapidation reporting, undertaken by others, be prepared for all adjacent structures, and council infrastructure, before the demolition, excavation or construction commence onsite.
Excavation	<p>Significant excavation is proposed for the installation of the lift shaft, and to accommodate levels 2 and 3. The excavation is expected to extend to an approximate maximum depth of 7.0m from ~RL 19.0 - ~26.0. Based on the results of our testing, and the known geology of the area, we would expect the excavation to extend through ~1-2m of fill, silty soil, and silt clay, before medium strength weathered shale and sandstone is encountered, with the possibility of encountering bands of high strength indurated sandstone (ironstone). Abundant sandstone boulders/floaters, of varying sizes are expected to be located on and in the upper profile of the slope, with embedment depth likely to increase downslope.</p> <p>Given the likely composition of the excavation cut, and the gradient of the slope temporary and permanent support systems will be required before bulk excavations commence on site.</p> <p>It is strongly recommended that an excavation contractor with demonstrable experience in this type of project be engaged to undertake the proposed works with the appropriate care and diligence.</p> <p>Further ground testing, in the form of mechanical drilling, may be deemed necessary to provide additional information for structural design of retention and footing systems.</p>

Recommendation	Description
Soil Excavation	<p>Soil excavation will be required to establish new pad levels and new footings across the site. It is anticipated that these excavations will encounter shallow uncontrolled fill and silty topsoil, silty clay, and weathered bedrock. The excavation of soil, clay and extremely weathered rock should be possible with the use of bucket excavators, rippers, or for piered footings, traditional auger attachments.</p> <p>For shallow excavations (<1.0m), provided the residual soil is battered back to a minimum of 45 degrees, they should remain stable without support for a short period until permanent support is in place. Permanent batters are not considered appropriate for this site.</p> <p>For deep excavations, it is recommended that an array of closely spaced piers, socketed into at least medium strength sandstone bedrock, with reinforced shotcrete infill panels be installed before, and as part of a controlled, staged excavation.</p>
Rock Excavation	<p>It is strongly recommended that an excavation contractor with demonstrable experience in this type of project be engaged to undertake the proposed works with the appropriate care and diligence. Ascent are happy to recommend professional contractors who have completed similar projects to a high standard in the past.</p> <p>All excavation recommendations as outlined below should be read in conjunction with Safe Work Australia's <i>Code of Practice: Excavation Work</i>, published October 2018.</p> <p>It is essential that any excavation through rock that cannot be readily achieved with a bucket excavator or ripper should be carried out initially using a rock saw to minimise the vibration impact and disturbance on the adjoining properties, and existing structures, and rock outcrops. Any rock breaking must be carried out only after the rock has been sawed, and in short bursts (2–5 seconds), to prevent the vibration amplifying. The break in the rock from the saw must be between the rock to be broken and the closest adjoining structure.</p> <p>All excavated material is to be removed from the site in accordance with current Office of Environment and Heritage (OEH) regulations.</p>

Recommendation	Description
Vibrations	<p>Australian Standard AS2670.1–2001 ‘Evaluation of human exposure to whole-body vibration General requirements. Part 1: General requirements,’ suggests a daytime limit of 5 mm/s component PPV for human comfort is acceptable.</p> <p>Where rock hammers are adopted for bulk rock excavation there is an inherent risk that the vibrations may result in damage to existing and adjacent structures, and rock outcrops. To mitigate such risk, we recommend vibration monitoring be carried out as part of the proposed works.</p> <p>We would suggest allowable vibration limits be set at 5mm/s PPV, and monitoring devices installed at the footing level of any adjacent structures, as well as at sensitive rock outcrops. It is expected that rock hammers with an approximate weight of 300–500kg will be adequate to operate within these tolerances. It may be necessary to move to smaller rock hammers or to rotary grinders or rock saws if vibrations limits cannot be met. Manufactures of the plant should be contacted for information regarding peak vibration output.</p> <p>The propagation of vibrations can be mitigated by pulsing the use of rock hammers, i.e. short bursts, utilising line sawing along boundaries.</p>
Excavation Support	<p>Temporary batter slopes of 1.0V : 1.0H are recommended for excavations in soil and clay up to 1.0m. Due to the gradient and composition of the site, excavations >1.0m are to be supported by permanent supporting systems, prior to or as part of a staged excavation.</p> <p>The excavation will require temporary and permanent support to be installed prior to, and as part of a controlled excavation. It is recommended that an array of spaced concrete piers be installed and socketed at least 500mm into at least medium strength bedrock prior to any bulk excavations. As the excavation progresses, regular pre-determined hold points (1.5m drops) should be established for reinforced shotcrete infill panels, rear wall drainage, and rock anchors, or structural bracing as required.</p> <p>Medium to large sandstone boulders/floaters are expected to be found on and in the slope. Where possible the removal of these boulders before commencement of excavation works would be advantageous.</p> <p>Where removal of boulders is not possible, or deeply embedded boulders are encountered in the wall of the excavation, these may require over excavation and underpinning or rock bolting to ensure no movement is possible that might result in detrimental point loads being applied to retaining systems.</p>

Recommendation	Description
Retaining Structures	<p>Proposed new retaining walls should be designed using the following preliminary design parameters:</p> <ul style="list-style-type: none"> • Cantilever walls should be designed adopting a triangular lateral earth pressure and an ‘at rest’ earth pressure coefficient (K_0) of 0.55 and 1.35 for retained soils and extremely weathered rock, assuming a maximum backfill surface angle of 25 degrees, respectively. • Bulk unit weights of 20kN/m^3 and 22kN/m^3 should be adopted for the retained soil and weathered rock respectively and should consider possible surcharge loading from above. • Any retaining structures to be constructed as part of the site works are to be backfilled with suitable free-draining materials and subsoil drains wrapped in a non-woven geotextile fabric (i.e. Bidim A34 or similar) to prevent the clogging of the drainage with fine-grained sediment. • Where required rock bolts within the low strength bedrock may be design for an allowable bond strength of 100 kPa.
Footings	<p>All pad, strip or piered footings should be founded on and socketed a minimum of 400mm into the insitu underlying bedrock. For fully cleaned footings, the maximum allowable bearing pressure is 600 kPa. Higher bearing capacities may be achievable subject to inspection and certification by Ascent.</p> <p>High level footings in very stiff clays can achieve allowable bearing pressures of 200 kPa subject to inspection and certification.</p> <p>Pier footings, where required, should be of sufficient diameter to enable effective base cleaning to be carried out during construction. Small diameter piers that cannot be cleaned should be designed for shaft friction, resulting in a longer rock socket.</p> <p>To mitigate the risk of differential settlement, it is essential that all footings are founded on competent bedrock of similar consistency. This may require the removal, or excavation through sandstone floaters or the relocation of planned footings.</p> <p>It is essential that the foundation materials of all footing excavations be inspected and approved before steel reinforcement and concrete is placed. This inspection should be scheduled while excavation plant and operators are still on site, and before steel reinforcement has been fixed or the concrete booked.</p>

Recommendation	Description
Sediment and Erosion Control	Appropriate design and construction methods shall be required during site works to minimise erosion and provide sediment control. Any stockpiled soil will require erosion control measures, such as siltation fencing and barriers, to be designed by others.
Fills	<p>Any fill that may be required is to comprise local soil, clay and weathered rock. Existing organic topsoil is to be cleared in preparation for the introduction of fill.</p> <p>Any new fill material is to be placed in layers not more than 250 mm thick and compacted to not less than 95% of Standard Optimum Dry Density at plus or minus 2% of Standard Optimum Moisture Content.</p> <p>All new fill placement is to be carried out in accordance with AS 3798–2007 ‘Guidelines on Earthworks for Commercial and Residential Developments’.</p> <p>Fill should not be placed on the site outside of the lateral extent of new engineered retaining walls. The retaining walls should be in place prior to the placement of new fill, with suitable permanent and effective drainage of backfill.</p>
Stormwater Disposal	<p>All stormwater collected from hard surfaces is to be collected and piped to Pittwater below through any storage tanks or onsite detention that may be required by the regulating authorities and in accordance with all relevant Australian Standards and the detailed stormwater management plan by others. It is essential that the discharging outlet does not permit concentrated water to erode soil or slope materials. This can be mitigated via level spreader, drain sock or similar.</p> <p>Onsite stormwater discharge is not considered appropriate for this site.</p>
Inspections	<p>Excavations should be inspected at vertical hold points of 1.5m as the excavations progress.</p> <p>It is essential that the foundation materials of all footing excavations be visually assessed and approved by Ascent before steel reinforcement and concrete is placed.</p> <p>Failure to engage Ascent for the required hold point/excavation/foundation material inspections will negate our ability to provide final geotechnical sign off or certification.</p>

Recommendation	Description
Recommended further work	<p>Under the supervision of JKG, the stabilisation works outlined in their site report should be carried out and certified.</p> <p>Further testing, and more detailed recommendations/design parameters may be required for detailed structural design of footings and retention systems. This should be discussed with the consulting structural engineers at post DA – pre CC stage.</p>
Conditions Relating to Design and Construction Monitoring	<p>To comply with Council conditions and enable the completion of Forms 2B and 3, as required in Council’s Geotechnical Risk Management Policy, it will be necessary for the Geotechnical Engineer to:</p> <ul style="list-style-type: none"> • review the geotechnical content of all structural designs prior to the issue of Construction Certificate – Form 2B • complete the abovementioned excavation hold point and foundation material inspections during construction to ensure compliance to design with respect to stability and geotechnical design parameters • at Occupation Certificate stage (project completion), Ascent must have inspected and certified excavations and foundation materials. A final site inspection will be required at this stage – Form 3.

Should you have any queries regarding this report, please do not hesitate to contact the author of this report, undersigned.

For and on behalf of **Ascent Geotechnical Consulting Pty Ltd,**



Ben Morgan BSc, MAIG RPGeo
General Manager | Engineering Geologist



4 References

Australian Geomechanics Society (March 2007), Landslide Risk Management, Australian Geomechanics 42(1).

Australian Standard 1289.6.3.2–1997 Methods of Testing Soils for Engineering Purposes.

Australian Standard 1726–2017 Geotechnical Site Investigations.

Australian Standard AS2670.1–2001 Evaluation of human exposure to whole-body vibration. Part 1: General requirements.

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Appendix A

Information Sheets

General Notes About This Report

INTRODUCTION

These notes have been prepared by Ascent Geotechnical Consulting Pty Ltd (Ascent) to help our Clients interpret and understand the limitations of this report. Not all sections below are necessarily relevant to all reports.

SCOPE OF SERVICES

This report has been prepared in accordance with the scope of services set out in Ascent's proposal under Ascent's Terms and Conditions, or as otherwise agreed with the Client. The scope of work may have been limited by a range of factors including time, budget, access and/or site constraints.

RELIANCE ON INFORMATION PROVIDED

In preparing the report, Ascent has necessarily relied upon information provided by the Client and/or their Agents. Such data may include surveys, analyses, designs, maps and design plans. Ascent has not verified the accuracy or completeness of the data except as stated in this report.

GEOTECHNICAL AND ENVIRONMENTAL REPORTING

Geotechnical and environmental reporting relies on the interpretation of factual information, based on judgment and opinion, and is far less exact than other engineering or design disciplines.

Geotechnical and environmental reports are prepared for a specific purpose, development, and site, as described in the report, and may not contain sufficient information for other purposes, developments, or sites (including adjacent sites), other than that described in the report.

SUBSURFACE CONDITIONS

Subsurface conditions can change with time and can vary between test locations. For example, the actual interface between the materials may be far more gradual or abrupt than indicated.

Therefore, actual conditions in areas not sampled may differ from those predicted, since no subsurface investigation, no matter how comprehensive, can reveal all subsurface details and anomalies.

Construction operations at or adjacent to the site and natural events such as floods, earthquakes or groundwater fluctuations can also affect subsurface conditions, and thus the continuing adequacy of a geotechnical report. Ascent should be kept informed of any such events, and should be retained to identify variances, conduct additional tests if required, and recommend solutions to problems encountered on site.

GROUNDWATER

Groundwater levels indicated on borehole and test pit logs are recorded at specific times. Depending on ground permeability, measured levels may or may not reflect actual levels if measured over a longer time period. Also, groundwater levels and seepage inflows may fluctuate with seasonal and environmental variations and construction activities.

INTERPRETATION OF DATA

Data obtained from nominated discrete locations, subsequent laboratory testing and empirical or external sources are interpreted by trained professionals in order to provide an opinion about overall site conditions, their likely impact with respect to the report purpose and recommended actions in accordance with any relevant industry standards, guidelines or procedures.

SOIL AND ROCK DESCRIPTIONS

Soil and rock descriptions are based on AS 1726 – 1993, using visual and tactile assessment, except at discrete locations where field and / or laboratory tests have been carried out. Refer to the accompanying soil and rock terms sheet for further information.

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FURTHER ADVICE

Ascent would be pleased to further discuss how any of the above issues could affect a specific project. We would also be pleased to provide further advice or assistance including:

- Assessment of suitability of designs and construction techniques;
- Contract documentation and specification;
- Construction advice (foundation assessments, excavation support).

Abbreviations, Notes & Symbols

SUBSURFACE INVESTIGATION

METHOD

Borehole Logs

AS#	Auger screwing (#-bit)
AD#	Auger drilling (#-bit)
B	Blank bit
V	V-bit
T	TC-bit
HA	Hand auger
R	Roller/tricone
W	Washbore
AH	Air hammer
AT	Air track
LB	Light bore push tube
MC	Macro core push tube
DT	Dual core push tube

Excavation Logs

BH	Backhoe/excavator bucket
NE	Natural exposure
HE	Hand excavation
X	Existing excavation

Cored Borehole Logs

NMLC	NMLC core drilling
NQ/HQ	Wireline core drilling

SUPPORT

Borehole Logs

C	Casing
M	Mud

Excavation Logs

S	Shoring
B	Benching

SAMPLING

B	Bulk sample
D	Disturbed sample
U#	Thin-walled tube sample (#mmdiameter)
ES	Environmental sample
EW	Environmental water sample

FIELD TESTING

PP	Pocket penetrometer (kPa)
DCP	Dynamic cone penetrometer
PSP	Perth sand penetrometer
SPT	Standard penetration test
PBT	Plate bearing test
s_u	Vane shear strength peak/residual (kPa) and vane size (mm)
N*	SPT (blows per 300mm)
Nc	SPT with solid cone
R	Refusal

*denotes sample taken

BOUNDARIES

————	Known
-----	Probable
.....	Possible

SOIL

MOISTURE CONDITION

D	Dry
M	Moist
W	Wet
Wp	Plastic Limit
Wl	Liquid Limit
MC	Moisture Content

CONSISTENCY

VS	Very Soft
S	Soft
F	Firm
St	Stiff
VSt	Very Stiff
H	Hard
Fb	Friable

DENSITY INDEX

VL	Very Loose
L	Loose
MD	Medium Dense
D	Dense
VD	Very Dense

USCS SYMBOLS

GW	Well graded gravels and gravel-sand mixtures, little or no fines
GP	Poorly graded gravels and gravel-sand mixtures, little or no fines
GM	Silty gravels, gravel-sand-silt mixtures
GC	Clayey gravels, gravel-sand-clay mixtures

SW	Well graded sands and gravelly sands, little or no fines
SP	Poorly graded sands and gravelly sands, little or no fines
SM	Silty sand, sand-silt mixtures
SC	Clayey sand, sand-clay mixtures
ML	Inorganic silts of low plasticity, very fine sands, rock flour, silty or clayey fine sands
CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays
OL	Organic silts and organic silty clays of low plasticity
MH	Inorganic silts of high plasticity
CH	Inorganic clays of high plasticity
OH	Organic clays of medium to high plasticity
PT	Peat muck and other highly organic soils

ROCK

WEATHERING

RS	Residual Soil
XW	Extremely Weathered
HW	Highly Weathered
MW	Moderately Weathered
DW*	Distinctly Weathered
SW	Slightly Weathered
FR	Fresh

*covers both HW & MW

STRENGTH

EL	Extremely Low
VL	Very Low
L	Low
M	Medium
H	High
VH	Very High
EH	Extremely High

ROCK QUALITY DESIGNATION (%)

= $\frac{\text{sum of intact core pieces} > 100\text{mm}}{\text{total length of section being evaluated}} \times 100$

CORE RECOVERY (%)

= $\frac{\text{core recovered}}{\text{core lift}} \times 100$

NATURAL FRACTURES

Type

JT	Joint
BP	Bedding plane
SM	Seam
FZ	Fractured zone
SZ	Shear zone
VN	Vein

Infill or Coating

Cn	Clean
St	Stained
Vn	Veneer
Co	Coating
Cl	Clay
Ca	Calcite
Fe	Iron oxide
Mi	Micaceous
Qz	Quartz

Shape

pl	Planar
cu	Curved
un	Undulose
st	Stepped
ir	Irregular

Roughness

pol	Polished
slk	Slickensided
smo	Smooth
rou	Rough

Soil & Rock Terms

SOIL

MOISTURE CONDITION

Term	Description
Dry	Looks and feels dry. Cohesive and cemented soils are hard, friable or powdery. Uncemented granular soils run freely through the hand.
Moist	Feels cool and darkened in colour. Cohesive soils can be moulded. Granular soils tend to cohere.
Wet	As for moist, but with free water forming on hands when handled.

For cohesive soils, moisture content may also be described in relation to plastic limit (W_p) or liquid limit (W_L). [$>$ much greater than, $>$ greater than, $<$ less than, $<<$ much less than].

CONSISTENCY

Term	c (kPa)	Term	c (kPa)
Very Soft	< 12	Very Stiff	$100 - 200$
Soft	$12 - 25$	Hard	> 200
Firm	$25 - 50$	Friable	-
Stiff	$50 - 100$		

DENSITY INDEX

Term	I_D (%)	Term	I_D (%)
Very Loose	< 15	Dense	$65 - 8$
Loose	$15 - 35$	Very Dense	> 85
Medium Dense	$35 - 65$		

PARTICLE SIZE

Name	Subdivision	Size (mm)
Boulders		> 200
Cobbles		$63 - 200$
Gravel	coarse	$20 - 63$
	medium	$6 - 20$
	fine	$2.36 - 6$
Sand	coarse	$0.6 - 2.36$
	medium	$0.2 - 0.6$
	fine	$0.075 - 0.2$
Silt & Clay		< 0.075

MINOR COMPONENTS

Term	Proportion by Mass coarse grained	fine grained
Trace	$\leq 5\%$	$\leq 15\%$
Some	$5 - 2\%$	$15 - 30\%$

SOIL ZONING

Layers	Continuous exposures
Lenses	Discontinuous layers of lenticular shape
Pockets	Irregular inclusions of different material

SOIL CEMENTING

Weakly	Easily broken up by hand
Moderately	Effort is required to break up the soil by hand

SOIL STRUCTURE

Massive	Coherent, with any partings both vertically and horizontally spaced at greater than 100mm
Weak	Peds indistinct and barely observable on pit face. When disturbed approx. 30% consist of peds smaller than 100mm
Strong	Peds are quite distinct in undisturbed soil. When disturbed $>60\%$ consists of peds smaller than 100mm

ROCK

SEDIMENTARY ROCK TYPE DEFINITIONS

Rock Type	Definition (more than 50% of rock consists of....)
Conglomerate	... gravel sized ($> 2\text{mm}$) fragments
Sandstone	... sand sized (0.06 to 2mm) grains
Siltstone	... silt sized ($< 0.06\text{mm}$) particles, rock is not laminated
Claystone	... clay, rock is not laminated
Shale	... silt or clay sized particles, rock is laminated

STRENGTH

Term	I_s50 (MPa)	Term	I_s50 (MPa)
Extremely Low	< 0.03	High	$1 - 3$
Very Low	$0.03 - 0.1$	Very High	$3 - 10$
Low	$0.1 - 0.3$	Extremely High	> 10
Medium	$0.3 - 1$		

WEATHERING

Term	Description
Residual Soil	Soil developed on extremely weathered rock; the mass structure and substance fabric are no longer evident
Extremely Weathered	Rock is weathered to such an extent that it has 'soil' properties, i.e. it either disintegrates or can be remoulded, in water. Fabric of original rock is still visible
Highly Weathered	Rock strength usually highly changed by weathering; rock may be highly discoloured
Moderately Weathered	Rock strength usually moderately changed by weathering; rock may be moderately discoloured
Distinctly Weathered	See 'Highly Weathered' or 'Moderately Weathered'
Slightly Weathered	Rock is slightly discoloured but shows little or no change of strength from fresh rock
Fresh	Rock shows no signs of decomposition or staining

NATURAL FRACTURES

Type	Description
Joint	A discontinuity or crack across which the rock has little or no tensile strength. May be open or closed
Bedding plane	Arrangement in layers of mineral grains of similar sizes or composition
Seam	Seam with deposited soil (infill), extremely weathered in situ rock (XW), or disoriented usually angular fragments of the host rock (crushed)
Shear zone	Zone with roughly parallel planar boundaries, of rock material intersected by closely spaced (generally $< 50\text{mm}$) joints and /or microscopic fracture (cleavage) planes
Vein	Intrusion of any shape dissimilar to the adjoining rock mass. Usually igneous

Shape

Shape	Description
Planar	Consistent orientation
Curved	Gradual change in orientation
Undulose	Wavy surface
Stepped	One or more well defined steps
Irregular	Many sharp changes in orientation

Infill or Coating








































Infill or Coating	Description
Clean	No visible coating or discolouring
Stained	No visible coating but surfaces are discoloured
Veneer	A visible coating of soil or mineral, too thin to measure; may be patchy
Coating	Visible coating $\leq 1\text{mm}$ thick. Ticker soil material described as seam

Roughness

Roughness	Description
Polished	Shiny smooth surface
Slickensided	Grooved or striated surface, usually polished
Smooth	Smooth to touch. Few or no surface irregularities
Rough	Many small surface irregularities (amplitude generally $< 1\text{mm}$). Feels like fine to coarse sandpaper

Note: soil and rock descriptions are generally in accordance with AS1726-1993 Geotechnical Site Investigations

Graphic Symbols Index

Soil	Rock	Water Measurements
 Fill	 Sandstone	 Level at time of drilling
 Peat, Topsoil	 Shale	 Level after drilling
 Clay	 Clayey Shale	 Inflow
 Silty Clay	 Siltstone	 Outflow
 Gravelly Clay	 Conglomerate	
 Sandy Clay	 Claystone	
 Silt	 Dolerite, Basalt	
 Sandy Silt	 Granite	
 Clayey Silt	 Limestone	
 Gravelly Silt	 Tuff	
 Gravel	 Coarse grained Metamorphic	
 Sandy Gravel	 Medium grained Metamorphic	
 Clayey Gravel	 Fine grained Metamorphic	
 Silty Gravel	 Coal	
 Sand	Other	
 Gravelly Sand	 Asphalt	
 Silty Sand	 Concrete	
 Clayey Sand	 Brick	

Foundation Maintenance and Footing Performance: A Homeowner's Guide



CSIRO

BTF 18
replaces
Information
Sheet 10/91

Buildings can and often do move. This movement can be up, down, lateral or rotational. The fundamental cause of movement in buildings can usually be related to one or more problems in the foundation soil. It is important for the homeowner to identify the soil type in order to ascertain the measures that should be put in place in order to ensure that problems in the foundation soil can be prevented, thus protecting against building movement.

This Building Technology File is designed to identify causes of soil-related building movement, and to suggest methods of prevention of resultant cracking in buildings.

Soil Types

The types of soils usually present under the topsoil in land zoned for residential buildings can be split into two approximate groups – granular and clay. Quite often, foundation soil is a mixture of both types. The general problems associated with soils having granular content are usually caused by erosion. Clay soils are subject to saturation and swell/shrink problems.

Classifications for a given area can generally be obtained by application to the local authority, but these are sometimes unreliable and if there is doubt, a geotechnical report should be commissioned. As most buildings suffering movement problems are founded on clay soils, there is an emphasis on classification of soils according to the amount of swell and shrinkage they experience with variations of water content. The table below is Table 2.1 from AS 2870, the Residential Slab and Footing Code.

Causes of Movement

Settlement due to construction

There are two types of settlement that occur as a result of construction:

- Immediate settlement occurs when a building is first placed on its foundation soil, as a result of compaction of the soil under the weight of the structure. The cohesive quality of clay soil mitigates against this, but granular (particularly sandy) soil is susceptible.
- Consolidation settlement is a feature of clay soil and may take place because of the expulsion of moisture from the soil or because of the soil's lack of resistance to local compressive or shear stresses. This will usually take place during the first few months after construction, but has been known to take many years in exceptional cases.

These problems are the province of the builder and should be taken into consideration as part of the preparation of the site for construction. Building Technology File 19 (BTF 19) deals with these problems.

Erosion

All soils are prone to erosion, but sandy soil is particularly susceptible to being washed away. Even clay with a sand component of say 10% or more can suffer from erosion.

Saturation

This is particularly a problem in clay soils. Saturation creates a bog-like suspension of the soil that causes it to lose virtually all of its bearing capacity. To a lesser degree, sand is affected by saturation because saturated sand may undergo a reduction in volume – particularly imported sand fill for bedding and blinding layers. However, this usually occurs as immediate settlement and should normally be the province of the builder.

Seasonal swelling and shrinkage of soil

All clays react to the presence of water by slowly absorbing it, making the soil increase in volume (see table below). The degree of increase varies considerably between different clays, as does the degree of decrease during the subsequent drying out caused by fair weather periods. Because of the low absorption and expulsion rate, this phenomenon will not usually be noticeable unless there are prolonged rainy or dry periods, usually of weeks or months, depending on the land and soil characteristics.

The swelling of soil creates an upward force on the footings of the building, and shrinkage creates subsidence that takes away the support needed by the footing to retain equilibrium.

Shear failure

This phenomenon occurs when the foundation soil does not have sufficient strength to support the weight of the footing. There are two major post-construction causes:

- Significant load increase.
- Reduction of lateral support of the soil under the footing due to erosion or excavation.
- In clay soil, shear failure can be caused by saturation of the soil adjacent to or under the footing.

GENERAL DEFINITIONS OF SITE CLASSES

Class	Foundation
A	Most sand and rock sites with little or no ground movement from moisture changes
S	Slightly reactive clay sites with only slight ground movement from moisture changes
M	Moderately reactive clay or silt sites, which can experience moderate ground movement from moisture changes
H	Highly reactive clay sites, which can experience high ground movement from moisture changes
E	Extremely reactive sites, which can experience extreme ground movement from moisture changes
A to P	Filled sites
P	Sites which include soft soils, such as soft clay or silt or loose sands; landslip; mine subsidence; collapsing soils; soils subject to erosion; reactive sites subject to abnormal moisture conditions or sites which cannot be classified otherwise

Tree root growth

Trees and shrubs that are allowed to grow in the vicinity of footings can cause foundation soil movement in two ways

- Roots that grow under footings may increase in cross-sectional size, exerting upward pressure on footings.
- Roots in the vicinity of footings will absorb much of the moisture in the foundation soil, causing shrinkage or subsidence.

Unevenness of Movement

The types of ground movement described above usually occur unevenly throughout the building's foundation soil. Settlement due to construction tends to be uneven because of:

- Differing compaction of foundation soil prior to construction.
- Differing moisture content of foundation soil prior to construction.

Movement due to non-construction causes is usually more uneven still. Erosion can undermine a footing that traverses the flow or can create the conditions for shear failure by eroding soil adjacent to a footing that runs in the same direction as the flow.

Saturation of clay foundation soil may occur where subfloor walls create a dam that makes water pond. It can also occur wherever there is a source of water near footings in clay soil. This leads to a severe reduction in the strength of the soil which may create local shear failure.

Seasonal swelling and shrinkage of clay soil affects the perimeter of the building first, then gradually spreads to the interior. The swelling process will usually begin at the uphill extreme of the building, or on the weather side where the land is flat. Swelling gradually reaches the interior soil as absorption continues. Shrinkage usually begins where the sun's heat is greatest.

Effects of Uneven Soil Movement on Structures

Erosion and saturation

Erosion removes the support from under footings, tending to create subsidence of the part of the structure under which it occurs. Brickwork walls will resist the stress created by this removal of support by bridging the gap or cantilevering until the bricks or the mortar bedding fail. Older masonry has little resistance. Evidence of failure varies according to circumstances and symptoms may include:

- Step cracking in the mortar beds in the body of the wall or above/below openings such as doors or windows.
- Vertical cracking in the bricks (usually but not necessarily in line with the vertical beds or pendants).

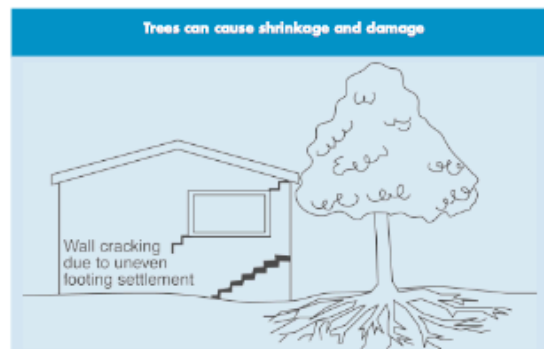
Isolated piers affected by erosion or saturation of foundations will eventually lose contact with the bearers they support and may tilt or fall over. The floors that have lost this support will become bouncy, sometimes rattling ornaments etc.

Seasonal swelling/shrinkage in clay

Swelling foundation soil due to rainy periods first lifts the most exposed extremities of the footing system, then the remainder of the perimeter footings while gradually permeating inside the building footprint to lift internal footings. This swelling first tends to create a dish effect, because the external footings are pushed higher than the internal ones.

The first noticeable symptom may be that the floor appears slightly dished. This is often accompanied by some doors binding on the floor or the door head, together with some cracking of cornice mitres. In buildings with timber flooring supported by bearers and joists, the floor can be bouncy. Externally there may be visible dishing of the hip or ridge lines.

As the moisture absorption process completes its journey to the innermost areas of the building, the internal footings will rise. If the spread of moisture is roughly even, it may be that the symptoms will temporarily disappear, but it is more likely that swelling will be uneven, creating a difference rather than a disappearance in symptoms. In buildings with timber flooring supported by bearers and joists, the isolated piers will rise more easily than the strip footings or piers under walls, creating noticeable doming of flooring.



As the weather pattern changes and the soil begins to dry out, the external footings will be first affected, beginning with the locations where the sun's effect is strongest. This has the effect of lowering the external footings. The doming is accentuated and cracking reduces or disappears where it occurred because of dishing, but other cracks open up. The roof lines may become convex.

Doming and dishing are also affected by weather in other ways. In areas where warm, wet summers and cooler dry winters prevail, water migration tends to be toward the interior and doming will be accentuated, whereas where summers are dry and winters are cold and wet, migration tends to be toward the exterior and the underlying propensity is toward dishing.

Movement caused by tree roots

In general, growing roots will exert an upward pressure on footings, whereas soil subject to drying because of tree or shrub roots will tend to remove support from under footings by inducing shrinkage.

Complications caused by the structure itself

Most forces that the soil causes to be exerted on structures are vertical – i.e. either up or down. However, because these forces are seldom spread evenly around the footings, and because the building resists uneven movement because of its rigidity, forces are exerted from one part of the building to another. The net result of all these forces is usually rotational. This resultant force often complicates the diagnosis because the visible symptoms do not simply reflect the original cause. A common symptom is binding of doors on the vertical member of the frame.

Effects on full masonry structures

Brickwork will resist cracking where it can. It will attempt to span areas that lose support because of subsided foundations or raised points. It is therefore usual to see cracking at weak points, such as openings for windows or doors.

In the event of construction settlement, cracking will usually remain unchanged after the process of settlement has ceased.

With local shear or erosion, cracking will usually continue to develop until the original cause has been remedied, or until the subsidence has completely neutralised the affected portion of footing and the structure has stabilised on other footings that remain effective.

In the case of swell/shrink effects, the brickwork will in some cases return to its original position after completion of a cycle, however it is more likely that the rotational effect will not be exactly reversed, and it is also usual that brickwork will settle in its new position and will resist the forces trying to return it to its original position. This means that in a case where swelling takes place after construction and cracking occurs, the cracking is likely to at least partly remain after the shrink segment of the cycle is complete. Thus, each time the cycle is repeated, the likelihood is that the cracking will become wider until the sections of brickwork become virtually independent.

With repeated cycles, once the cracking is established, if there is no other complication, it is normal for the incidence of cracking to stabilise, as the building has the articulation it needs to cope with the problem. This is by no means always the case, however, and monitoring of cracks in walls and floors should always be treated seriously.

Uplift caused by growth of tree roots under footings is not a simple vertical shear stress. There is a tendency for the root to also exert lateral forces that attempt to separate sections of brickwork after initial cracking has occurred.

The normal structural arrangement is that the inner leaf of brickwork in the external walls and at least some of the internal walls (depending on the roof type) comprise the load-bearing structure on which any upper floors, ceilings and the roof are supported. In these cases, it is internally visible cracking that should be the main focus of attention, however there are a few examples of dwellings whose external leaf of masonry plays some supporting role, so this should be checked if there is any doubt. In any case, externally visible cracking is important as a guide to stresses on the structure generally, and it should also be remembered that the external walls must be capable of supporting themselves.

Effects on framed structures

Timber or steel framed buildings are less likely to exhibit cracking due to swell/shrink than masonry buildings because of their flexibility. Also, the doming/dishing effects tend to be lower because of the lighter weight of walls. The main risks to framed buildings are encountered because of the isolated pier footings used under walls. Where erosion or saturation cause a footing to fall away this can double the span which a wall must bridge. This additional stress can create cracking in wall linings, particularly where there is a weak point in the structure caused by a door or window opening. It is, however, unlikely that framed structures will be so stressed as to suffer serious damage without first exhibiting some or all of the above symptoms for a considerable period. The same warning period should apply in the case of upheaval. It should be noted, however, that where framed buildings are supported by strip footings there is only one leaf of brickwork and therefore the externally visible walls are the supporting structure for the building. In this case, the subfloor masonry walls can be expected to behave as full brickwork walls.

Effects on brick veneer structures

Because the load-bearing structure of a brick veneer building is the frame that makes up the interior leaf of the external walls plus perhaps the internal walls, depending on the type of roof, the building can be expected to behave as a framed structure, except that the external masonry will behave in a similar way to the external leaf of a full masonry structure.

Water Service and Drainage

Where a water service pipe, a sewer or stormwater drainage pipe is in the vicinity of a building, a water leak can cause erosion, swelling or saturation of susceptible soil. Even a minuscule leak can be enough to saturate a clay foundation. A leaking tap near a building can have the same effect. In addition, trenches containing pipes can become watercourses even though backfilled, particularly where broken rubble is used as fill. Water that runs along these trenches can be responsible for serious erosion, interstrata seepage into subfloor areas and saturation.

Pipe leakage and trench water flows also encourage tree and shrub roots to the source of water, complicating and exacerbating the problem.

Poor roof plumbing can result in large volumes of rainwater being concentrated in a small area of soil:

- Incorrect falls in roof guttering may result in overflows, as may gutters blocked with leaves etc.

- Corroded guttering or downpipes can spill water to ground.
- Downpipes not positively connected to a proper stormwater collection system will direct a concentration of water to soil that is directly adjacent to footings, sometimes causing large-scale problems such as erosion, saturation and migration of water under the building.

Seriousness of Cracking

In general, most cracking found in masonry walls is a cosmetic nuisance only and can be kept in repair or even ignored. The table below is a reproduction of Table C1 of AS 2870.

AS 2870 also publishes figures relating to cracking in concrete floors, however because wall cracking will usually reach the critical point significantly earlier than cracking in slabs, this table is not reproduced here.

Prevention/Cure

Plumbing

Where building movement is caused by water service, roof plumbing, sewer or stormwater failure, the remedy is to repair the problem. It is prudent, however, to consider also rerouting pipes away from the building where possible, and relocating taps to positions where any leakage will not direct water to the building vicinity. Even where gully traps are present, there is sometimes sufficient spill to create erosion or saturation, particularly in modern installations using smaller diameter PVC fixtures. Indeed, some gully traps are not situated directly under the taps that are installed to charge them, with the result that water from the tap may enter the backfilled trench that houses the sewer piping. If the trench has been poorly backfilled, the water will either pond or flow along the bottom of the trench. As these trenches usually run alongside the footings and can be at a similar depth, it is not hard to see how any water that is thus directed into a trench can easily affect the foundation's ability to support footings or even gain entry to the subfloor area.

Ground drainage

In all soils there is the capacity for water to travel on the surface and below it. Surface water flows can be established by inspection during and after heavy or prolonged rain. If necessary, a grated drain system connected to the stormwater collection system is usually an easy solution.

It is, however, sometimes necessary when attempting to prevent water migration that testing be carried out to establish watertable height and subsoil water flows. This subject is referred to in BTF 19 and may properly be regarded as an area for an expert consultant.

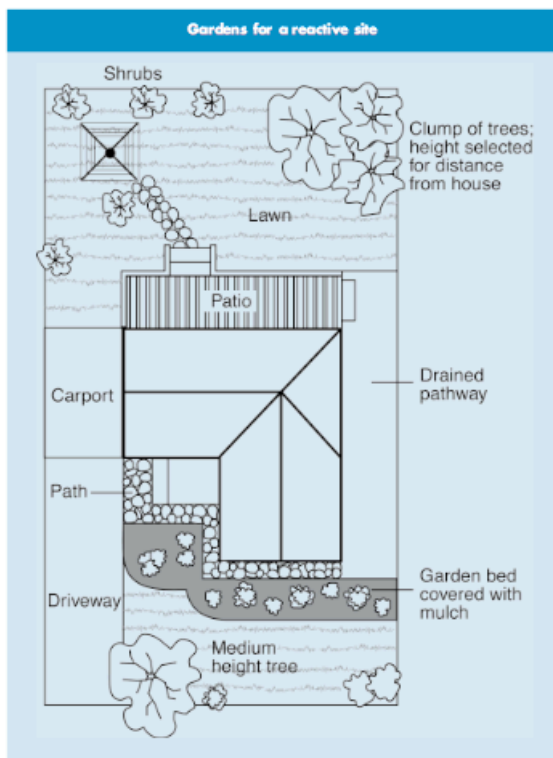
Protection of the building perimeter

It is essential to remember that the soil that affects footings extends well beyond the actual building line. Watering of garden plants, shrubs and trees causes some of the most serious water problems.

For this reason, particularly where problems exist or are likely to occur, it is recommended that an apron of paving be installed around as much of the building perimeter as necessary. This paving

CLASSIFICATION OF DAMAGE WITH REFERENCE TO WALLS

Description of typical damage and required repair	Approximate crack width limit (see Note 3)	Damage category
Hairline cracks	<0.1 mm	0
Fine cracks which do not need repair	<1 mm	1
Cracks noticeable but easily filled. Doors and windows stick slightly	<5 mm	2
Cracks can be repaired and possibly a small amount of wall will need to be replaced. Doors and windows stick. Service pipes can fracture. Weathertightness often impaired	5–15 mm (or a number of cracks 3 mm or more in one group)	3
Extensive repair work involving breaking-out and replacing sections of walls, especially over doors and windows. Window and door frames distort. Walls lean or bulge noticeably, some loss of bearing in beams. Service pipes disrupted	15–25 mm but also depend on number of cracks	4



- Water that is transmitted into masonry, metal or timber building elements causes damage and/or decay to those elements.
- High subfloor humidity and moisture content create an ideal environment for various pests, including termites and spiders.
- Where high moisture levels are transmitted to the flooring and walls, an increase in the dust mite count can ensue within the living areas. Dust mites, as well as dampness in general, can be a health hazard to inhabitants, particularly those who are abnormally susceptible to respiratory ailments.

The garden

The ideal vegetation layout is to have lawn or plants that require only light watering immediately adjacent to the drainage or paving edge, then more demanding plants, shrubs and trees spread out in that order.

Overwatering due to misuse of automatic watering systems is a common cause of saturation and water migration under footings. If it is necessary to use these systems, it is important to remove garden beds to a completely safe distance from buildings.

Existing trees

Where a tree is causing a problem of soil drying or there is the existence or threat of upheaval of footings, if the offending roots are subsidiary and their removal will not significantly damage the tree, they should be severed and a concrete or metal barrier placed vertically in the soil to prevent future root growth in the direction of the building. If it is not possible to remove the relevant roots without damage to the tree, an application to remove the tree should be made to the local authority. A prudent plan is to transplant likely offenders before they become a problem.

Information on trees, plants and shrubs

State departments overseeing agriculture can give information regarding root patterns, volume of water needed and safe distance from buildings of most species. Botanic gardens are also sources of information. For information on plant roots and drains, see Building Technology File 17.

Excavation

Excavation around footings must be properly engineered. Soil supporting footings can only be safely excavated at an angle that allows the soil under the footing to remain stable. This angle is called the angle of repose (or friction) and varies significantly between soil types and conditions. Removal of soil within the angle of repose will cause subsidence.

Remediation

Where erosion has occurred that has washed away soil adjacent to footings, soil of the same classification should be introduced and compacted to the same density. Where footings have been undermined, augmentation or other specialist work may be required. Remediation of footings and foundations is generally the realm of a specialist consultant.

Where isolated footings rise and fall because of swell/shrink effect, the homeowner may be tempted to alleviate floor bounce by filling the gap that has appeared between the bearer and the pier with blocking. The danger here is that when the next swell segment of the cycle occurs, the extra blocking will push the floor up into an accentuated dome and may also cause local shear failure in the soil. If it is necessary to use blocking, it should be by a pair of fine wedges and monitoring should be carried out fortnightly.

This BTF was prepared by John Lewer FAIB, MIAMA, Partner, Construction Diagnosis.

should extend outwards a minimum of 900 mm (more in highly reactive soil) and should have a minimum fall away from the building of 1:60. The finished paving should be no less than 100 mm below brick vent bases.

It is prudent to relocate drainage pipes away from this paving, if possible, to avoid complications from future leakage. If this is not practical, earthenware pipes should be replaced by PVC and backfilling should be of the same soil type as the surrounding soil and compacted to the same density.

Except in areas where freezing of water is an issue, it is wise to remove taps in the building area and relocate them well away from the building – preferably not uphill from it (see BTF 19).

It may be desirable to install a grated drain at the outside edge of the paving on the uphill side of the building. If subsoil drainage is needed this can be installed under the surface drain.

Condensation

In buildings with a subfloor void such as where bearers and joists support flooring, insufficient ventilation creates ideal conditions for condensation, particularly where there is little clearance between the floor and the ground. Condensation adds to the moisture already present in the subfloor and significantly slows the process of drying out. Installation of an adequate subfloor ventilation system, either natural or mechanical, is desirable.

Warning: Although this Building Technology File deals with cracking in buildings, it should be said that subfloor moisture can result in the development of other problems, notably:

The information in this and other issues in the series was derived from various sources and was believed to be correct when published.

The information is advisory. It is provided in good faith and not claimed to be an exhaustive treatment of the relevant subject.

Further professional advice needs to be obtained before taking any action based on the information provided.

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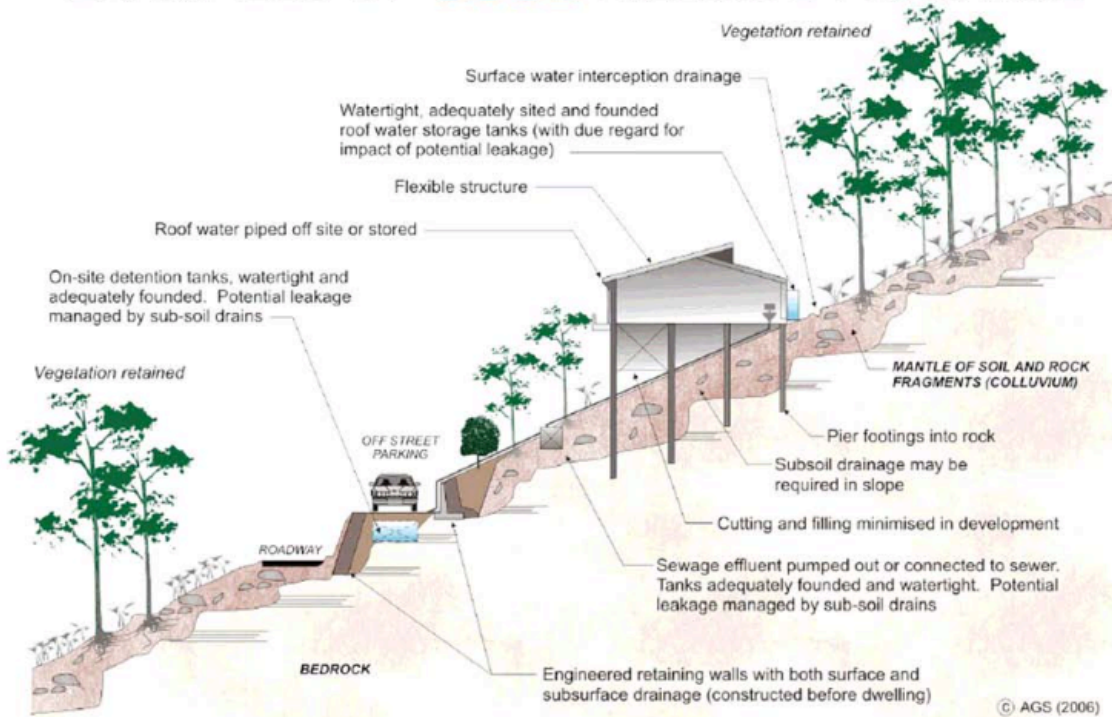
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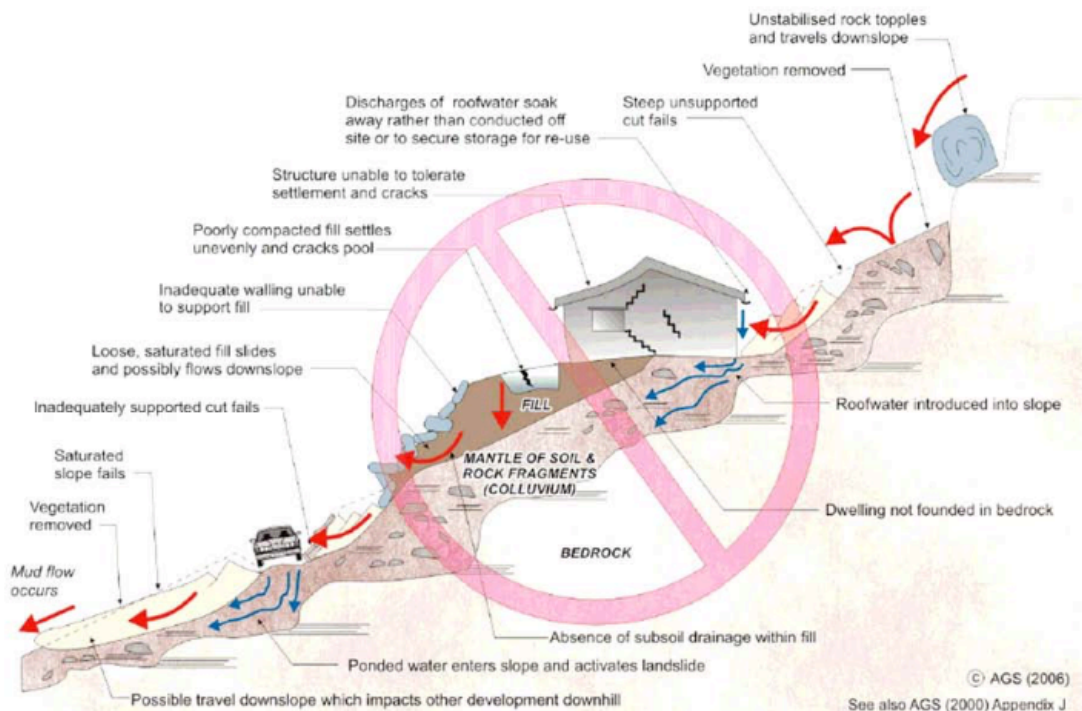
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EXAMPLES OF **GOOD** HILLSIDE PRACTICE



EXAMPLES OF **POOR** HILLSIDE PRACTICE



PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007
APPENDIX C: LANDSLIDE RISK ASSESSMENT
QUALITATIVE TERMINOLOGY FOR USE IN ASSESSING RISK TO PROPERTY

QUALITATIVE MEASURES OF LIKELIHOOD

Approximate Annual Probability		Implied Indicative Landslide Recurrence Interval	Description	Descriptor	Level
Indicative Value	Notional Boundary				
10 ⁻¹	5x10 ⁻²	10 years	The event is expected to occur over the design life.	ALMOST CERTAIN	A
10 ⁻²	5x10 ⁻³	100 years	The event will probably occur under adverse conditions over the design life.	LIKELY	B
10 ⁻³	5x10 ⁻⁴	1000 years	The event could occur under adverse conditions over the design life.	POSSIBLE	C
10 ⁻⁴	5x10 ⁻⁵	10,000 years	The event might occur under very adverse circumstances over the design life.	UNLIKELY	D
10 ⁻⁵	5x10 ⁻⁶	100,000 years	The event is conceivable but only under exceptional circumstances over the design life.	RARE	E
10 ⁻⁶	5x10 ⁻⁶	1,000,000 years	The event is inconceivable or fanciful over the design life.	BARELY CREDIBLE	F

Note: (1) The table should be used from left to right; use Approximate Annual Probability or Description to assign Descriptor, not *vice versa*.

QUALITATIVE MEASURES OF CONSEQUENCES TO PROPERTY

Approximate Cost of Damage		Description	Descriptor	Level
Indicative Value	Notional Boundary			
200%	100%	Structure(s) completely destroyed and/or large scale damage requiring major engineering works for stabilisation. Could cause at least one adjacent property major consequence damage.	CATASTROPHIC	1
60%	40%	Extensive damage to most of structure, and/or extending beyond site boundaries requiring significant stabilisation works. Could cause at least one adjacent property medium consequence damage.	MAJOR	2
20%	10%	Moderate damage to some of structure, and/or significant part of site requiring large stabilisation works.	MEDIUM	3
5%	1%	Could cause at least one adjacent property minor consequence damage.	MINOR	4
0.5%		Limited damage to part of structure, and/or part of site requiring some reinstatement stabilisation works. Little damage. (Note for high probability event (Almost Certain), this category may be subdivided at a notional boundary of 0.1%. See Risk Matrix.)	INSIGNIFICANT	5

Notes: (2) The Approximate Cost of Damage is expressed as a percentage of market value, being the cost of the improved property which includes the land plus the unaffected structures.

(3) The Approximate Cost is to be an estimate of the direct cost of the damage, such as the cost of reinstatement of the damaged portion of the property (land plus structures), stabilisation works required to render the site to tolerable risk level for the landslide which has occurred and professional design fees, and consequential costs such as legal fees, temporary accommodation. It does not include additional stabilisation works to address other landslides which may affect the property.

(4) The table should be used from left to right; use Approximate Cost of Damage or Description to assign Descriptor, not *vice versa*

PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007
APPENDIX C: – QUALITATIVE TERMINOLOGY FOR USE IN ASSESSING RISK TO PROPERTY (CONTINUED)

QUALITATIVE RISK ANALYSIS MATRIX – LEVEL OF RISK TO PROPERTY

LIKELIHOOD		CONSEQUENCES TO PROPERTY (With Indicative Approximate Cost of Damage)				
	Indicative Value of Approximate Annual Probability	1: CATASTROPHIC 200%	2: MAJOR 60%	3: MEDIUM 20%	4: MINOR 5%	5: INSIGNIFICANT 0.5%
A - ALMOST CERTAIN	10 ⁻¹	VH	VH	VH	H	M or L (S)
B - LIKELY	10 ⁻²	VH	VH	H	M	L
C - POSSIBLE	10 ⁻³	VH	H	M	M	VL
D - UNLIKELY	10 ⁻⁴	H	M	L	L	VL
E - RARE	10 ⁻⁵	M	L	L	VL	VL
F - BARELY CREDIBLE	10 ⁻⁶	L	VL	VL	VL	VL

Notes: (5) For Cell A5, may be subdivided such that a consequence of less than 0.1% is Low Risk.

(6) When considering a risk assessment it must be clearly stated whether it is for existing conditions or with risk control measures which may not be implemented at the current time.

RISK LEVEL IMPLICATIONS

Risk Level	Example Implications (7)
VH	Unacceptable without treatment. Extensive detailed investigation and research, planning and implementation of treatment options essential to reduce risk to Low; may be too expensive and not practical. Work likely to cost more than value of the property.
H	Unacceptable without treatment. Detailed investigation, planning and implementation of treatment options required to reduce risk to Low. Work would cost a substantial sum in relation to the value of the property.
M	May be tolerated in certain circumstances (subject to regulator's approval) but requires investigation, planning and implementation of treatment options to reduce the risk to Low. Treatment options to reduce to Low risk should be implemented as soon as practicable.
L	Usually acceptable to regulators. Where treatment has been required to reduce the risk to this level, ongoing maintenance is required.
VL	Acceptable. Manage by normal slope maintenance procedures.

Note: (7) The implications for a particular situation are to be determined by all parties to the risk assessment and may depend on the nature of the property at risk; these are only given as a general guide.



Appendix B

Site Plan | Testing Locations



LEGEND



DCP LOCATIONS



EXPOSED SANDSTONE
BEDROCK/LARGE
DETACHED FLOATERS



SITE PLAN/GROUND TEST LOCATIONS
SCALE NTS

REV	DATE	REVISION DESCRIPTION	REV BY	CHKD
A	20.02.22	PRELIMINARY ISSUE	VT	BM

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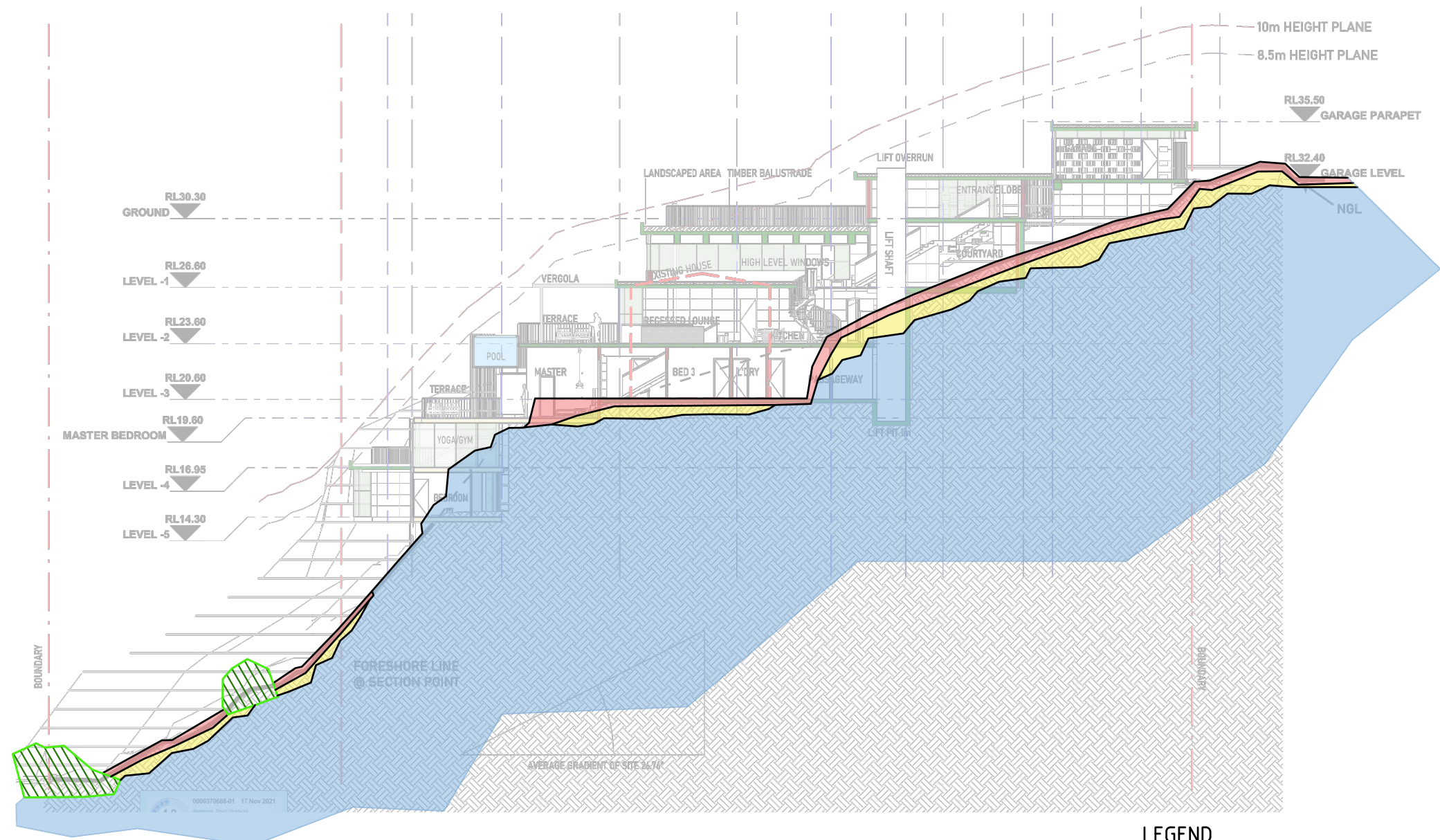
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SITE PLAN/GROUND TEST LOCATIONS
AT 189 RIVERVIEW ROAD
AVALON BEACH NSW

DATE:	20/02/2022
SCALE:	AS SHOWN @ A3
DRAWING TITLE:	SITE PLAN
DRAWING NO:	AG 20235- S1

INTERPRETED SUBSURFACE SECTION ONLY.
ACTUAL GROUND CONDITIONS MAY VARY.



- LEGEND**
- SHALLOW UNCONTROLLED FILL
 - SANDY/SILTY CLAY
 - NEWPORT FORMATION GEOLOGY
 - LARGE DETACHED SANDSTONE FLOATERS

INFERRED GEOLOGICAL SECTION
SCALE NTS

REV	DATE	REVISION DESCRIPTION	REV BY	CHKD
A	20.02.22	PRELIMINARY ISSUE	VT	BM


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INFERRED GEOLOGICAL SECTION
AT 189 RIVERVIEW ROAD
AVALON BEACH NSW

DATE:	20/02/2022
SCALE:	AS SHOWN @ A3
DRAWING TITLE:	SECTION
DRAWING NO:	AG 20235- S2



Appendix C

Bore Logs | DCP Test Results

Dynamic Cone Penetration Test Report

Client: Jamie Durie		Job No: AG 20235							
Project: New Dwelling		Date: 17/9/20							
Location: 189 Riverview Road, Avalon Beach		Operator: BM & KG							
Test Procedure: AS 1289.6.3.2 – 1997									
Test Data									
Test No: DCP 1		Test No: DCP 2		Test No: DCP 3		Test No: DCP 4		Test No: DCP 5	
Test Location: Refer to Site Plan		Test Location: Refer to Site Plan		Test Location: Refer to Site Plan		Test Location: Refer to Site Plan		Test Location: Refer to Site Plan	
RL: 5.5m		RL: 7.5m		RL: 7.5m		RL: 25m		RL: 25.4m	
Soil Classification: P		Soil Classification: P		Soil Classification: P		Soil Classification: P		Soil Classification: P	
Depth (m)	Blows	Depth (m)	Blows	Depth (m)	Blows	Depth (m)	Blows	Depth (m)	Blows
0.0 - 0.3	1D	0.0 - 0.3	D	0.0 - 0.3	D	0.0 - 0.3	10	0.0 - 0.3	14
0.3 - 0.6	3	0.3 - 0.6	1D	0.3 - 0.6	D	0.3 - 0.6	22 Rs	0.3 - 0.6	23
0.6 - 0.9	2D	0.6 - 0.9	D	0.6 - 0.9	3	0.6 - 0.9		0.6 - 0.9	30
0.9 - 1.2	4D	0.9 - 1.2	6	0.9 - 1.2	3	0.9 - 1.2		0.9 - 1.2	23
1.2 - 1.5	10	1.2 - 1.5	20 Rs	1.2 - 1.5	19	1.2 - 1.5		1.2 - 1.5	30
1.5 - 1.8	21	1.5 - 1.8		1.5 - 1.8	15 Rs	1.5 - 1.8		1.5 - 1.8	14 Rs
1.8 - 2.1	27 Pr	1.8 - 2.1		1.8 - 2.1		1.8 - 2.1		1.8 - 2.1	
2.1 - 2.4		2.1 - 2.4		2.1 - 2.4		2.1 - 2.4		2.1 - 2.4	
2.4 - 2.7		2.4 - 2.7		2.4 - 2.7		2.4 - 2.7		2.4 - 2.7	
2.7 - 3.0		2.7 - 3.0		2.7 - 3.0		2.7 - 3.0		2.7 - 3.0	
3.0 - 3.3		3.0 - 3.3		3.0 - 3.3		3.0 - 3.3		3.0 - 3.3	
3.3 - 3.6		3.3 - 3.6		3.3 - 3.6		3.3 - 3.6		3.3 - 3.6	
3.6 - 3.9		3.6 - 3.9		3.6 - 3.9		3.6 - 3.9		3.6 - 3.9	
3.9 - 4.2		3.9 - 4.2		3.9 - 4.2		3.9 - 4.2		3.9 - 4.2	
4.2 - 4.5		4.2 - 4.5		4.2 - 4.5		4.2 - 4.5		4.2 - 4.5	
4.5 - 4.8		4.5 - 4.8		4.5 - 4.8		4.5 - 4.8		4.5 - 4.8	
DCP 1: Refusal @ 1.95m Bouncing on bedrock. Orange brown fine-grained impact dust on wet tip.		DCP 2: Refusal @ 1.5m Bouncing on bedrock. Wet rods to 1.3m. Orange brown fine-grained impact dust on wet tip.		DCP 3: Refusal @ 1.65m Bouncing on bedrock. Damp rods to 0.9m. White impact dust on wet tip.		DCP 4: Refusal @ 0.50m Bouncing on floater. Clean and dry tip		DCP 5: Refusal @ 1.55m Bouncing on bedrock. White impact dust on dry tip.	
Remarks: Available test locations limited by large trees, existing hard surfaces and possible buried services . No groundwater encountered.						Weight: 9 kg			
						Drop: 510 mm			
						Rod Diameter: 16 mm			

Rs = Solid Refusal - Typically indicates sandstone bedrock

Pr = Practical Refusal - still penetrating slowly - typically indicates extremely weathered or low strength rock

D = Dropped - rods penetrate under weight of equipment only - typically indicates voids or very low compaction

Dynamic Cone Penetration Test Report

Client: Jamie Durie		Job No: AG 20235							
Project: Alterations and Additions		Date: 17/9/20							
Location: 189 Riverview Road, Avalon Beach		Operator: BM & KG							
Test Procedure: AS 1289.6.3.2 – 1997									
Test Data									
Test No: DCP 6		Test No: DCP 7		Test No: DCP 8		Test No:		Test No:	
Test Location: Refer to Site Plan		Test Location: Refer to Site Plan		Test Location: Refer to Site Plan		Test Location: Refer to Site Plan		Test Location: Refer to Site Plan	
RL: 28m		RL: 31m		RL: 31.5m		RL:		RL:	
Soil Classification: P		Soil Classification: P		Soil Classification: P		Soil Classification:		Soil Classification:	
Depth (m)	Blows	Depth (m)	Blows	Depth (m)	Blows				
0.0 - 0.3	8	0.0 - 0.3	15	0.0 - 0.3	20				
0.3 - 0.6	17	0.3 - 0.6	25	0.3 - 0.6	24				
0.6 - 0.9	22	0.6 - 0.9	23	0.6 - 0.9	28				
0.9 - 1.2	18 Rs	0.9 - 1.2	20	0.9 - 1.2	20				
1.2 - 1.5		1.2 - 1.5	24	1.2 - 1.5	21				
1.5 - 1.8		1.5 - 1.8	25 Rs	1.5 - 1.8	11				
1.8 - 2.1		1.8 - 2.1		1.8 - 2.1	13				
2.1 - 2.4		2.1 - 2.4		2.1 - 2.4	17 Rs				
2.4 - 2.7		2.4 - 2.7		2.4 - 2.7					
2.7 - 3.0		2.7 - 3.0		2.7 - 3.0					
3.0 - 3.3		3.0 - 3.3		3.0 - 3.3					
3.3 - 3.6		3.3 - 3.6		3.3 - 3.6					
3.6 - 3.9		3.6 - 3.9		3.6 - 3.9					
3.9 - 4.2		3.9 - 4.2		3.9 - 4.2					
4.2 - 4.5		4.2 - 4.5		4.2 - 4.5					
4.5 - 4.8		4.5 - 4.8		4.5 - 4.8					
DCP 6: Refusal @ 0.50m Bouncing on bedrock. Orange white silt on damp tip.		DCP 7: Refusal @ 1.65m Bouncing on bedrock. Orange yellow silt on dry tip.		DCP 8: Refusal @ 2.3m Bouncing on bedrock. Orange white silt on dry tip.					
Remarks: Available test locations limited by large trees, existing hard surfaces and possible buried services . No groundwater encountered.				Weight: 9 kg		Drop: 510 mm		Rod Diameter: 16 mm	

Rs = Solid Refusal - Typically indicates sandstone bedrock

Pr = Practical Refusal - still penetrating slowly - typically indicates extremely weathered or low strength rock

D = Dropped - rods penetrate under weight of equipment only - typically indicates voids or very low compaction



Appendix D

Geotechnical Forms 1 & 1A
Northern Beaches Council | Pittwater LEP

**GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER
FORM NO. 1 – To be submitted with Development Application**

Development Application for	<u>Mr Jamie Durie</u>
	Name of Applicant
Address of site	<u>189 Riverview Road, Avalon Beach NSW</u>

Declaration made by geotechnical engineer or engineering geologist or coastal engineer (where applicable) as part of a geotechnical report

I, KAREN ALLAN on behalf of Ascent Geotechnical Consulting P/L
(insert name) (Trading or Company Name)

on this the 23/09/2020 certify that I am a geotechnical engineer or engineering geologist or coastal engineer as defined by the Geotechnical Risk Management Policy for Pittwater - 2009 and I am authorised by the above organisation/company to issue this document and to certify that the organisation/company has a current professional indemnity policy of at least \$2million.

Please mark appropriate box

- Prepared the detailed Geotechnical Report referenced below in accordance with the Australia Geomechanics Society's Landslide Risk Management Guidelines (AGS 2007) and the Geotechnical Risk Management Policy for Pittwater - 2009
- I am willing to technically verify that the detailed Geotechnical Report referenced below has been prepared in accordance with the Australian Geomechanics Society's Landslide Risk Management Guidelines (AGS 2007) and the Geotechnical Risk Management Policy for Pittwater - 2009
- Have examined the site and the proposed development in detail and have carried out a risk assessment in accordance with paragraph 6.0 of the Geotechnical Risk Management Policy for Pittwater - 2009. I confirm the results of the risk assessment for the proposed development are in compliance with the Geotechnical Risk Management Policy for Pittwater - 2009 and further detailed geotechnical reporting is not required for the subject site.
- Have examined the site and the proposed development/alteration in detail and am of the opinion that the Development Application only involves Minor Development/Alterations that do not require a Detailed Geotechnical Risk Assessment and hence my report is in accordance with the Geotechnical Risk Management Policy for Pittwater – 2009 requirements for Minor Development/Alterations.
- Have examined the site and the proposed development/alteration is separate form and not affected by a Geotechnical Hazard and does not require a Geotechnical report or Risk Assessment and hence my Report is in accordance with the Geotechnical Risk Management Policy for Pittwater – 2009 requirements
- Provided the coastal process and coastal forces analysis for inclusion in the Geotechnical Report

Geotechnical Report Details:

Report Title: Geotechnical Assessment Report for a New Dwelling at 189 Riverview Road, Avalon Beach NSW (AG 20235) Report Date: 21 February 2022 Author: Ben Morgan Author's Company/Organisation: Ascent Geotechnical Consulting PTY LTD
--

Documentation which relate to or are relied upon in report preparation:

Durie Design, Job DD, drawing numbers 001: 050, 100-102, 120-121, 200-206, 210-212, 240-241, 250-271, 300-303, 310-311, 320-321, revision K, dated 15 November 2021; and architectural plans for proposed inclined lift prepared by Durie Design, drawing number 001: 050, revision A, undated.

I am aware that the above Geotechnical Report, prepared for the abovementioned site is to be submitted in support of a Development Application for this site and will be relied on by Northern Beaches Council as the basis for ensuring that the Geotechnical Risk Management aspects of the proposed development have been adequately addressed to achieve an "Acceptable Risk Management" level for the life of the structure, taken as at least 100 years unless otherwise stated and justified in the Report and that reasonable and practical measures have been identified to remove foreseeable risk.

Signature



Name Ben Morgan

Chartered Professional Status MAIG RPGeo (Geotechnical & Engineering)

Membership No. 10269

Company Ascent Geotechnical Consulting Pty Ltd

**GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER
FORM NO. 1(a) - Checklist of Requirements for Geotechnical Risk Management Report for
Development Application**

Development Application for	Mr Jamie Durie
	Name of Applicant
Address of site	189 Riverview Road, Avalon Beach

The following checklist covers the minimum requirements to be addressed in a Geotechnical Risk Management Geotechnical Report. This checklist is to accompany the Geotechnical Report and its certification (Form No. 1).


Geotechnical Report Details:

Report Title: Geotechnical Assessment Report for a New Dwelling at 189 Riverview Road, Avalon Beach NSW (AG 20235)
Report Date: 21 February 2022
Author: Ben Morgan
Author's Company/Organisation: Ascent Geotechnical Consulting PTY LTD

Please mark appropriate box

- Comprehensive site mapping conducted 17/09/2020
(date)
- Mapping details presented on contoured site plan with geomorphic mapping to a minimum scale of 1:200 (as appropriate)
- Subsurface investigation required
 - No Justification
 - Yes Date conducted 17/09/2020
- Geotechnical model developed and reported as an inferred subsurface type-section
- Geotechnical hazards identified
 - Above the site
 - On the site
 - Below the site
 - Beside the site
- Geotechnical hazards described and reported
- Risk assessment conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
 - Consequence analysis
 - Frequency analysis
- Risk calculation
- Risk assessment for property conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
- Risk assessment for loss of life conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
- Assessed risks have been compared to "Acceptable Risk Management" criteria as defined in the Geotechnical Risk Management Policy for Pittwater - 2009
- Opinion has been provided that the design can achieve the "Acceptable Risk Management" criteria provided that the specified conditions are achieved.
- Design Life Adopted:
 - 100 years
 - Other
specify
- Geotechnical Conditions to be applied to all four phases as described in the Geotechnical Risk Management Policy for Pittwater – 2009 have been specified
- Additional action to remove risk where reasonable and practical have been identified and included in the report.
- Risk Assessment within Bushfire Asset Protection Zone

I am aware that Pittwater Council will rely on the Geotechnical Report, to which this checklist applies, as the basis for ensuring that the geotechnical risk management aspects of the proposal have been adequately addressed to achieve an "Acceptable Risk Management" level for the life of the structure, taken as at least 100 years unless otherwise stated, and justified in the Report and that reasonable and practical measures have been identified to remove foreseeable risk.



Signature _____

Name Ben Morgan

Chartered Professional Status MAIG RPGeo (Geotechnical & Engineering)

Membership No. 10269

Company Ascent Geotechnical Consulting Pty Ltd