

GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER
FORM NO. 1 – To be submitted with Development Application

Development Application for	Name of Applicant
Address of site	<u>34 Narrabeen Park Parade, Warriewood</u>

The following checklist covers the minimum requirements to be addressed in a Geotechnical Risk Declaration made by geotechnical engineer or engineering geologist or coastal engineer (where applicable) as part of a geotechnical report

I, Ben White on behalf of White Geotechnical Group Pty Ltd
(Insert Name) (Trading or Company Name)

on this the 19/5/22 certify that I am a geotechnical engineer or engineering geologist or coastal engineer as defined by the Geotechnical Risk Management Policy for Pittwater - 2009 and I am authorised by the above organisation/company to issue this document and to certify that the organisation/company has a current professional indemnity policy of at least \$10million.

I:

Please mark appropriate box

- ☒ have prepared the detailed Geotechnical Report referenced below in accordance with the Australia Geomechanics Society's Landslide Risk Management Guidelines (AGS 2007) and the Geotechnical Risk Management Policy for Pittwater - 2009
- ☒ am willing to technically verify that the detailed Geotechnical Report referenced below has been prepared in accordance with the Australian Geomechanics Society's Landslide Risk Management Guidelines (AGS 2007) and the Geotechnical Risk Management Policy for Pittwater - 2009
- ☐ have examined the site and the proposed development in detail and have carried out a risk assessment in accordance with Section 6.0 of the Geotechnical Risk Management Policy for Pittwater - 2009. I confirm that the results of the risk assessment for the proposed development are in compliance with the Geotechnical Risk Management Policy for Pittwater - 2009 and further detailed geotechnical reporting is not required for the subject site.
- ☐ have examined the site and the proposed development/alteration in detail and I am of the opinion that the Development Application only involves Minor Development/Alteration that does not require a Geotechnical Report or Risk Assessment and hence my Report is in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009 requirements.
- ☐ have examined the site and the proposed development/alteration is separate from and is not affected by a Geotechnical Hazard and does not require a Geotechnical Report or Risk Assessment and hence my Report is in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009 requirements.
- ☐ have provided the coastal process and coastal forces analysis for inclusion in the Geotechnical Report

Geotechnical Report Details:

Report Title: Geotechnical Report <u>34 Narrabeen Park Parade, Warriewood</u>
Report Date: <u>19/5/22</u>
Author: <u>BEN WHITE</u>
Author's Company/Organisation: <u>WHITE GEOTECHNICAL GROUP PTY LTD</u>

Documentation which relate to or are relied upon in report preparation:

<u>Australian Geomechanics Society Landslide Risk Management March 2007.</u>
<u>White Geotechnical Group company archives.</u>

I am aware that the above Geotechnical Report, prepared for the abovementioned site is to be submitted in support of a Development Application for this site and will be relied on by Pittwater Council as the basis for ensuring that the Geotechnical Risk Management aspects of the proposed development have been adequately addressed to achieve an "Acceptable Risk Management" level for the life of the structure, taken as at least 100 years unless otherwise stated and justified in the Report and that reasonable and practical measures have been identified to remove foreseeable risk.

Signature



Name

Ben White

Chartered Professional Status

MScGEOLAusIMM CP GEOL

Membership No.

222757

Company

White Geotechnical Group Pty Ltd

GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER
FORM NO. 1(a) - Checklist of Requirements for Geotechnical Risk Management Report for
Development Application

Development Application for	Name of Applicant
Address of site	<u>34 Narrabeen Park Parade, Warriewood</u>

The following checklist covers the minimum requirements to be addressed in a Geotechnical Risk Management Geotechnical Report. This checklist is to accompany the Geotechnical Report and its certification (Form No. 1).


Geotechnical Report Details:

Report Title: Geotechnical Report <u>34 Narrabeen Park Parade, Warriewood</u>
Report Date: <u>19/5/22</u>
Author: <u>BEN WHITE</u>
Author's Company/Organisation: <u>WHITE GEOTECHNICAL GROUP PTY LTD</u>

Please mark appropriate box

- ☒ Comprehensive site mapping conducted 16/5/22
(date)
- ☒ Mapping details presented on contoured site plan with geomorphic mapping to a minimum scale of 1:200 (as appropriate)
- ☒ Subsurface investigation required
 - ☐ No Justification _____
 - ☒ Yes Date conducted 16/5/22
- ☒ Geotechnical model developed and reported as an inferred subsurface type-section
- ☒ Geotechnical hazards identified
 - ☐ Above the site
 - ☒ On the site
 - ☒ Below the site
 - ☐ Beside the site
- ☒ Geotechnical hazards described and reported
- ☒ Risk assessment conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
 - ☒ Consequence analysis
 - ☒ Frequency analysis
- ☒ Risk calculation
- ☒ Risk assessment for property conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
- ☒ Risk assessment for loss of life conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
- ☒ Assessed risks have been compared to "Acceptable Risk Management" criteria as defined in the Geotechnical Risk Management Policy for Pittwater - 2009
- ☒ Opinion has been provided that the design can achieve the "Acceptable Risk Management" criteria provided that the specified conditions are achieved.
- ☒ Design Life Adopted:
 - ☒ 100 years
 - ☐ Other _____ specify
- ☒ Geotechnical Conditions to be applied to all four phases as described in the Geotechnical Risk Management Policy for Pittwater - 2009 have been specified
- ☒ Additional action to remove risk where reasonable and practical have been identified and included in the report.
- ☐ Risk assessment within Bushfire Asset Protection Zone.

I am aware that Pittwater Council will rely on the Geotechnical Report, to which this checklist applies, as the basis for ensuring that the geotechnical risk management aspects of the proposal have been adequately addressed to achieve an "Acceptable Risk Management" level for the life of the structure, taken as at least 100 years unless otherwise stated, and justified in the Report and that reasonable and practical measures have been identified to remove foreseeable risk.


Signature _____

Name Ben White

Chartered Professional Status MScGEOLAusIMM CP GEOL

Membership No. 222757

Company White Geotechnical Group Pty Ltd

GEOTECHNICAL INVESTIGATION:

Alts. & Adds., New Pool, and Carport at **34 Narrabeen Park Parade, Warriewood**

1. Proposed Development

- 1.1** Construct an extension to the downhill side of the house.
- 1.2** Construct a patio area on the uphill side of the property.
- 1.3** Construct a carport on the downhill side of the property.
- 1.4** Remove the existing pool and install a new pool by excavating to a maximum depth of ~1.8m.
- 1.5** Various other minor internal and external alterations and additions.
- 1.6** Details of the proposed development are shown on 13 drawings prepared by J.D Evans and company, project number 2088, drawings numbered 1 to 13, dated 15/2/22.

2. Site Description

- 2.1** The site was inspected on the 16th May, 2022.
- 2.2** This residential property is on the low side of the road and has a W aspect. It is located on the moderate to steeply graded upper reaches of a hillslope. The natural slope falls across the property at an average angle of ~14°. The slope above the property eases to the crest of the slope, and the slope below the property continues at similar angles.
- 2.3** At the road frontage, a concrete driveway runs past the house to a carport and garage underneath the downhill side of the house (Photo 1). The concrete blocks supporting the driveway displays minor stepped cracking through the mortar (Photo 2). A ~1.3m high stack rock retaining wall supports a fill for the road reserve and a cut

for a gently sloping lawn area in between the road frontage and the house (Photo 3). The stack rock wall is covered in dense vegetation and could not be assessed in terms of stability. The two-storey timber framed and clad house is supported on brick piers (Photo 4). The brick piers appear to stand vertical. A stable ~0.6m high concrete block retaining wall supports a fill for a pathway and cut for a level platform for the carport (Photo 5). An above ground steel pool with a liner has been partially cut into the slope on the downhill side of the property (Photo 6). This pool is to be removed as part of the proposed works. A gently sloping lawn extends from the pool area to the lower common boundary (Photo 7).

3. Geology

The Sydney 1:100 000 Geological sheet indicates the site is underlain by the Newport Formation of the Narrabeen Group. This is described as interbedded laminite, shale and quartz to lithic quartz sandstone.

4. Subsurface Investigation

One hand Auger Hole (AH) was put down to identify soil materials. Five Dynamic Cone Penetrometer (DCP) tests were put down to determine the relative density of the overlying soil and the depth to weathered rock. The locations of the tests are shown on the site plan attached. It should be noted that a level of caution should be applied when interpreting DCP test results. The test will not pass through hard buried objects so in some instances it can be difficult to determine whether refusal has occurred on an obstruction in the profile or on the natural rock surface. This is not expected to be an issue for the testing on this site. However, excavation and foundation budgets should always allow for the possibility that the interpreted ground conditions in this report vary from those encountered during excavations. See the appended "Important information about your report" for a more comprehensive explanation. The results are as follows:

AUGER HOLE 1 (~RL20.6) – AH1 (Photo 8)

Depth (m)	Material Encountered
0.0 to 0.4	TOPSOIL , dark brown clayey soil, medium grained, loose, fine trace of organic matter, dry.
0.4 to 0.8	CLAY , brown, fine grained, firm to stiff, dry.

End of test @ 0.8m. No water table encountered.

DCP TEST RESULTS – Dynamic Cone Penetrometer					
Equipment: 9kg hammer, 510mm drop, conical tip.			Standard: AS1289.6.3.2 - 1997		
Depth(m) Blows/0.3m	DCP 1 (~RL20.5)	DCP 2 (~RL19.5)	DCP 3 (~RL22.9)	DCP 4 (~RL25.7)	DCP 5 (~RL25.4)
0.0 to 0.3	4	4	3	4	4
0.3 to 0.6	4	5	6	4	6
0.6 to 0.9	12	9	9	14	18
0.9 to 1.2	24	16	12	26	32
1.2 to 1.5	32	28	21	31	#
1.5 to 1.8	#	36	29	#	
1.8 to 2.1		#	36		
			#		
	End of Test @ 1.5m	End of Test @ 1.8m	End of Test @ 2.1m	End of Test @ 1.5m	End of Test @ 1.5m

#refusal/end of test. F=DCP fell after being struck showing little resistance through all or part of the interval.

DCP Notes:

DCP1 – End of test @ 1.5m, DCP still going down slowly, orange and red clay on dry tip.

DCP2 – End of test @ 1.5m, DCP still going down slowly, orange and red clay on dry tip.

DCP3 – End of test @ 1.8m, DCP still going down slowly, orange and red clay on dry tip.

DCP4 – End of test @ 1.5m, DCP still going down slowly, orange and red clay on dry tip.

DCP5 – End of test @ 1.5m, DCP still going down slowly, orange and red clay on dry tip.

5. Geological Observations/Interpretation

The slope materials are colluvial at the near surface and residual at depth. In the test locations, the ground materials consist of shallow soils over clays. The clay merges into the underlying weathered rock at depths of between ~1.2m to ~1.8m below the current surface. The weathered zone is interpreted to be Extremely Low Strength Shale. See Type Section attached for a diagrammatical representation of the expected ground materials.

6. Groundwater

Normal ground water seepage is expected to move over the buried surface of the rock and through the cracks. Due to the slope and elevation of the block, the water table is expected to be many metres below the base of the proposed works.

7. Surface Water

The owner informed us on site that overland flows entered the property from above during the recent heavy rainfall. As the weather on the east coast is modelled to become more extreme in the future, it is recommended that a dish drain at least 0.6m in diameter and raised at the downhill edge so flows can't skip over the drain be installed. This is to be installed across the uphill side of the property. Any pits and piping to direct flows to the stormwater system for the house are to be oversized and designed to cope with extreme rainfall events. The drain is to be designed by a stormwater/civil engineer in consultation with the geotechnical consultant. It is a condition of the risk assessment in **Section 8** that this be done.

8. Geotechnical Hazards and Risk Analysis

No geotechnical hazards were observed above or beside the property. The moderate to steeply graded slope that falls across the property and continues below is a potential hazard (**Hazard One**). The proposed excavation for the pool is a potential hazard until retaining structures are in place (**Hazard Two**).

Risk Analysis Summary

HAZARDS	Hazard One	Hazard Two
TYPE	The moderate to steep slope that falls across the property and continues below failing and impacting on the proposed works.	The excavation for the proposed pool (up to a maximum depth of ~1.8m) collapsing onto the work site before retaining structures are in place.
LIKELIHOOD	'Unlikely' (10^{-4})	'Possible' (10^{-3})
CONSEQUENCES TO PROPERTY	'Medium' (15%)	'Medium' (15%)
RISK TO PROPERTY	'Low' (2×10^{-5})	'Moderate' (2×10^{-4})
RISK TO LIFE	9.1×10^{-7} /annum	8.3×10^{-6} /annum
COMMENTS	This level of risk is 'ACCEPTABLE', provided the recommendations in Section 7 are followed.	This level of risk to life and property is 'UNNACCEPTABLE'. To move risk to 'ACCEPTABLE' levels, the recommendations in Section 13 and 14 are to be followed.

(See Aust. Geomech. Jnl. Mar 2007 Vol. 42 No 1, for full explanation of terms)

9. Suitability of the Proposed Development for the Site

The proposed development is suitable for the site. No geotechnical hazards will be created by the completion of the proposed development provided it is carried out in accordance with the requirements of this report and good engineering and building practice.

10. Stormwater

The fall is away from the street. The stormwater engineer is to refer to council stormwater policy for suitable options for stormwater disposal.

11. Excavations

An excavation to a maximum depth of ~1.8m is required for the proposed pool on the downhill side of the property.

The excavations are expected to be through shallow soil over clay with Extremely Low Strength Shale expected at depths of between ~1.2m and ~1.8m. It is envisaged that excavations through soil, clay, and Extremely Low Strength Shale can be carried out with an excavator and bucket.

12. Vibrations

No excessive vibrations will be generated by excavation through soil, clay, and Extremely Low Strength Shale. Any vibrations generated by a domestic machine and bucket up to 16 ton carrying out excavation works will be below the threshold limit for infrastructure or building damage.

13. Excavation Support Advice

The excavations for the proposed pool will reach a maximum depth of ~1.8m. The setbacks are as follows:

- ~1.3m from the proposed carport.
- ~3.0m from the upslope concrete block retaining wall.
- ~3.8m from the N common boundary.
- ~4.0m from the upslope proposed extension.

As such, provided the pool excavation is carried out prior to the construction of the proposed carport, no structures or boundaries are expected to lie within the zone of influence of the proposed excavation.

The sides of the proposed pool excavation are expected to stand at near-vertical angles for short periods of time until the pool structure is installed, provided the cut batters are kept from becoming saturated. If the cut batters through soil and clay remain unsupported for more than a few days before pool construction commences, they are to be supported with typical pool shoring until the pool structure is in place.

Upslope runoff is to be diverted from the cut faces by sandbag mounds or other diversion works. Unsupported cut batters through soil, and clay are to be covered to prevent access of water in wet weather and loss of moisture in dry weather. The covers are to be tied down with metal pegs or other suitable fixtures so they can't blow off in a storm. The materials and labour to construct the retaining structures are to be organised so on completion of the excavations they can be constructed as soon as possible. The excavations are to be carried out during a dry period. No excavations are to commence if heavy or prolonged rainfall is forecast.

All excavation spoil is to be removed from site following the current Environmental Protection Agency (EPA) waste classification guidelines.

14. Retaining Structures

For cantilever or singly-propped retaining structures, it is suggested the design be based on a triangular pressure distribution of lateral pressures using the parameters shown in Table 1.

Table 1 – Likely Earth Pressures for Retaining Structures

Unit	Earth Pressure Coefficients		
	Unit weight (kN/m ³)	'Active' K_a	'At Rest' K_0
Soil, and Residual Clays	20	0.35	0.45
Extremely Low Strength Shale	22	0.3	0.25

For rock classes refer to Pells et al "Design Loadings for Foundations on Shale and Sandstone in the Sydney Region". Australian Geomechanics Journal 1978.

It is to be noted that the earth pressures in Table 1 assume a level surface above the structure, do not account for any surcharge loads, and assume retaining structures are fully drained. Rock strength and relevant earth pressure coefficients are to be confirmed on site by the geotechnical consultant.

All retaining structures are to have sufficient back-wall drainage and be backfilled immediately behind the structure with free-draining material (such as gravel). This material is to be wrapped in a non-woven Geotextile fabric (i.e., Bidim A34 or similar), to prevent the drainage from becoming clogged with silt and clay. If no back-wall drainage is installed in retaining structures, the likely hydrostatic pressures are to be accounted for in the structural design.

15. Foundations

The level patio area on the uphill side of the house can be supported on a thickened edge/raft slab supported on the natural clays.

A maximum allowable pressure of 200kPa can be assumed for footings supported on the underlying firm to hard clays.

The proposed carport and extensions to the downhill side of the house can be supported on piers taken to the underlying Extremely Low Strength Shale.

The proposed pool is expected to be partially seated in Extremely Low Strength Shale. This is a suitable foundation material. It is expected to be exposed across the uphill side of the proposed excavation. Where it is not exposed, and where weathered rock drops away with the slope, piers taken to Extremely Low Strength Shale will be required to maintain a uniform foundation material across the structure.

Extremely Low Strength Shale is expected at depths of between 1.2m to 1.8m below the current surface in the area of the proposed works.

A maximum allowable bearing pressure of 600kPa can be assumed for footings on Extremely Low Strength Shale. It should be noted that this material is a soft rock and a rock auger will cut through it so the builders should not be looking for refusal to end the footings.

As the bearing capacity of clay and shale reduces when it is wet, we recommend the footings be dug, inspected, and poured in quick succession (ideally the same day if possible). If the

footings get wet, they will have to be drained and the soft layer of wet clay or shale on the footing surface will have to be removed before concrete is poured.

If a rapid turnaround from footing excavation to the concrete pour is not possible, a sealing layer of concrete may be added to the footing surface after it has been cleaned.

NOTE: If the contractor is unsure of the footing material required, it is more cost-effective to get the geotechnical consultant on site at the start of the footing excavation to advise on footing depth and material. This mostly prevents unnecessary over-excavation in clay-like shaly-rock but can be valuable in all types of geology.

16. Geotechnical Review

The structural plans are to be checked and certified by the geotechnical engineer as being in accordance with the geotechnical recommendations. On completion, a Form 2B will be issued. This form is required for the Construction Certificate to proceed.

17. Inspections

The client and builder are to familiarise themselves with the following required inspections as well as council geotechnical policy. We cannot provide geotechnical certification for the owners and Occupation Certificate if the following inspections have not been carried out during the construction process.

- All footings are to be inspected and approved by the geotechnical consultant while the excavation equipment and contractors are still onsite and before steel reinforcing is placed or concrete is poured.

White Geotechnical Group Pty Ltd.



Ben White M.Sc. Geol.,
AusIMM., CP GEOL.
No. 222757
Engineering Geologist.



Photo 1



Photo 2



Photo 3



Photo 4



Photo 5



Photo 6



Photo 7



Photo 8

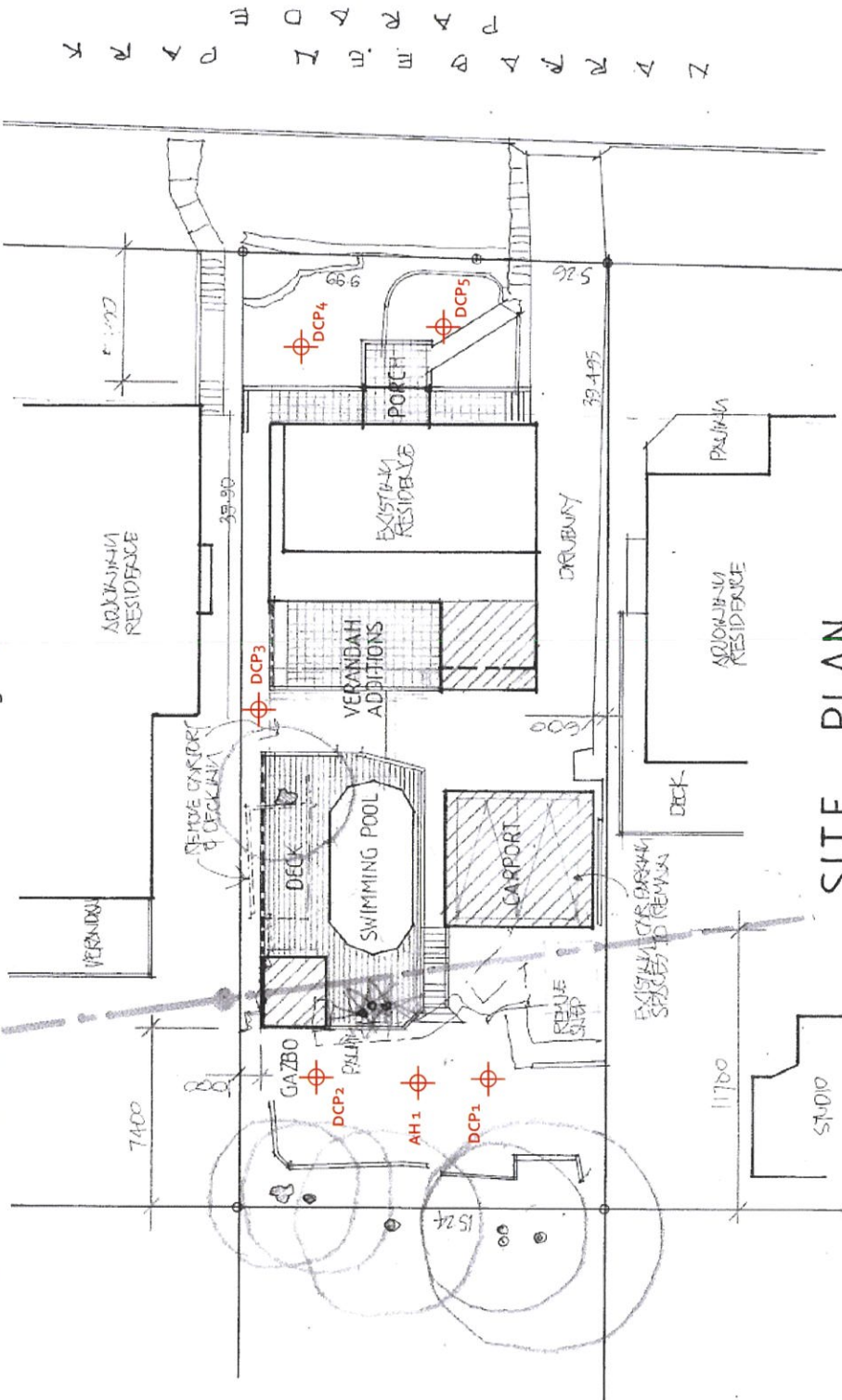
Important Information about Your Report

It should be noted that Geotechnical Reports are documents that build a picture of the subsurface conditions from the observation of surface features and testing carried out at specific points on the site. The spacing and location of the test points can be limited by the location of existing structures on the site or by budget and time constraints of the client. Additionally, the test themselves, although chosen for their suitability for the particular project, have their own limiting factors. The testing gives accurate information at the location of the test, within the confines of the test's capability. A geological interpretation or model is developed by joining these test points using all available data and drawing on previous experience of the geotechnical consultant. Even the most experienced practitioners cannot determine every possible feature or change that may lie below the earth. All of the subsurface features can only be known when they are revealed by excavation. As such, a Geotechnical report can be considered an interpretive document. It is based on factual data but also on opinion and judgement that comes with a level of uncertainty. This information is provided to help explain the nature and limitations of your report.

With this in mind, the following points are to be noted:

- If upon the commencement of the works the subsurface ground or ground water conditions prove different from those described in this report, it is advisable to contact White Geotechnical Group immediately, as problems relating to the ground works phase of construction are far easier and less costly to overcome if they are addressed early.
- If this report is used by other professionals during the design or construction process, any questions should be directed to White Geotechnical Group as only we understand the full methodology behind the report's conclusions.
- The report addresses issues relating to your specific design and site. If the proposed project design changes, aspects of the report may no longer apply. Contact White Geotechnical if this occurs.
- This report should not be applied to any other project other than that outlined in section 1.0.
- This report is to be read in full and should not have sections removed or included in other documents as this can result in misinterpretation of the data by others.
- It is common for the design and construction process to be adapted as it progresses (sometimes to suit the previous experience of the contractors involved). If alternative design and construction processes are required to those described in this report, contact White Geotechnical Group. We are familiar with a variety of techniques to reduce risk and can advise if your proposed methods are suitable for the site conditions.

SITE PLAN – showing test locations



SITE PLAN

LOT 21 H.O.P. 157629

DEVELOPMENT CALCULATIONS			
SITE AREA	605.30	SQUARE METRES	
DESCRIPTION	EXISTING SQM	PROPOSED SQM	
RESIDENCE FLOOR	207.10	220.70	
RESIDENCE ROOF	95.20	99.20	
VERANDAH	11.20	26.65	
DRIVEWAY & TURNING	109.20	77.60	
CARPORT	15.00	—	
AWNING	8.10	—	
PORCH	—	14.32	
GAZBO	—	8.10	
SHED	6.00	—	
SWIMMING POOL & DECKING	39.46	70.78	
PARKING SPACES	27.00	—	
TOTAL HARD SURFACE	312.06	(51.55%)	331.77 (54.77%)

NOTES:

- NOTES:
1. THE BUILDER IS TO CHECK AND CONFIRM ALL NECESSARY DIMENSIONS AND LEVELS ON SITE PRIOR TO ORDERING MATERIALS AND COMMENCING CONSTRUCTION. DO NOT SCALE OFF THE DRAWING.
 2. SHOULD ANY DEVELOPMENT OR CONSTRUCTION OCCUR ON OR NEAR BOUNDARIES, THE BOUNDARIES SHOULD BE CLEARLY MARKED ON SITE BY THE REGISTERED LAND SURVEYORS.

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Before it could end services of temporary structures at this point to construction, the staff made the discovery. At the moment that the members of the *Asamblea* were subject to verification by the state agency. At that time it was reported that the *Asamblea* had been in the vicinity of some military headquarters & that soldiers were being recruited. The *Asamblea* was then taken to the station and the leader to the state's apartment, used for any emergency situation, in which he was to be captured by a Divisional Troop.

That night a military helicopter was dispatched to the apartment where it was thought to have caught the fugitive. An exact point of light relative to be determined by moon.

J.D. EVANS and COMPANY

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№	Адресат	Дата

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Final writer & sub-unit envelope to be disposed of in the approved manner or as directed by local council inspectors.
All electrical power & light outlets to be determined by owner.

PROJECT
PROPOSED ALTERATION/ADDITIONS
NO. 34 NARRABEEN PARK PARADE
WARRIEWOOD N. S. W. 2102

CLIENT
JAMES & AMANDA WHITEHEAD

2088-1

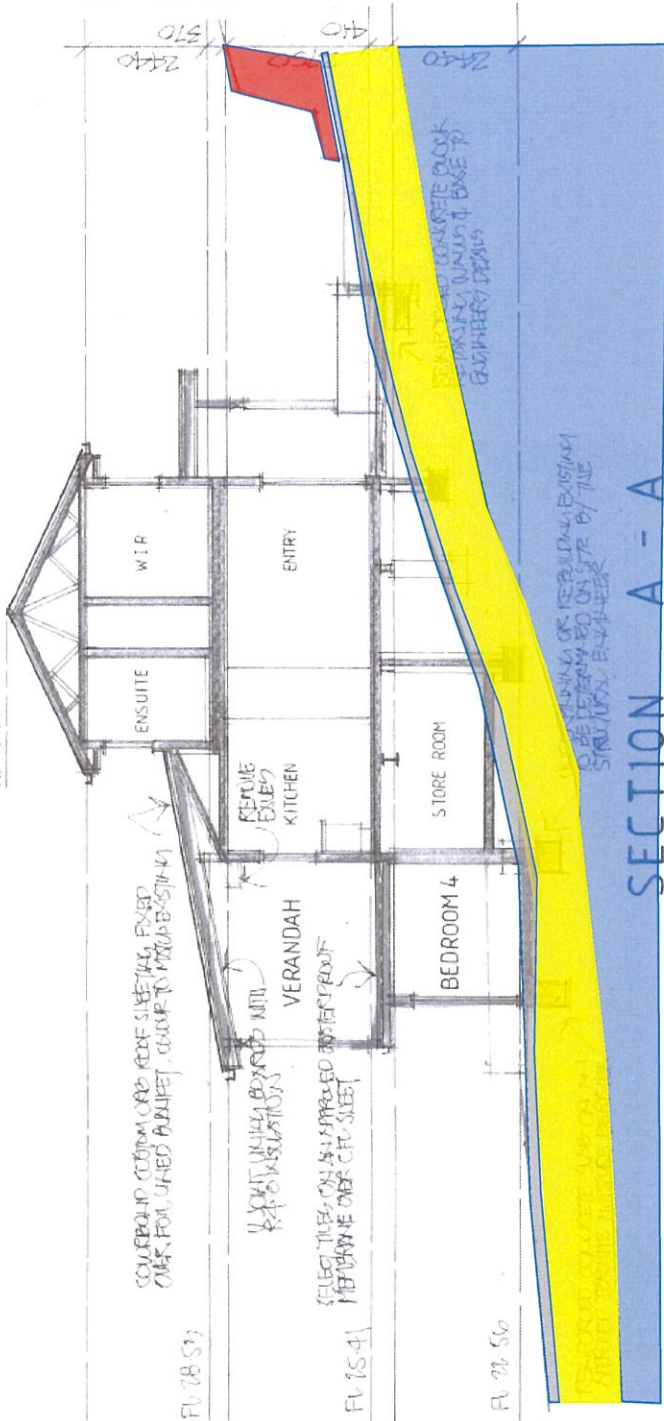
TYPE SECTION - Diagrammatical Interpretation of expected Ground Materials

R. 31.246

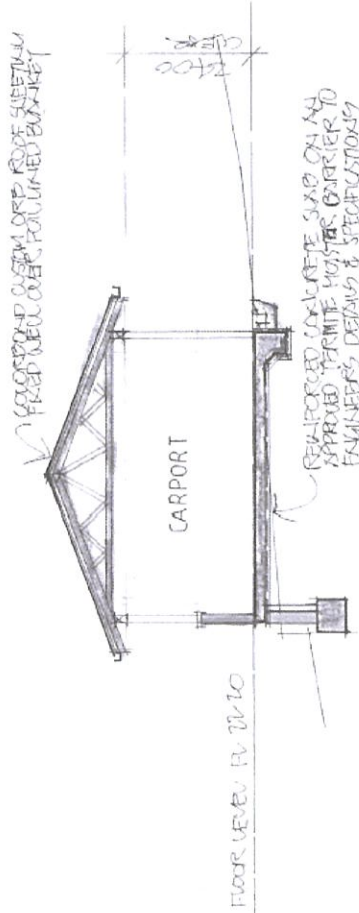
Expected Ground Materials



Narraheen Group Rocks - Extremely Low Strength Shale - after being cut up by excavation equipment can resemble a stiff to hard clay.



SECTION A - A



SECTION C - C

PROJECT: 15/02/2022
 DRAWN: J.E.
 SCALE: 1:100
 SHEET: 2088-7

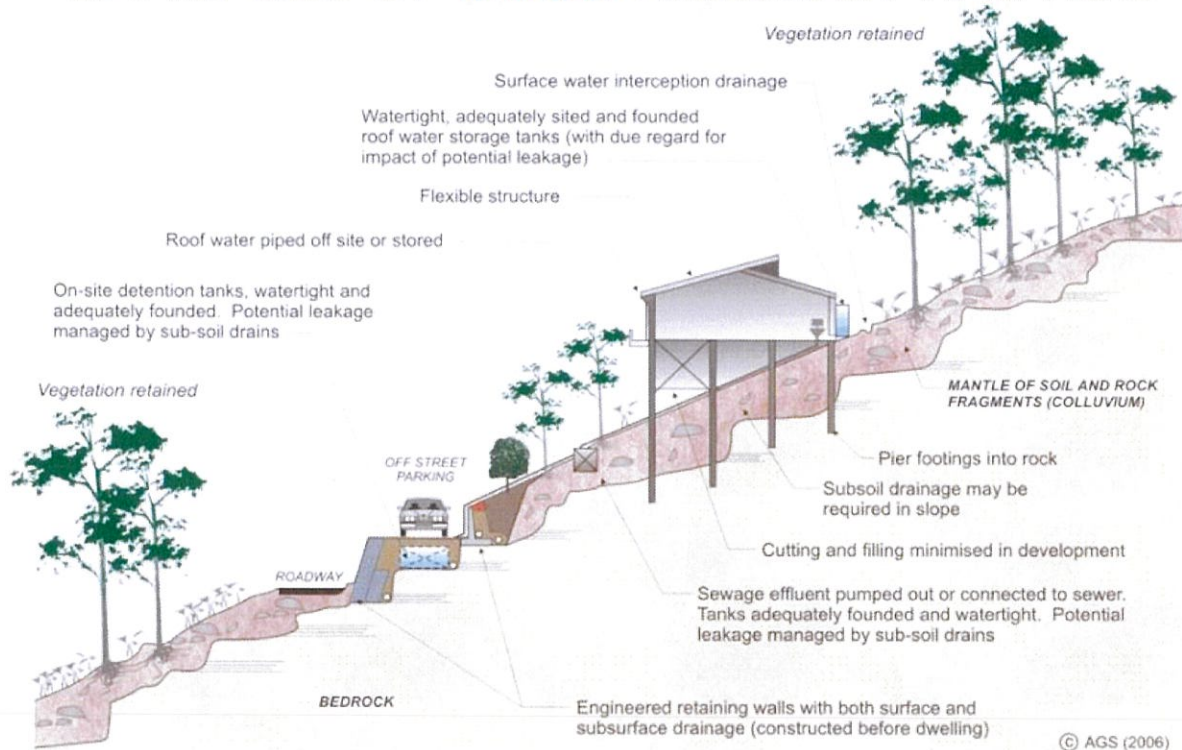
PROPOSED ALTERATION/ADDITIONS
 NO. 34 NARRABEEN PARK PARADE
 WARRIEWOOD N.S.W. 2102
 CLIENT: JAMES & AMANDA WHITEHEAD

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 4. All dimensions are to be taken from the face of the wall unless otherwise indicated.
 5. All dimensions are to be taken from the face of the wall unless otherwise indicated.
 6. All dimensions are to be taken from the face of the wall unless otherwise indicated.

EXAMPLES OF **GOOD** HILLSIDE PRACTICE



EXAMPLES OF **POOR** HILLSIDE PRACTICE

