GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER FORM NO. 1 – To be submitted with Development Application

vel	opment Application	on forName of Applicant			
ddress of site 23 Park Avenue, Avalon Beach					
	·				
		vers the minimum requirements to be addressed in a Geotechnical Risk Declaration made by r engineering geologist or coastal engineer (where applicable) as part of a geotechnical report			
	Ben White (Insert Name)	on behalf of White Geotechnical Group Pty Ltd (Trading or Company Name)			
	(moon ramo)	(mading of company mains)			
nee inis	er as defined by the	28/9/21 certify that I am a geotechnical engineer or engineering geologist or coastance Geotechnical Risk Management Policy for Pittwater - 2009 and I am authorised by the above ssue this document and to certify that the organisation/company has a current professional indemnity.			
	mark appropriato	hav			
156	mark appropriate	box			
		the detailed Geotechnical Report referenced below in accordance with the Australia Geomechanics lide Risk Management Guidelines (AGS 2007) and the Geotechnical Risk Management Policy for			
	accordance with	echnically verify that the detailed Geotechnical Report referenced below has been prepared in the Australian Geomechanics Society's Landslide Risk Management Guidelines (AGS 2007) and the sk Management Policy for Pittwater - 2009			
	with Section 6.0 assessment for	the site and the proposed development in detail and have carried out a risk assessment in accordance of the Geotechnical Risk Management Policy for Pittwater - 2009. I confirm that the results of the risl the proposed development are in compliance with the Geotechnical Risk Management Policy fo and further detailed geotechnical reporting is not required for the subject site.			
	have examined to Application only	the site and the proposed development/alteration in detail and I am of the opinion that the Development involves Minor Development/Alteration that does not require a Geotechnical Report or Risk thence my Report is in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009			
	have examined the Hazard and doe	the site and the proposed development/alteration is separate from and is not affected by a Geotechnical s not require a Geotechnical Report or Risk Assessment and hence my Report is in accordance with Il Risk Management Policy for Pittwater - 2009 requirements.			
		ne coastal process and coastal forces analysis for inclusion in the Geotechnical Report			
toc	hnical Report Det	aile:			
100		otechnical Report 23 Park Avenue, Avalon Beach			
	Author: BEN WI	HITE			
	Author's Compar	ny/Organisation: WHITE GEOTECHNICAL GROUP PTY LTD			
um	entation which re	late to or are relied upon in report preparation:			
	Australian G	eomechanics Society Landslide Risk Management March 2007.			
	White Geote	echnical Group company archives.			
elop Mage	pment Application f anagement aspect ement" level for the	We Geotechnical Report, prepared for the abovementioned site is to be submitted in support of a for this site and will be relied on by Pittwater Council as the basis for ensuring that the Geotechnical softhe proposed development have been adequately addressed to achieve an "Acceptable Ris life of the structure, taken as at least 100 years unless otherwise stated and justified in the Report and call measures have been identified to remove foreseeable risk.			

Signature

Name Ben White

Chartered Professional Status MScGEOLAusIMM CP GEOL

Membership No. 222757

Company White Geotechnical Group Pty Ltd

GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER FORM NO. 1(a) - Checklist of Requirements for Geotechnical Risk Management Report for Development Application

Develo	pment Application	n for	11		
Name of Applicant					
Addres	ss of site	23 Park Avenue, Avalon I	3each		
Report. 7	This checklist is to a	ccompany the Geotechnical Repo	e addressed in a Geotechnical Risk Management Geotechnical rt and its certification (Form No. 1).		
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		Report 23 Park Avenue, Avak	оп веасп		
	Date: 28/9/21 BEN WHITE				
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		nisation: WHITE GEOTECHNICA	L GROUP PIT LID		
Please n	nark appropriate b	ox			
\boxtimes	Comprehensive site	e mapping conducted 17/9/21 (date)			
\boxtimes	Mapping details pre	,	eomorphic mapping to a minimum scale of 1:200 (as appropriate)		
\boxtimes	Subsurface investig	gation required			
	□ No	Justification			
	⊠ Yes	Date conducted 17/9/21			
		el developed and reported as an infer	red subsurface type-section		
\boxtimes	Geotechnical hazar				
		e the site			
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		rds described and reported	stackwisel Diels Management Delies for Ditturator 2000		
\boxtimes		equence analysis	otechnical Risk Management Policy for Pittwater - 2009		
		uency analysis			
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		or property conducted in accordance	with the Geotechnical Risk Management Policy for Pittwater - 2009		
			e with the Geotechnical Risk Management Policy for Pittwater - 2009		
			k Management" criteria as defined in the Geotechnical Risk		
		for Pittwater - 2009	K Management offeria as defined in the Secteon floar No.		
\boxtimes			the "Acceptable Risk Management" criteria provided that the		
	specified conditions	· ·			
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	0	specify			
			as described in the Geotechnical Risk Management Policy for		
	Pittwater - 2009 ha	•	practical have been identified and included in the report.		
		rithin Bushfire Asset Protection Zone	·		
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that the g Manager	geotechnical risk ma ment" level for the li	anagement aspects of the proposal ife of the structure, taken as at lea ctical measures have been identifi			
		Signature	elit		
		Name	Ben White		
		Chartered Professional Status	MScGEOLAusIMM CP GEOL		

Company White Geotechnical Group Pty Ltd

Membership No.



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GEOTECHNICAL INVESTIGATION:

Alterations and Additions at 23 Park Avenue, Avalon Beach

1. Proposed Development

- **1.1** Construct a new upper level over the existing footprint of the house.
- **1.2** Construct a ground floor extension on the uphill side of the house.
- **1.3** Various other internal and external alterations and additions.
- 1.4 Details of the proposed development are shown on 8 drawings by JJ Drafting, project number 923/21, drawings numbered DA.01 to DA.8, dated July 2021.

2. Site Description

- **2.1** The site was inspected on the 17th September, 2021.
- 2.2 This residential property is on the high side of the road and has a SW aspect. It is located on the gently graded upper reaches and crest of a NW-SE trending ridgeline. The slope rises to the house then falls across the property at angles averaging \sim 5°. The slope below continues at increasing angles and the slope above levels at the crest of the ridgeline.
- 2.3 At the road frontage, a gravel driveway runs across the slope to a timber framed carport on the uphill side of the property (Photo 1). The part two-storey brick and timber clad house is supported on brick walls and brick piers. The brick walls show no significant signs of movement and the brick piers stand vertical (Photo 2). Some of the supporting piers were observed to be supported directly onto outcropping Medium Strength Sandstone (Photo 3). A gently sloping lawn extends from the downhill side of the house to the lower common boundary (Photo 4). Sandstone can be seen outcropping through the slope in several places (Photo 5).



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3. Geology

The Sydney 1:100 000 Geological sheet indicates the site is underlain by the Newport Formation of the Narrabeen Group. This is described as interbedded laminite, shale and quartz to lithic quartz sandstone. A band of Medium Strength Sandstone underlies the location of the proposed works and extends through the otherwise shale-dominated profile.

4. Subsurface Investigation

One hand Auger Hole (AH) was put down to identify the ground materials. Four Dynamic Cone Penetrometer (DCP) tests were put down to determine the relative density of the overlying soil and the depth to weathered rock. The locations of the tests are shown on the site plan attached. It should be noted that a level of caution should be applied when interpreting DCP test results. The test will not pass through hard buried objects so in some instances it can be difficult to determine whether refusal has occurred on an obstruction in the profile or on the natural rock surface. This is not expected to be an issue for the testing on this site. However, excavation and foundation budgets should always allow for the possibility that the interpreted ground conditions in this report vary from those encountered during excavations. See the appended "Important information about your report" for a more comprehensive explanation. The results are as follows:

AUGER HOLE 1 (~RL37.8) – AH1 (Photo 6)

Depth (m)	Material Encountered		
0.0 to 0.15	SILTY SOIL, dark brown, orange, grey, fine to coarse grained, loose to		
	medium dense, dry.		
0.15 to 0.3	CLAYEY SAND, brown, rock fragments, fine to medium grained,		
	medium dense, dry.		

Refusal on rock @ 0.3m. No water table encountered.



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DCP TEST RESULTS – Dynamic Cone Penetrometer					
Equipment: 9kg ham	nmer, 510mm drop, cor	nical tip.	Standard: AS1289.6.3.2 - 1997		
Depth(m) Blows/0.3m	DCP 1 (~RL37.8)	DCP 2 (~RL38.5)	DCP 3 (~RL38.9)	DCP 4 (~RL39.9)	
0.0 to 0.3	6	11	5	8	
0.3 to 0.6	#	13	16	10	
0.6 to 0.9		21	#	#	
0.9 to 1.2		#			
	Refusal on Rock @ 0.3m	Refusal on Rock @ 0.9m	Refusal on Rock @ 0.6m	Refusal on Rock @ 0.4m	

#refusal/end of test. F=DCP fell after being struck showing little resistance through all or part of the interval.

DCP Notes:

DCP1 – Refusal on rock @ 0.3m, DCP bouncing off rock surface, white impact dust on dry tip.

DCP2 – Refusal on rock @ 0.9m, DCP bouncing off rock surface, orange and white impact dust on dry tip.

DCP3 – Refusal on rock @ 0.6m, DCP bouncing off rock surface, orange clay on dry tip.

DCP4 – Refusal on rock @ 0.4m, DCP bouncing off rock surface, white impact on dry tip.

5. Geological Observations/Interpretation

Sandstone bedrock outcrops under the house and through the slope below the property. This is an unusually thick sandstone bed within the Narrabeen Group of rocks. The rock is overlain by silty soils and clayey sands that fill the bench-step formation. In the test locations, the depth to Medium Strength Sandstone ranged between 0.3 to 0.9m below the current surface, being deeper due to the stepped nature of the underlying bedrock. The sandstone underlying the area of the proposed development is estimated to be medium strength. See Type Section attached for a diagrammatical representation of the expected ground materials.



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6. Groundwater

Normal ground water seepage is expected to move over the buried surface of the rock and through the cracks. Due to the slope and elevation of the block, the water table is expected to be many metres below the area of the proposed works.

7. Surface Water

No evidence of surface flows were observed on the property during the inspection. As the property encompasses the crest of the hill, any surface flows will be generated on the property and will flow away from the property.

8. Geotechnical Hazards and Risk Analysis

No geotechnical hazards were observed above or beside the property. The gently graded land surface that falls across the property and continues below is a potential hazard (Hazard One).

Risk Analysis Summary

HAZARDS	Hazard One	
TYPE	The gentle slope that falls across the property and continues below failing and impacting on the proposed works.	
LIKELIHOOD	'Unlikely' (10 ⁻⁴)	
CONSEQUENCES TO PROPERTY	'low' (5%)	
RISK TO PROPERTY	'Low' (2 x 10 ⁻⁵)	
RISK TO LIFE	5.5 x 10 ⁻⁷ /annum	
COMMENTS	This level of risk is 'ACCEPTABLE'.	

(See Aust. Geomech. Jnl. Mar 2007 Vol. 42 No 1, for full explanation of terms)

9. Suitability of the Proposed Development for the Site

The proposed development is suitable for the site. No geotechnical hazards will be created by the completion of the proposed development provided it is carried out in accordance with the requirements of this report and good engineering and building practice.



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10. Stormwater

The roofing of the proposed works adds less than ~50m² to the current roof area so it is

possible the existing stormwater system can be used with the approval of the stormwater

engineer.

11. Excavations

Apart from those for footings, no excavations are required.

12. Foundations

Any additional footings required for the proposed works are to be supported on pads or piers

taken to Medium Strength Sandstone. This material is expected at depths of between ~0.4m

and ~0.9m below the current surface.

A maximum allowable bearing pressure of 600kPa can be assumed for footings on Medium

Strength Sandstone.

Naturally occurring vertical cracks (known as joints) commonly occur in sandstone. These are

generally filled with soil and are the natural seepage paths through the rock. They can extend

to depths of several metres and are usually relatively narrow but can range between 0.1 to

0.8m wide. If a footing falls over a joint in the rock, the construction process is simplified if

with the approval of the structural engineer the joint can be spanned or alternatively the

footing can be repositioned so it does not fall over the joint.

NOTE: If the contractor is unsure of the footing material required, it is more cost-effective to

get the geotechnical consultant on site at the start of the footing excavation to advise on

footing depth and material. This mostly prevents unnecessary over-excavation in clay-like

shaly-rock but can be valuable in all types of geology.



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13. Geotechnical Review

The structural plans are to be checked and certified by the geotechnical engineer as being in accordance with the geotechnical recommendations. On completion, a Form 2B will be issued. This form is required for the Construction Certificate to proceed.

14. Inspection

The client and builder are to familiarise themselves with the following required inspection as well as council geotechnical policy. We cannot provide geotechnical certification for the owners and Occupation Certificate if the following inspection has not been carried out during the construction process.

 All footings are to be inspected and approved by the geotechnical consultant while the excavation equipment and contractors are still onsite and before steel reinforcing is placed or concrete is poured.

White Geotechnical Group Pty Ltd.

Bulit

Ben White M.Sc. Geol., AusIMM., CP GEOL.

No. 222757

Engineering Geologist.



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Photo 1



Photo 2



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Photo 3



Photo 4



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Photo 5



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Photo 6 (Top is top of hole)



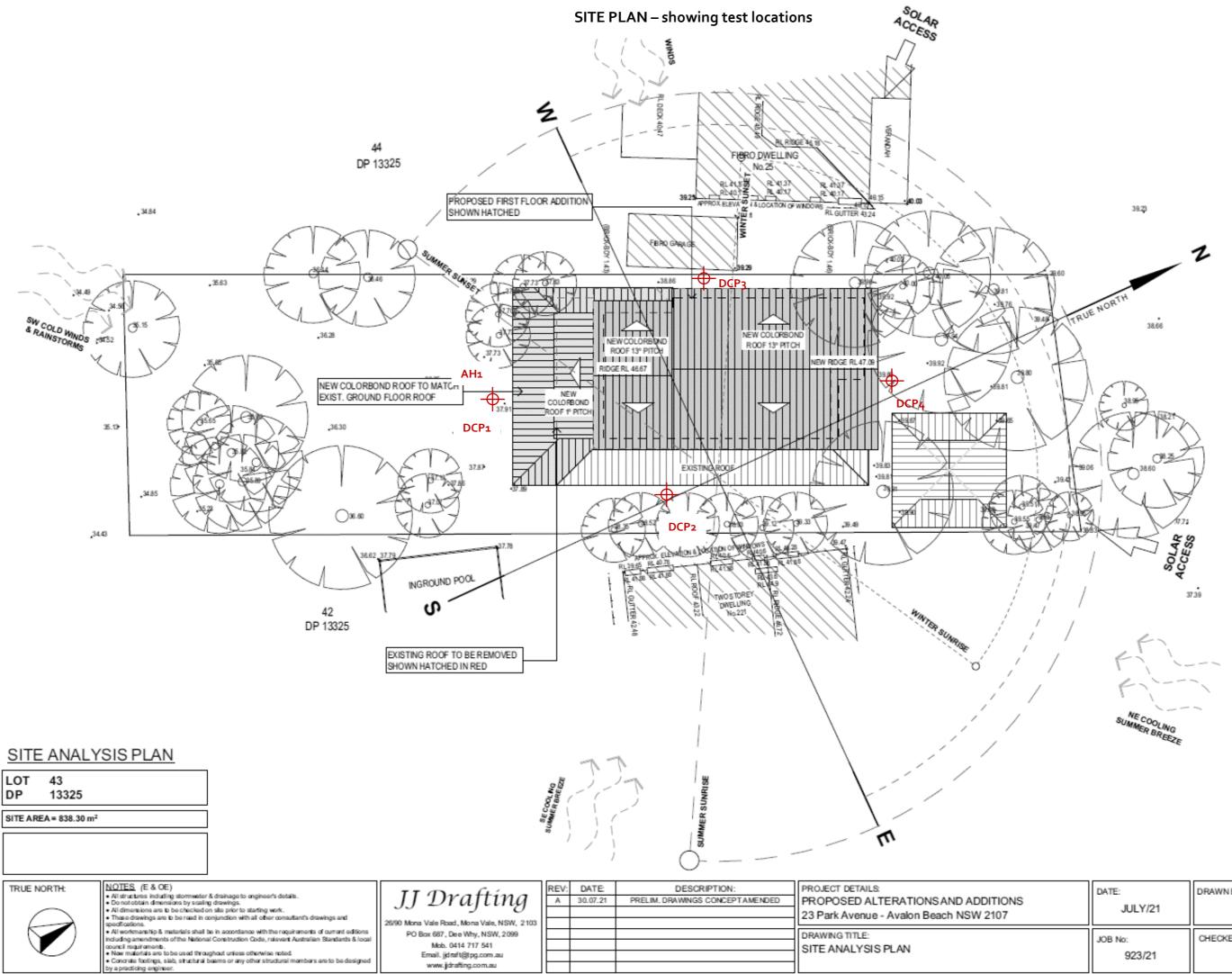
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Important Information about Your Report

It should be noted that Geotechnical Reports are documents that build a picture of the subsurface conditions from the observation of surface features and testing carried out at specific points on the site. The spacing and location of the test points can be limited by the location of existing structures on the site or by budget and time constraints of the client. Additionally, the test themselves, although chosen for their suitability for the particular project, have their own limiting factors. The testing gives accurate information at the location of the test, within the confines of the test's capability. A geological interpretation or model is developed by joining these test points using all available data and drawing on previous experience of the geotechnical consultant. Even the most experienced practitioners cannot determine every possible feature or change that may lie below the earth. All of the subsurface features can only be known when they are revealed by excavation. As such, a Geotechnical report can be considered an interpretive document. It is based on factual data but also on opinion and judgement that comes with a level of uncertainty. This information is provided to help explain the nature and limitations of your report.

With this in mind, the following points are to be noted:

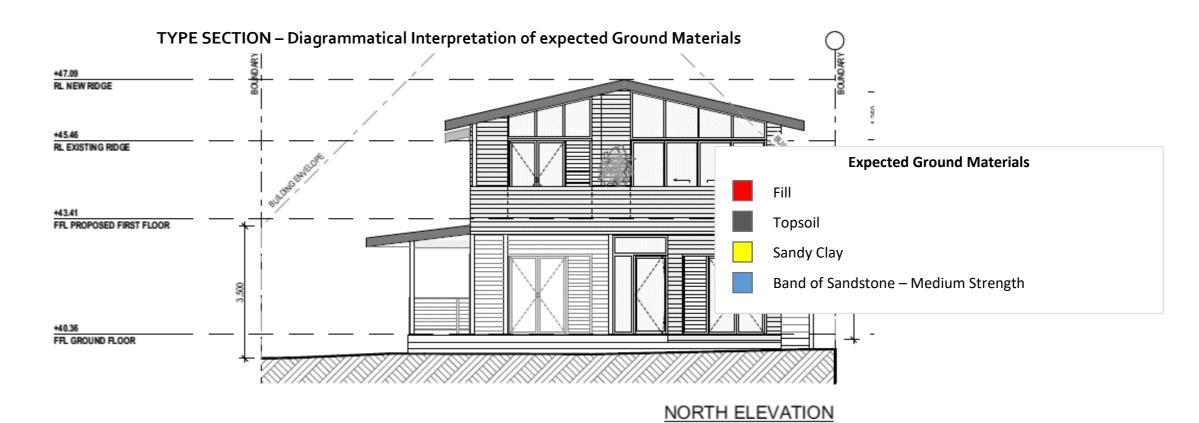
- If upon the commencement of the works the subsurface ground or ground water conditions prove different from those described in this report, it is advisable to contact White Geotechnical Group immediately, as problems relating to the ground works phase of construction are far easier and less costly to overcome if they are addressed early.
- If this report is used by other professionals during the design or construction process, any questions should be directed to White Geotechnical Group as only we understand the full methodology behind the report's conclusions.
- The report addresses issues relating to your specific design and site. If the proposed project design changes, aspects of the report may no longer apply. Contact White Geotechnical if this occurs.
- This report should not be applied to any other project other than that outlined in section 1.0.
- This report is to be read in full and should not have sections removed or included in other documents as this can result in misinterpretation of the data by others.
- It is common for the design and construction process to be adapted as it progresses (sometimes to suit the previous experience of the contractors involved). If alternative design and construction processes are required to those described in this report, contact White Geotechnical Group. We are familiar with a variety of techniques to reduce risk and can advise if your proposed methods are suitable for the site conditions.

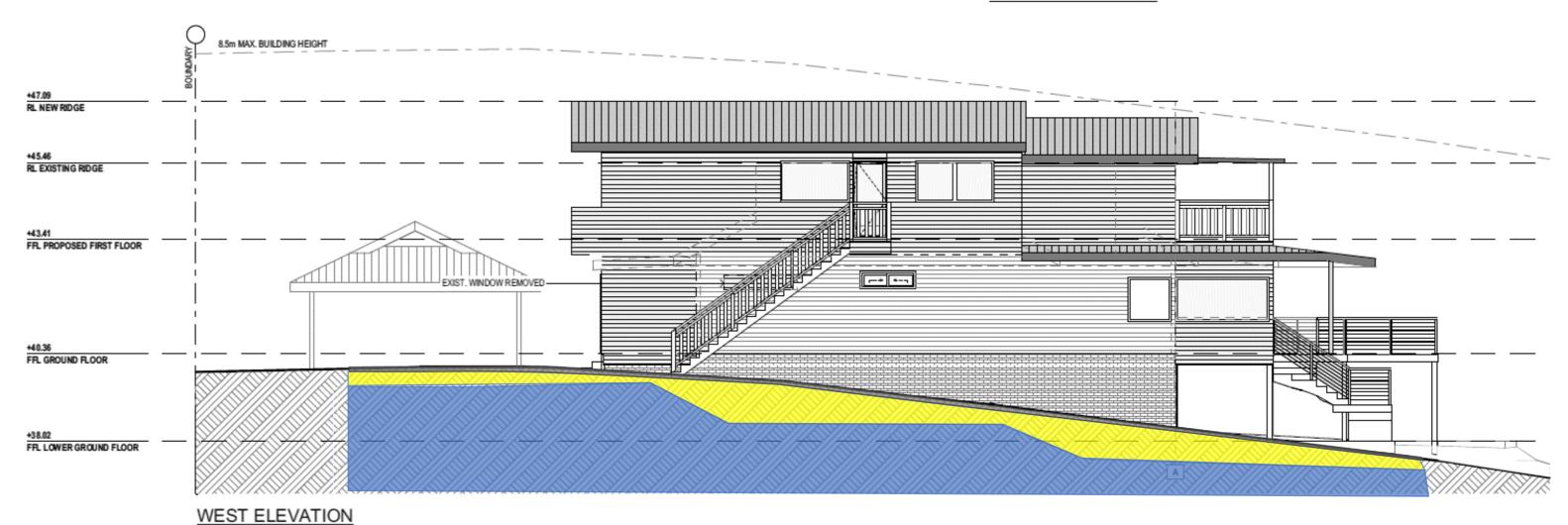


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- 1	REV.	DATE	DESCRIPTION:
- 1	Α	30.07.21	PRELIM. DRAWINGS CONCEPTAMENDED
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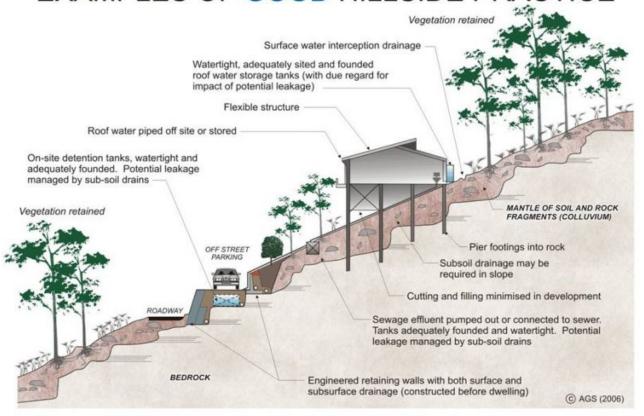
PROJECT DETAILS: PROPOSED ALTERATIONS AND ADDITIONS 23 Park Avenue - Avalon Beach NSW 2107	DATE: JULY/21	DRAWNBY:	SCALE: 1:200 @ A3
DRAWING TITLE: SITE ANALYSIS PLAN	JOB No: 923/21	CHECKED BY:	DRAWING No: DA.01





NOTES (E & OE) REV: DATE: DESCRIPTION: PROJECT DETAILS: DATE: DRAWN BY: SCALE: JJ Drafting 30.07.21 PRELIM. DRAWINGS CONCEPTAMENDED PROPOSED ALTERATIONS AND ADDITIONS JULY/21 1:100 @ A3 LB 23 Park Avenue - Avalon Beach NSW 2107 174 Garden St, North Narrabe en, NSW, 2101 All workmanship & materials shall be in accordance with the requirements of current edition PO Box 687, Dee Why, NSW, 2099 DRAWING TITLE: CHECKED BY: DRAWING No: JOB No: Mob. 0414 717 541 council requirements. New materials are to be used throughout unless otherwise noted. Concrete facilings, slab, structural beams or any other structural members are to be design NORTH AND WEST ELEVATIONS Email. jjdraft@tpg.com.au 923/21 JJ DA.07 www.jdrafting.com.au

EXAMPLES OF GOOD HILLSIDE PRACTICE



EXAMPLES OF POOR HILLSIDE PRACTICE

