

**REPORT ON GEOTECHNICAL ASSESSMENT**

**for**

**PROPOSED NEW ALTERATIONS AND ADDITIONS**

**at**

**11 FLORENCE TERRACE, SCOTLAND ISLAND, NSW**

**Prepared For**

**James Wish**

**Project No.: 2024-172**

**Document Revision Record**

<b>Issue No</b>	<b>Date</b>	<b>Details of Revisions</b>
0	18 September 2024	Original issue

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**GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER  
FORM NO. 1 – To be submitted with Development Application**

Development Application for _____	James Wish
Name of Applicant	
Address of site _____ 11 Florence Terrace, Scotland Island, NSW	

**Declaration made by geotechnical engineer or engineering geologist or coastal engineer (where applicable) as part of a geotechnical report**

I, Troy Crozier on behalf of Crozier Geotechnical Consultants 18 September 2024 certify that I am a geotechnical engineer or engineering geologist or coastal engineer as defined by the Geotechnical Risk Management Policy for Pittwater - 2009 and I am authorised by the above organisation/company to issue this document and to certify that the organisation/company has a current professional indemnity policy of at least \$2million.  
I:

- have prepared the detailed Geotechnical Report referenced below in accordance with the Australia Geomechanics Society's Landslide Risk Management Guidelines (AGS 2007) and the Geotechnical Risk Management Policy for Pittwater - 2009
- am willing to technically verify that the detailed Geotechnical Report referenced below has been prepared in accordance with the Australian Geomechanics Society's Landslide Risk Management Guidelines (AGS 2007) and the Geotechnical Risk Management Policy for Pittwater - 2009
- have examined the site and the proposed development in detail and have carried out a risk assessment in accordance with Section 6.0 of the Geotechnical Risk Management Policy for Pittwater - 2009. I confirm that the results of the risk assessment for the proposed development are in compliance with the Geotechnical Risk Management Policy for Pittwater - 2009 and further detailed geotechnical reporting is not required for the subject site.
- have examined the site and the proposed development/alteration in detail and I am of the opinion that the Development Application only involves Minor Development/Alteration that does not require a Geotechnical Report or Risk Assessment and hence my Report is in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009 requirements.
- have examined the site and the proposed development/alteration is separate from and is not affected by a Geotechnical Hazard and does not require a Geotechnical Report or Risk Assessment and hence my Report is in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009 requirements.
- have provided the coastal process and coastal forces analysis for inclusion in the Geotechnical Report


**Geotechnical Report Details:**

<b>Report Title:</b> Geotechnical Report for Proposed Alterations and Additions	
<b>Report Date:</b> 18 September 2024	<b>Project No.:</b> 2024-172
<b>Author:</b> Ben Taylor	
<b>Author's Company/Organisation:</b> Crozier Geotechnical Consultants	

**Documentation which relate to or are relied upon in report preparation:**

Architectural Drawings – SHED Architects, Project No.: 2401, Drawing No.: DA 001, 003, 010 – 015, 050, 105 – 106, 111, 150, 200 – 202, 300 – 301, 310 – 311, 320 – 321, 330 – 331, 400, 500 – 506, 600 – 601, 605, 610, 620, 700, Dated: 07/2024
Survey Drawing – Waterview Surveying Services, Project No.: 1866, Dated: 25/03/2024

I am aware that the above Geotechnical Report, prepared for the abovementioned site is to be submitted in support of a Development Application for this site and will be relied on by Pittwater Council as the basis for ensuring that the Geotechnical Risk Management aspects of the proposed development have been adequately addressed to achieve an "Acceptable Risk Management" level for the life of the structure, taken as at least 100 years unless otherwise stated and justified in the Report and that reasonable and practical measures have been identified to remove foreseeable risk.

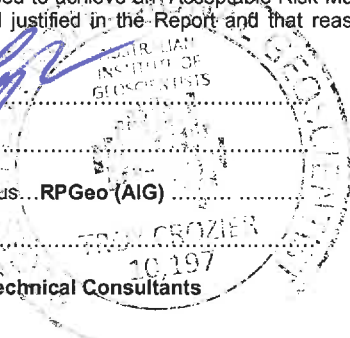
Signature ..... 

Name ...Troy Crozier.....

Chartered Professional Status...RPGeo (AIG).....

Membership No.: ...10197.....

Company... Crozier Geotechnical Consultants



**GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER**  
**FORM NO. 1(a) - Checklist of Requirements For Geotechnical Risk Management Report for Development Application**

Development Application for _____	James Wish
Name of Applicant	
Address of site ___11 Florence Terrace, Scotland Island, NSW, NSW	

The following checklist covers the minimum requirements to be addressed in a Geotechnical Risk Management Geotechnical Report. This checklist is to accompany the Geotechnical Report and its certification (Form No. 1).

**Geotechnical Report Details:**

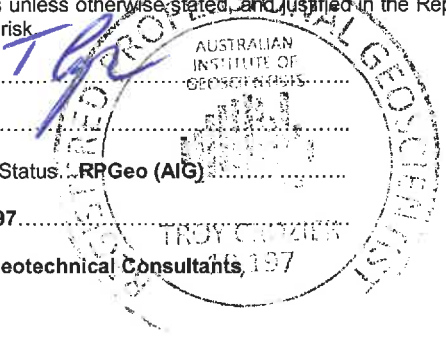
<b>Report Title:</b> Geotechnical Report for Proposed Alterations and Additions	<b>Project No.:</b> 2024-172
<b>Report Date:</b> 18 September 2024	
<b>Author:</b> Ben Taylor	
<b>Author's Company/Organisation:</b> Crozier Geotechnical Consultants	

**Please mark appropriate box**

- Comprehensive site mapping conducted \_\_\_13 September 2024\_\_\_\_\_
- Mapping details presented on contoured site plan with geomorphic mapping to a minimum scale of 1:200 (as appropriate)
- Subsurface investigation required
  - No Justification ..... no significant excavation.
  - Yes Date conducted .....
- Geotechnical model developed and reported as an inferred subsurface type-section
- Geotechnical hazards identified
  - Above the site
  - On the site
  - Below the site
  - Beside the site
- Geotechnical hazards described and reported
- Risk assessment conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
  - Consequence analysis
  - Frequency analysis
- Risk calculation
- Risk assessment for property conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
- Risk assessment for loss of life conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
- Assessed risks have been compared to "Acceptable Risk Management" criteria as defined in the Geotechnical Risk Management Policy for Pittwater - 2009
- Opinion has been provided that the design can achieve the "Acceptable Risk Management" criteria provided that the specified conditions are achieved.
- Design Life Adopted:
  - 100 years
  - Other ...50 years for remnant structure post renovations.  
specify
- Geotechnical Conditions to be applied to all four phases as described in the Geotechnical Risk Management Policy for Pittwater - 2009 have been specified
- Additional action to remove risk where reasonable and practical have been identified and included in the report.
- Risk assessment within Bushfire Asset Protection Zone.

I am aware that Pittwater Council will rely on the Geotechnical Report, to which this checklist applies, as the basis for ensuring that the geotechnical risk management aspects of the proposal have been adequately addressed to achieve an "Acceptable Risk Management" level for the life of the structure, taken as at least 100 years unless otherwise stated, and justified in the Report and that reasonable and practical measures have been identified to remove foreseeable risk

Signature .....  
 Name ...**Troy Crozier**.....  
 Chartered Professional Status...**RPGeo (AIG)**.....  
 Membership No. ....**10197**.....  
 Company... **Crozier Geotechnical Consultants**.....



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**Date:** 18<sup>th</sup> September 2024

**Project No:** 2024-172

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**GEOTECHNICAL REPORT FOR PROPOSED NEW ALTERATIONS AND ADDITIONS AT  
11 FLORENCE TERRACE, SCOTLAND ISLAND, NSW**

**1. INTRODUCTION:**

This report details the results of a landslip assessment required by Northern Beaches Council for proposed alterations and additions at 11 Florence Terrace, Scotland Island, NSW. The assessment was undertaken by Crozier Geotechnical Consultants (CGC) at the written request of Shed Architects on behalf of the owner James Wish.

It is understood that the proposed works involve alterations and additions which include a northern and northwestern extension to the main floor level. The proposed extension will be above ground requiring isolated excavation for new footings only.

The site is located within the H1 (highest category) landslip hazard zone as identified within Northern Beaches Councils precinct (Geotechnical Risk Management Policy for Pittwater – 2009). This report has been prepared to meet the Council Policy Requirements of Paragraph 6.5. This geotechnical report is provided in support of the Development Application (DA) and assesses the landslip risk to ensure ‘Acceptable’ risk levels are achieved and can be maintained for the remnant design life of the existing structure.

The investigation comprised:

- a) A detailed geotechnical inspection and mapping of the site and adjacent properties by a Senior Geotechnical Engineer.
- b) Review of CGC database on local geotechnical conditions

The following plans were supplied and relied upon for the work:

- Architectural Drawings – SHED Architects, Project No.: 2401, Drawing No.: DA 001, 003, 010 – 015, 050, 105 – 106, 111, 150, 200 – 202, 300 – 301, 310 – 311, 320 – 321, 330 – 331, 400, 500 – 506, 600 – 601, 605, 610, 620, 700, Dated: 07/2024
- Survey Drawing – Waterview Surveying Services, Project No.: 1866, Dated: 25/03/2024

## **2. SITE FEATURES:**

### **2.1. Description:**

The site is an irregular shaped block of land located on the low, waterfront side of Florence Terrace on the north side of Scotland Island.

It is situated within gentle to very steeply north dipping topography with the site levels varying from approximately RL14.20 at the southwest corner to RL1.50 at the paved area on the northern site boundary on the Pittwater foreshore. It has an irregular northern boundary of approximately 18.20m, a southern boundary of 10.06m, and side eastern and western boundaries of 58.67m and 60.655m respectively, as referenced from the provided Survey Plan.

### **2.2. Geology:**

Reference to the Sydney 1:100,000 Geological Series sheet (9130) indicates that the site is underlain by Newport Formation (Upper Narrabeen Group) rock (Rnn) which is of middle Triassic Age. The Newport Formation typically comprises interbedded laminite, shale and quartz to lithic quartz sandstones and pink clay pellet sandstones. The rock unit was identified and mapped on the present and adjacent sites.

Narrabeen Group rocks are dominated by shales and thin siltstone beds and often form rounded convex ridge tops with moderate angle (<20°) side slopes. These side slopes can be either concave or convex depending on geology, internally they comprise shale beds with close spaced bedding partings that have either close spaced vertical joints or in extreme cases large space convex joints. The shale often forms deeply weathered silty clay soil profiles (medium to high plasticity) with thin silty colluvial cover.

## **3. FIELD WORK:**

### **3.1. Methods:**

The field investigation comprised a walk over inspection and mapping of the site and limited inspection of adjacent properties on the 13<sup>th</sup> September 2024 by a Senior Geotechnical Engineer which included a photographic record of site conditions as well as geological/geomorphological mapping of the site and adjacent land.

Explanatory notes are included in Appendix: 1.

### **3.2. Field Observations:**

The site is situated on the northern side of the island with access via Florence Terrace to the south and water access on the northern site boundary into Pittwater. Florence Terrace comprises an unsealed road which slopes gently down towards Catherine Park to the west in front of the site. The site is located at the base of a slope which is gently ( $<7^{\circ}$ ) north dipping across the majority of the site, with a narrow section of very steeply ( $\sim 32^{\circ}$ ) north dipping slope at the northern end of the site near the waterfront.

The southern portion of the site comprises gently ( $\sim 6^{\circ}$ ) north sloping lawn, with two water tanks near the southwest corner and a lightweight shed near the western site boundary. A low ( $<0.50\text{m}$ ) timber retaining wall supports the northern end of the lawn area, below which is a timber deck which extends from the front, southern end of the dwelling around the eastern side to the rear, northern end.

The site dwelling comprises one-storey timber log residence with the floor level at roughly the natural ground level on the southern end and is raised on timber posts at the northern end where the topography slopes down. The structure appears to be in good condition and the external features of the structure did not indicate any signs of geotechnical instability with no signs of excessive settlement or cracking.

Below the house to the north is a lawn area divided by a  $0.70\text{m}$  high timber retaining wall running east-west across the site, showing some signs of minor deflection. Another timber retaining wall of similar height supports the lower section of lawn, below which the site slopes very steeply ( $\sim 32^{\circ}$ ) down towards the waterfront to the north. This portion of the site is heavily vegetated with timber stairs and low ( $<0.40\text{m}$  high) timber retaining walls in some locations. No detached boulders were identified in this area although the dense vegetation restricted visibility.

A  $2.30\text{m}$  high concrete crib retaining wall which appeared to be in good condition supports the slope at the base, with a weatherboard boathouse structure built adjacent to the retaining wall. The structure appears to be in good condition, with no indications of excess deterioration, erosion or deformation to suggest underlying slope instability or geotechnical hazards. The northern end of the site comprises a level paved area supported by a sandstone block seawall.

General views of the site at the time of investigation are provided in Photograph-1 to Photograph-4.



Photograph: 1 – Southern side of site showing lawn area and existing dwelling structure, looking north



Photograph: 2 – Southern side of site looking towards Florence Terrace to south



Photograph: 3 – Northwest corner of existing dwelling structure and western side of site as viewed from northern end lawn area



Photograph: 4 – Pathway leading down from existing dwelling to boathouse, including low (<0.40m) timber retaining walls

The property to the west (No. 9 Florence Terrace) contains a one and two-storey rendered and weatherboard clad dwelling positioned centrally within the property and set back approximately 0.80m from the common boundary, with a timber deck extending along the eastern side of the structure. The house appeared in good structural condition and there were no signs of geotechnical instability. The levels within this property matched those within the site along the common boundary although levels were not visible beneath the timber deck.



The neighbouring property to the east (No. 13 Florence Terrace) contains a one and two-storey weatherboard dwelling positioned centrally within the property. The dwelling is set approximately 1.0m from the common boundary and appears in good condition. The levels within the property appeared generally similar to those within the site along the common boundary.

The neighbouring buildings and properties were only inspected from within the site, from the water or from the road reserve however the visible aspects did not show any signs of large-scale slope instability or other major geotechnical concerns which would impact the site.

#### **4. COMMENTS:**

##### **4.1. Geotechnical Model:**

Based on conditions exposed within the site and from previous investigations nearby, the sub-surface conditions within the site are expected to comprise minor fill soils over silty clay to sandy clayey soils containing ironstone bands with very low strength siltstone/sandstone bedrock (including iron rich bands) likely within 1.0 to 2.0m depth of surface. The bedrock is expected to grade slowly to low strength by approximately 3.0m depth, however variability can be high within the local geological sequence.

##### **4.2. Geotechnical Assessment:**

The geotechnical inspection did not identify any signs of previous or impending large scale or deep-seated landslip instability within the site or adjacent properties. The existing main residential structure appears to be >20 years of age and shows no signs of slope movement whilst there are no indications of excess surface stormwater flow, groundwater seepage or erosion.

The proposed works involve alterations and additions which include a northern and northwestern extension to the ground floor level of the dwelling. The proposed extension will be above ground requiring isolated excavation for new footings only.

It is recommended that the new footings be extended through any fill and colluvium and be founded onto/within residual clay/sandy clay material of similar bearing capacity (at least very stiff) or bedrock of at least very low strength. Very stiff residual soils are suitable for 200kPa allowable bearing pressures whilst the very low strength bedrock can be designed for 800kPa.

Geotechnical inspection is required following footing excavation and prior to the placement of steel reinforcement or concrete to assess the insitu nature of the founding material and its bearing capacity against the structural engineering design requirements.

The recommendations and conclusions in this report are based on an assessment utilizing only surface observations and a limited inspection of neighbouring properties. However, based on the site geological and topographical conditions, along with existing structures. It is considered that the updated/modernized residential structure will have a design life of 50 years from completion of the proposed works.

#### **4.3. Slope Stability & Risk Assessment:**

Based on our site mapping and understanding of the proposed works no credible geological/geotechnical landslip hazards were identified which need to be considered in relation to the existing site and proposed development works. As such a risk assessment is not required as the works are considered separate from, and not affected by a geotechnical landslip hazard.

#### **4.4. Conditions Relating to Design and Construction Monitoring:**

If requested by Council to complete Forms: 2b and 3 as part of construction, building and post-construction certificate requirements of the Councils Geotechnical Risk Management Policy 2009, it will be necessary for Crozier Geotechnical Consultants to:

1. Review and approve the structural drawings for compliance with the recommendations of this report,
2. Inspect new footings before concrete or steel are placed to verify their bearing capacity and the in-situ nature of the founding strata,

The client and builder should make themselves familiar with the Councils Geotechnical Policy. Crozier Geotechnical Consultants can not sign Form: 3 of the policy for an occupation certificate if it has not reviewed structural designs and been called to site to undertake any required inspections.

#### **4.5. Design Life of Structure:**

We have interpreted the design life requirements specified within Council's Risk Management Policy to refer to structural elements designed to support the existing structures, control stormwater and maintain the risk of instability within acceptable limits. Specific structures and features that may affect the maintenance and stability of the site in relation to the proposed and existing development are considered to comprise:

- stormwater and subsoil drainage systems,
- retaining walls and instability,
- maintenance of trees/vegetation on this and adjacent properties.

Man-made features should be designed and maintained for a design life consistent with surrounding structures (as per AS2870 – 2011 (100 years)). It will be necessary for the structural and geotechnical engineers to incorporate appropriate design and inspection procedures during the construction period. Additionally, the property owner should adopt and implement a maintenance and inspection program.

If this maintenance and inspection schedule are not maintained the design life of the property cannot be attained. A recommended program should include the following guidelines.

- The conditions on the block don't change from those present at the time this report was prepared, except for the changes due to this development.
- There is no change to the property due to an extraordinary event external to this site
- The property is maintained in good order and in accordance with the guidelines set out in;
  - a) CSIRO sheet BTF 18
  - b) Australian Geomechanics "Landslide Risk Management" Volume 42, March 2007.
  - c) AS 2870 – 2011, Australian Standard for Residential Slabs and Footings

Where changes to site conditions are identified during the maintenance and inspection program, reference should be made to relevant professionals (e.g. structural engineer, geotechnical engineer or Council). Where the property owner has any lack of understanding or concerns about the implementation of any component of the maintenance and inspection program the relevant engineer should be contacted for advice or to complete the component. It is assumed that Council will control development on neighbouring properties, carry out regular inspections and maintenance of the road verge, stormwater systems and large trees on public land adjacent to the site so as to ensure that stability conditions do not deteriorate with potential increase in risk level to the site.

Also, individual Government Departments will maintain public utilities in the form of power lines, water, and sewer mains to ensure they don't leak and increase either the local groundwater level or landslide potential.

## 5. CONCLUSION:

The inspection and assessment identified no obvious slope movement, excess surface stormwater flow or seepage, erosion or instability within the site or adjacent properties. The entire site and surrounding slopes have been assessed as per the Northern Beaches Council - Geotechnical Risk Management Policy for Pittwater and no credible landslip hazards were identified whilst the proposed works are relatively minor from a geotechnical perspective and should not create any landslip hazards. Therefore, the proposed works are separate from and not affected by a geotechnical hazard, and no further reporting is required as part of this DA.

It is considered that the site will meet the 'Acceptable' risk management criteria for the remnant 50 years design life of the development provided the property is maintained as per the recommendations of this report.

Prepared by:



Ben Taylor  
Senior Geotechnical Engineer

Reviewed by:



Troy Crozier  
Principal  
MAIG. RPGeo: Geotechnical and Engineering

## 6.0. REFERENCES:

1. Australian Geomechanics Society 2007, "Landslide Risk Assessment and Management", Australian Geomechanics Journal Vol 42, No 1, March 2007.
2. Geotechnical Risk Management Policy for Pittwater, 2009.

# Appendix 1

## NOTES RELATING TO THIS REPORT

### Introduction

These notes have been provided to amplify the geotechnical report in regard to classification methods, specialist field procedures and certain matters relating to the Discussion and Comments section. Not all, of course, are necessarily relevant to all reports.

Geotechnical reports are based on information gained from limited subsurface test boring and sampling, supplemented by knowledge of local geology and experience. For this reason, they must be regarded as interpretive rather than factual documents, limited to some extent by the scope of information on which they rely.

### Description and classification Methods

The methods of description and classification of soils and rocks used in this report are based on Australian Standard 1726, Geotechnical Site Investigation Code. In general, descriptions cover the following properties - strength or density, colour, structure, soil or rock type and inclusions.

Soil types are described according to the predominating particle size, qualified by the grading of other particles present (eg. Sandy clay) on the following bases:

<u>Soil Classification</u>	<u>Particle Size</u>
Clay	less than 0.002 mm
Silt	0.002 to 0.06 mm
Sand	0.06 to 2.00 mm
Gravel	2.00 to 60.00mm

Cohesive soils are classified on the basis of strength either by laboratory testing or engineering examination. The strength terms are defined as follows:

<u>Classification</u>	<u>Undrained Shear Strength kPa</u>
Very soft	Less than 12
Soft	12 - 25
Firm	25 - 50
Stiff	50 - 100
Very stiff	100 - 200
Hard	Greater than 200

Non-cohesive soils are classified on the basis of relative density, generally from the results of standard penetration tests (SPT) or Dutch cone penetrometer tests (CPT) as below:

<u>Relative Density</u>	<u>SPT "N" Value (blows/300mm)</u>	<u>CPT Cone Value (Qc - MPa)</u>
Very loose	less than 5	less than 2
Loose	5 - 10	2 - 5
Medium dense	10 - 30	5 - 15
Dense	30 - 50	15 - 25
Very dense	greater than 50	greater than 25

Rock types are classified by their geological names. Where relevant, further information regarding rock classification is given on the following sheet.

## Sampling

Sampling is carried out during drilling to allow engineering examination (and laboratory testing where required) of the soil or rock.

Disturbed samples taken during drilling to allow information on colour, type, inclusions and, depending upon the degree of disturbance, some information on strength and structure.

Undisturbed samples are taken by pushing a thin-walled sample tube into the soil and withdrawing a sample of the soil in a relatively undisturbed state. Such samples yield information on structure and strength, and are necessary for laboratory determination of shear strength and compressibility. Undisturbed sampling is generally effective only in cohesive soils.

## Drilling Methods

The following is a brief summary of drilling methods currently adopted by the company and some comments on their use and application.

**Test Pits** – these are excavated with a backhoe or a tracked excavator, allowing close examination of the insitu soils if it is safe to descent into the pit. The depth of penetration is limited to about 3m for a backhoe and up to 6m for an excavator. A potential disadvantage is the disturbance caused by the excavation.

**Large Diameter Auger (eg. Pengo)** – the hole is advanced by a rotating plate or short spiral auger, generally 300mm or larger in diameter. The cuttings are returned to the surface at intervals (generally of not more than 0.5m) and are disturbed but usually unchanged in moisture content. Identification of soil strata is generally much more reliable than with continuous spiral flight augers, and is usually supplemented by occasional undisturbed tube sampling.

**Continuous Sample Drilling** – the hole is advanced by pushing a 100mm diameter socket into the ground and withdrawing it at intervals to extrude the sample. This is the most reliable method of drilling soils, since moisture content is unchanged and soil structure, strength, etc. is only marginally affected.

**Continuous Spiral Flight Augers** – the hole is advanced using 90 – 115mm diameter continuous spiral flight augers which are withdrawn at intervals to allow sampling or insitu testing. This is a relatively economical means of drilling in clays and in sands above the water table. Samples are returned to the surface, or may be collected after withdrawal of the auger flights, but they are very disturbed and may be contaminated. Information from the drilling (as distinct from specific sampling by SPT's or undisturbed samples) is of relatively lower reliability, due to remoulding, contamination or softening of samples by ground water.

**Non-core Rotary Drilling** - the hole is advanced by a rotary bit, with water being pumped down the drill rods and returned up the annulus, carrying the drill cuttings. Only major changes in stratification can be determined from the cuttings, together with some information from 'feel' and rate of penetration.

**Rotary Mud Drilling** – similar to rotary drilling, but using drilling mud as a circulating fluid. The mud tends to mask the cuttings and reliable identification is again only possible from separate intact sampling (eg. From SPT).

**Continuous Core Drilling** – a continuous core sample is obtained using a diamond-tipped core barrel, usually 50mm internal diameter. Provided full core recovery is achieved (which is not always possible in very weak rocks and granular soils), this technique provides a very reliable (but relatively expensive) method of investigation.

## Standard Penetration Tests

Standard penetration tests (abbreviated as SPT) are used mainly in non-cohesive soils, but occasionally also in cohesive soils as a means of determining density or strength and also of obtaining a relatively undisturbed sample. The test procedures is described in Australian Standard 1289, "Methods of Testing Soils for Engineering Purposes" – Test 6.3.1.

The test is carried out in a borehole by driving a 50mm diameter split sample tube under the impact of a 63kg hammer with a free fall of 760mm. It is normal for the tube to be driven in three successive 150mm increments and the 'N' value is taken

as the number of blows for the last 300mm. In dense sands, very hard clays or weak rock, the full 450mm penetration may not be practicable and the test is discontinued.

The test results are reported in the following form.

- In the case where full penetration is obtained with successive blow counts for each 150mm of say 4, 6 and 7 as 4, 6, 7 then  $N = 13$
- In the case where the test is discontinued short of full penetration, say after 15 blows for the first 150mm and 30 blows for the next 40mm then as 15, 30/40mm.

The results of the test can be related empirically to the engineering properties of the soil. Occasionally, the test method is used to obtain samples in 50mm diameter thin wall sample tubes in clay. In such circumstances, the test results are shown on the borelogs in brackets.

## Cone Penetrometer Testing and Interpretation

Cone penetrometer testing (sometimes referred to as Dutch Cone – abbreviated as CPT) described in this report has been carried out using an electrical friction cone penetrometer. The test is described in Australia Standard 1289, Test 6.4.1.

In tests, a 35mm diameter rod with a cone-tipped end is pushed continually into the soil, the reaction being provided by a specially designed truck or rig which is fitted with an hydraulic ram system. Measurements are made of the end bearing resistance on the cone and the friction resistance on a separate 130mm long sleeve, immediately behind the cone. Transducers in the tip of the assembly are connected by electrical wires passing through the centre of the push rods to an amplifier and recorder unit mounted on the control truck.

As penetration occurs (at a rate of approximately 20mm per second) their information is plotted on a computer screen and at the end of the test is stored on the computer for later plotting of the results.

The information provided on the plotted results comprises: -

- Cone resistance – the actual end bearing force divided by the cross-sectional area of the cone – expressed in MPa.
- Sleeve friction – the frictional force on the sleeve divided by the surface area – expressed in kPa.
- Friction ratio - the ratio of sleeve friction to cone resistance, expressed in percent.

There are two scales available for measurement of cone resistance. The lower scale (0 – 5 MPa) is used in very soft soils where increased sensitivity is required and is shown in the graphs as a dotted line. The main scale (0 – 50 MPa) is less sensitive and is shown as a full line. The ratios of the sleeve friction to cone resistance will vary with the type of soil encountered, with higher relative friction in clays than in sands. Friction ratios 1% - 2% are commonly encountered in sands and very soft clays rising to 4% - 10% in stiff clays.

In sands, the relationship between cone resistance and SPT value is commonly in the range: -

$$Q_c \text{ (MPa)} = (0.4 \text{ to } 0.6) N \text{ blows (blows per 300mm)}$$

In clays, the relationship between undrained shear strength and cone resistance is commonly in the range: -

$$Q_c = (12 \text{ to } 18) C_u$$

Interpretation of CPT values can also be made to allow estimation of modulus or compressibility values to allow calculations of foundation settlements.

Inferred stratification as shown on the attached reports is assessed from the cone and friction traces and from experience and information from nearby boreholes, etc. This information is presented for general guidance, but must be regarded as being to some extent interpretive. The test method provides a continuous profile of engineering properties, and where precise information on soil classification is required, direct drilling and sampling may be preferable.

## Dynamic Penetrometers

Dynamic penetrometer tests are carried out by driving a rod into the ground with a falling weight hammer and measuring the blows for successive 150mm increments of penetration. Normally, there is a depth limitation of 1.2m but this may be extended in certain conditions by the use of extension rods.



Two relatively similar tests are used.

- Perth sand penetrometer – a 16mm diameter flattened rod is driven with a 9kg hammer, dropping 600mm (AS1289, Test 6.3.3). The test was developed for testing the density of sands (originating in Perth) and is mainly used in granular soils and filling.
- Cone penetrometer (sometimes known as Scala Penetrometer) – a 16mm rod with a 20mm diameter cone end is driven with a 9kg hammer dropping 510mm (AS 1289, Test 6.3.2). The test was developed initially for pavement sub-grade investigations, and published correlations of the test results with California bearing ratio have been published by various Road Authorities.

## Laboratory Testing

Laboratory testing is generally carried out in accordance with Australian Standard 1289 “Methods of Testing Soil for Engineering Purposes”. Details of the test procedure used are given on the individual report forms.

## Borehole Logs

The bore logs presented herein are an engineering and/or geological interpretation of the subsurface conditions, and their reliability will depend to some extent on frequency of sampling and the method of drilling. Ideally, continuous undisturbed sampling or core drilling will provide the most reliable assessment, but this is not always practicable, or possible to justify on economic grounds. In any case, the boreholes represent only a very small sample of the total subsurface profile.

Interpretation of the information and its application to design and construction should therefore take into account the spacing of boreholes, the frequency of sampling and the possibility of other than ‘straight line’ variations between the boreholes.

Details of the type and method of sampling are given in the report and the following sample codes are on the borehole logs where applicable:

D	Disturbed Sample	E	Environmental sample	DT	Diatube
B	Bulk Sample	PP	Pocket Penetrometer Test		
U50	50mm Undisturbed Tube Sample	SPT	Standard Penetration Test		
U63	63mm “ “ “ “ “	C	Core		

## Ground Water

Where ground water levels are measured in boreholes there are several potential problems:

- In low permeability soils, ground water although present, may enter the hole slowly or perhaps not at all during the time it is left open.
- A localised perched water table may lead to an erroneous indication of the true water table.
- Water table levels will vary from time to time with seasons or recent weather changes. They may not be the same at the time of construction as are indicated in the report.
- The use of water or mud as a drilling fluid will mask any ground water inflow. Water has to be blown out of the hole and drilling mud must first be washed out of the hole if water observations are to be made. More reliable measurements can be made by installing standpipes which are read at intervals over several days, or perhaps weeks for low permeability soils. Piezometers, sealed in a particular stratum, may be interference from a perched water table.

## Engineering Reports

Engineering reports are prepared by qualified personnel and are based on the information obtained and on current engineering standards of interpretation and analysis. Where the report has been prepared for a specific design proposal (eg. A three-storey building), the information and interpretation may not be relevant if the design proposal is changed (eg. to a twenty-storey building). If this happens, the Company will be pleased to review the report and the sufficiency of the investigation work.

Every care is taken with the report as it relates to interpretation of subsurface condition, discussion of geotechnical aspects and recommendations or suggestions for design and construction. However, the Company cannot always anticipate or assume responsibility for:

- unexpected variations in ground conditions – the potential for this will depend partly on bore spacing and sampling frequency,
- changes in policy or interpretation of policy by statutory authorities,
- the actions of contractors responding to commercial pressures,

If these occur, the Company will be pleased to assist with investigation or advice to resolve the matter.

### **Site Anomalies**

In the event that conditions encountered on site during construction appear to vary from those which were expected from the information contained in the report, the Company requests that it immediately be notified. Most problems are much more readily resolved when conditions are exposed than at some later stage, well after the event.

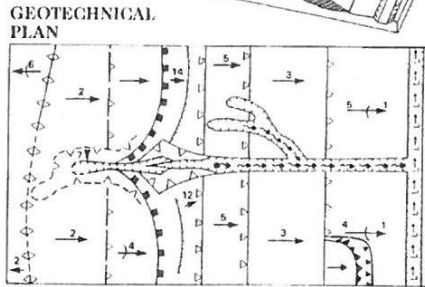
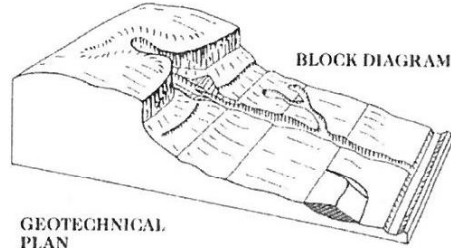
### **Reproduction of Information for Contractual Purposes**

Attention is drawn to the document “Guidelines for the Provision of Geotechnical Information in Tender Documents”, published by the Institution of Engineers Australia. Where information obtained from this investigation is provided for tendering purposes, it is recommended that all information, including the written report and discussion, be made available. In circumstances where the discussion or comments section is not relevant to the contractual situation, it may be appropriate to prepare a special ally edited document. The Company would be pleased to assist in this regard and/or to make additional report copies available for contract purposes at a nominal charge.

### **Site Inspection**

The Company will always be pleased to provide engineering inspection services for geotechnical aspects of work to which this report is related. This could range from a site visit to confirm that conditions exposed are as expected, to full time engineering presence on site.

PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007



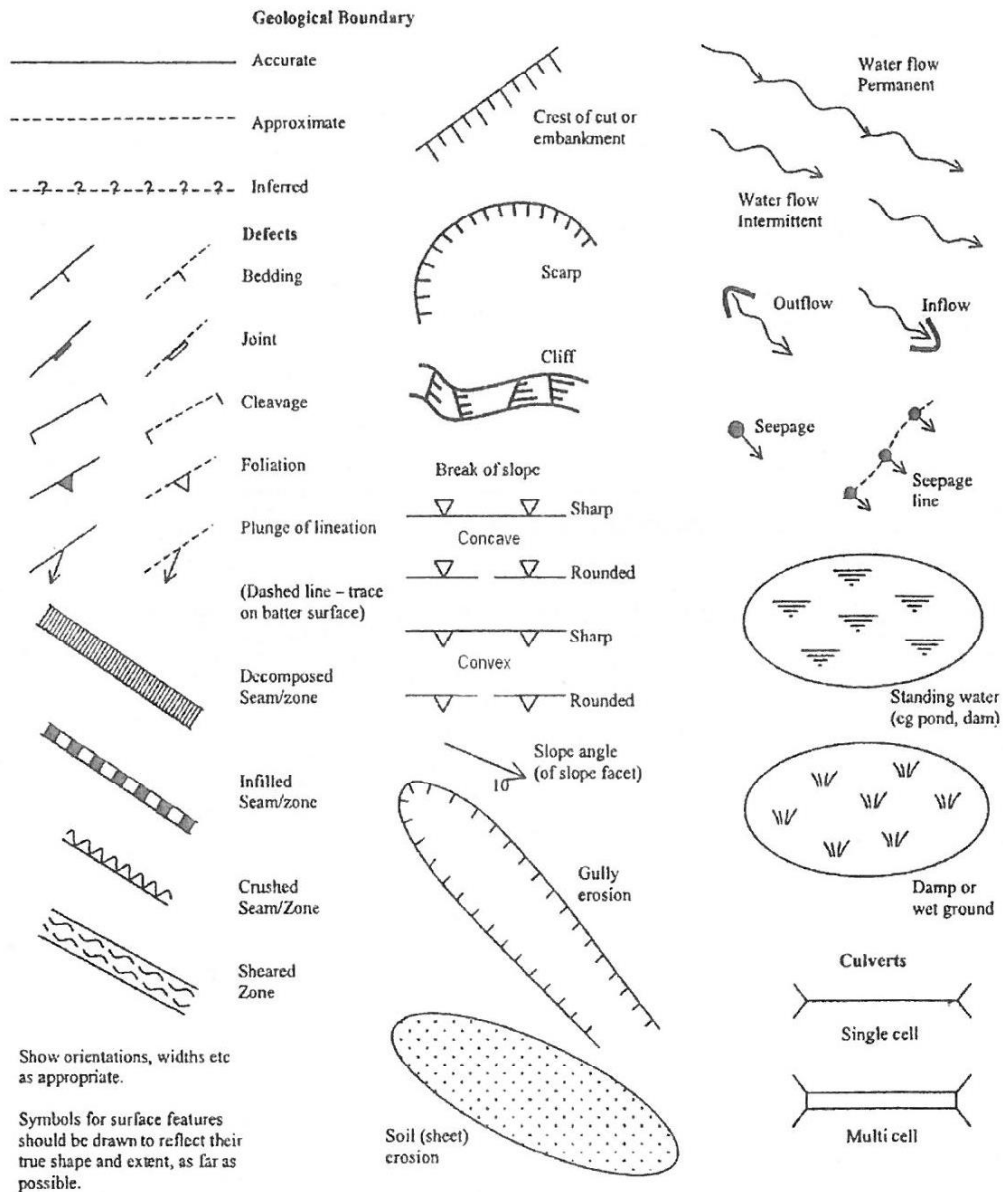
SYMBOL	GROUND PROFILE		
		Convex	Well defined or angular break of slope
		Concave	
		Convex	Poorly defined or smooth change of slope
		Concave	
		Breaks of slope	Convex and concave too close together to allow the use of separate symbols
		Changes of slope	
		Sharp	Ridge crest
		Rounded	
		Cliff or escarpment or sharp break 40° or more (estimated height in metres)	
		Uniform slope	Slope direction and angle (Degrees)
		Concave slope	
		Convex slope	
		Top	Cut or fill slope, arrows pointing down slope
		Bottom	
		Hummocky or irregular ground	
		Open drain, unlined	
		Open drain, lined	
		Fence line	
		Property boundary	
		Dry stone wall	
		Major joint in rock face (opening in millimetres)	
		Tension crack (opening in millimetres)	

Example of Mapping Symbols

(after V Gardiner & R V Dackombe (1983). Geomorphological Field Manual. George Allen & Unwin).

PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007

APPENDIX E - GEOLOGICAL AND GEOMORPHOLOGICAL MAPPING SYMBOLS AND TERMINOLOGY



Examples of Mapping Symbols (after Guide to Slope Risk Analysis Version 3.1 November 2001, Roads and Traffic Authority of New South Wales).

# Appendix 2

## APPENDIX A

## DEFINITION OF TERMS

INTERNATIONAL UNION OF GEOLOGICAL SCIENCES WORKING GROUP  
ON LANDSLIDES, COMMITTEE ON RISK ASSESSMENT

- Risk** – A measure of the probability and severity of an adverse effect to health, property or the environment. Risk is often estimated by the product of probability x consequences. However, a more general interpretation of risk involves a comparison of the probability and consequences in a non-product form.
- Hazard** – A condition with the potential for causing an undesirable consequence (*the landslide*). The description of landslide hazard should include the location, volume (or area), classification and velocity of the potential landslides and any resultant detached material, and the likelihood of their occurrence within a given period of time.
- Elements at Risk** – Meaning the population, buildings and engineering works, economic activities, public services utilities, infrastructure and environmental features in the area potentially affected by landslides.
- Probability** – The likelihood of a specific outcome, measured by the ratio of specific outcomes to the total number of possible outcomes. Probability is expressed as a number between 0 and 1, with 0 indicating an impossible outcome, and 1 indicating that an outcome is certain.
- Frequency** – A measure of likelihood expressed as the number of occurrences of an event in a given time. See also Likelihood and Probability.
- Likelihood** – used as a qualitative description of probability or frequency.
- Temporal Probability** – The probability that the element at risk is in the area affected by the landsliding, at the time of the landslide.
- Vulnerability** – The degree of loss to a given element or set of elements within the area affected by the landslide hazard. It is expressed on a scale of 0 (no loss) to 1 (total loss). For property, the loss will be the value of the damage relative to the value of the property; for persons, it will be the probability that a particular life (the element at risk) will be lost, given the person(s) is affected by the landslide.
- Consequence** – The outcomes or potential outcomes arising from the occurrence of a landslide expressed qualitatively or quantitatively, in terms of loss, disadvantage or gain, damage, injury or loss of life.
- Risk Analysis** – The use of available information to estimate the risk to individuals or populations, property, or the environment, from hazards. Risk analyses generally contain the following steps: scope definition, hazard identification, and risk estimation.
- Risk Estimation** – The process used to produce a measure of the level of health, property, or environmental risks being analysed. Risk estimation contains the following steps: frequency analysis, consequence analysis, and their integration.
- Risk Evaluation** – The stage at which values and judgements enter the decision process, explicitly or implicitly, by including consideration of the importance of the estimated risks and the associated social, environmental, and economic consequences, in order to identify a range of alternatives for managing the risks.
- Risk Assessment** – The process of risk analysis and risk evaluation.
- Risk Control or Risk Treatment** – The process of decision making for managing risk, and the implementation, or enforcement of risk mitigation measures and the re-evaluation of its effectiveness from time to time, using the results of risk assessment as one input.
- Risk Management** – The complete process of risk assessment and risk control (*or risk treatment*).

**Individual Risk** – The risk of fatality or injury to any identifiable (named) individual who lives within the zone impacted by the landslide; or who follows a particular pattern of life that might subject him or her to the consequences of the landslide.

**Societal Risk** – The risk of multiple fatalities or injuries in society as a whole: one where society would have to carry the burden of a landslide causing a number of deaths, injuries, financial, environmental, and other losses.

**Acceptable Risk** – A risk for which, for the purposes of life or work, we are prepared to accept as it is with no regard to its management. Society does not generally consider expenditure in further reducing such risks justifiable.

**Tolerable Risk** – A risk that society is willing to live with so as to secure certain net benefits in the confidence that it is being properly controlled, kept under review and further reduced as and when possible.

In some situations risk may be tolerated because the individuals at risk cannot afford to reduce risk even though they recognise it is not properly controlled.

**Landslide Intensity** – A set of spatially distributed parameters related to the destructive power of a landslide. The parameters may be described quantitatively or qualitatively and may include maximum movement velocity, total displacement, differential displacement, depth of the moving mass, peak discharge per unit width, kinetic energy per unit area.

**Note:** Reference should also be made to Figure 1 which shows the inter-relationship of many of these terms and the relevant portion of Landslide Risk Management.

# PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007

## APPENDIX C: LANDSLIDE RISK ASSESSMENT

### QUALITATIVE TERMINOLOGY FOR USE IN ASSESSING RISK TO PROPERTY

#### QUALITATIVE MEASURES OF LIKELIHOOD

Approximate Annual Probability		Implied Indicative Landslide Recurrence Interval		Description	Descriptor	Level
Indicative Value	Notional Boundary					
10 <sup>-1</sup>	5x10 <sup>-2</sup>	10 years	20 years	The event is expected to occur over the design life.	ALMOST CERTAIN	A
10 <sup>-2</sup>		100 years		The event will probably occur under adverse conditions over the design life.	LIKELY	B
10 <sup>-3</sup>	5x10 <sup>-3</sup>	1000 years	200 years	The event could occur under adverse conditions over the design life.	POSSIBLE	C
10 <sup>-4</sup>	5x10 <sup>-4</sup>	10,000 years	2000 years	The event might occur under very adverse circumstances over the design life.	UNLIKELY	D
10 <sup>-5</sup>	5x10 <sup>-5</sup>	100,000 years	20,000 years	The event is conceivable but only under exceptional circumstances over the design life.	RARE	E
10 <sup>-6</sup>	5x10 <sup>-6</sup>	1,000,000 years	200,000 years	The event is inconceivable or fanciful over the design life.	BARELY CREDIBLE	F

**Note:** (1) The table should be used from left to right; use Approximate Annual Probability or Description to assign Descriptor, not *vice versa*.

#### QUALITATIVE MEASURES OF CONSEQUENCES TO PROPERTY

Approximate Cost of Damage		Description	Descriptor	Level
Indicative Value	Notional Boundary			
200%	100%	Structure(s) completely destroyed and/or large scale damage requiring major engineering works for stabilisation. Could cause at least one adjacent property major consequence damage.	CATASTROPHIC	1
60%		Extensive damage to most of structure, and/or extending beyond site boundaries requiring significant stabilisation works. Could cause at least one adjacent property medium consequence damage.	MAJOR	2
20%	40%	Moderate damage to some of structure, and/or significant part of site requiring large stabilisation works. Could cause at least one adjacent property minor consequence damage.	MEDIUM	3
5%	10%	Limited damage to part of structure, and/or part of site requiring some reinstatement stabilisation works.	MINOR	4
0.5%	1%	Little damage. (Note for high probability event (Almost Certain), this category may be subdivided at a notional boundary of 0.1%. See Risk Matrix.)	INSIGNIFICANT	5

**Notes:** (2) The Approximate Cost of Damage is expressed as a percentage of market value, being the cost of the improved value of the unaffected property which includes the land plus the unaffected structures.

(3) The Approximate Cost is to be an estimate of the direct cost of the damage, such as the cost of reinstatement of the damaged portion of the property (land plus structures), stabilisation works required to render the site to tolerable risk level for the landslide which has occurred and professional design fees, and consequential costs such as legal fees, temporary accommodation. It does not include additional stabilisation works to address other landslides which may affect the property.

(4) The table should be used from left to right; use Approximate Cost of Damage or Description to assign Descriptor, not vice versa



## PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007

### APPENDIX C: – QUALITATIVE TERMINOLOGY FOR USE IN ASSESSING RISK TO PROPERTY (CONTINUED)

#### *QUALITATIVE RISK ANALYSIS MATRIX – LEVEL OF RISK TO PROPERTY*

LIKELIHOOD		CONSEQUENCES TO PROPERTY (With Indicative Approximate Cost of Damage)				
	Indicative Value of Approximate Annual Probability	1: CATASTROPHIC 200%	2: MAJOR 60%	3: MEDIUM 20%	4: MINOR 5%	5: INSIGNIFICANT 0.5%
<b>A – ALMOST CERTAIN</b>	10 <sup>-1</sup>	VH	VH	VH	H	M or L (5)
<b>B - LIKELY</b>	10 <sup>-2</sup>	VH	VH	H	M	L
<b>C - POSSIBLE</b>	10 <sup>-3</sup>	VH	H	M	M	VL
<b>D - UNLIKELY</b>	10 <sup>-4</sup>	H	M	L	L	VL
<b>E - RARE</b>	10 <sup>-5</sup>	M	L	L	VL	VL
<b>F - BARELY CREDIBLE</b>	10 <sup>-6</sup>	L	VL	VL	VL	VL

**Notes:** (5) For Cell A5, may be subdivided such that a consequence of less than 0.1% is Low Risk.

(6) When considering a risk assessment it must be clearly stated whether it is for existing conditions or with risk control measures which may not be implemented at the current time.

#### *RISK LEVEL IMPLICATIONS*

Risk Level		Example Implications (7)
VH	VERY HIGH RISK	Unacceptable without treatment. Extensive detailed investigation and research, planning and implementation of treatment options essential to reduce risk to Low; may be too expensive and not practical. Work likely to cost more than value of the property.
H	HIGH RISK	Unacceptable without treatment. Detailed investigation, planning and implementation of treatment options required to reduce risk to Low. Work would cost a substantial sum in relation to the value of the property.
M	MODERATE RISK	May be tolerated in certain circumstances (subject to regulator's approval) but requires investigation, planning and implementation of treatment options to reduce the risk to Low. Treatment options to reduce to Low risk should be implemented as soon as practicable.
L	LOW RISK	Usually acceptable to regulators. Where treatment has been required to reduce the risk to this level, ongoing maintenance is required.
VL	VERY LOW RISK	Acceptable. Manage by normal slope maintenance procedures.

**Note:** (7) The implications for a particular situation are to be determined by all parties to the risk assessment and may depend on the nature of the property at risk; these are only given as a general guide.

# Appendix 3

# PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007

## APPENDIX G - SOME GUIDELINES FOR HILLSIDE CONSTRUCTION

### GOOD ENGINEERING PRACTICE

### POOR ENGINEERING PRACTICE

#### ADVICE

GEOTECHNICAL ASSESSMENT	Obtain advice from a qualified, experienced geotechnical practitioner at early stage of planning and before site works.	Prepare detailed plan and start site works before geotechnical advice.
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#### PLANNING

SITE PLANNING	Having obtained geotechnical advice, plan the development with the risk arising from the identified hazards and consequences in mind.	Plan development without regard for the Risk.
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#### DESIGN AND CONSTRUCTION

HOUSE DESIGN	Use flexible structures which incorporate properly designed brickwork, timber or steel frames, timber or panel cladding. Consider use of split levels. Use decks for recreational areas where appropriate.	Floor plans which require extensive cutting and filling. Movement intolerant structures.
SITE CLEARING	Retain natural vegetation wherever practicable.	Indiscriminately clear the site.
ACCESS & DRIVEWAYS	Satisfy requirements below for cuts, fills, retaining walls and drainage. Council specifications for grades may need to be modified. Driveways and parking areas may need to be fully supported on piers.	Excavate and fill for site access before geotechnical advice.
EARTHWORKS	Retain natural contours wherever possible.	Indiscriminatory bulk earthworks.
CUTS	Minimise depth. Support with engineered retaining walls or batter to appropriate slope. Provide drainage measures and erosion control.	Large scale cuts and benching. Unsupported cuts. Ignore drainage requirements
FILLS	Minimise height. Strip vegetation and topsoil and key into natural slopes prior to filling. Use clean fill materials and compact to engineering standards. Batter to appropriate slope or support with engineered retaining wall. Provide surface drainage and appropriate subsurface drainage.	Loose or poorly compacted fill, which if it fails, may flow a considerable distance including onto property below. Block natural drainage lines. Fill over existing vegetation and topsoil. Include stumps, trees, vegetation, topsoil, boulders, building rubble etc in fill.
ROCK OUTCROPS & BOULDERS	Remove or stabilise boulders which may have unacceptable risk. Support rock faces where necessary.	Disturb or undercut detached blocks or boulders.
RETAINING WALLS	Engineer design to resist applied soil and water forces. Found on rock where practicable. Provide subsurface drainage within wall backfill and surface drainage on slope above. Construct wall as soon as possible after cut/fill operation.	Construct a structurally inadequate wall such as sandstone flagging, brick or unreinforced blockwork. Lack of subsurface drains and weepholes.
FOOTINGS	Found within rock where practicable. Use rows of piers or strip footings oriented up and down slope. Design for lateral creep pressures if necessary. Backfill footing excavations to exclude ingress of surface water.	Found on topsoil, loose fill, detached boulders or undercut cliffs.
SWIMMING POOLS	Engineer designed. Support on piers to rock where practicable. Provide with under-drainage and gravity drain outlet where practicable. Design for high soil pressures which may develop on uphill side whilst there may be little or no lateral support on downhill side.	
DRAINAGE		
SURFACE	Provide at tops of cut and fill slopes. Discharge to street drainage or natural water courses. Provide general falls to prevent blockage by siltation and incorporate silt traps. Line to minimise infiltration and make flexible where possible. Special structures to dissipate energy at changes of slope and/or direction.	Discharge at top of fills and cuts. Allow water to pond on bench areas.
SUBSURFACE	Provide filter around subsurface drain. Provide drain behind retaining walls. Use flexible pipelines with access for maintenance. Prevent inflow of surface water.	Discharge roof runoff into absorption trenches.
SEPTIC & SULLAGE	Usually requires pump-out or mains sewer systems; absorption trenches may be possible in some areas if risk is acceptable. Storage tanks should be water-tight and adequately founded.	Discharge sullage directly onto and into slopes. Use absorption trenches without consideration of landslide risk.
EROSION CONTROL & LANDSCAPING	Control erosion as this may lead to instability. Revegetate cleared area.	Failure to observe earthworks and drainage recommendations when landscaping.

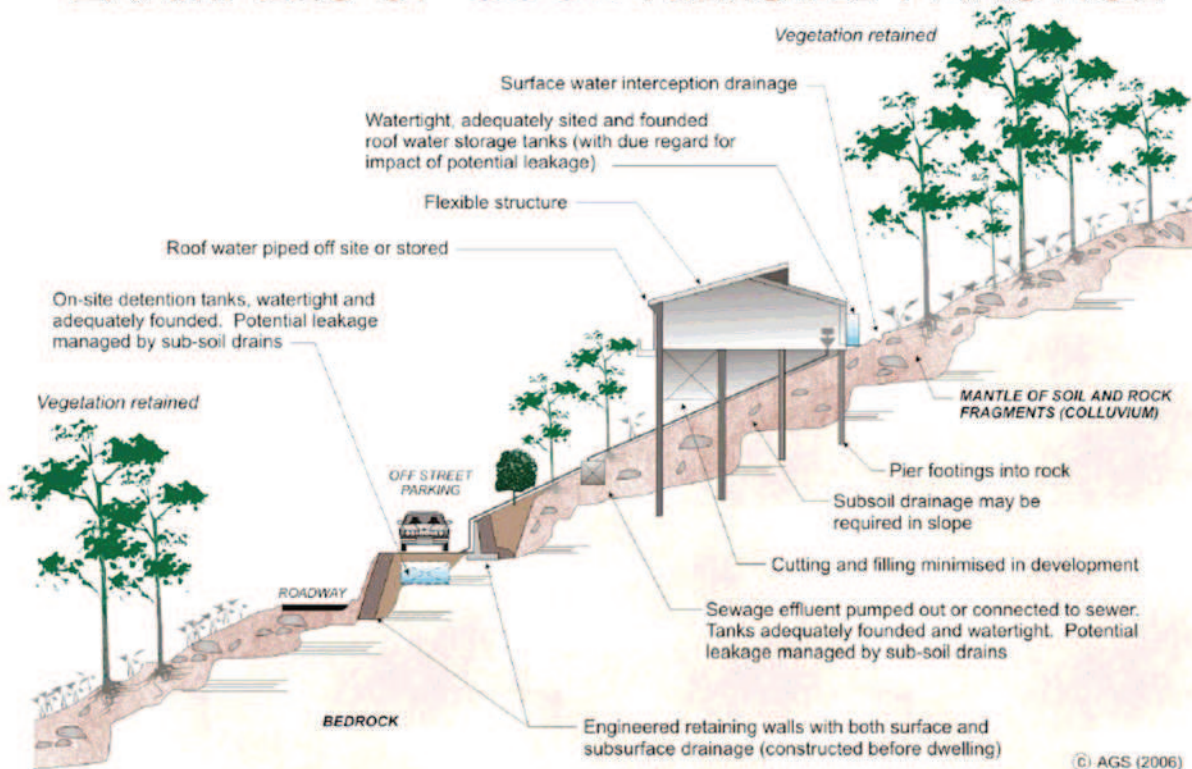
#### DRAWINGS AND SITE VISITS DURING CONSTRUCTION

DRAWINGS	Building Application drawings should be viewed by geotechnical consultant	
SITE VISITS	Site Visits by consultant may be appropriate during construction/	

#### INSPECTION AND MAINTENANCE BY OWNER

OWNER'S RESPONSIBILITY	Clean drainage systems; repair broken joints in drains and leaks in supply pipes. Where structural distress is evident see advice. If seepage observed, determine causes or seek advice on consequences.	
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## EXAMPLES OF **GOOD** HILLSIDE PRACTICE



## EXAMPLES OF **POOR** HILLSIDE PRACTICE

