

**GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER
FORM NO. 1 – To be submitted with Development Application**

Development Application for _____
Name of Applicant

Address of site 11 Gilwinga Drive, Bayview

The following checklist covers the minimum requirements to be addressed in a Geotechnical Risk Declaration made by geotechnical engineer or engineering geologist or coastal engineer (where applicable) as part of a geotechnical report

I, Ben White on behalf of White Geotechnical Group Pty Ltd
(Insert Name) (Trading or Company Name)

on this the 9/2/22 certify that I am a geotechnical engineer or engineering geologist or coastal engineer as defined by the Geotechnical Risk Management Policy for Pittwater - 2009 and I am authorised by the above organisation/company to issue this document and to certify that the organisation/company has a current professional indemnity policy of at least \$10million.

I:

Please mark appropriate box

- have prepared the detailed Geotechnical Report referenced below in accordance with the Australia Geomechanics Society's Landslide Risk Management Guidelines (AGS 2007) and the Geotechnical Risk Management Policy for Pittwater - 2009
- am willing to technically verify that the detailed Geotechnical Report referenced below has been prepared in accordance with the Australian Geomechanics Society's Landslide Risk Management Guidelines (AGS 2007) and the Geotechnical Risk Management Policy for Pittwater - 2009
- have examined the site and the proposed development in detail and have carried out a risk assessment in accordance with Section 6.0 of the Geotechnical Risk Management Policy for Pittwater - 2009. I confirm that the results of the risk assessment for the proposed development are in compliance with the Geotechnical Risk Management Policy for Pittwater - 2009 and further detailed geotechnical reporting is not required for the subject site.
- have examined the site and the proposed development/alteration in detail and I am of the opinion that the Development Application only involves Minor Development/Alteration that does not require a Geotechnical Report or Risk Assessment and hence my Report is in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009 requirements.
- have examined the site and the proposed development/alteration is separate from and is not affected by a Geotechnical Hazard and does not require a Geotechnical Report or Risk Assessment and hence my Report is in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009 requirements.
- have provided the coastal process and coastal forces analysis for inclusion in the Geotechnical Report

Geotechnical Report Details:

Report Title: Geotechnical Report 11 Gilwinga Drive, Bayview
Report Date: 9/2/22

Author: BEN WHITE

Author's Company/Organisation: WHITE GEOTECHNICAL GROUP PTY LTD

Documentation which relate to or are relied upon in report preparation:

Australian Geomechanics Society Landslide Risk Management March 2007.

White Geotechnical Group company archives.

I am aware that the above Geotechnical Report, prepared for the abovementioned site is to be submitted in support of a Development Application for this site and will be relied on by Pittwater Council as the basis for ensuring that the Geotechnical Risk Management aspects of the proposed development have been adequately addressed to achieve an "Acceptable Risk Management" level for the life of the structure, taken as at least 100 years unless otherwise stated and justified in the Report and that reasonable and practical measures have been identified to remove foreseeable risk.

Signature 
Name Ben White
Chartered Professional Status MScGEOLAusIMM CP GEOL
Membership No. 222757
Company White Geotechnical Group Pty Ltd

GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER
FORM NO. 1(a) - Checklist of Requirements for Geotechnical Risk Management Report for
Development Application

Development Application for _____	Name of Applicant
Address of site <u>11 Gilwinga Drive, Bayview</u>	

The following checklist covers the minimum requirements to be addressed in a Geotechnical Risk Management Geotechnical Report. This checklist is to accompany the Geotechnical Report and its certification (Form No. 1).

Geotechnical Report Details:

Report Title: Geotechnical Report <u>11 Gilwinga Drive, Bayview</u>
Report Date: <u>9/2/22</u>
Author: <u>BEN WHITE</u>
Author's Company/Organisation: <u>WHITE GEOTECHNICAL GROUP PTY LTD</u>

Please mark appropriate box

- Comprehensive site mapping conducted 2/2/22
(date)
- Mapping details presented on contoured site plan with geomorphic mapping to a minimum scale of 1:200 (as appropriate)
- Subsurface investigation required
 - No Justification _____
 - Yes Date conducted 2/2/22
- Geotechnical model developed and reported as an inferred subsurface type-section
- Geotechnical hazards identified
 - Above the site
 - On the site
 - Below the site
 - Beside the site
- Geotechnical hazards described and reported
- Risk assessment conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
 - Consequence analysis
 - Frequency analysis
- Risk calculation
- Risk assessment for property conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
- Risk assessment for loss of life conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
- Assessed risks have been compared to "Acceptable Risk Management" criteria as defined in the Geotechnical Risk Management Policy for Pittwater - 2009
- Opinion has been provided that the design can achieve the "Acceptable Risk Management" criteria provided that the specified conditions are achieved.
- Design Life Adopted:
 - 100 years
 - Other _____
specify
- Geotechnical Conditions to be applied to all four phases as described in the Geotechnical Risk Management Policy for Pittwater - 2009 have been specified
- Additional action to remove risk where reasonable and practical have been identified and included in the report.
- Risk assessment within Bushfire Asset Protection Zone.

I am aware that Pittwater Council will rely on the Geotechnical Report, to which this checklist applies, as the basis for ensuring that the geotechnical risk management aspects of the proposal have been adequately addressed to achieve an "Acceptable Risk Management" level for the life of the structure, taken as at least 100 years unless otherwise stated, and justified in the Report and that reasonable and practical measures have been identified to remove foreseeable risk.


Signature _____
Name Ben White
Chartered Professional Status MScGEOLAusIMM CP GEOL
Membership No. 222757
Company White Geotechnical Group Pty Ltd

GEOTECHNICAL INVESTIGATION:

Additions and Alterations at 11 Gilwinga Drive, Bayview

1. Proposed Development

- 1.1** Construct an upper floor addition on the uphill side of the house.
- 1.2** Construct a terrace off the downhill side of the house.
- 1.3** Various other minor internal and external additions and alterations.
- 1.4** Details of the proposed development are shown on 20 drawings prepared by Archolm Holdings, project number 3/21, drawings numbered 1a, 1 to 6, 6, 8, 16, 18, and 19, dated 29.11.21, and 9 to 16, dated 8.12.21.

2. Site Description

- 2.1** The site was inspected on the 2nd February, 2022.
- 2.2** This residential property is on the low side of the road and has a NW aspect. It is located on the moderate to steeply graded middle reaches of a hillslope. The slope falls across the property at angles averaging $\sim 17^\circ$. The slope above and below the property continues at similar angles.
- 2.3** At the road frontage, a concrete driveway runs down the slope to a garage on the uphill side of the house (Photo 1). The cut for the driveway has been partly taken through outcropping Medium Strength Sandstone (Photo 2). A series of stable low-lying sandstone rock walls terrace the slope between the road frontage and the house (Photo 3). Sandstone floaters outcrop through the slope in several places. Competent Medium Strength Sandstone bedrock can be seen stepping down the property in this location. The two-storey brick house is supported on brick walls. The external supporting brick walls of the house show no significant signs of movement. A large council stormwater pipe discharges water into a channel cut into the slope down the

W side of the property (Photo 4). A stable ~1.0m high stack rock retaining wall supports a fill for a level lawn area and two buried water tanks on the downhill side of the property (Photo 5). A suspended pool extends off the downhill side of the house and is supported by stable concrete columns (Photo 6). Immediately below the pool is a stable ~3.0m high sandstone block retaining wall that supports the fill for a level area underneath the house (Photo 7). A large Sandstone rock face steps down the property some ~3.0m (Photo 8). This rock face is slightly undercut at the E end but displays no other significant geological defects and is considered stable. A gentle to moderately sloping lawn area extends to a densely vegetated bushland area which continues some ~40m to the lower common boundary (Photo 9).

3. Geology

The Sydney 1:100 000 Geological sheet indicates the site is underlain by Hawkesbury Sandstone. It is described as a medium to coarse grained quartz sandstone with very minor shale and laminite lenses.

4. Subsurface Investigation

Four Dynamic Cone Penetrometer (DCP) tests were put down to determine the relative density of the overlying soil and the depth to bedrock. The locations of the tests are shown on the site plan attached. It should be noted that a level of caution should be applied when interpreting DCP test results. The test will not pass through hard buried objects so in some instances it can be difficult to determine whether refusal has occurred on an obstruction in the profile or on the natural rock surface. This is not expected to be an issue for the testing on this site. However, excavation and foundation budgets should always allow for the possibility that the interpreted ground conditions in this report vary from those encountered during excavations. See the appended "Important information about your report" for a more comprehensive explanation. The results are as follows:

DCP TEST RESULTS – Dynamic Cone Penetrometer				
Equipment: 9kg hammer, 510mm drop, conical tip.			Standard: AS1289.6.3.2 - 1997	
Depth(m) Blows/0.3m	DCP 1 (~RL91.3)	DCP 2 (~RL93.1)	DCP 3 (~RL95.0)	DCP 4 (~RL92.1)
0.0 to 0.3	8	14	4	9
0.3 to 0.6	17	#	9	12F
0.6 to 0.9	42		7	4
0.9 to 1.2	10		14	15
1.2 to 1.5	#		21	18
1.5 to 1.8			#	#
	Refusal on Rock @ 1.0m	Refusal on Rock @ 0.3m	Refusal on Rock @ 1.4m	Refusal on Rock @ 1.5m

#refusal/end of test. F=DCP fell after being struck showing little resistance through all or part of the interval.

DCP Notes:

DCP1 – Refusal on rock @ 1.0m, DCP bouncing off rock surface, white clay on dry tip.

DCP2 – Refusal on rock @ 0.3m, DCP bouncing off rock surface, white impact dust on dry tip.

DCP3 – Refusal on rock @ 1.4m, DCP thudding, orange and white sandy clay on damp tip.

DCP4 – Refusal on rock @ 1.5m, DCP bouncing off rock surface, orange and white sandy clay on damp tip.

5. Geological Observations/Interpretation

The surface features of the block are controlled by the outcropping and underlying sandstone bedrock that steps down the property forming sub-horizontal benches between the steps. Where the grade is steeper, the steps are larger and the benches narrower. Where the slope eases, the opposite is true. Where the rock is not exposed, it is overlain by soils and sandy clays that fill the bench step formation. Filling has been placed beside the house for landscaping. In the test locations, where the rock was not exposed, it was encountered at depths of between 0.3m and 1.5m below the current surface, being slightly deeper due to the presence of fill and the stepped nature of the underlying bedrock. The outcropping sandstone on the property is estimated to be medium strength or better and similar strength rock is

expected to underlie the entire site. See Type Section attached for a diagrammatical representation of the expected ground materials.

6. Groundwater

Normal ground water seepage is expected to move over the buried surface of the rock and through the cracks. Due to the slope and elevation of the block, the water table is expected to be many metres below the base of the proposed works.

7. Surface Water

Apart from the large council stormwater pipe that discharges onto the slope to the W of the house and natural watercourse that runs across the downhill side of the block, no evidence of significant surface flows were observed on the property during the inspection. Normal sheet wash from the slope above will be intercepted by the street drainage system for Gilwinga Drive above.

8. Geotechnical Hazards and Risk Analysis

No geotechnical hazards were observed beside the property. The moderate to steeply graded slope that falls across the property and continues above and below is a potential hazard (**Hazard One**). The large sandstone face that steps ~3.0m down the slope is a potential hazard (**Hazard Two**).

RISK ANALYSIS SUMMARY ON THE NEXT PAGE

Risk Analysis Summary

HAZARDS	Hazard One	Hazard Two
TYPE	The moderate to steep slope that falls across the property and continues above and below failing and impacting on the proposed works.	The sandstone face that steps ~3.0m down the slope failing and impacting on the proposed works (Photo 8).
LIKELIHOOD	'Unlikely' (10^{-4})	'Unlikely' (10^{-4})
CONSEQUENCES TO PROPERTY	'Medium' (15%)	'Medium' (25%)
RISK TO PROPERTY	'Low' (2×10^{-5})	'Low' (2×10^{-5})
RISK TO LIFE	9.1×10^{-7} /annum	2.9×10^{-7} /annum
COMMENTS	This level of risk is 'ACCEPTABLE'.	This level of risk is 'ACCEPTABLE'.

(See Aust. Geomech. Jnl. Mar 2007 Vol. 42 No 1, for full explanation of terms)

9. Suitability of the Proposed Development for the Site

The proposed development is suitable for the site. No geotechnical hazards will be created by the completion of the proposed development provided it is carried out in accordance with the requirements of this report and good engineering and building practice.

10. Stormwater

The fall is away from the street. Roof water from the development is to be piped to the existing stormwater channel on the W side of the property through any tanks that may be required by the regulating authorities.

11. Excavations

Apart from those for footings, no excavations are required.

12. Foundations

Due to the steep slope across the site, any additional footings for the proposed additions to the house and terrace area are to be supported on piers taken to and potted some ~0.3m into

the underlying Medium Strength Sandstone. Where this material is not exposed, it is expected at depths of between ~0.3m and ~1.5m below the current surface. Where footings are over an exposed sloping rock surface, they may be supported off level pads cut into the rock and fixed with suitable bar grouted / epoxied ~0.4m into the rock.

A maximum allowable bearing pressure of 1000kPa can be assumed for footings on Medium Strength Sandstone.

The depth and material of the existing house foundations are unknown. Where the foundation material changes across the structure it is recommended that construction joints or similar be installed to allow for differential movement, where the structure cannot tolerate such movement in line with a class S site.

Naturally occurring vertical cracks (known as joints) commonly occur in sandstone. These are generally filled with soil and are the natural seepage paths through the rock. They can extend to depths of several metres and are usually relatively narrow but can range between 0.1 to 0.8m wide. If a footing falls over a joint in the rock, the construction process is simplified if, with the approval of the structural engineer, the joint can be spanned or, alternatively, the footing can be repositioned so it does not fall over the joint.

NOTE: If the contractor is unsure of the footing material required, it is more cost effective to get the geotechnical consultant on site at the start of the footing excavation to advise on footing depth and material. This mostly prevents unnecessary over-excavation in clay like shaly rock but can be valuable in all types of geology.

13. Geotechnical Review

The structural plans are to be checked and certified by the geotechnical engineer as being in accordance with the geotechnical recommendations. On completion, a Form 2B will be issued. This form is required for the Construction Certificate to proceed.

14. Inspection

The client and builder are to familiarise themselves with the following required inspection as well as council geotechnical policy. We cannot provide geotechnical certification for the owners and Occupation Certificate if the following inspection has not been carried out during the construction process.

- All footings are to be inspected and approved by the geotechnical consultant while the excavation equipment and contractors are still onsite and before steel reinforcing is placed or concrete is poured.

White Geotechnical Group Pty Ltd.



Ben White M.Sc. Geol.,
AusIMM., CP GEOL.
No. 222757
Engineering Geologist.



Photo 1



Photo 2



Photo 3



Photo 4

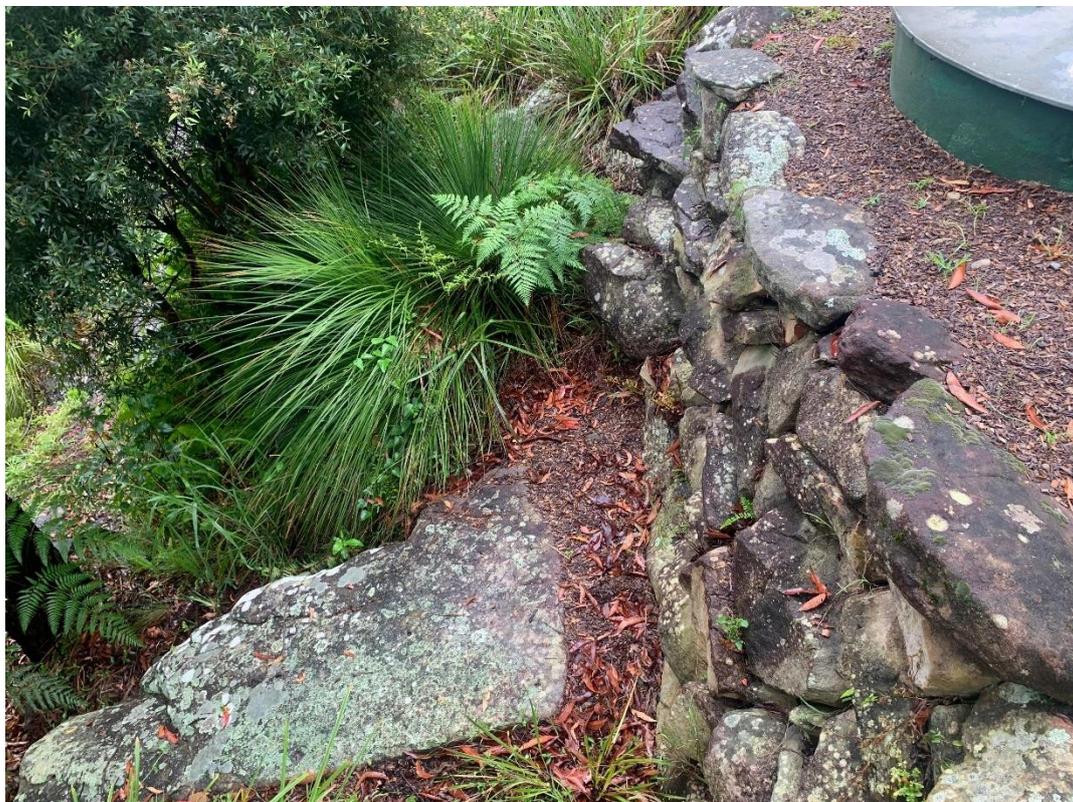


Photo 5



Photo 6



Photo 7



Photo 8



Photo 9

Important Information about Your Report

It should be noted that Geotechnical Reports are documents that build a picture of the subsurface conditions from the observation of surface features and testing carried out at specific points on the site. The spacing and location of the test points can be limited by the location of existing structures on the site or by budget and time constraints of the client. Additionally, the test themselves, although chosen for their suitability for the particular project, have their own limiting factors. The testing gives accurate information at the location of the test, within the confines of the test's capability. A geological interpretation or model is developed by joining these test points using all available data and drawing on previous experience of the geotechnical consultant. Even the most experienced practitioners cannot determine every possible feature or change that may lie below the earth. All of the subsurface features can only be known when they are revealed by excavation. As such, a Geotechnical report can be considered an interpretive document. It is based on factual data but also on opinion and judgement that comes with a level of uncertainty. This information is provided to help explain the nature and limitations of your report.

With this in mind, the following points are to be noted:

- If upon the commencement of the works the subsurface ground or ground water conditions prove different from those described in this report, it is advisable to contact White Geotechnical Group immediately, as problems relating to the ground works phase of construction are far easier and less costly to overcome if they are addressed early.
- If this report is used by other professionals during the design or construction process, any questions should be directed to White Geotechnical Group as only we understand the full methodology behind the report's conclusions.
- The report addresses issues relating to your specific design and site. If the proposed project design changes, aspects of the report may no longer apply. Contact White Geotechnical if this occurs.
- This report should not be applied to any other project other than that outlined in section 1.0.
- This report is to be read in full and should not have sections removed or included in other documents as this can result in misinterpretation of the data by others.
- It is common for the design and construction process to be adapted as it progresses (sometimes to suit the previous experience of the contractors involved). If alternative design and construction processes are required to those described in this report, contact White Geotechnical Group. We are familiar with a variety of techniques to reduce risk and can advise if your proposed methods are suitable for the site conditions.

PROPOSED DA -UPPER LEVEL EXTENSION- F1 SITE PLAN – showing test locations /E- BAYVIEW NSW 2104

LOT 39 SEC DP 241518

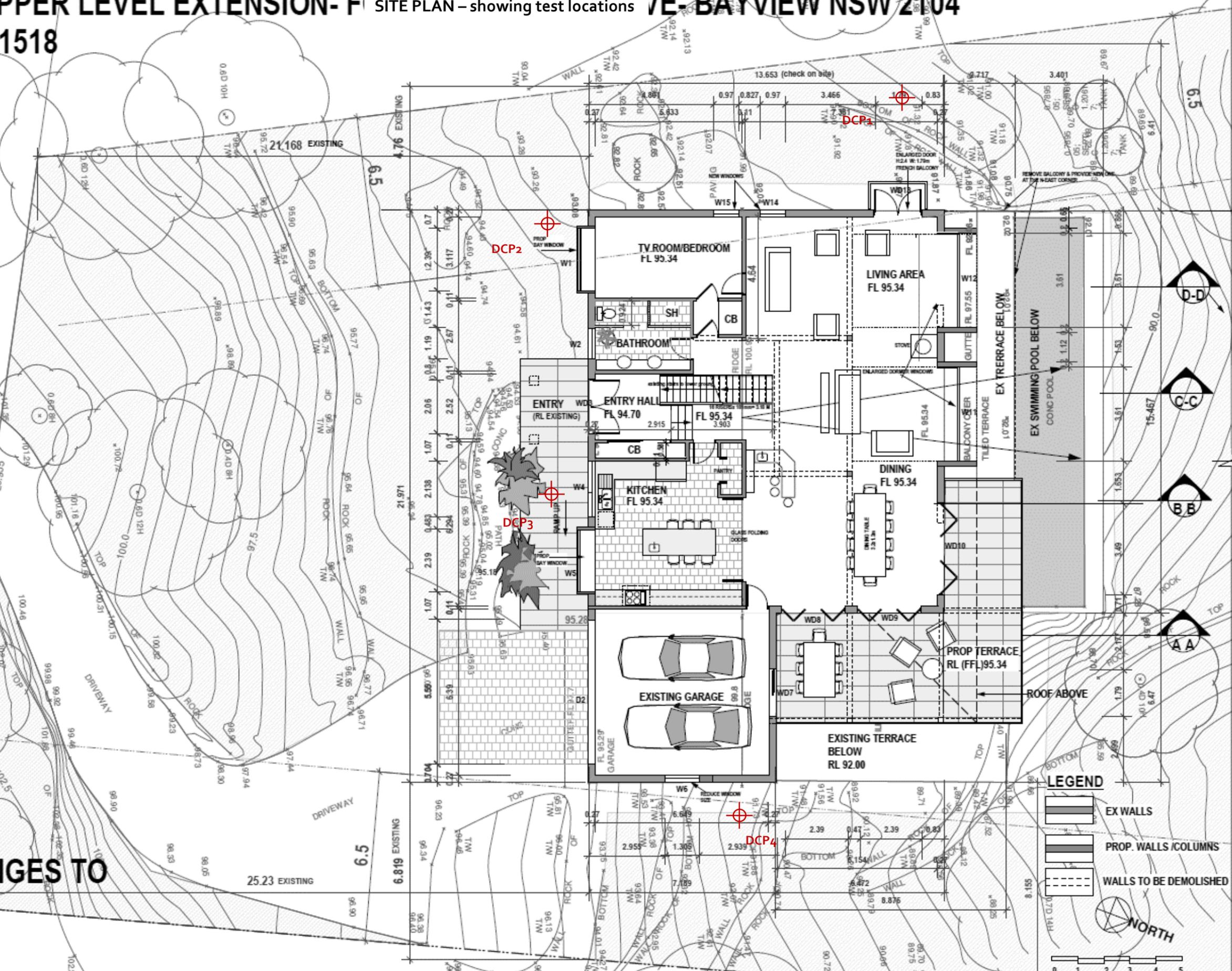
3.5133; No.11

GILWINGA DRIVE

PROPOSED CHANGES TO GROUND LEVEL

ENCLOSED AREA=215.82 M2
BALCONY =55 M2

NO	Hx W	Remarks
W1	1534x 2390	AWNING
W2	2100x 1190	AWNING
WD3	2900x 2060	glazed door/ fixed glass
W4	1000x 2138	awning
W5	1534x 2390	awning
W6	1372x 1330	SLIDING
WD7	2143x 1790	SLIDING DOOR
WD8	2143x 2390	FOLDING DOOR
WD9	2143x 2390	FOLDING DOOR
WD10	2143x 3490	FOLDING DOOR
W11	3330x 3610	sliding/ fixed
W12	3330x 3610	sliding/ fixed
WD13	2400x 1790	glazed doors
W14	3260x 970	awning/ fixed
W15	2270x 970	awning



LEGEND

- EX WALLS
- PROP. WALLS /COLUMNS
- WALLS TO BE DEMOLISHED

NORTH

0 1 2 3 4 5

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ISS.	DATE	REVISION	DR.	APP.
1				
2				

NOTES:
ALL DIMENSIONS TO BE CHECKED ON SITE

PLANNING	STRUCTURAL/CIVIL	LANDSCAPING	SEWTECHNICAL	BIOSECURE	BASIX

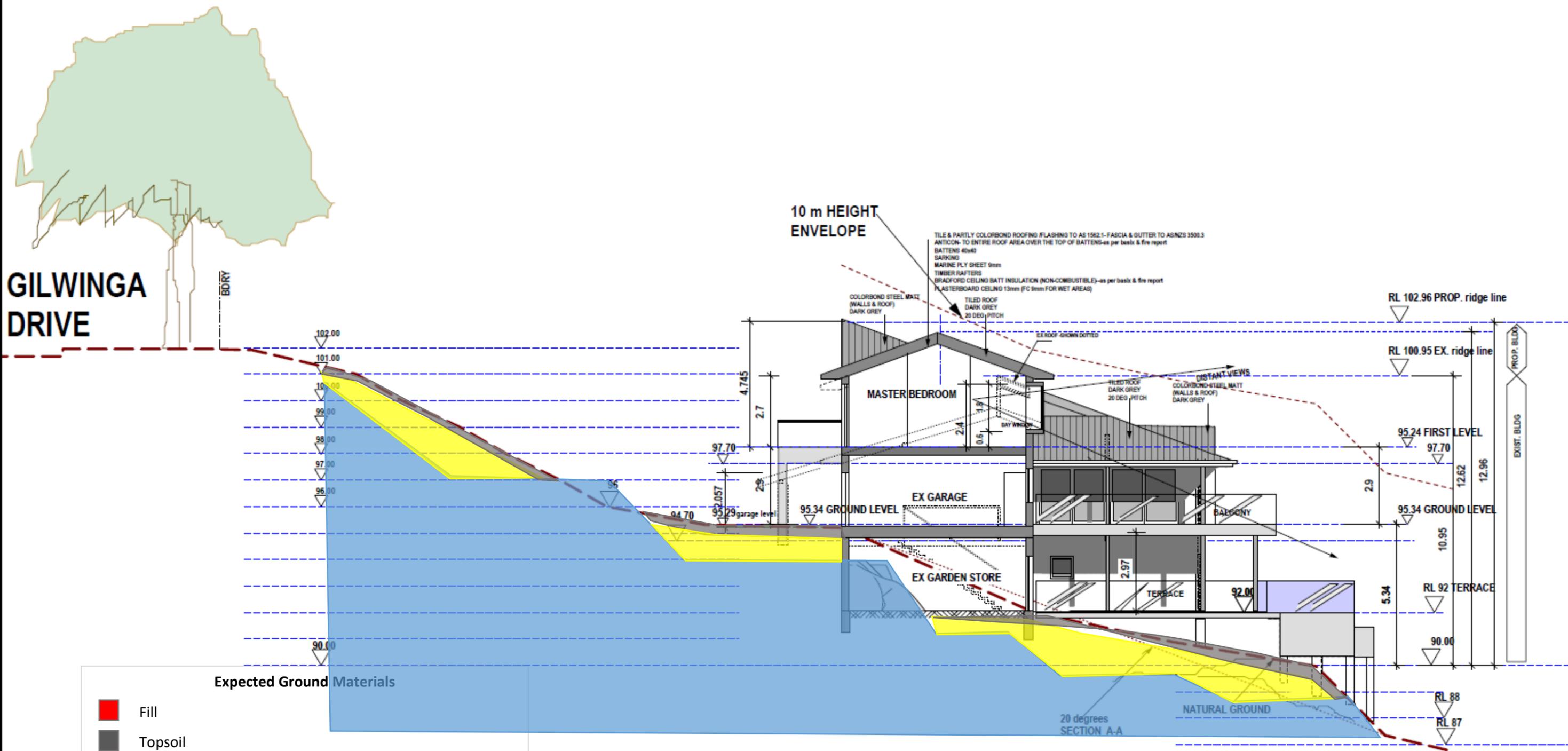
PROJECT:
PROP FIRST FLOOR ADDITION & SOME ALTERATION TO EX. RESIDENCE
AT NO. 11 GILWINGA DR BAYVIEW
LOT 39 SEC DP 241518

DRAWING TITLE:
PROPOSED CHANGES TO GROUND LEVEL

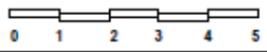
SCALE	DRAWN	CHECKED	APPROVED	PLOT DATE	DATE
1:100 @ A2	M.G.	M.B.	MB	29.11.2021	3/21

ARCOM HOLDINGS PTY LTD
DR MAZHAR BEKHE
BA-MSc & PhD IN ARCH
3/6 ROBEY RD-ABBOTSFORD NSW 2046
PH: 0414 321 044

PROPOSED DA -UPPER LEVEL EXT TYPE SECTION – Diagrammatical Interpretation of expected Ground Materials **NSW 2104**
LOT 39 SEC DP 241518

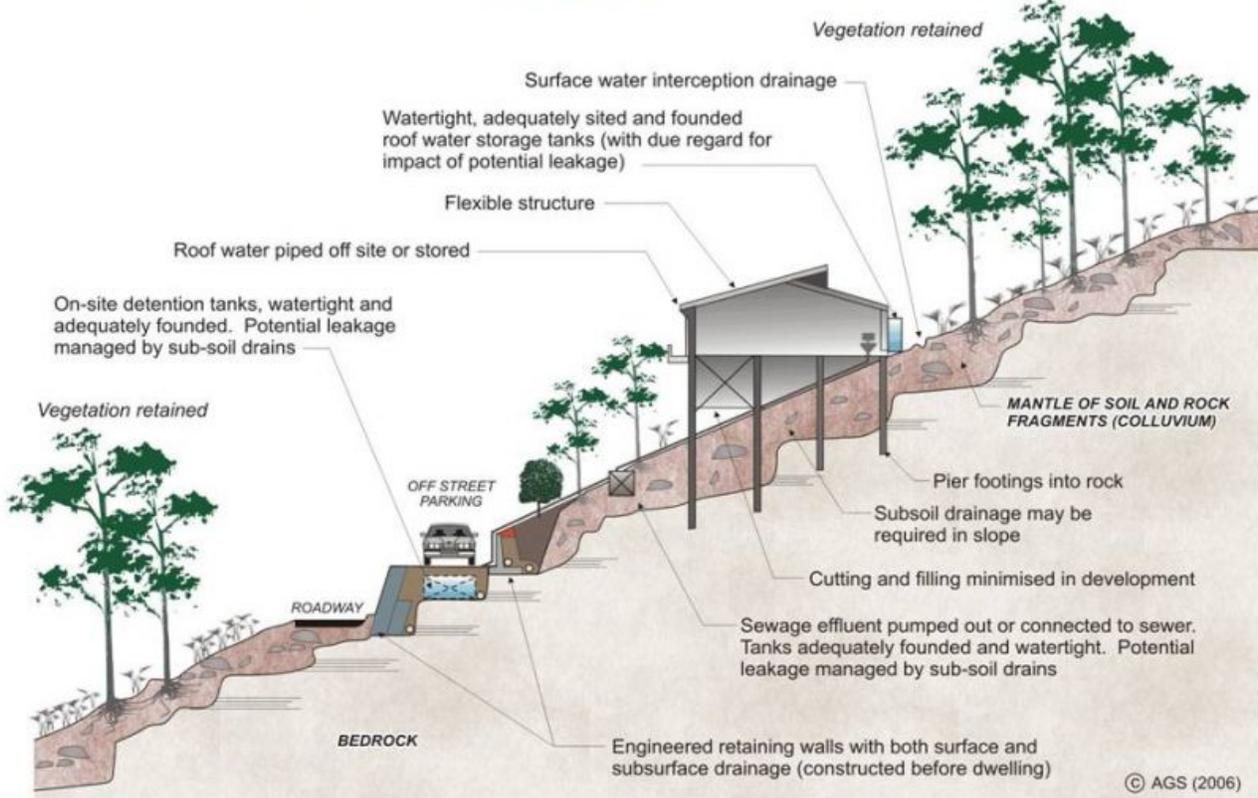


CROSS SECTION A-A



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	1	8-12-21	FINISHES		M.B.							
	2						LANDSCAPING	BASIX			APPROVED: MB	PLOT DATE 29.11.2021
											JOB NO: 3/21	ISSUE DA

EXAMPLES OF **GOOD** HILLSIDE PRACTICE



EXAMPLES OF **POOR** HILLSIDE PRACTICE

