### **Sirsi Marina Overland Flow Study**

**Prepared for Essex Develop** 

October 2023 Project Number S22042

21 1A

Y





#### Contents

1.	Intro	oduction	1
	1.1 1.2	Purpose of Report Referenced Standards	1 2
2.	Site	Context	3
	2.1 2.2 2.3 2.4	Existing Scenario Existing Site Information and Description Existing Council Stormwater pipe Existing Overland Flow and Flooding	3 3 3 4
3.	Indu	Istry standard practices	7
	3.1	Australian Rainfall and Runoff 2019	7
4.	Prop	posed Development	10
	4.1 4.2 4.3 4.4 4.5	Design Proposal Catchment Plan Proposed Council Pipe Diversion Proposed Overland Flow Path OSD Requirement	10 10 11 12 13
5.	Hyd	raulics	14
	5.1 5.2 5.3 5.1 5.2	Methodology Assumptions & Parameters Pipe Diversion Results Overland Flow Results Summary of Results	14 14 15 16 18
6.	Res	ponse to Council comments	20
7.	Con	clusion	22
Ap	oendi	ices	

Appendix A – Architectural Plans24Appendix B - Civil Plans25Appendix C - Survey26

Document Control				
Revision	Date	Prepared	Reviewed	Approved
А	30/10/2023	SM	SH	SH
В	03/11/2023	SM	SH	SH

A person using BG&E Pty Limited (BG&E) documents or data accepts the risks of:

a) using the documents or data in electronic form without requesting and checking them for accuracy against the original hard copy version; and

b) using the documents or data for any purpose not agreed to in writing by BG&E.

#### 1. Introduction

#### 1.1 Purpose of Report

BG&E have been engaged by Essex Develop to prepare an overland flow assessment along with the assessment for the proposed Council drainage diversion for the subdivision development at 122-128 Crescent Road, Newport (Site).

This report will outline the approach BG&E has adopted to evaluate the following:

- Assessment of the following:
  - Existing Council Stormwater pipe
  - o Existing overland flows on-site
  - Proposed Council Drainage Diversion
  - Proposed Overland flow paths.
- And Address the following council comments:
  - The site is affected by overland stormwater flows in larger storm events predominately along the line the of the existing Council 375mm Concrete stormwater line which runs from the sag point in Crescent Road to the existing seawall.

An overland stormwater flow study is to be prepared by a NER/RPENG qualified civil engineer to determine the extent of the overland flow path and impacts on the development including the proposed construction of dwellings. The study is to be prepared by a suitable computer hydrological/hydraulic model. Councils preferred model is DRAINS. The overland flow cross-sections are to be determined by the HEC Ras programme or similar.

- It is noted that the design engineers BG&E propose to re divert Councils existing 375mm RCP stormwater (SW) line which runs through the current site to the southern boundary with a 3m wide stormwater drainage easement over the re diverted drainage line. This proposal is supported by Councils stormwater assets team.
- The current proposed location of the overland flow swale is not acceptable as the overland flow path needs to be aligned with the sag pit Crescent road ie the lowest point . Any overland flow originating from this location cannot physical enter the proposed swale given its separation from the current sag point. Any over land flow path should be aligned from the current sag point in the Crescent Road and follow the alignment of the proposed re diverted Council SW line to the discharge point.
- The following needs be addressed by the study:
  - The existing overland flow regime is to be mapped. All proposed residential housing is to be keep clear of the overland flow path.
  - The re diverted Council stormwater line running through the site is to be upgraded to have a minimum hydraulic capacity of 5/100 AEP. Flows in excess of this event are to be controlled via an above overland flow path to the 1 /100 AEP.
  - The overland flow path is to be contained with a widened 3m easement from the sag point in Crescent Road to the outlet.
  - The study is to also review velocity depth ratio in relation to pedestrian safety.
  - The overland flow study/report is to be prepared in accordance with ARR 2019 Section Flood estimation and use the Initial/ Continuing Hydraulic Loss model.
  - The Council stormwater line upgrade works are to be designed in accordance with Councils Auspec one design guideline.
- Construction of the Headwall outlet and upgraded pipeline requires a controlled activity permit.
   Comments from Councils Coast and Catchment Team will be required on the new headwall/pipe construction.
- A Stormwater management plan will need to detail the provision of On site stormwater detention (OSD) in accordance with Councis water management policy for development. A DRAINS model is required to determine the required OSD storage volumes and post developed discharge rates to the 1/100AEP storm event. The pre developed scenario for the model is to be state of nature. Additionally water quality treatment devices are to be provided for the proposed subdivision in



accordance Councils water management policy for development. Councils Coast and Catchment Team can be contacted for more detailed comments.

#### 1.2 Referenced Standards

The following documents have been used as part of this report:

- Pittwater 21 Development Control Plan
- Northern Beaches Water Management for Development Policy (February 2021)
- Australian Rainfall and Runoff Guidelines (Geoscience Australia, 2019)



#### 2. Site Context

#### 2.1 Existing Scenario

The site is located at 122-128 Crescent Road, Newport, within the Northern Beaches Council LGA. As shown in the locality plan in **Figure 1**, the site is bordered to the north by a public reserve known as 'The Avenue', to the east by Crescent Road, and to the west by the Pittwater Waterway. The site is zoned as C4 environmental living and is part of a low-density residential neighbourhood.



Figure 1 Site Locality Plan (Sixmaps)

#### 2.2 Existing Site Information and Description

The site currently consists of 4 residential dwellings, a commercial building, a workshop and various small structures such as storage containers and sheds. The commercial building is fronted by a large bitumen carpark. There is a large concrete slab at the back of the site, and there are three pontoons extending into Pittwater Waterway.

#### 2.3 Existing Council Stormwater pipe

The site contains an existing 375mm drainage pipe, which belongs to the Northern Beaches Council. The drainage pipe extends from the kerb inlet pit along Crescent Road and, navigates further west between lots 112 & 126 and discharges to Pittwater through an existing headwall. Refer to Figure 2, which indicates the existing pipe size and location based on Northern Beaches Council's Planning Maps.





Figure 2: Northern Beaches Council- Planning Maps

#### 2.4 Existing Overland Flow and Flooding

The entire site generally slopes from the east to the west at grades approximately 17% towards Pittwater Waterway. There is an existing kerb inlet pit adjacent to Lot 112, Crescent Road. The pit is currently located at a low sag point along Crescent Road and is currently servicing surface runoff flows from upstream catchments, including nearby residential houses. The survey provided by Boxall Surveyors, dated 20.05.22, suggests an existing overland flow path is located along the existing driveway at 122 Crescent Rd Newport. The existing overland flow path is expected to carry external site catchments and discharge into Pittwater. Refer to Figure 3 for the exiting location of the overland flow path.

The western side of the site is affected by coastal flood inundation per the Newport Flood Study (2019). The overland flow path is also picked up in council's flood maps, shown in Figure 4 below.





Figure 3: Existing Overland Flow Path





Figure 4: Existing Overland Flow Path



#### 3. Industry standard practices

Due to the Site being in close proximity to Pittwater waterway while being within an overland flow path zone married in with council-limited guidance in technical standards around to assess this current scenario, it is worth investigating what the literature says about these individually and them coming to a reasonable conclusion.

#### 3.1 Australian Rainfall and Runoff 2019

Australian Rainfall and Runoff 2019 (ARR2019) is the most current document Australia has on how to assess the stormwater design, modelling and theory, superseding Australian Rainfall and Runoff 1987 with the draft release in 2016 and being made the formalised documents moving forward in Australia since. ARR2019 can provide the base drainage of Site at the bottom of the catchment to assess flood-prone areas and when OSD should be used.

#### 3.1.1 Catchment drainage

Chapter 4 of ARR2019 talks about stormwater volume management, where the aim of managing stormwater at catchment wides analysis is to think about the volume of water being moved. **Figure 5** shows the impact of implementing different stormwater volume management devices, such as OSD and retention, compared to the undeveloped and unmanaged flows.



Figure 4: Developed Catchment with Retentions as compared to Detention and slow Drainage Strategies (From ARR2019 Figure 9.4.3)

The above figure communicates that the idea with the implementation of OSD is to help manage the volume of water in a rain event as to no overwhelm the existing network or to contribute to a larger catchment peak flow event which can lead to flooding event. The major aim is to reduce the peak flow discharge rate and to extend the period out over which stormwater is released into the network.

#### 3.1.2 Flood prone hazards

The following is a section form ARR 7.2.7 General Flood Hazard Curves that is shown in **Figure 6**. "The combined flood hazard curves presented in Figure 6.7.9 set hazard thresholds that relate to the vulnerability of the community when interacting with floodwaters. The combined curves are divided into hazard classifications that relate to specific vulnerability thresholds .... provides the limits for the classifications in Table 6.7.3







Hazard Vulnerability Classification	Description
H1	Generally safe for vehicles, people and buildings.
H2	Unsafe for small vehicles.
НЗ	Unsafe for vehicles. children and the elderly.
H4	Unsafe for vehicles and people.
Н5	Unsafe for vehicles and people. All buildings vulnerable to structural damage. Some less robust buildings subject to failure.
H6	Unsafe for vehicles and people. All building types considered vulnerable to failure.

This information will be used to help understand any of the results from overland flow management.

#### 3.1.3 On Site Detention

Table 9.4.1 Summary of Volume Management Design Objectives from ARR2019 say the following:

"This objective seeks to limit the peak flood flows and volumes discharging from a catchment to a pre-determined and acceptable level. Commonly, the acceptable level is set at the natural or 'pre-development' condition. In some cases, the acceptable level may be set below the natural condition in order to achieve a net benefit or offset an impact elsewhere. In highly developed catchments (infill development), the acceptable level may correspond to flows from the original development.

These objectives may seek to change the total volume of stormwater leaving a site (retention), or delay the volume for a short period of time (hours) (detention or retarding) which may reduce the peak of the flood hydrograph discharging from a catchment.

Careful consideration of the spectrum of design flood events needs to be given and its impact on downstream receiving systems (for example, stream forming flows and flood flows), which can result in 'slow release' systems. Emerging stormwater management practices seek to reduce the volume and timing of stormwater discharges from catchments. This combined approach is particularly relevant for managing stormwater runoff from increasing urban density."



This is a good framework to help understand the objectives of when OSD should be used and where it is appropriate and should be used in lieu of the council not having any OSD design standards or requirements.



#### 4. Proposed Development

#### 4.1 Design Proposal

The proposed development consists of a subdivision of the site into 8 residential lots and a shared internal road for access, as shown in Figure 7. The proposed development will need to account for an overland flow location and the diversion of the existing Council's 375mm pipe crossing the site to be diverted on site to consider future developments on site.



#### Figure 7 Proposed Subdivision Plan (Scott Carver)

#### 4.2 Catchment Plan

Figure 8 illustrates the upstream catchment area responsible for contributing surface stormwater runoff to the existing kerb inlet pit adjacent to Lot 122 on Crescent Road and the proposed developed catchment. The internal catchment area has been estimated based on the site's existing topography.





**Figure 8: Overland Flow Catchments** 

The following catchment data was collected for defining the catchment areas:

- External catchment was found to be 7109sq.m with:
  - An impervious percentage of 52.60%
  - Previous percentage of 47.40%
- Internal catchment was found to be 3960sq.m with
  - An expected impervious percentage of 54.0%
  - Expected Previous percentage of 46.00%

#### 4.3 Proposed Council Pipe Diversion

The proposed development aims to reroute an existing council stormwater 375mm diameter pipe around the site and eventually discharge through the existing headwall at Pittwater Waterway to the site's west. The site lacks any natural water courses, and the existing seawall is expected to remain in good, non-degraded condition for the purpose of re-use. Refer figure 9 for the proposed Council pipe diversion adjacent to the northern boundary of the proposed lot 4. A new kerb inlet pit is proposed north of the existing kerb pit along Crescent Road, which will form part of the pipe diversion network.



#### **Figure 9 Proposed Council Pipe Diversion**



#### 4.4 Proposed Overland Flow Path

The Site is subjected to overland flow that needs to be managed on-site for safety and compliance with council requirements. Northern Beaches Council states in the Water Management for Development Policy that an overland flow path through the property is to be provided for all storms in excess of the 5% AEP, up to and including the 1% AEP.

The Site currently contains an overland flow path along the existing driveway at 122 Crescent Road. The overland flow path is approximately 3m South of Lot 112's Northern boundary, which is located between the carpark and driveway regions.

To manage the overland flow, a proposed grassed swales along the boundaries of the subdivision lots 3-4 & 5-6. The current route is proposed as the existing site falls in this direction from the council sag pit, as shown in the survey. This also does not clash with any of the proposed driveways in future housing developments. A bund is to be installed as part of the subdivision works until the property is developed, at which time the bund needs to be made permanent, or a wall to direct the stormwater is to be installed. This is in line with the location for the existing start of the overland flow path and will result in the smallest impact no any ponding in the roadway. See Figure 10 for the proposed layout on top of the existing survey.

If the overland flow path were to be placed in the same location as the proposed stormwater pipe, this would require the stormwater to pond to a level of RL14.7, which is more than 100mm higher than the existing and proposed overland flow route.

The Council's Water Management for Development Policy states that the width of any easement for overland flow is controlled by the minimum practical width necessary for standard machinery to carry out reconstruction of the public drainage system.





Figure 10 Close up of the Proposed Overland Flow path and Stormwater Diversion

#### 4.5 **OSD Requirement**

Northern Beaches Council Water Management for Development Policy (2021) says the following:

- The proposed site is located within Council's Region 1- Northern Stormwater Region.
  - Any sites located within region 1 which are affected by the 1% AEP flood plain do not require an OSD tank.
  - The proposed site is located within Region 1 1% AEP flood plain. Refer to section 2.4 for the Council flood map.

BG&E has provided this information to council in previous letters dated 14 July 2023.



#### 5. Hydraulics

#### 5.1 Methodology

The DRAINS software package has been used to model the hydrologic and hydraulic characteristics of stormwater runoff and flow to and from the Site. The model has been prepared to assess the 50 Yr. ARI storm events for the pipe diversion and 1% ARI for the overland flow.

The following models have been prepared to evaluate the proposed developments of the Site on councils existing infrastructure. The pipe diversion was developed based on using council's minimum pipe size of 375 dia to start, which was incrementally increased as required.

The swale sizing was developed through initial stormwater discharge from the entire catchment and refined through an iterative process during modelling.

#### 5.2 Assumptions & Parameters

The following assumptions and parameters were adopted from councils Water Management for Development Policy, Section 9.9:

- Soil Type = 2.5
- Antecedent moisture content, AMC = 3
- Infiltration rates: Initial paved = 1 mm, grassed = 5 mm
- Pit losses have been applied as per
- Blockage factor has been adopted from ARR 2019 of 0.5 for sag pits
- Minimum pipe size is to be 375mm except where noted otherwise
- Minimum pipe grade is to be 1%
- Ku factors from ARR 2019
- Rainfall Data from BOM IFT
- Temporal Pattern from ARR Data Hub



#### 5.3 **Pipe Diversion Results**

The following shows the results from the DRAINS models based on the above information. Refer to figures 11-14 for the 5% and 1% AEP DRAINS modelling results, respectively.









#### 5.1 Overland Flow Results

The following shows the results from the DRAINS models based on the above information. Refer to figures 15-17 for the 1% AEP DRAINS modelling results, respectively.





Figure 15 DRAINS Model – 1% AEP Overland Flow Result



Figure 16 DRAINS Model – 1% AEP Upper Overland Flow Result



Figure 17 DRAINS Model – 1% AEP Lower Overland Flow Result



#### 5.2 Summary of Results

#### 5.2.1 Pipe Diversion

The DRAINS modelling shown in section 5.3 indicates that the current pipe division is adequate to meet coucnil's requirements. The modelling also showed that the pipe sizing, being council's minimum size, would also cater for the 1% AEP event being 50% blocked.

#### 5.2.2 Overland Flow

The overland flow can be contained within 1.36m wide and 0.26m deep swales running along the boundary of lots 3-4 & 5-6. The hazard classifications are shown in Table 2 below based on Figure 6 from earlier in this report.

Channel Number	Velocity	Depth ( <i>m</i> )	Classification Limit (D * V)	Hazard Vulnerability
	(m/s)			Classification
Upper Swale	2.87	0.25	0.72 (<1.0)	H4 – Unsafe for vehicles &
				people
Lower Swale	2.65	0.25	0.66 (<1.0)	H4 - Unsafe for vehicles &
				people

Table 2: Velocity/Depth Hazard Analysis

The hazard rating for the swale through the size of H4, which, as defined in table 1, is "Unsafe for vehicles and people. All buildings vulnerable to structural damage. Some less robust buildings subject to failure." The location of the swale results in only property fences being in this area. This is not an access path for pedestrians or vehicles. It's located out of the way of property and driveway access.

Section 4.4 Council's Water Management for Development Policy states that the width of any easement for overland flow is controlled by the minimum practical width necessary for standard machinery to carry out reconstruction of the public drainage system. Due to this being an earthworks swale with a total width of 1.36m, it is recommended that an easement of 2m in total would be all that is required to maintain or amened any damage to the area as it does not require any extensive excavation.

#### 5.2.3 OSD

In section 3 it outlines why an OSD tank is not required in line with council's development requirements. It is also not recommended under ARR 2019.

Chapter 4 of ARR2019 talks about stormwater volume management, where the aim of managing stormwater at catchment wide analysis is to think about the volume of water being moved. Figure 4 shows the impact of implementing different stormwater volume management devices, such as OSD and retention, compared to the undeveloped and unmanaged flows.

Figure 4 communicates that the idea with the implementation of OSD is to help manage the volume of water in a rain event so as not to overwhelm the existing network or to contribute to a larger catchment peak flow event which can lead to flooding events. The major aim is to reduce the peak flow discharge rate and to extend the period over which stormwater is released into the network.

The main issue with our site being located at the downstream extent of the catchment is if we are required to implement an OSD tank, this would have a significant impedance on the existing council network as it would slow down the discharge rate from our site and align the stormwater discharge from upper catchment.

Thus, it would result in the headwall having to deal with both peak flow rates from the catchment at the same point in time instead of letting them be naturally staggered.

As a result of both council and ARR2019 best practise we are not proposing OSD for DA2022/2152 - 122 Crescent Road.



In addition as mentioned earlier the western side of the site is affected by coastal flood inundation per the Newport Flood Study (2019). Hence, the allocation of any OSD tank at the downstream end being the western boundary, would also be within the coastal inundation zone.



#### 6. Response to Council comments

Council comments are shown below inline with the

• And Address the following council comments:

Council Comment	BG&E Reply
<ol> <li>The site is affected by overland stormwater flows in larger storm events predominately along the line the of the existing Council 375mm Concrete stormwater line which runs from the sag point in Crescent Road to the existing seawall. An overland stormwater flow study is to be prepared by a NER/RPENG qualified civil engineer to determine the extent of the overland flow path and impacts on the development including the proposed construction of dwellings. The study is to be prepared by a suitable computer hydrological/hydraulic model. Councils preferred model is DRAINS. The overland flow cross- sections are to be determined by the HEC Ras programme or similar.</li> <li>It is noted that the design engineers BG&amp;E propose to re divert Councils existing 375mm RCP stormwater (SW) line which runs through the current site to the southern boundary with a 3m wide stormwater drainage easement over the re diverted drainage line. This proposal is supported by Councils stormwater assets team.</li> <li>The current proposed location of the overland flow swale is not acceptable as the overland flow swale is not acceptable as the overland flow originating from this location cannot physical enter the proposed swale given its separation from the current sag point. Any over land flow path should be aligned from the current sag point in the Crescent Road and follow the alignment of the proposed re diverted Council SW line to the discharge point.</li> </ol>	<ul> <li>1.a. BG&amp;E accepts that council accepts the location of the stormwater pipe diversion and as shown in this report meets council's standards.</li> <li>1.b. The location of the overland flow path is the best location on the site due to the following discussed in this report: <ul> <li>Location due to low point on site and inline with where the existing overland flow path is.</li> <li>Moving the overland flow to the same location as the stormwater uppie diversion will force stormwater uphill and cause a large amount of ponding in the roadway</li> <li>Can achieve a safe flow route <ul> <li>No Vehicle access</li> <li>No Vehicle access</li> <li>No vehicle access</li> <li>No impact on the future driveway</li> </ul> </li> <li>The overland flow is proposed to flow by a bund into the proposed swales in the temporary case and in the premiant case can be a wall or bund to control the direction of flow.</li> </ul> </li> <li>Council's request that the overland flow and stormwater diversion align does not work for this site due to the existing road levels. Refer to section 4.4 of this report.</li> </ul>
<ol> <li>The following needs be addressed by the study:         <ul> <li>The existing overland flow regime is to be mapped. All proposed residential housing is to be keep clear of the overland flow path.</li> <li>The re diverted Council stormwater line running through the site is to be upgraded to have a minimum hydraulic capacity of 5/100 AEP. Flows in excess of this event are to be controlled via an above overland flow path to the 1 /100 AEP.</li> </ul> </li> </ol>	<ul> <li>2.</li> <li>a. Council is to refer to updated drawings in the appendix of this report.</li> <li>b. This report shows that this requirement has met this requirement.</li> <li>c. The Council drainage pipe has been positioned to align with the proposed overland flow path within a 3m easement.</li> </ul>
<ul> <li>c. The overland flow path is to be contained with a widened 3m easement from the sag point in Crescent Road to the outlet.</li> <li>d. The study is to also review velocity depth ratio in relation to pedestrian safety.</li> </ul>	<ul> <li>d. The report has addressed the velocity depth ratio and complies with the safety requirements of its location.</li> <li>e. Refer to this report.</li> </ul>
e. The overland flow study/report is to be prepared in accordance with ARR 2019 - Section Flood	f. This has been complied with.



	f. g.	estimation and use the Initial/ Continuing Hydraulic Loss model. The Council stormwater line upgrade works are to be designed in accordance with Councils Auspec one design guideline. Construction of the Headwall outlet and upgraded pipeline requires a controlled activity permit. Comments from Councils Coast and Catchment Team will be required on the new headwall/pipe construction.	g. The existing headwall is not proposed to have any work done to it. This is not required as part of this project.
3.	A the pc re vc 1/ sc Ac be ac de Te cc	Stormwater management plan will need to detail e provision of On site stormwater detention (OSD a accordance with Councis water management blicy for development. A DRAINS model is quired to determine the required OSD storage blumes and post developed discharge rates to the 100AEP storm event. The pre developed cenario for the model is to be state of nature. dditionally water quality treatment devices are to a provided for the proposed subdivision in ccordance Councils water management policy for evelopment. Councils Coast and Catchment eam can be contacted for more detailed omments.	<ul> <li>ODS is not required on the Site based on the following information:</li> <li>Northern Beaches Council Water Management for Development Policy (2021) for being within a 1% AEP floodplain</li> <li>ARR does not recommend OSD to be used in downstream catchments due to putting more stain on council's infrastructure.</li> <li>The Site is subject to coastal flood inundation, which would undermine any OSD use on-site as it would be subject to inundation.</li> </ul>

#### 7. Conclusion

BG&E has been engaged by Essex Develop to prepare an overland flow assessment along with the assessment for the proposed Council drainage diversion for the subdivision development at 122-128 Crescent Road, Newport.

This report outlined the methodology adopted and associated results of the:

- Existing Council Stormwater pipe
- Existing overland flows on-site
- Proposed Council Drainage Diversion
- Proposed Overland flow paths.

As outlined in this report, the following key items have been identified:

- The proposed Council 375mm diameter pipe diversion has sufficient capacity to meet 5% AEP per council requirements and has been checked against the 1% AEP.
- The relocation of the overland flow path is not achievable due to the existing road levels, and the proposed overland flow location matches up with the existing.
- The Overland flow path is able to safely capture and divert the stormwater from upstream catchments through the proposed development.
- OSD is not required on-site due to the council's requirements and is not recommended by ARR 2019



### Appendices



### **Appendix A – Architectural Plans**





# **NEWPORT MARINA AND RESIDENTIAL DEVELOPMENT** 122-128 CRESCENT RD & 55-57 THE AVENUE DP210342, DP503390, DP545339 & DP556902 ESSEX DEVELOP

# SUBDIVISION DEVELOPMENT APPLICATION 20220005: DRAWING LIST

#### DRAWING LIST

 NUMBER
 SI

 AD-DA900
 CC

 AD-DA901
 LC

 AD-DA902
 DE

 AD-DA903
 AF

 AD-DA904
 AF

 AD-DA905
 SL

 AD-DA906
 OV

 AD-DA910
 DE

 AD-DA911
 3D

SHEET NAME COVER PAGE LOCATION PLAN DEMOLITION PLAN ARCHITECTURAL ENVELOPE PLAN ARCHITECTURAL ENVELOPE SECTIONS SUBDIVISION PLAN OVERLAY DEMO & NEW DEMOLITION WASTE MANAGEMENT PLAN 3D ENVELOPE DIAGRAM LANDSCAPE PRINCIPLES FOR LOT LAYOUTS

#### **GENERAL NOTES**

1. DO NOT SCALE FROM DRAWINGS. WRITTEN DIMENSIONS GOVERN. IF IN DOUBT OBTAIN WRITTEN ADVICE FROM SCOTT CARVER OR WHERE APPLICABLE VIA THE PRINCIPAL'S REPRESENTATIVE

2. ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE STATED. ALL DIMENSIONS ARE MINIMUM SETTING OUT REQUIREMENTS

3. ALL DIMENSIONS SHOULD BE VERIFIED ON SITE PRIOR TO PROCEEDING WITH THE WORKS. NOTIFY THE PRINCIPALS REPRESENTATIVE IN WRITING OF ANY DISCREPANCIES

4. ALL ARCHITECTURAL DRAWINGS MUST BE READ IN CONJUNCTION WITH RELEVANT CONTRACTS, ARCHITECTURAL REPORTS, SCHEDULES AND SPECIFICATIONS AND ALL OTHER CONSULTANT / CONTRACT DOCUMENTATION. NOTIFY THE PRINCIPALS REPRESENTATIVE OF ANY DISCREPANCIES BETWEEN DOCUMENTATION IN WRITING TO OBTAIN CLARIFICATION DIRECTION

5. ALL WORK TO COMPLY WITH THE NATIONAL CONSTRUCTION CODE INCLUDING RELEVANT AUSTRALIAN STANDARDS AND REQUIREMENTS OF THE BUILDING CODE OF AUSTRALIA AND AUSTRALIAN WORK HEALTH AND SAFETY LEGISLATION

6. DESIGN DRAWINGS ARE BASED ON SURVEY INFORMATION. PRIOR TO DETAILED DESIGN AND CONSTRUCTION, THE CONTRACTOR IS TO UNDERTAKE A FULL SURVEY TO VERIFY ALL DIMENSIONS AND CONFIRM LOCATION OF EXISTING STRUCTURE, SERVICES, BUILDING FABRIC AND SITE FEATURES.

7. ANY CONSULTANT DRAWING INFORMATION SHOWN ON THE ARCHITECTURAL DRAWINGS ARE SUBJECT TO LATEST REQUIREMENT AND ARE TO BE USED AS A GUIDE ONLY

8. ALL WALL AND ROOF CONSTRUCTION IS TO BE CONSTRUCTED IN ACCORDANCE WITH THE REQUIREMENTS SET OUT IN THE ACCOMPANYING BASIX CERTIFICATION AND BCA CLASS 1A.

SURVEY PLAN: BY BOXALL SURVEYORS

## **OCTOBER 2023**





[Project] NEWPORT MARINA AND RESIDENTIAL DEVELOPMENT [Client] ESSEX DEVELOP

### PRELIMINARY [Nom. Architect] DOUG SOUTHWELL /7362 [File]

[Print Date]

24/10/2023 2:39:33 PM

History

[Rev#]	[Description]	[Date]dd.mm.yy
1	SUBDIVISION DA	07.06.2022
2	SUBDIVISION DA	12.07.2023
3	REVISED SUBDIVISION DA	06.10.2023
4	REVISED SUBDIVISION DA	24.10.2023









#### PRELIMINARY

DOUG SOUTHWELL /7362 [Nom. Architect]

[File]

[Print Date]

[Status]

24/10/2023 1:54:13 PM

History

[Rev#]	[Description]	[Date]dd.mm.yy
1	SUBDIVISION DA	07.06.2022
2	SUBDIVISION DA	12.07.2023
3	REVISED SUBDIVISION DA	06.10.2023
4	REVISED SUBDIVISION DA	24.10.2023



BOUNDARY LINE DEMOLITION – – – HIGH WATER MARK - - - FORESHORE SETBACK 01 TREE TO REMAIN TREE TO BE DEMOLISHED FOR DETAILED ARBORIST REPORT REFER TO REPORT 6322 BY RAINTREE CONSULTING - DATED 25.05.2022 EXISTING EASEMENTS A. TRANSMISSION LINE **B. RIGHT OF CARRIAGEWAY** 

C. DRAIN WATER (1.83M WIDE)

BASED ON SURVEY BY BOXALL DATED 05.05.2022 - DRAWING NO. 11369-001-A

### DEMOLITION PLAN







[Project] NEWPORT MARINA AND RESIDENTIAL DEVELOPMENT

[Client] ESSEX DEVELOP

#### PRELIMINARY

DOUG SOUTHWELL /7362 [Nom. Architect]

[File]

[Print Date]

2 AD-DA904

1 AD-DA904

 $\square$ 

 $\triangleleft$ 

0

 $\mathbf{C}$ 

Ζ

ш

 $\mathbf{O}$ 

လ

ш

 $\mathbf{r}$ 

 $\mathbf{O}$ 

[Status]

24/10/2023 1:54:41 PM

History

[Rev#]	[Description]	[Date]dd.mm.yy
1	SUBDIVISION DA	07.06.2022
2	SUBDIVISION DA	12.07.2023
3	UPDATE FOR 8 LOTS	12.09.2023
4	REVISED SUBDIVISION DA	06.10.2023
5	REVISED SUBDIVISION DA	24.10.2023

### NOTE

ALL HOUSE FOOTPRINTS CAN ACHIEVE MIN BUILDING AREA OF 175M<sup>2</sup> AS PER B2.2

#### LEGEND

	BOUNDARY LINE
	HIGH WATER MARK
	FORESHORE SETBACK
	RIGHT OF WAY
	DRAINAGE EASEMENTS REFER TO SITEWORKS AND DRAINAGE PLAN FROM BG&E
ZZ	UNDERGROUND CABLE EASEMENT REFER TO PROPOSED UNDERGROUND SERVICES PLAN FROM IGS
	INDICATIVE BUILDING FOOTPRINT SUBJECT TO FUTURE DWELLING DA
	INDICATIVE LANDSCAPED AREA
r	BUILDING SETBACK
01	TREE TO REMAIN
	FOR DETAILED ARBORIST REPORT REFER TO REPORT 6322 BY RAINTREE CONSULTING - DATED 25.05.2022

BASED ON SURVEY BY BOXALL DATED 05.05.2022 - DRAWING NO. 11369-001-A



### ARCHITECTURAL ENVELOPE PLAN

[Ref] 20220005

[Dwg No] AD-DA903 [Rev] 5



www.scottcarver.com.au +61 2 9957 3988

[Client] ESSEX DEVELOP

[Scale] As indicated @ A1



#### PRELIMINARY

DOUG SOUTHWELL /7362

#### 24/10/2023 1:55:33 PM

#### History

Rev#]	[Description]	[Date]dd.mm.yy
1	SUBDIVISION DA	07.06.2022
2	SUBDIVISION DA	12.07.2023
3	REVISED SUBDIVISION DA	06.10.2023
4	REVISED SUBDIVISION DA	24.10.2023

MAX BUILDING ENVELOPE

PERMISSIBLE BUILDING MASS. INDICATIVE ABOVE GROUND AREA DEMONSTRATES WHAT FLOOR AREA CAN BE ACHIEVED WITH SIMPLE MASSING FLOOR PLATES. THIS DOES NOT INDICATE FINAL SIZE OF DWELLING, A DESIGN EXERCISE FOR EACH LOT WILL NEED TO BE UNDERTAKEN TO DETERMINE COMPOSITION INCLUDING POOL, INTERNAL COURTYARD ET.C

INDICATIVE EXCAVATION ZONE OVER 1M DEPTH, PENDING FINAL HOUSE DESIGN

### ARCHITECTURAL ENVELOPE SECTIONS

[Ref] 20220005

[Dwg No] AD-DA904 [Rev] 4



[Project] NEWPORT MARINA AND RESIDENTIAL DEVELOPMENT [Client] ESSEX DEVELOP

<sup>[Scale]</sup>1:250 @ A1

#### PRELIMINARY

[Nom. Architect] DOUG SOUTHWELL /7362

[File]

[Print Date]

[Status]

24/10/2023 1:55:57 PM

#### History

[Rev#]	[Description]	[Date]dd.mm.yy
1	SUBDIVISION DA	07.06.2022
2	SUBDIVISION DA	12.07.2023
3	REVISED SUBDIVISION DA	06.10.2023
4	REVISED SUBDIVISION DA	24.10.2023

#### LEGEND

BOUNDARY LINE
HIGH WATER MARK
FORESHORE SETBACK
RIGHT OF WAY
DRAINAGE EASEMENTS REFER TO SITEWORKS AND DRAINAGE PLAN FROM BG&E
UNDERGROUND CABLE EASEMENT REFER TO PROPOSED UNDERGROUND SERVICES PLAN FROM IGS

BASED ON SURVEY BY BOXALL DATED 05.05.2022 - DRAWING NO. 11369-001-A



SUBDIVISION PLAN





[Client] ESSEX DEVELOP

<sup>[Scale]</sup>1:250@A1

#### PRELIMINARY

[Nom. Architect] DOUG SOUTHWELL /7362

[File]

[Print Date]

[Status]

24/10/2023 1:56:17 PM

History

[Rev#]	[Description]	[Date]dd.mm.yy
1	SUBDIVISION DA	07.06.2022
2	SUBDIVISION DA	12.07.2023
3	REVISED SUBDIVISION DA	06.10.2023
4	REVISED SUBDIVISION DA	24.10.2023

### $\boldsymbol{\square}$ $\triangleleft$ 0 $\mathbf{C}$ Ζ ш $\mathbf{O}$ S ш $\mathbf{C}$ $\mathbf{O}$

#### LEGEND

	BOUNDARY LINE
	DEMOLITION
	HIGH WATER MARK
	FORESHORE SETBACK
	RIGHT OF WAY
[]]	DRAINAGE EASEMENTS REFER TO SITEWORKS AND DRAINAGE PLAN FROM BG&E
<u> </u>	UNDERGROUND CABLE EASEMENT REFER TO PROPOSED UNDERGROUND SERVICES PLAN FROM IGS
	INDICATIVE BUILDING FOOTPRINT SUBJECT TO FUTURE DWELLING DA
01	TREE TO REMAIN
01	TREE TO BE DEMOLISHED
	FOR DETAILED ARBORIST REPORT REFER TO REPORT 6322 BY RAINTREE CONSULTING - DATED 25.05.2022
	EXISTING EASEMENTS A. TRANSMISSION LINE B. RIGHT OF CARRIAGEWAY C. DRAIN WATER (1.83M WIDE)

BASED ON SURVEY BY BOXALL DATED 05.05.2022 - DRAWING NO. 11369-001-A

### **OVERLAY DEMO & NEW**



[Ref] 20220005

[Dwg No] AD-DA906 [Rev] 4

#### **NOTES**

- DISCONNECT POWER SUPPLY, GAS, WATER AND TELEPHONE LINE.
- PROVIDE 1.8m TEMPORARY FENCE TO SECURE DEMOLITION SITE
- PROVIDE INDICATOR AND WARNING SIGNS BEFORE WORKS BEGIN BUILDER TO PROVIDE SILT FENCE TO PREVENT SEDIMENTATION
- RUN-OFF
- BUILDER TO MINIMISE DUST BY SPRAYING WATER DURING **DEMOLITION**
- WORKS TO BE DONE BY HAND SUCH AS: ROOF TILES, ROOF TIMBER, EXTERNAL CLADDING AND TIMBER FRAME.
- A HEAVY EXCAVATOR WILL BE USED TO DEMOLISH THE PROPERTY AND TO LOAD THE TRUKS.
- WORK SHALL BE DONE BETWEEN 7.00am. AND 5.00pm. FROM MONDAY TO FRIDAY INCLUSIVE.
- TIPPER TRUCKS WILL PARK ON DRIVEWAYS AND CROSSOVERS PROTECTED WITH TIMBER.
- LOADS SHOULD BE COVERED BEFORE LEAVING THE SITE. ADJACENT PROPERTIES AND PUBLIC RIGHT OF WAYS SHALL BE PROTECTED.
- THE ACCESS POINT FOR THE SITE WILL BE FROM EXISTING DRIVEWAYS
- THE SITE SHALL HAVE BINS TO SEGREGATE MATERIALS SUCH AS A SCRAP METAL BIN AND SPOIL STOCKPILE. THIS WILL ALLOW MATERIAL TO BE RECYCLED IN EFFECTIVELY
- NO REQUIREMENTS ON THE SITE FOR UNDER PINNING OR SHORING WILL BE UNDERTAKEN.

NOTE: ASBESTOS IF ANY SHOULD BE REMOVED MANUALLY BY WET METHOD, AS REQUIRED UNDER WORKCOVER AND SAFTY REGULATIONS TO AUSTRALIAN STANDARD 2601-1991, WRAPPED WITH PLASTIC AND PLACED IN A DEDICATED BIN. THE BIN IS SECURELY SEALED AND TIPPED AT A SUITABLY APPROVED SITE.

#### DETAIL OF EXISTING STRUCTURES

LOT 122, DP 556902 / 122 CRESCENT ROAD, 2 STOREY RENDERED COMMERCIAL BUILDING, CLAD WORKSHOP AND BITUMEN CARPARK

LOT 111, DP 556902 / 124 CRESCENT ROAD, CONCRETE DRIVEWAY ANDTHREE CONTAINERS

LOT 1, DP503390 / 126 CRESCENT ROAD, SINGLE STOREY FIBRO RESIDENCE AND **FIBRO GARAGE** 

LOT 3, DP210342 / 128 CRESCENT ROAD, SINGLE STOREY BRICK RESIDENCE LOT 2, DP210342 / 55 THE AVENUE, SINGLE STOREY WEATHERBOARD CLAD RESIDENCE

NAME AND DETAILS OF DEMOLISHER: TO BE ADVISED.

FINAL LOCATION SUBJECT TO DEMOLITION CONTRATOR.

LOT 21, DP545339 / 57 THE AVENUE, ONE AND TWO STOREY BRICK RESIDENCE

- EROSION CONTROL NOTES ALL EROSION SEDIMENT CONTROL MEASURES ARE TO BE INSTALLED AND MAINTAINED IN ACCORDANCE WITH 'MANAGING URBAN STORMWATER, 3rd EDITION' PRODUCED BY THE NSW DEPARTMENT OF HOUSING. STANDARD DRAWING (SD) NUMBERS REFERENCED ON THIS DRAWING CAN BE OBTAINED FROM THIS PUBLICATION
- ALL EROSION AND SILATION CONTROL DEVICES ARE TO BE PLACED 2 PRIOR TO THE COMMENCEMENT OF ANY CONSTRUCTION WORKS. AND ALL SILT TRAPS ARE TO HAVE DEPOSITED SILT REMOVED REGULARLY DURING CONSTRUCTION.
- ALL TREES ARE TO BE PRESERVED UNLESS INDICATED OTHERWISE 3 ON THIS PLAN.
- INSTALL TEMPORARY SEDIMENT BARRIERS TO ALL INLET PITS LIKELY TO COLLECT SILT LADEN WATER
- NOT WITHSTANDING DETAILS SHOWN IT IS THE CONTRACTORS SOLE 5 RESPONSIBILITY TO ENSURE THAT ALL SITE ACTIVITIES COMPLY WITH THE REQUIREMENTS OF THE CLEAN WATERS ACT.





- - TEMPORARY FENCE
- SEDIMENT CONTROL



Level One, One Chifley Square Sydney NSW 2000 Australia

www.scottcarver.com.au +61 2 9957 3988

[Nom. Architect] DOUG SOUTHWELL /7362 [Rev#] [Description] 1 SUBDIVISION DA 2 REVISED SUBDIVISION DA [Date]dd.mm.yy [Project] 16.08.2022

24.10.2023

NEWPORT MARINA AND RESIDENTIAL DE2/E2800 RESIDENT RD & 55-57 THE AVENUE DE210342, DP503390, DP545339 & DP556902

No. 50

TWO STOREY

**BRICK & CLAD** 

RESIDENCE

Ø

21

DP 545339

840.9m<sup>2</sup>

112

DP 556902

1979m<sup>2</sup>

22

DP 553811

No.120 1 & 2 STOREY RENDERED RESIDENCE

S

PONTOON

PITTWATER

PONTOON

295

DP 820302

2498m<sup>2</sup>

PONTOON



THE

2

DP 210342

828.3m<sup>2</sup>

111

DP 556902

980.1m<sup>2</sup>

(A)(C

 $\bigcirc$ 

8

ESSEX DEVELOP







[Client] ESSEX DEVELOP





[Project] NEWPORT MARINA AND RESIDENTIAL DEVELOPMENT [Client] ESSEX DEVELOP

<sup>[Scale]</sup>1:250@A1

### PRELIMINARY [Status] [Nom. Architect] DOUG SOUTHWELL /7362 [File] 24/10/2023 1:56:32 PM [Print Date] History

[Rev#]	[Description]	[Date]dd.mm.yy
1	SUBDIVISION DA	12.07.2023
2	REVISED SUBDIVISION DA	06.10.2023
3	REVISED SUBDIVISION DA	24.10.2023

### LEGEND



TO BE READ IN CONJUNCTION WITH ARCHITECTURAL SUBDIVISION PACKAGE.



 $\square$ 

 $\triangleleft$ 

0

 $\mathbf{r}$ 

**—** 

Ζ

ш

 $\mathbf{O}$ 

S

ш

 $\mathbf{r}$ 

 $\mathbf{O}$ 

LANDSCAPE PRINCIPLES FOR LOT LAYOUTS [Ref] 20220005

[Dwg No] AD-DA912 [Rev] 3

### **Appendix B - Civil Plans**



# NEWPORT MARINA AND RESIDENTIAL DEVELOPMENT

LGA: NORTHERN BEACHES COUNCIL DEVELOPMENT APPLICATION ISSUE





LOCALITY PLAN SCALE 1:1000



								CLIENT	
								]	
								]	
D	27.10.23	ISSUED FOR DEVELOPMENT APPROVAL	SH					1	
C	09.10.23	ISSUED FOR DEVELOPMENT APPROVAL	SH					1	
В	12.07.23	ISSUED FOR DEVELOPMENT APPROVAL	SH					1	
Α	31.05.22	ISSUED FOR DEVELOPMENT APPROVAL	SH					1	
REV	DATE	DESCRIPTION	RVD	REV	DATE	DESCRIPTION	RVD	]	
REVISIONS REVISIONS							]		
\\BGESYD 27/10/202	\\BGESYDF\$03\PR0JECT\$\\BGE\\SYD\\S22042 SIRSI MARINA\100 DRAW\100.2 CIVIL\AUTOCAD\\S22042-DRG-CI-0000-TEMP.DWG 27/10/2023 5:30:01 PM								



	DRAWING INDEX
DRG No.	DESCRIPTION
CI-0000	COVER SHEET, LOCALITY PLAN AND DRAWING
CI-0200	SITEWORKS AND DRAINAGE PLAN
CI-0300	OVERLAND FLOW CATCHMENT PLAN
CI-0310	OVERLAND FLOW PLAN
CI-0320	PROPOSED INTERNAL ROAD PLAN
CI-0330	GENERAL ROADWORKS PLAN
CI-0340	DRAINAGE DETAILS
CI-0341	COUNCIL PIPE DIVERSION DRAINAGE LONGITUD
CI-0350	INTERNAL DRIVEWAY LONGITUDINAL SECTION
CI-0360	INTERNAL DRIVEWAY CROSS SECTIONS
CI-0370	THE AVENUE LONGITUDINAL SECTION - CLO2
CI-0371	THE AVENUE CROSS SECTIONS
CI-0380	MUSIC CATCHMENT PLAN
CI-0700	EROSION AND SEDIMENT CONTROL PLAN
CI-0710	EROSION AND SEDIMENT CONTROL DETAILS



Level One, One Chifley Square Sydney NSW 2000 Australia www.scottcarver.com.au +61 2 9957 3988

Sydney Office – L2, 8 Windmill St Sydney NSW 2000 P/+61297703300 E/info@bgeeng.com bgeeng.com<del>\_</del>



NEWPORT MARINA AND RESIDENTIAL DEVELOPMENT



INDEX
NAL SECTION
- CL01



ISSUED FOR APPROVAL					COVER SH	EET, PLAN	
Δ			APPROVED				
	GDA2020 MGA-56	SCALE 1:1000	<u>Ι</u>	A1 SIZE	PROJECT NO. S22042	DRAWING NO.	REV.
	<u> </u>	1		,, JIZL			C BG&E Pty Limited



ISSUED FOR APPROVAL						SITEWORKS AND DRAINAGE			GE
	DESIGNED	CHECKED	APPROVED						
	AM	SH							
	GRID	SCALE				PROJECT No.	DRAWING No.		REV.
כ	MGA-56	1:250		AT	A1 size	S22042	CI_(	)200	G



L	E	G	E	Ν	D
_	_	_	_	•••	_

SITE BOUNDARY

CADASTRAL

EXISTING PIT/ KERB INLET PIT

PROPOSED PIT/ KERB INLET PIT

EXTERNAL CATCHMENT OVERLAND FLOW TO PROPOSED GRASSED SWALE ON SITE INTERNAL CATCHMENT OVERLAND FLOW TO PROPOSED GRASSED SWALE ON SITE

PROPOSED GRASSED SWALE

EXTERNAL CATCHMENT FLOWS CALCULATION:

- STORM EVENT = 100 YR ARI TIME OF CONCENTRATION = 5 MINS
- INTENSITY= 273 mm/hr CATCHMENT AREA = 0.7109 Ha
- C=0.75

QOVERLAND FLOW = 0.404 m3/s

INTERNAL CATCHMENT FLOWS CALCULATION: STORM EVENT = 100 YR ARI • TIME OF CONCENTRATION = 5 mins • INTENSITY= 273 mm/hr

- CATCHMENT AREA = 0.3960 Ha
   C=0.54

QOVERLAND FLOW = 0.161 m3/s QTOTAL (INCLUDING EXTERNAL CATCHMENT FLOWS) = 0.404+0.161 = 0.566 m3/s

> 12 16 SCALE 1:400 AT A1 SIZE

ISSUED FOR APPROVAL							) FLOW C	ATCHM	1ENT
	DESIGNED	CHECKED	APPROVED						
A	SM	SH							
łD	GDA2020 MGA-56	scale AS SHC	)WN	AT /	A1 size	<sup>PR0JECT №.</sup>	DRAWING NO.	00	rev.



C BG&E Pty Limited

![](_page_41_Figure_0.jpeg)

![](_page_42_Figure_0.jpeg)

### LEGEND

![](_page_42_Picture_7.jpeg)

EXTENT OF WORKS

EXISTING SURVEY FEATURES

ARCHITECTURAL

L.O.K I.O.K T.O.K B.O.K ROAD C.L

PROPOSED ROADWORKS

#### EXISTING SERVICES

— — NBN — —	EXISTING	NBN
- G G	EXISTING	GAS
S S	EXISTING	SEWER
	EXISTING	STORMWATER

Q	1	2	, <u>3</u>	4		5 6n
	S	CALE 1:	100 A	AT A1	SIZE	

15	SSUED NOT TO	FOR A		L			ROADWORKS	
	DESIGNED	CHECKED	APPROVED					
A	SM	SH						
HD	GDA2020 MGA-56	SCALE AS SHC	WN	AT	A1 size	PROJECT №. S22042	CI-0330	REV.

![](_page_43_Figure_0.jpeg)

REVISIONS

REVISIONS

![](_page_43_Figure_2.jpeg)

![](_page_43_Figure_3.jpeg)

![](_page_43_Figure_4.jpeg)

*P* / +61 2 9770 3300 *E* / info@bgeeng.com bgeeng.com—

![](_page_43_Picture_8.jpeg)

NEWPORT MARINA AND RESIDENTIAL DEVELOPMENT

![](_page_43_Picture_10.jpeg)

### NOTES

![](_page_44_Figure_0.jpeg)

Image: Constraint of the second of the se									
Image: style									CLI
Image: Note of the second se									
Image: constraint of the system of the sy									
Image: Constraint of the state of the st									
B     03.11.23     ISSUED FOR DEVELOPMENT APPROVAL     SH     Image: Constraint of the state of									
A     27.10.23     ISSUED FOR DEVELOPMENT APPROVAL     SH     Image: Constraint of the state of	В	03.11.23	ISSUED FOR DEVELOPMENT APPROVAL	SH					
REV     DATE     DESCRIPTION     RVD     REV     DATE     DESCRIPTION     RVD       REVISIONS	Α	27.10.23	ISSUED FOR DEVELOPMENT APPROVAL	SH					
REVISIONS     REVISIONS       \\BGESYDFS03\\PR0JECTS\\BGE\\SYD\\S22042 SIRSI MARINA\100 DRAW\100.2 CIVIL\AUTOCAD\\S22042-DRG-CI-341.DWG     REVISIONS	REV	DATE	DESCRIPTION	RVD	REV	DATE	DESCRIPTION	RVD	1
\\BGESYDFS03\PR0JECTS\BGE\SYD\S22042 SIRSI MARINA\100 DRAW\100.2 CIVIL\AUTOCAD\S22042-DRG-CI-341.DWG 3/11/2023 12:21:48 PM			REVISIONS				REVISIONS		
	\\BGESYD 3/11/2023	FS03\PROJECTS 12:21:48 PM	SAGE/SYD/S22042 SIRSI MARINA/100 DRAW/100.2 CIVIL/AUTOCA	D\S22042-DRG-CI-	-341.DWG				

![](_page_44_Picture_2.jpeg)

![](_page_44_Picture_4.jpeg)

Scott\_ Level One, One Chifley Square Sydney NSW 2000 Australia www.scottcarver.com.au +61 2 9957 3988

Sydney Office — L2, 8 Windmill St Sydney NSW 2000 P / +61 2 9770 3300 E / info@bgeeng.com bgeeng.com—

![](_page_44_Picture_7.jpeg)

NEWPORT MARINA AND RESIDENTIAL DEVELOPMENT

![](_page_44_Picture_9.jpeg)

![](_page_44_Picture_10.jpeg)

DRAINAGE PIT DRAINAGE PIPE ------ HYDRAULIC GRADE LINE

					0 5 	10 15 20 25 LE 1:500 AT A1 SIZE	30m	
	SSUED NOT TO	FOR A				PIPE DIVERS		
rawn SR	DESIGNED	CHECKED SH	APPROVED		SECTION	LONGITODI	NAL	
ATUM AHD	GDA2020 MGA-56	SCALE AS SHO	DWN AT	A1 size	S22042	CI-0341	REV.	;
							C BG&E Pty L	imited

![](_page_45_Figure_0.jpeg)

							SCALE 1:200 AT A1 SIZE	
15	SSUED NOT TO	FOR A		VAL			DRIVEWAY	_ ۲۱ 01
	DESIGNED	CHECKED	APPROVED				INAL SECTIONS-	
IA	SM	SH						
HD	GDA2020 MGA-56	SCALE AS SHO	OWN	AT	A1 size	PROJECT No. S22042	CI-0350	REV.

C BG&E Pty Limited

![](_page_46_Figure_0.jpeg)

CH 16.00

![](_page_46_Figure_2.jpeg)

DESIGN LEVELS

EXISTING LEVELS

CHAINAGE

DESIGN LEVELS

EXISTING LEVELS

CHAINAGE

![](_page_46_Figure_4.jpeg)

С	27.10.23	ISSUED FOR DEVELOPMENT APPROVAL	SH				
В	09.10.23	ISSUED FOR DEVELOPMENT APPROVAL	SH				
А	19.09.23	ISSUED FOR DEVELOPMENT APPROVAL	SH				
REV	DATE	DESCRIPTION	RVD	REV	DATE	DESCRIPTION	RVD
		REVISIONS				REVISIONS	

![](_page_46_Picture_6.jpeg)

![](_page_46_Figure_8.jpeg)

CH 36.00

DESIGN

LEVELS

EXISTING LEVELS

CHAINAGE

![](_page_46_Figure_10.jpeg)

CH 26.00

![](_page_46_Picture_12.jpeg)

DESIGN LEVELS

EXISTING LEVELS

CHAINAGE

ARCHITECT

Sydney Office – L2, 8 Windmill St Sydney NSW 2000 P/+61 2 9770 3300 E/ info@bgeeng.com bgeeng.com–

![](_page_46_Picture_14.jpeg)

NEWPORT MARINA AND RESIDENTIAL DEVELOPMENT

DESIGN LEVELS

EXISTING LEVELS

CHAINAGE

![](_page_46_Picture_16.jpeg)

DESIGN LEVELS EXISTING LEVELS CHAINAGE

![](_page_46_Figure_18.jpeg)

![](_page_46_Figure_19.jpeg)

![](_page_46_Figure_20.jpeg)

						SCALE 1:100 AT A1 S	IZE			
15	SSUED NOT TO	FOR A			INTERNAL DRIVEWAY					
	DESIGNED	CHECKED	APPROVED							
4	SM	SH								
D	GDA2020 MGA-56	SCALE AS SHO	DWN AT	A1 size	<sup>PROJECT №.</sup> S22042	CI-0360	<sup>REV.</sup>			
							<u></u>			

C BG&E Pty Limited

![](_page_47_Figure_0.jpeg)

NEWPORT MARINA AND RESIDENTIAL DEVELOPMENT

![](_page_47_Picture_5.jpeg)

STATUS ISSUED FOR APPROVAL NOT TO BE USED FOR CONSTRUCTION						THE AVENUE LONGITUDINAL			
DRAWN	DESIGNED	CHECKED	APPROVED			JECHONS	= CLVZ		
HA	SM	SH							
DATUM	GRID	SCALE				PROJECT No.	DRAWING No.	REV.	
AHD	MGA-56	AS SHO	)WN	AT	A1 size	S22042	CI-0370	C	
								C BG&E Pty Limite	

0 1 2 3 4 5 SCALE 1:100 AT A1 SIZE

EXISTING ROAD EXTENT	PROPOSED ROAD EXTENSION	PROPOSED ROAD CENTRELINE	PROPOSED KERB	PROPOSED ROAD EXTENSION	SITE BOUNDARY
DATUM 10.00	52			8 88	
	17.3	- 17.3		- 17.3	
	17.322	- 17.326		- 17.302 - 17.298 - 17.298	
0.000	4.138	5.000		9.349 9.799 10.000	12.725

### DESIGN LEVELS EXISTING LEVELS

CHAINAGE

CH 16.30

EXISTING ROAD EXTENT	18.479 PROPOSED ROAD EXTENSION	PROPOSED ROAD CENTRELINE	PROPOSED KERB - AND GUTTER	PROPOSED ROAD EXTENSION		
					<u> </u>	(
DATUM 10.00						
	- 18.479 -	- 18.498		- 18.537 - 18.497	- 18.489	
18.460	- 18.479	18.498		18.473	18.418	
0000	3.416 -	5.000		9.000 - 9.450 -	10.000	

DESIGN LEVELS EXISTING LEVELS

CHAINAGE

		REVISIONS						
REV	DATE	DESCRIPTION	RVD	REV	DATE	DESCRIPTION	RVD	
А	20.09.23	ISSUED DEVELOPMENT APPROVAL	SH					
В	29.09.23	ISSUED DEVELOPMENT APPROVAL	SH					
C	27.10.23	ISSUED FOR DEVELOPMENT APPROVAL	SH					
								CLIENT

![](_page_48_Picture_8.jpeg)

![](_page_48_Figure_10.jpeg)

CH 36.30

DESIGN LEVELS

EXISTING LEVELS

CHAINAGE

PROPOSED KERB DATUM 10.00 16.201 16.181 16.255 16.245 16.204 16.174 - 16.095 - 16.125 - 16.146 16.245 16.204 16.174 9.230 9.680 10.000 3.790 5.000 5.650

CH 26.30

![](_page_48_Figure_14.jpeg)

CH 47.779

DESIGN LEVELS

EXISTING LEVELS

CHAINAGE

![](_page_48_Picture_16.jpeg)

DESIGN LEVELS

EXISTING LEVELS

CHAINAGE

Level One, One Chifley Square Sydney NSW 2000 Australia www.scottcarver.com.au +61 2 9957 3988

Sydney Office – L2, 8 Windmill St Sydney NSW 2000 P / +61 2 9770 3300 E / info@bgeeng.com bgeeng.com—

![](_page_48_Picture_19.jpeg)

NEWPORT MARINA AND RESIDENTIAL DEVELOPMENT

![](_page_48_Picture_21.jpeg)

		PROPOSED VEHICLE CROSS	SITE BOUNDARY	
13.862	13.633	13.500		
13.862	13.655 -	13.500 -		
7.146	10.000	11.493	14.291	

SOVER

						SCALL 1.100 AT AT SIZE	
ISSUED FOR APPROVAL					THE AVENUE CROSS		
	DESIGNED	CHECKED	APPROVED				
A	SM	SH					
HD	GDA2020 MGA-56	SCALE AS SHO	DWN AT	A1 size	PROJECT No. S22042	CI-0371	REV.

C BG&E Pty Limited

![](_page_49_Figure_0.jpeg)

NEWPORT MARINA AND RESIDENTIAL DEVELOPMENT

![](_page_49_Picture_4.jpeg)

STATUS

![](_page_49_Figure_5.jpeg)

![](_page_49_Figure_6.jpeg)

### SITE BOUNDARY SURVEY LINE WORK

ROOF AREA (2438.57m<sup>2</sup>)

DRIVEWAY AREA (232.32m²)

LANDSCAPE AREA (3665.59m²)

ROAD AREA (188.82m<sup>2</sup>)

(TOTAL 6525.3m<sup>2</sup>)

### WATER QUALITY

TREATMENT NODES:

STORMFILTER CHAMBER WITH 10xOCEAN PROTECT 690mm PSORB (MCC) STORMFILTER CARTRIDGES OR EQUIVALENT PRODUCT.

TREATMENT STANDARDS									
POLLUTANT	POST(kg/yr)	REDUCTION (%)	COUNCIL REQUIREMENTS(%)						
GROSS POLLUTANT	0.0	100	90						
TOTAL SUSPENDED SOLIDS	48.9	85.2	85						
TOTAL PHOSPHORUS	0.217	77.6	65						
TOTAL NITROGEN	4.19	56.6	45						

STORMWATER DRAINAGE REQUIREMENTS HAVE BEEN CALCULATED IN ACCORDANCE WITH NORTHERN BEACHES COUNCIL'S WSUD & MUSIC MODELLING GUIDELINES.

![](_page_49_Picture_18.jpeg)

2,5 5,0 7,5 10 12.5 15m SCALE 1:250 AT A1 SIZE

ISSUED FOR APPROVAL					MUSIC CA	TCHMENT PL	AN
	DESIGNED	CHECKED	APPROVED		]		
	AM	SH					
)	GDA2020 MGA-56	scale 1:250	AT	A1 size	<sup>PROJECT №.</sup> S22042	DRAWING NO.	REV.
					•		C BG&E Pty Limited

![](_page_50_Picture_0.jpeg)

	SITE BOUNDARY
	STABILISED SITE ACCESS
-00	SEDIMENT FENCE
	SECURITY FENCE
	SUGGESTED TEMPORARY STOCKPILE LOCATION
	GROUND FLOOR ARCHITECTURAL
Constant and	MESH & GRAVEL INLET FILTER

#### EXISTING SERVICES

LEGEND

— – NBN – —	EXISTING NBN
G G	EXISTING GAS
S S	EXISTING SEWER
——————————————————————————————————————	EXISTING STORMWATER

#### NOTES

- REFER DRAWING CI-0710 FOR EROSION AND SEDIMENT CONTROL DETAILS.
- 2. CONTRACTOR TO ENSURE SITE DRAINAGE IS NOT ADVERSELY IMPACTED DURING CONSTRUCTION.
- 3. CONTRACTOR TO PROVIDE 'SANDBAG SEDIMENT TRAP' TO ALL PAVED / ROAD AREAS (BOTH PROPOSED AND EXISTING) IN ACCORDANCE WITH THE 'BLUE BOOK'.
- 4. CONTRACTOR TO PROVIDE 'GEOTEXTILE INLET FILTER TRAPS' TO ALL STORMWATER DRAINAGE INLETS (BOTH PROPOSED AND EXISTING) IN ACCORDANCE WITH THE 'BLUE BOOK'.
- 5. INSTALL AND MAINTAIN SANDBAG FILTERS ACROSS ALL PAVEMENT INTERFACES.

2.5 5.0 7.5 10 12.5 15m SCALE 1:250 AT A1 SIZE

ISSUED FOR APPROVAL					EROSION AND SEDIMENT				
	DESIGNED	CHECKED	APPROVED			CONTROL			
IA	AM	SH							
HD	GDA2020 MGA-56	scale 1:250		ат А1	SIZE	PROJECT No. S22042	DRAWING NO. CI-0700	rev.	

![](_page_51_Figure_0.jpeg)

CONTOURS OF THE SITE, BUT WITH SMALL RETURNS AS SHOWN IN THE DRAWING TO LIMIT

THE CATCHMENT AREA OF ANY ONE SECTION. THE CATCHMENT AREA SHOULD BE SMALL

ENOUGH TO LIMIT WATER FLOW IF CONCENTRATED AT ONE POINT TO 50 LITRES PER

2. CUT A 150mm DEEP TRENCH ALONG THE UPSLOPE LINE OF THE FENCE FOR THE BOTTOM

DRIVE 1.5m LONG STAR PICKETS INTO GROUND AT 2.5m INTERVALS (MAX.) AT THE

4. FIX SELF-SUPPORTING GEOTEXTILE TO THE UPSLOPE SIDE OF THE POSTS ENSURING IT

FOR SEDIMENT FENCING. THE USE OF SHADE CLOTH FOR THIS PURPOSE IS NOT

6. BACKFILL THE TRENCH OVER THE BASE OF THE FABRIC AND COMPACT IT THOROUGHLY

5. JOIN SECTIONS OF FABRIC AT A SUPPORT POST WITH A 150mm OVERLAP.

GOES TO THE BASE OF THE TRENCH. FIX THE GEOTEXTILE WITH WIRE TIES OR AS

RECOMMENDED BY THE MANUFACTURER. ONLY USE GEOTEXTILE SPECIFICALLY PRODUCED

DOWNSLOPE EDGE OF THE TRENCH. ENSURE ANY STAR PICKETS ARE FITTED WITH SAFETY

SECOND IN THE DESIGN STORM EVENT, USUALLY THE 10-YEAR EVENT.

![](_page_51_Figure_1.jpeg)

EARTH BANK—	
FLOW	2:1 SLOPE MAX

- STOCKPILE CONSTRUCTION NOTES:
- 3. WHERE THERE IS SUFFICIENT AREA, TOPSOIL STOCKPILES SHALL BE LESS THAN 2 METRES IN HEIGHT.
- 4. WHERE THEY ARE TO BE PLACE FOR MORE THAN 10 DAYS, STABILISE FOLLOWING THE APPROVED ESCP OR SWMP TO REDUCE THE C-FACTOR TO LESS THAN 0.10.
- STOCKPILES

SCALE N.T.S

SEDIMENT FENCE

OVER THE GEOTEXTILE.

SATISFACTORY.

OF THE FABRIC TO BE ENTRENCHED.

SCALE N.T.S

З.

CAPS.

								CLIENT	
								-	$\sim$
B	12 07 23		СН					-	ANPN
A	31.05.22	ISSUED FOR DEVELOPMENT APPROVAL	SH					-	
REV	DATE	DESCRIPTION	RVD	REV	DATE	DESCRIPTION	RVD	_	DEVELOP
REVISIONS				REVISIONS					
P:\BGE\S 20/07/20	YD\S22042 SIRS 23 10:57:49 AM	5I MARINA\100 DRAW\100.2 CIVIL\AUTOCAD\S22042-DRG-CI-0	)710.DWG						

![](_page_51_Picture_12.jpeg)

![](_page_51_Figure_13.jpeg)

### STABILISED SITE ACCESS - SHAKER GRID SCALE N.T.S.

![](_page_51_Figure_15.jpeg)

![](_page_51_Figure_16.jpeg)

![](_page_51_Figure_17.jpeg)

1. PLACE STOCKPILES MORE THAN 2 (PREFERABLY 5) METRES FROM EXISTING VEGETATION, CONCENTRATED WATER FLOW, ROADS AND HAZARD AREAS.

- 2. CONSTRUCT ON THE CONTOUR AS LOW, FLAT, ELONGATED MOUNDS.
- 5. CONSTRUCT EARTH BANKS ON THE UPSLOPE SIDE TO DIVERT WATER AROUND STOCKPILES AND SEDIMENT FENCES 1 TO 2 METRES DOWNSLOPE.

![](_page_51_Figure_21.jpeg)

#### GEOTEXTILE INLET FILTER CONSTRUCTION NOTES: 1. FABRICATE A SEDIMENT BARRIER MADE FROM GEOTEXTILE OR STRAW BALES.

- 2. PICKET SPACING TO BE A MAXIMUM 1.0m CENTRES.
- 3. IN WATERWAYS, ARTIFICIAL SAG POINTS CAN BE CREATED WITH SANDBAGS OR EARTH BANKS AS SHOWN IN THE DRAWING.
- 4. DO NOT COVER THE INLET WITH GEOTEXTILES UNLESS THE DESIGN IS ADEQUATE TO ALLOW FOR ALL WATERS TO BYPASS IT.

#### GEOTEXTILE INLET FILTER SCALE N.T.S

![](_page_51_Picture_27.jpeg)

CHITECT

Scottee Level One, One Chifley Square Sydney NSW 2000 Australia www.scottcarver.com.au +61 2 9957 3988

Sydney Office — L2, 8 Windmill St Sydney NSW 2000 P/+61 2 9770 3300 E/info@bgeeng.com bgeeng.com—

![](_page_51_Picture_30.jpeg)

![](_page_51_Picture_32.jpeg)

ł	1	0	0	n	۲	Ŧ	
0	8	R	5	t	١		

44

ŢŢ

- WATER

TO SUIT - GRAVEL-FILLED WIRE MESH OR GEOTEXTILE "SAUSAGE" OVERFLOW TIMBER SPACER RUNOFF WATER TO SUIT WITH SEDIMENT SEDIMENT<sup>-</sup> FILTERED WATER GRAVEL-FILLED WIRE MESH-OR GEOTEXTILE "SAUSAGE"

- KERB-SIDE INLET

MESH & GRAVEL INLET FILTER CONSTRUCTION NOTES: 1. INSTALL FILTERS TO KERB INLETS ONLY AT SAG POINTS.

TIMBER SPACER

- 2. FABRICATE A SLEEVE MADE FROM GEOTEXTILE OR WIRE MESH LONGER THAN THE LENGTH OF THE INLET PIT AND FILL IT WITH 25mm TO 50mm GRAVEL.
- 3. FORM AN ELLIPTICAL CROSS-SECTION ABOUT 150mm HIGH x 400mm WIDE.
- 4. PLACE THE FILTER AT THE OPENING LEAVING AT LEAST A 100mm SPACE BETWEEN IT AND THE KERB INLET. MAINTAIN THE OPENING WITH SPACER BLOCKS.
- 5. FORM A SEAL WITH THE KERB TO PREVENT SEDIMENT BYPASSING THE FILTER.
- 6. SANDBAGS FILLED WITH GRAVEL CAN SUBSTITUTE FOR THE MESH OR GEOTEXTILE PROVIDING THEY ARE PLACED SO THAT THEY CAN FIRMLY ABUT EACH OTHER AND SEDIMENT-LADEN WATERS CANNOT PASS BETWEEN.

MESH & GRAVEL INLET FILTER SCALE N.T.S

S ISSUED FOR APPROVAL					EROSION AND SEDIMENT		
	DESIGNED	CHECKED	APPROVED			DLIAILS	
HA	AM	SH					
\HD	GDA2020 MGA-56	scale NOT TC	SCALE	AT A1 SIZE	<sup>PROJECT №.</sup> S22042	CI-0710	REV. B
							C BG&E Pty Limited

### **Appendix C** - **Survey**

![](_page_52_Figure_1.jpeg)

![](_page_53_Figure_0.jpeg)

![](_page_54_Picture_0.jpeg)

At BG&E, we are united by a common purpose – we believe that truly great engineering takes curiosity, bravery and trust, and is the key to creating extraordinary built environments.

Our teams in Australia, New Zealand, South East Asia, the United Kingdom and the Middle East, design and deliver engineering solutions for clients in the Property, Transport, Ports and Marine, Water, Defence, Renewables and Resources sectors.

We collaborate with leading contractors, developers, architects, planners, financiers and government agencies, to create projects for today and future generations.

ABN 67 150 804 603

![](_page_55_Picture_4.jpeg)