

GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER
FORM NO. 1 – To be submitted with Development Application

Development Application for _____
Name of Applicant

Address of site 78 Powderworks Road, Narrabeen

The following checklist covers the minimum requirements to be addressed in a Geotechnical Risk Declaration made by geotechnical engineer or engineering geologist or coastal engineer (where applicable) as part of a geotechnical report

I, Ben White on behalf of White Geotechnical Group Pty Ltd
(Insert Name) (Trading or Company Name)

on this the 1/03/21 certify that I am a geotechnical engineer or engineering geologist or coastal engineer as defined by the Geotechnical Risk Management Policy for Pittwater - 2009 and I am authorised by the above organisation/company to issue this document and to certify that the organisation/company has a current professional indemnity policy of at least \$10million.

I:

Please mark appropriate box

- ☒ have prepared the detailed Geotechnical Report referenced below in accordance with the Australia Geomechanics Society's Landslide Risk Management Guidelines (AGS 2007) and the Geotechnical Risk Management Policy for Pittwater - 2009
- ☒ am willing to technically verify that the detailed Geotechnical Report referenced below has been prepared in accordance with the Australian Geomechanics Society's Landslide Risk Management Guidelines (AGS 2007) and the Geotechnical Risk Management Policy for Pittwater - 2009
- ☐ have examined the site and the proposed development in detail and have carried out a risk assessment in accordance with Section 6.0 of the Geotechnical Risk Management Policy for Pittwater - 2009. I confirm that the results of the risk assessment for the proposed development are in compliance with the Geotechnical Risk Management Policy for Pittwater - 2009 and further detailed geotechnical reporting is not required for the subject site.
- ☐ have examined the site and the proposed development/alteration in detail and I am of the opinion that the Development Application only involves Minor Development/Alteration that does not require a Geotechnical Report or Risk Assessment and hence my Report is in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009 requirements.
- ☐ have examined the site and the proposed development/alteration is separate from and is not affected by a Geotechnical Hazard and does not require a Geotechnical Report or Risk Assessment and hence my Report is in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009 requirements.
- ☐ have provided the coastal process and coastal forces analysis for inclusion in the Geotechnical Report

Geotechnical Report Details:

Report Title: Geotechnical Report 78 Powderworks Road, Narrabeen
Report Date: 01/03/21


Author: BEN WHITE

Author's Company/Organisation: WHITE GEOTECHNICAL GROUP PTY LTD

Documentation which relate to or are relied upon in report preparation:

Australian Geomechanics Society Landslide Risk Management March 2007.
White Geotechnical Group company archives.

I am aware that the above Geotechnical Report, prepared for the abovementioned site is to be submitted in support of a Development Application for this site and will be relied on by Pittwater Council as the basis for ensuring that the Geotechnical Risk Management aspects of the proposed development have been adequately addressed to achieve an "Acceptable Risk Management" level for the life of the structure, taken as at least 100 years unless otherwise stated and justified in the Report and that reasonable and practical measures have been identified to remove foreseeable risk.

Signature 
Name Ben White
Chartered Professional Status MScGEOLAusIMM CP GEOL
Membership No. 222757
Company White Geotechnical Group Pty Ltd

GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER
FORM NO. 1(a) - Checklist of Requirements for Geotechnical Risk Management Report for Development Application

| | |
|-----------------------------|---------------------------------------|
| Development Application for | _____ |
| | Name of Applicant |
| Address of site | <u>78 Powderworks Road, Narrabeen</u> |

The following checklist covers the minimum requirements to be addressed in a Geotechnical Risk Management Geotechnical Report. This checklist is to accompany the Geotechnical Report and its certification (Form No. 1).

Geotechnical Report Details:

| |
|---|
| Report Title: Geotechnical Report <u>78 Powderworks Road, Narrabeen</u> |
| Report Date: <u>1/03/2021</u> |
| Author: <u>BEN WHITE</u> |
| Author's Company/Organisation: <u>WHITE GEOTECHNICAL GROUP PTY LTD</u> |

Please mark appropriate box

- ☒ Comprehensive site mapping conducted 26/02/21
(date)
- ☒ Mapping details presented on contoured site plan with geomorphic mapping to a minimum scale of 1:200 (as appropriate)
- ☒ Subsurface investigation required
 - ☐ No Justification _____
 - ☒ Yes Date conducted 26/02/21
- ☒ Geotechnical model developed and reported as an inferred subsurface type-section
- ☒ Geotechnical hazards identified
 - ☐ Above the site
 - ☒ On the site
 - ☒ Below the site
 - ☐ Beside the site
- ☒ Geotechnical hazards described and reported
- ☒ Risk assessment conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
 - ☒ Consequence analysis
 - ☒ Frequency analysis
- ☒ Risk calculation
- ☒ Risk assessment for property conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
- ☒ Risk assessment for loss of life conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
- ☒ Assessed risks have been compared to "Acceptable Risk Management" criteria as defined in the Geotechnical Risk Management Policy for Pittwater - 2009
- ☒ Opinion has been provided that the design can achieve the "Acceptable Risk Management" criteria provided that the specified conditions are achieved.
- ☒ Design Life Adopted:
 - ☒ 100 years
 - ☐ Other _____
specify
- ☒ Geotechnical Conditions to be applied to all four phases as described in the Geotechnical Risk Management Policy for Pittwater - 2009 have been specified
- ☒ Additional action to remove risk where reasonable and practical have been identified and included in the report.
- ☐ Risk assessment within Bushfire Asset Protection Zone.

I am aware that Pittwater Council will rely on the Geotechnical Report, to which this checklist applies, as the basis for ensuring that the geotechnical risk management aspects of the proposal have been adequately addressed to achieve an "Acceptable Risk Management" level for the life of the structure, taken as at least 100 years unless otherwise stated, and justified in the Report and that reasonable and practical measures have been identified to remove foreseeable risk.



Signature

Name Ben White

Chartered Professional Status MScGEOLAusIMM CP GEOL

Membership No. 222757

Company White Geotechnical Group Pty Ltd

GEOTECHNICAL INVESTIGATION:

New Dwelling and Pool at 78 Powderworks Road, North Narrabeen

1. Proposed Development

- 1.1** Demolish existing house and shed and construct a new dwelling and pool on the property.
- 1.2** Details of the proposed development are shown on 4 drawings prepared by High Design, Job number 910 20, drawings numbered 1 to 3 and 3a, dated November 2020.

2. Site Description

- 2.1** The site was inspected on the 25th February 2021.
- 2.2** This residential property is on the uphill side of the road and has a N aspect. It is located on the upper reaches of a hillslope. The slope falls across the site at an average angle of ~11°. The slope above the property continues at gentle angles for a short period of time before encountering the crest of the slope. The grade below the property continues at moderate angles.
- 2.3** At the road frontage, the concrete driveway runs up past the house to a parking area on the uphill side of the property (Photos 1 & 2). The driveway and road are both cut into the slope. Both excavations are taken entirely through sandstone bedrock. The excavations reach a maximum height of ~3.5m (Photo 3). The majority of the cut face is through Medium Strength Sandstone that displays no significant geological defects. However, there are 3 areas of interest where several rock bolts and sprayed concrete have shored jointed and softer sections of rock (Photos 4 & 5). The existing house (photo 6) is partially supported directly on to outcropping Medium Strength Sandstone (Photo 7). The house will be demolished as part of the proposed works and the remainder of the site will be cleared.

3. Geology

The Sydney 1:100 000 Geological sheet indicates the site is underlain by Hawkesbury Sandstone. It is described as a medium to coarse grained quartz sandstone with very minor shale and laminite lenses.

4. Subsurface Investigation

Six DCP (Dynamic Cone Penetrometer) tests were put down to determine the relative density of the overlying soil and the depth to bedrock. The locations of the tests are shown on the site plan attached. It should be noted that a level of caution should be applied when interpreting DCP test results. The test will not pass through hard buried objects so in some instances it can be difficult to determine whether refusal has occurred on an obstruction in the profile or on the natural rock surface. This is not expected to be an issue for the testing on this site. However, excavation and foundation budgets should always allow for the possibility that the interpreted ground conditions in this report vary from those encountered during excavations. See the appended "Important information about your report" for a more comprehensive explanation. The results are as follows:

DCP RESULTS ON THE NEXT PAGE

| DCP TEST RESULTS – Dynamic Cone Penetrometer | | | | | | |
|---|-------------------------------|---------------------------|---------------------------|-------------------------------|-------------------------------|---------------------------|
| Equipment: 9kg hammer, 510mm drop, conical tip. | | | | Standard: AS1289.6.3.2 - 1997 | | |
| Depth(m) Blows/0.3m | DCP 1 (~RL68.2) | DCP 2 (~RL68.2) | DCP 3 (~RL67.2) | DCP 4 (~RL65.1) | DCP 5 (~RL66.7) | DCP 6 (~RL67.4) |
| 0.0 to 0.3 | 3 | 3 | 2 | 2 | 2 | 20 |
| 0.3 to 0.6 | # | 6 | 1 | 2 | 3 | 13 |
| 0.6 to 0.9 | | 17 | # | # | 10 | 9 |
| 0.9 to 1.2 | | 13 | | | 5 | # |
| 1.2 to 1.5 | | 8 | | | 12 | |
| 1.5 to 1.8 | | # | | | 12 | |
| 1.8 to 2.1 | | | | | 10 | |
| 2.1 to 2.4 | | | | | # | |
| | Refusal on rock @ 0.15m | Refusal on rock @ 1.3m | Refusal on rock @ 0.4m | Refusal on rock @ 0.5m | Refusal on rock @ 2.0 m | Refusal on rock @ 0.8m |

#refusal/end of test. F=DCP fell after being struck showing little resistance through all or part of the interval.

DCP Notes:

DCP1 – Refusal on rock @ 0.15m, DCP bouncing off rock surface, dark brown soil on damp tip.

DCP2 – Refusal on rock @ 1.3m, DCP bouncing off rock surface, maroon sandstone fragments on dry tip.

DCP3 – Refusal on rock @ 0.8m, DCP bouncing off rock surface, clean dry tip.

DCP4 – Refusal on rock @ 0.5m, DCP bouncing off rock surface, white impact dust on dry tip.

DCP5 – Refusal on rock @ 2.0m, DCP bouncing off rock surface, brown and maroon sandstone fragments on damp tip.

DCP6 – Refusal on rock @ 0.8m, DCP bouncing off rock surface, white impact dust on dry tip.

5. Geological Observations/Interpretation

The surface features of the block are controlled by the outcropping and underlying sandstone bedrock that steps down the property forming sub-horizontal benches between the steps. Where the grade is steeper, the steps are larger and the benches narrower. Where the slope eases, the opposite is true. Where the rock is not exposed, it is overlain by fill and natural soil.

Filling has been placed below the house for landscaping. In the test locations, the depth to rock ranged between 0.15 to 2.0m below the current surface, being slightly deeper due to the presence of fill and due to the stepped nature of the underlying bedrock. The outcropping sandstone on the property is estimated to be Medium Strength or better and similar strength rock is expected to underlie the entire site. See Type Section attached for a diagrammatical representation of the expected ground materials.

6. Groundwater

Normal ground water seepage is expected to move over the buried surface of the rock and through the cracks. Due to the slope and elevation of the block, the water table is expected to be many metres below the base of the proposed works.

7. Surface Water

No evidence of surface flows was observed on the property during the inspection. It is expected that there will be minimal to no sheet wash as the site is near the crest of the hill.

8. Geotechnical Hazards and Risk Analysis

No current geotechnical hazards were observed beside the property. The gentle to steeply graded land surface that falls across the property and continues below is a potential hazard (**Hazard One**). The cut face that rises ~3.5m above the road is a potential hazard (**Hazard two**).

RISK ANALYSIS SUMMARY ON NEXT PAGE

Risk Analysis Summary

| HAZARDS | Hazard One | Hazard Two |
|--------------------------|---|--|
| TYPE | The gentle to steep slope that rises across the property impacting on the proposed works. | The sandstone face that drops ~3.5m to the road below failing and impacting on the road. |
| LIKELIHOOD | 'Unlikely' (10^{-4}) | 'Unlikely' (10^{-4}) |
| CONSEQUENCES TO PROPERTY | 'Medium' (15%) | 'Medium' (25%) |
| RISK TO PROPERTY | 'Low' (2×10^{-5}) | 'Low' (2×10^{-5}) |
| RISK TO LIFE | 5.5×10^{-7} /annum | 2.9×10^{-7} /annum |
| COMMENTS | This level of risk is 'ACCEPTABLE'. | This level of risk is 'ACCEPTABLE'. |

(See Aust. Geomech. Jnl. Mar 2007 Vol. 42 No 1, for full explanation of terms)

9. Suitability of the Proposed Development for the Site

The proposed development is suitable for the site. No geotechnical hazards will be created by the completion of the proposed development provided it is carried out in accordance with the requirements of this report and good engineering and building practice.

10. Stormwater

The fall is to Powderworks Road. Roof water from the development is to be piped to the street drainage system through any tanks that may be required by the regulating authorities.

11. Excavations

Apart from those for footings, no excavations are required.

12. Foundations

Spread footings or piers supported off Medium Strength Sandstone are suitable footings for the proposed dwelling and pool. This material is expected to be exposed at the current surface or is expected at a depth of between ~0.1 and ~2.0m below the current surface. Where footings are over an exposed sloping rock surface, they may be supported off level pads cut

into the rock. Assume a maximum allowable bearing pressure of 1000kPa for footings supported off Medium Strength Sandstone.

Naturally occurring vertical cracks (known as joints) commonly occur in sandstone. These are generally filled with soil and are the natural seepage paths through the rock. They can extend to depths of several metres and are usually relatively narrow but can range between 0.1 to 0.8m wide. If a footing falls over a joint in the rock, the construction process is simplified if, with the approval of the structural engineer, the joint can be spanned or alternatively the footing can be repositioned so it does not fall over the joint.

NOTE: If the contractor is unsure of the footing material required, it is more cost-effective to get the geotechnical consultant on site at the start of the footing excavation to advise on footing depth and material. This mostly prevents unnecessary over-excavation in clay-like shaly-rock but can be valuable in all types of geology.

13. Inspections

The client and builder are to familiarise themselves with the following required inspections as well as council geotechnical policy. We cannot provide geotechnical certification for the owner or the regulating authorities if the following inspections have not been carried out during the construction process.

- All footings are to be inspected and approved by the geotechnical consultant while the excavation equipment is still onsite and before steel reinforcing is placed or concrete is poured.

White Geotechnical Group Pty Ltd.



Ben White M.Sc. Geol.,
AusIMM., CP GEOL.
No. 222757
Engineering Geologist



Photo 1



Photo 2



Photo 3

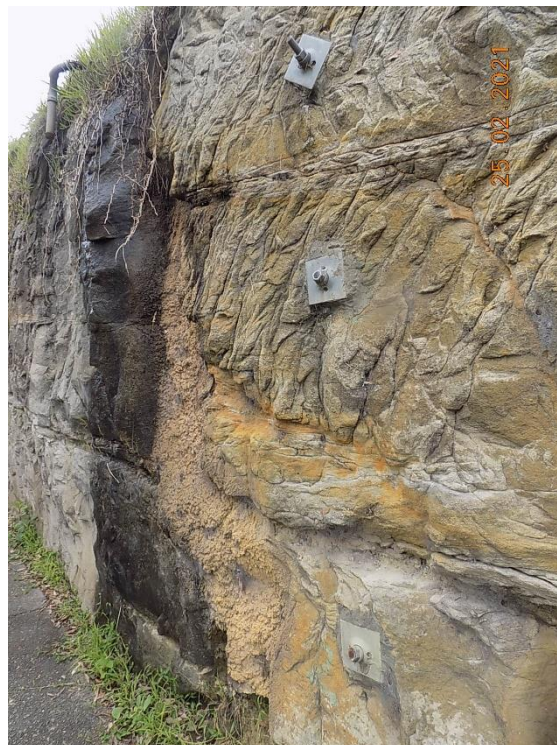


Photo 4



Photo 5



Photo 6



Photo 7



Photo 8

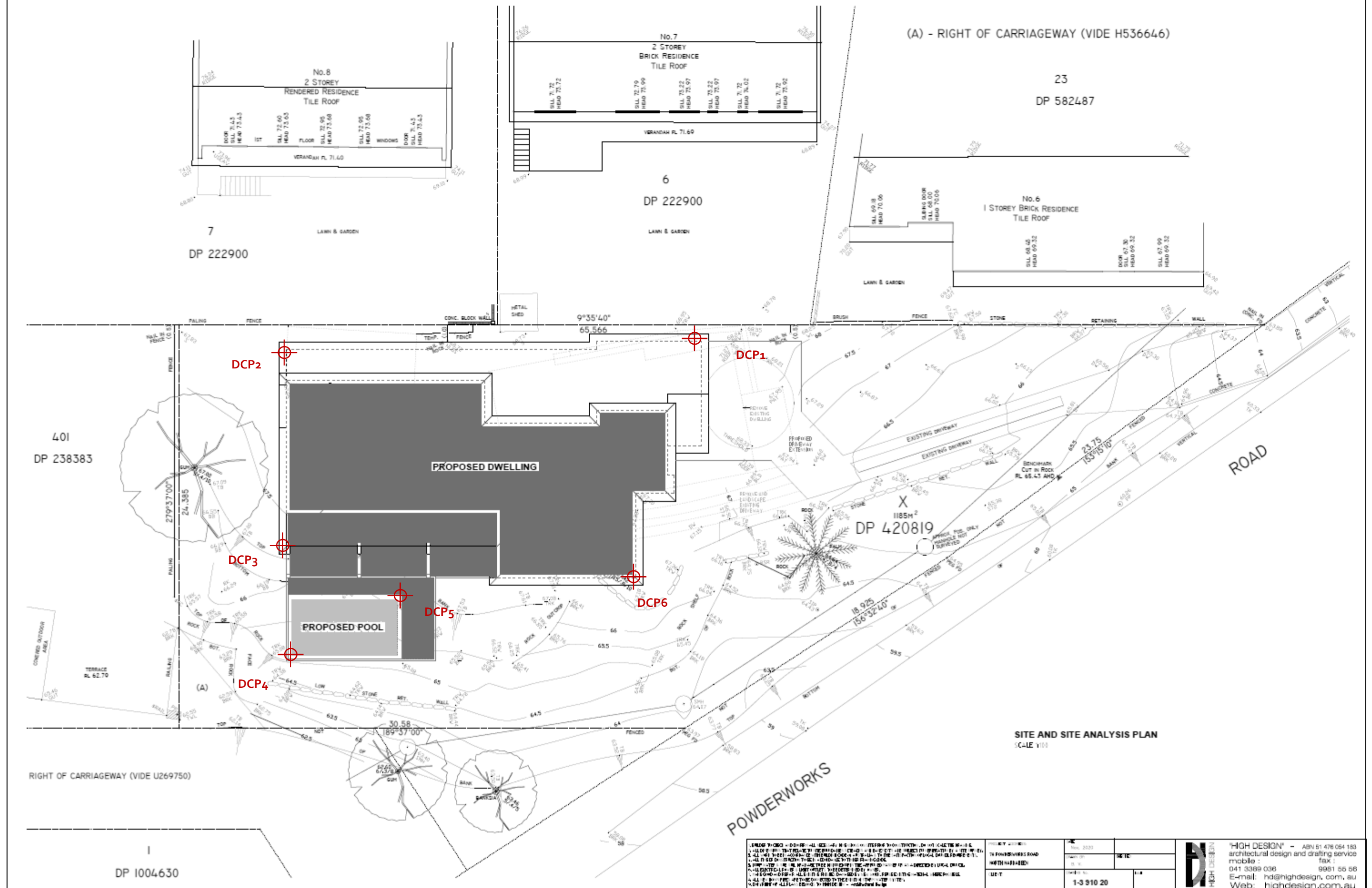
Important Information about Your Report

It should be noted that Geotechnical Reports are documents that build a picture of the subsurface conditions from the observation of surface features and testing carried out at specific points on the site. The spacing and location of the test points can be limited by the location of existing structures on the site or by budget and time constraints of the client. Additionally, the test themselves, although chosen for their suitability for the particular project, have their own limiting factors. The testing gives accurate information at the location of the test, within the confines of the test's capability. A geological interpretation or model is developed by joining these test points using all available data and drawing on previous experience of the geotechnical consultant. Even the most experienced practitioners cannot determine every possible feature or change that may lie below the earth. All of the subsurface features can only be known when they are revealed by excavation. As such, a Geotechnical report can be considered an interpretive document. It is based on factual data but also on opinion and judgement that comes with a level of uncertainty. This information is provided to help explain the nature and limitations of your report.

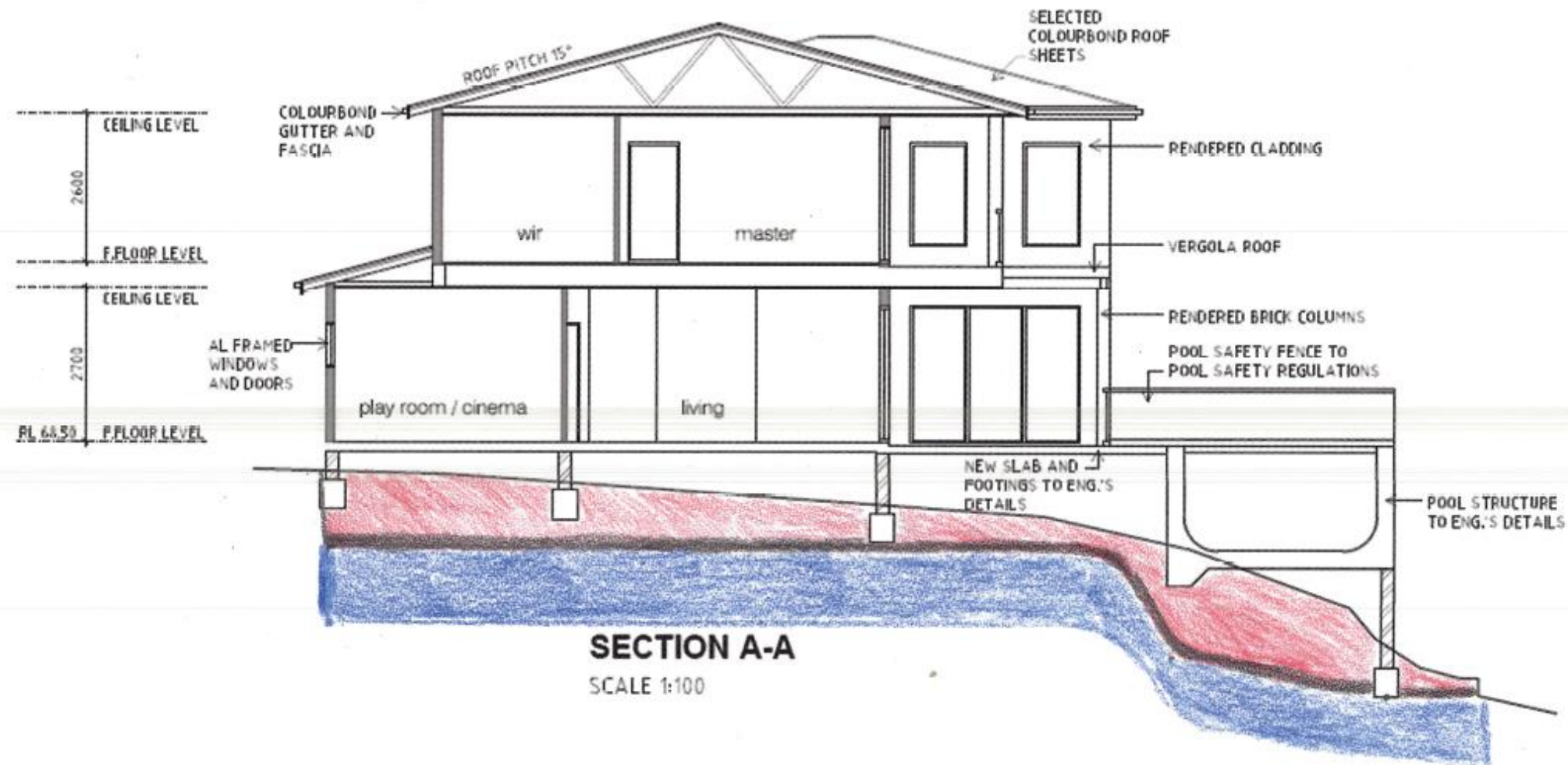
With this in mind, the following points are to be noted:

- If upon the commencement of the works the subsurface ground or ground water conditions prove different from those described in this report, it is advisable to contact White Geotechnical Group immediately, as problems relating to the ground works phase of construction are far easier and less costly to overcome if they are addressed early.
- If this report is used by other professionals during the design or construction process, any questions should be directed to White Geotechnical Group as only we understand the full methodology behind the report's conclusions.
- The report addresses issues relating to your specific design and site. If the proposed project design changes, aspects of the report may no longer apply. Contact White Geotechnical if this occurs.
- This report should not be applied to any other project other than that outlined in section 1.0.
- This report is to be read in full and should not have sections removed or included in other documents as this can result in misinterpretation of the data by others.
- It is common for the design and construction process to be adapted as it progresses (sometimes to suit the previous experience of the contractors involved). If alternative design and construction processes are required to those described in this report, contact White Geotechnical Group. We are familiar with a variety of techniques to reduce risk and can advise if your proposed methods are suitable for the site conditions.

SITE PLAN – showing test locations



TYPE SECTION – Diagrammatical Interpretation of expected Ground Materials



- Fill
- Topsoil
- Hawkesbury Sandstone – Medium Strength

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NORTH HAVESIDE
VIC 3102

DATE: 2020
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CHECKED BY: H.D.
SCALE: 1:3 910 20



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EXAMPLES OF **GOOD** HILLSIDE PRACTICE



EXAMPLES OF **POOR** HILLSIDE PRACTICE

