

**GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER**  
**FORM NO. 1 – To be submitted with Development Application**

Development Application for \_\_\_\_\_  
Name of Applicant

Address of site 9 Wakooka Avenue, Elanora Heights

*The following checklist covers the minimum requirements to be addressed in a Geotechnical Risk Declaration made by geotechnical engineer or engineering geologist or coastal engineer (where applicable) as part of a geotechnical report*

I, Ben White on behalf of White Geotechnical Group Pty Ltd  
(Insert Name) (Trading or Company Name)

on this the 26/6/19 certify that I am a geotechnical engineer or engineering geologist or coastal engineer as defined by the Geotechnical Risk Management Policy for Pittwater - 2009 and I am authorised by the above organisation/company to issue this document and to certify that the organisation/company has a current professional indemnity policy of at least \$10million.

I:

**Please mark appropriate box**

- ☒ have prepared the detailed Geotechnical Report referenced below in accordance with the Australia Geomechanics Society's Landslide Risk Management Guidelines (AGS 2007) and the Geotechnical Risk Management Policy for Pittwater - 2009
- ☒ am willing to technically verify that the detailed Geotechnical Report referenced below has been prepared in accordance with the Australian Geomechanics Society's Landslide Risk Management Guidelines (AGS 2007) and the Geotechnical Risk Management Policy for Pittwater - 2009
- ☐ have examined the site and the proposed development in detail and have carried out a risk assessment in accordance with Section 6.0 of the Geotechnical Risk Management Policy for Pittwater - 2009. I confirm that the results of the risk assessment for the proposed development are in compliance with the Geotechnical Risk Management Policy for Pittwater - 2009 and further detailed geotechnical reporting is not required for the subject site.
- ☐ have examined the site and the proposed development/alteration in detail and I am of the opinion that the Development Application only involves Minor Development/Alteration that does not require a Geotechnical Report or Risk Assessment and hence my Report is in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009 requirements.
- ☐ have examined the site and the proposed development/alteration is separate from and is not affected by a Geotechnical Hazard and does not require a Geotechnical Report or Risk Assessment and hence my Report is in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009 requirements.
- ☐ have provided the coastal process and coastal forces analysis for inclusion in the Geotechnical Report

**Geotechnical Report Details:**

Report Title: Geotechnical Report 9 Wakooka Avenue, Elanora Heights

Report Date: 25/6/19

Author: BEN WHITE

Author's Company/Organisation: WHITE GEOTECHNICAL GROUP PTY LTD

**Documentation which relate to or are relied upon in report preparation:**

Australian Geomechanics Society Landslide Risk Management March 2007.

White Geotechnical Group company archives.

I am aware that the above Geotechnical Report, prepared for the abovementioned site is to be submitted in support of a Development Application for this site and will be relied on by Pittwater Council as the basis for ensuring that the Geotechnical Risk Management aspects of the proposed development have been adequately addressed to achieve an "Acceptable Risk Management" level for the life of the structure, taken as at least 100 years unless otherwise stated and justified in the Report and that reasonable and practical measures have been identified to remove foreseeable risk.

Signature



Name

Ben White

Chartered Professional Status

MScGEOLAusIMM CP GEOL

Membership No.

222757

Company

White Geotechnical Group Pty Ltd

**GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER**  
**FORM NO. 1(a) - Checklist of Requirements for Geotechnical Risk Management Report for Development Application**

Development Application for	_____
	Name of Applicant
Address of site	<u>9 Wakooka Avenue, Elanora Heights</u>

The following checklist covers the minimum requirements to be addressed in a Geotechnical Risk Management Geotechnical Report. This checklist is to accompany the Geotechnical Report and its certification (Form No. 1).

**Geotechnical Report Details:**

Report Title: Geotechnical Report <u>9 Wakooka Avenue, Elanora Heights</u>
Report Date: <u>25/6/19</u>
Author: <u>BEN WHITE</u>
Author's Company/Organisation: <u>WHITE GEOTECHNICAL GROUP PTY LTD</u>

**Please mark appropriate box**

- ☒ Comprehensive site mapping conducted 20/6/19  
(date)
- ☒ Mapping details presented on contoured site plan with geomorphic mapping to a minimum scale of 1:200 (as appropriate)
- ☒ Subsurface investigation required
  - ☐ No Justification \_\_\_\_\_
  - ☒ Yes Date conducted 20/6/19
- ☒ Geotechnical model developed and reported as an inferred subsurface type-section
- ☒ Geotechnical hazards identified
  - ☒ Above the site
  - ☒ On the site
  - ☒ Below the site
  - ☐ Beside the site
- ☒ Geotechnical hazards described and reported
- ☒ Risk assessment conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
  - ☒ Consequence analysis
  - ☒ Frequency analysis
- ☒ Risk calculation
- ☒ Risk assessment for property conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
- ☒ Risk assessment for loss of life conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
- ☒ Assessed risks have been compared to "Acceptable Risk Management" criteria as defined in the Geotechnical Risk Management Policy for Pittwater - 2009
- ☒ Opinion has been provided that the design can achieve the "Acceptable Risk Management" criteria provided that the specified conditions are achieved.
- ☒ Design Life Adopted:
  - ☒ 100 years
  - ☐ Other \_\_\_\_\_  
specify
- ☒ Geotechnical Conditions to be applied to all four phases as described in the Geotechnical Risk Management Policy for Pittwater - 2009 have been specified
- ☒ Additional action to remove risk where reasonable and practical have been identified and included in the report.
- ☐ Risk assessment within Bushfire Asset Protection Zone.

I am aware that Pittwater Council will rely on the Geotechnical Report, to which this checklist applies, as the basis for ensuring that the geotechnical risk management aspects of the proposal have been adequately addressed to achieve an "Acceptable Risk Management" level for the life of the structure, taken as at least 100 years unless otherwise stated, and justified in the Report and that reasonable and practical measures have been identified to remove foreseeable risk.

  
\_\_\_\_\_  
Signature

\_\_\_\_\_  
Name Ben White

\_\_\_\_\_  
Chartered Professional Status MScGEOLAusIMM CP GEOL

\_\_\_\_\_  
Membership No. 222757

\_\_\_\_\_  
Company White Geotechnical Group Pty Ltd

## **GEOTECHNICAL INVESTIGATION:**

### **New Secondary Dwelling at 9 Wakooka Avenue, Elanora Heights**

#### **1. Proposed Development**

- 1.1** Construct a new secondary dwelling on the W side of the property.
- 1.2** Details of the proposed development are shown on 5 drawings prepared by Envirotecture, Project number K33, drawings numbered 11.01, 21.01, 21.02, 30.01, and 40.01, Issue 1, dated 13/2/19.

#### **2. Site Description**

- 2.1** The site was inspected on the 20<sup>th</sup> June, 2019.
- 2.2** This residential property has a SE aspect in the location of the proposed works. It is located on the gently graded middle reaches of a hillslope. The slope falls from the road frontage to a creek that cuts the property from N to S. The slope to the E of the creek begins to rise gently again. The creek encounters a ~4.0m high waterfall that is located near the S boundary. The slopes above the property gradually increases in grade. The slope below the property continues at moderate to steep angles.
- 2.3** At the road frontage, a concrete driveway runs down the slope to a bridge over a creek channel that cuts through the property from N to S (Photos 1 & 2). On the W side of the creek, the driveway diverts to a stable masonry garage (Photo 3). Between the road frontage and the garage is a gentle, moderately vegetated slope (Photo 4). The bed of the creek consists of stable, competent Medium Strength Sandstone (Photo 7). The creek encounters a waterfall that flows over a ~4.0m high rock face that is close to the S boundary (Photo 5). The rock face is undercut ~5.0m at the base. However, the rock face is thickly bedded and the undercut is bridged at both ends by competent rock. Thus, we consider the undercut to currently be stable. Filling has been placed on both banks of the creek to level the property. The fills are supported

by stable sandstone block retaining walls (Photos 6 & 7). A pool has been constructed on the E side of the creek (Photo 8). No signs of movement were observed in the concrete shell of the pool. The single-storey brick house is supported on brick walls (Photo 9). No significant signs of movement were observed in the supporting brick walls. An excavation has been made in the slope to create a level platform for the house. The cut is supported by a brick retaining wall reaching ~1.2m high (Photo 10). This wall was observed to be directly supported off an outcrop of competent Medium Strength Sandstone bedrock.

### **3. Geology**

The Sydney 1:100 000 Geological sheet indicates the site is underlain by Hawkesbury Sandstone. It is described as a medium to coarse grained quartz sandstone with very minor shale and laminite lenses.

### **4. Subsurface Investigation**

Four Dynamic Cone Penetrometer (DCP) tests were put down to determine the relative density of the overlying soil and the depth to bedrock. The locations of the tests are shown on the site plan. It should be noted that a level of caution should be applied when interpreting DCP test results. The test will not pass through hard buried objects so in some instances it can be difficult to determine whether refusal has occurred on an obstruction in the profile or on the natural rock surface. This is not expected to be an issue for the testing on this site and the results are as follows:

## **DCP RESULTS ON THE NEXT PAGE**

DCP TEST RESULTS – Dynamic Cone Penetrometer				
Equipment: 9kg hammer, 510mm drop, conical tip.			Standard: AS1289.6.3.2 - 1997	
Depth(m) Blows/0.3m	DCP 1 (~RL56.5)	DCP 2 (~RL56.5)	DCP 3 (~RL55.4)	DCP 4 (~RL55.3)
0.0 to 0.3	11	2	10	2
0.3 to 0.6	20	7	12	30
0.6 to 0.9	15	#	22	23
0.9 to 1.2	32		23	15
1.2 to 1.5	13		#	#
1.5 to 1.8	#			
	Refusal on Rock @ 1.3m	Refusal on Rock @ 0.4m	Refusal on Rock @ 1.1m	Refusal on Rock @ 1.0m

#refusal/end of test. F = DCP fell after being struck showing little resistance through all or part of the interval.

#### DCP Notes:

DCP1 – Refusal on rock @ 1.3m, DCP bouncing off rock surface, clean dry tip.

DCP2 – Refusal on rock @ 0.4m, DCP bouncing off rock surface, white impact dust on dry tip.

DCP3 – Refusal on rock @ 1.1m, DCP bouncing off rock surface, white impact dust on dry tip.

DCP4 – Refusal on rock @ 1.0m, DCP bouncing off rock surface, white impact dust on dry tip.

## 5. Geological Observations/Interpretation

The surface features of the block are controlled by the outcropping and underlying sandstone bedrock that steps down the property forming sub-horizontal benches between the steps. Where the grade is steeper, the steps are larger, and the benches narrower. Where the slope eases, the opposite is true. The rock is overlain by sandy soils and firm to stiff sandy clays that fill the bench-step formation. In the test locations, the depth to Medium Strength Sandstone ranged between 0.4 to 1.3m below the current surface, being deeper due to the stepped nature of the underlying rock. See Type Section attached for a diagrammatical representation of the expected ground materials.

## 6. Groundwater

Ground water seepage is expected to move over the buried surface of the rock and through the cracks. As a creek flows down the centre of the block (Photo 7), we expect groundwater seepage to be slightly higher across the block as slope seepage will move toward the creek. Due to the slope and elevation of the block, the water table is expected to be many metres below the base of the proposed excavation.

## 7. Surface Water

Apart from the flowing creek down the centre of the site (Photo 7), no evidence of surface flows were observed on the property during the inspection. It is expected that normal sheet wash will move onto the site from above the property during heavy down pours.

## 8. Geotechnical Hazards and Risk Analysis

No geotechnical hazards were observed beside the property. The gently graded slope that falls across the property and continues above and below is a potential hazard (**Hazard One**). The undercut rock face below the property is a potential hazard (**Hazard Two**).

### Risk Analysis Summary

HAZARDS	Hazard One	Hazard Two
TYPE	The gentle slope that falls across the site and continues above and below failing and impacting on the proposed works.	The undercut rock face failing and impacting on the subject property (Photo 5).
LIKELIHOOD	'Unlikely' ( $10^{-4}$ )	'Rare' ( $10^{-5}$ )
CONSEQUENCES TO PROPERTY	'Medium' (12%)	'Major' (60%)
RISK TO PROPERTY	'Low' ( $2 \times 10^{-5}$ )	'Low' ( $6 \times 10^{-5}$ )
RISK TO LIFE	$5.5 \times 10^{-7}$ /annum	$8.3 \times 10^{-7}$ /annum
COMMENTS	This level of risk is 'ACCEPTABLE'.	This level of risk is 'ACCEPTABLE'.

(See Aust. Geomech. Jnl. Mar 2007 Vol. 42 No 1, for full explanation of terms)

## 9. Suitability of the Proposed Development for the Site

The proposed development is suitable for the site. No geotechnical hazards will be created by the completion of the proposed development provided it is carried out in accordance with the requirements of this report and good engineering and building practice.

## 10. Stormwater

There is fall to the creek that cuts through the property (Photo 7). Roof water from the development is to be piped to the creek through any tanks that may be required by the regulating authorities.

## 11. Excavations

Apart from those for footings, no excavations are required.

## 12. Foundations

The proposed secondary dwelling is to be supported on piers taken to the underlying Medium Strength Sandstone. This material is expected at depths of between 0.4 to 1.3m below the current ground level. A maximum allowable bearing pressure of 800kPa can be assumed for footings on Medium Strength Sandstone.

Naturally occurring vertical cracks (known as joints) commonly occur in sandstone. These are generally filled with soil and are the natural seepage paths through the rock. They can extend to depths of several metres and are usually relatively narrow but can range between 0.1 to 0.8m wide. If a footing falls over a joint in the rock, the construction process is simplified if with the approval of the structural engineer the joint can be spanned or alternatively the footing can be repositioned so it does not fall over the joint.

**NOTE:** If the contractor is unsure of the footing material required, it is more cost-effective to get the geotechnical consultant on site at the start of the footing excavation to advise on footing depth and material. This mostly prevents unnecessary over-excavation in clay-like shaly-rock but can be valuable in all types of geology.

## 13. Inspections

The client and builder are to familiarise themselves with the following required inspections as well as council geotechnical policy. We cannot provide geotechnical certification for the owner or the regulating authorities if the following inspections have not been carried out during the construction process.

- All footings are to be inspected and approved by the geotechnical consultant while the excavation equipment is still onsite and before steel reinforcing is placed or concrete is poured.

White Geotechnical Group Pty Ltd.



Ben White M.Sc. Geol.,  
AusIMM., CP GEOL.  
No. 222757  
Engineering Geologist





Photo 1



Photo 2





Photo 3

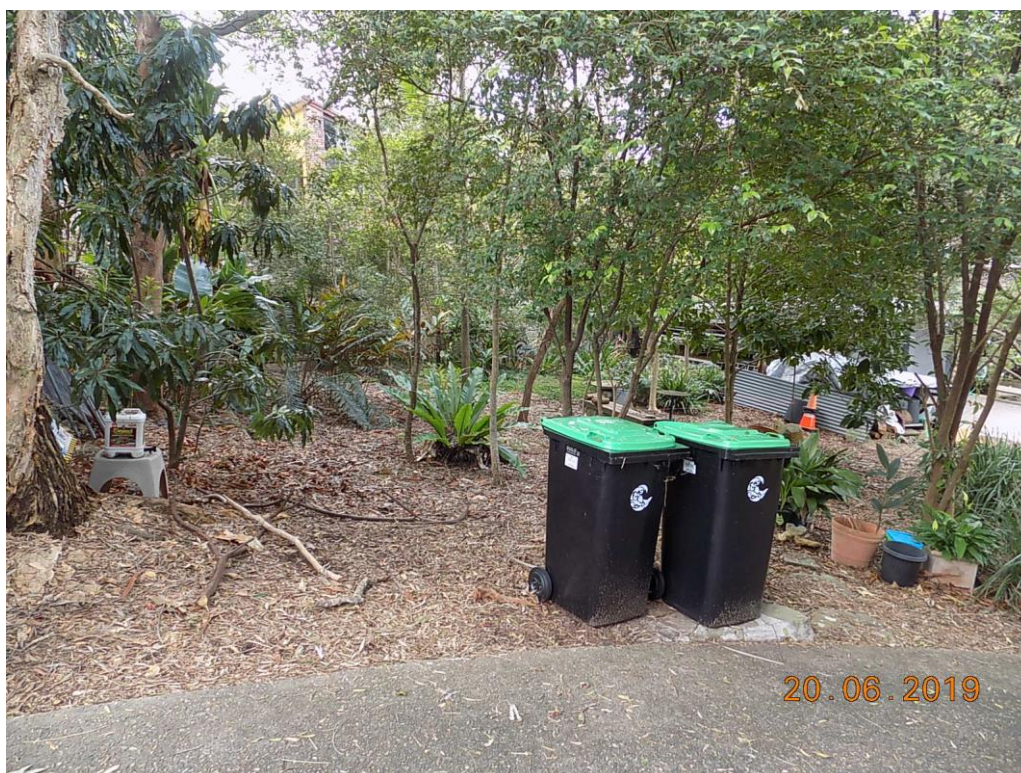


Photo 4





Photo 5



Photo 6





Photo 7



Photo 8





Photo 9



Photo 10

## Important Information about Your Report

It should be noted that Geotechnical Reports are documents that build a picture of the subsurface conditions from the observation of surface features and testing carried out at specific points on the site. The spacing and location of the test points can be limited by the location of existing structures on the site or by budget and time constraints of the client. Additionally, the test themselves, although chosen for their suitability for the particular project, have their own limiting factors. The testing gives accurate information at the location of the test, within the confines of the test's capability. A geological interpretation or model is developed by joining these test points using all available data and drawing on previous experience of the geotechnical consultant. Even the most experienced practitioners cannot determine every possible feature or change that may lie below the earth. All of the subsurface features can only be known when they are revealed by excavation. As such, a Geotechnical report can be considered an interpretive document. It is based on factual data but also on opinion and judgement that comes with a level of uncertainty. This information is provided to help explain the nature and limitations of your report.

With this in mind, the following points are to be noted:

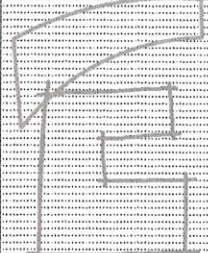

- If upon the commencement of the works the subsurface ground or ground water conditions prove different from those described in this report, it is advisable to contact White Geotechnical Group immediately, as problems relating to the ground works phase of construction are far easier and less costly to overcome if they are addressed early.
- If this report is used by other professionals during the design or construction process, any questions should be directed to White Geotechnical Group as only we understand the full methodology behind the report's conclusions.
- The report addresses issues relating to your specific design and site. If the proposed project design changes, aspects of the report may no longer apply. Contact White Geotechnical if this occurs.
- This report should not be applied to any other project other than that outlined in section 1.0.
- This report is to be read in full and should not have sections removed or included in other documents as this can result in misinterpretation of the data by others.
- It is common for the design and construction process to be adapted as it progresses (sometimes to suit the previous experience of the contractors involved). If alternative design and construction processes are required to those described in this report, contact White Geotechnical Group. We are familiar with a variety of techniques to reduce risk and can advise if your proposed methods are suitable for the site conditions.



D.P. 43271



- BUILDING TO INCORPORATE BASIX COMMITMENTS TO COMPLY WITH THE ATTACHED BASIX CERTIFICATE NO. 9828295 DATED 13 FEBRUARY 2019
- SMOKE ALARMS TO BE INSTALLED IN ACCORDANCE WITH AS 3786-1993 'SMOKE ALARMS' AND PART 3.7.2 - 'SMOKE ALARMS' OF THE BCA (NB. SMOKE ALARMS TO BE INTERCONNECTED WHERE THERE IS MORE THAN ONE ALARM)
- TERMITE MANAGEMENT TO COMPLY WITH AS3660-2000 'TERMITE MANAGEMENT - NEW BUILDING WORK'
- GLAZING TO COMPLY WITH AS1288-2006 'GLASS IN BUILDINGS - SELECTION AND INSTALLATION' AND AS 2047-1999 'WINDOWS IN BUILDINGS - SELECTION AND INSTALLATION'
- WATERPROOFING OF WET AREAS TO COMPLY WITH AS3740 'WATERPROOFING OF WET AREAS IN RESIDENTIAL BUILDINGS'. NO AIR DRIED LIQUID APPLIED TO MEMBRANES SHALL BE USED.
- ALL HOT WATER PIPES SHOULD BE INSULATED AS PER AS3500.4
- ALL REQUIRED FACILITIES FOR A CLASS 1 BUILDING TO BE INSTALLED AS REQUIRED BY PART 3.8.2.2 'REQUIRED FACILITIES' OF THE BCA
- DOORS TO FULLY ENCLOSED SANITARY COMPARTMENTS TO COMPLY WITH PART 3.8.3 'FACILITIES' OF THE BCA
- STAIR CONSTRUCTION TO COMPLY WITH PART 3.9.1 - 'STAIR CONSTRUCTION OF THE BCA (NB. ALL STAIR TREADS TO HAVE A SURFACE THAT IS SLIP RESISTANT IN ACCORDANCE WITH PART 3.9.1.3 OF THE BCA
- BALUSTRADES CONSTRUCTION TO COMPLY WITH PART 3.9.2.3 - 'BALUSTRADES' OF THE BCA
- ALL NEW OPENABLE WINDOWS WITHIN A BEDROOM WITH A FLOOR LEVEL 2M OR MORE ABOVE A SURFACE BENEATH TO BE PROTECTED IN ACCORDANCE WITH PART 3.9.2.5 OF THE BCA
- DAMP PROOF MEMBRANE MUST BE 'HIGH IMPACT', 0.2mm THICK POLYETHYLENE FILM
- ALL BUILDING WORK TO BE LOCATED WHOLLY WITHIN THE ALLOTMENT BOUNDARIES

	BSCE	DATE	02/0	FOR	PA	PROJECT #			
	PROJECT	NEW SECONDARY DWELLING						K33	
	ADDRESS	9 WAKOOKA AVE, ELANORA HEIGHTS							
	CLIENT	ANTJE KUEHNAST & MARCUS BISPING					DATE	2018-11-06	
							DWG #	A31101	
							SCALE	A5 SHOWN @ A3	
DWG	SITE PLAN					DRAWN	LP	DATE	1
						SHAD	AM		
14 KALANG ROAD, ELANORA HTS NSW 2111 OFFICE 02 9373 3997 <a href="http://www.envirolecture.com.au">www.envirolecture.com.au</a> envirolecture projects pty ltd ABN 49 078 853 572						ACCREDITED BUILDING DESIGNER DICK CLARKE #6008  NOMINATED ARCHITECT ANDY MASON #9597			



# TYPE SECTION – Diagrammatical Interpretation of expected Ground Materials

RIDGE RL60.72

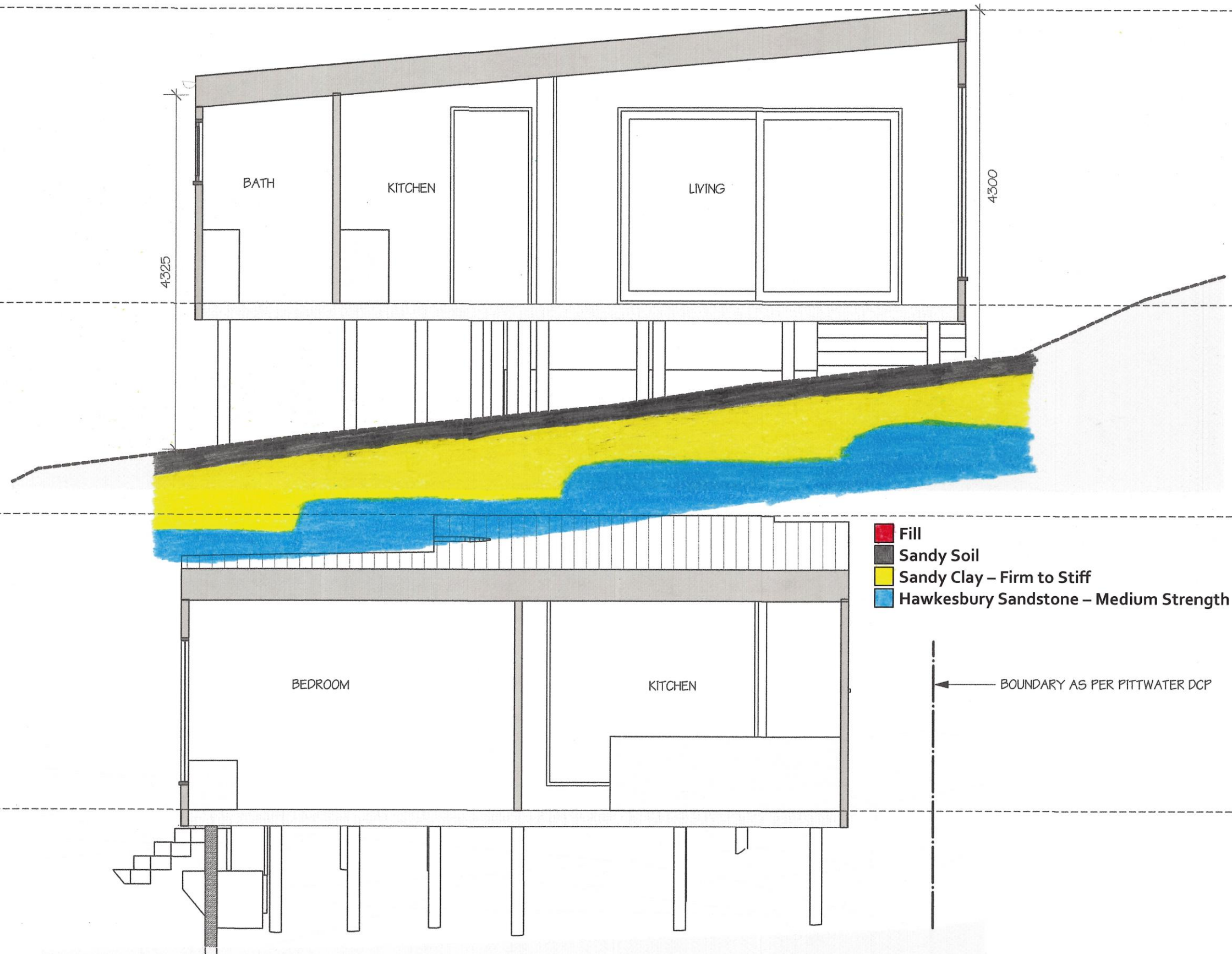
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1 SECTION 1  
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RIDGE RL60.72

FFL 57.10

2 SECTION 2  
1:50



- Fill
- Sandy Soil
- Sandy Clay – Firm to Stiff
- Hawkesbury Sandstone – Medium Strength

BOUNDARY AS PER PITTWATER DCP

## LEGEND

ROOF: PROVIDE COLORBOND ROOF SHEETING OVER TIMBER BATTENS AND COUNTERBATTENS WITH BREATHABLE MEMBRANE AS SPECIFIED. 100MM PIR BOARD FIXED FOIL FACE DOWN TO TOP OF PURLINS. LINE INTERNALLY AS SPECIFIED.

EXTERNAL WALLS: 90MM STUDS TO AS1684 WITH R2.5 INSULATION AND BREATHABLE MEMBRANE. BATTEN AND CLAD. LINE INTERNALLY WITH PLASTERBOARD (OR FC TO WET AREAS).

INTERNAL WALLS: TIMBER STUD FRAME 90mm THICK; CLAD EACH SIDE WITH PLASTERBOARD (OR FC SHEET IN WET AREAS). R2 QUIETSTUFF.

FLOOR: TIMBER BEARERS AND JOISTS TO ENGINEERS DETAIL. FLOORING AS SPECIFIED. INSULATE WITH R3 POLYESTER BATTS AS SPECIFIED.

JOINERY

ISSUE	DATE	FOR	PROJECT
1	08/08	FOR PA	NEW SECONDARY DWELLING
2	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
3	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
4	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
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73	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
74	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
75	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
76	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
77	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
78	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
79	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
80	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
81	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
82	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
83	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
84	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
85	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
86	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
87	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
88	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
89	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
90	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
91	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
92	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
93	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
94	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
95	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
96	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
97	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
98	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
99	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS
100	08/08	FOR PA	9 WAKOOKA AVE, ELANORA HEIGHTS



# EXAMPLES OF **GOOD** HILLSIDE PRACTICE



# EXAMPLES OF **POOR** HILLSIDE PRACTICE

