

# Overland Flow Assessment Report

49 Careel Head Road, Avalon

Job no. 200808

## Issue A

20 April 2022

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Prepared by: Hannah Stubley

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## 1. Summary

### 1.1 Site Characteristics

The 1,346m<sup>2</sup> residential site is located on the upper southern side of Careel Head Road in Avalon. The development site is bounded by residential buildings on the southern, eastern and western boundaries and naturally falls in a north-western trajectory towards Careel Head Road. There exists two localised sag points within Whale Beach Road above. Two grated kerb inlet pits are located at these sag points which form part of Council's stormwater drainage infrastructure. The two pits are the primary stormwater runoff collection points on the upper and lower side of Whale Beach Road which convey collected stormwater runoff towards Careel Creek (refer figure 2 for further detail).

### 1.2 Proposed Development Works

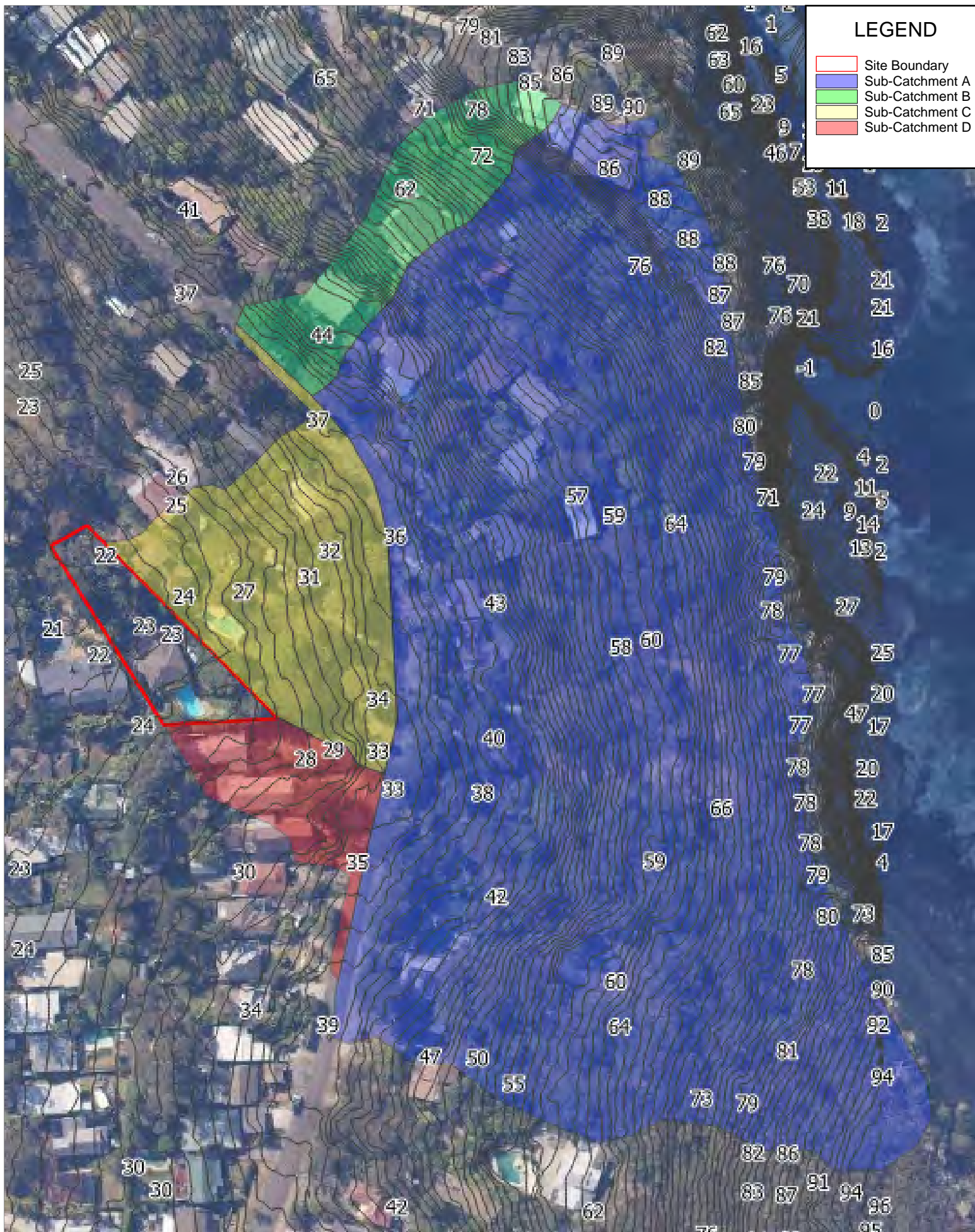
The proposed development works as detailed on the architectural plans prepared by Map Architecture & Interiors (dated September 2021) detail a proposed first floor addition and minor extensions to the ground floor level (refer appendix C).


### 1.3 Catchment Summary

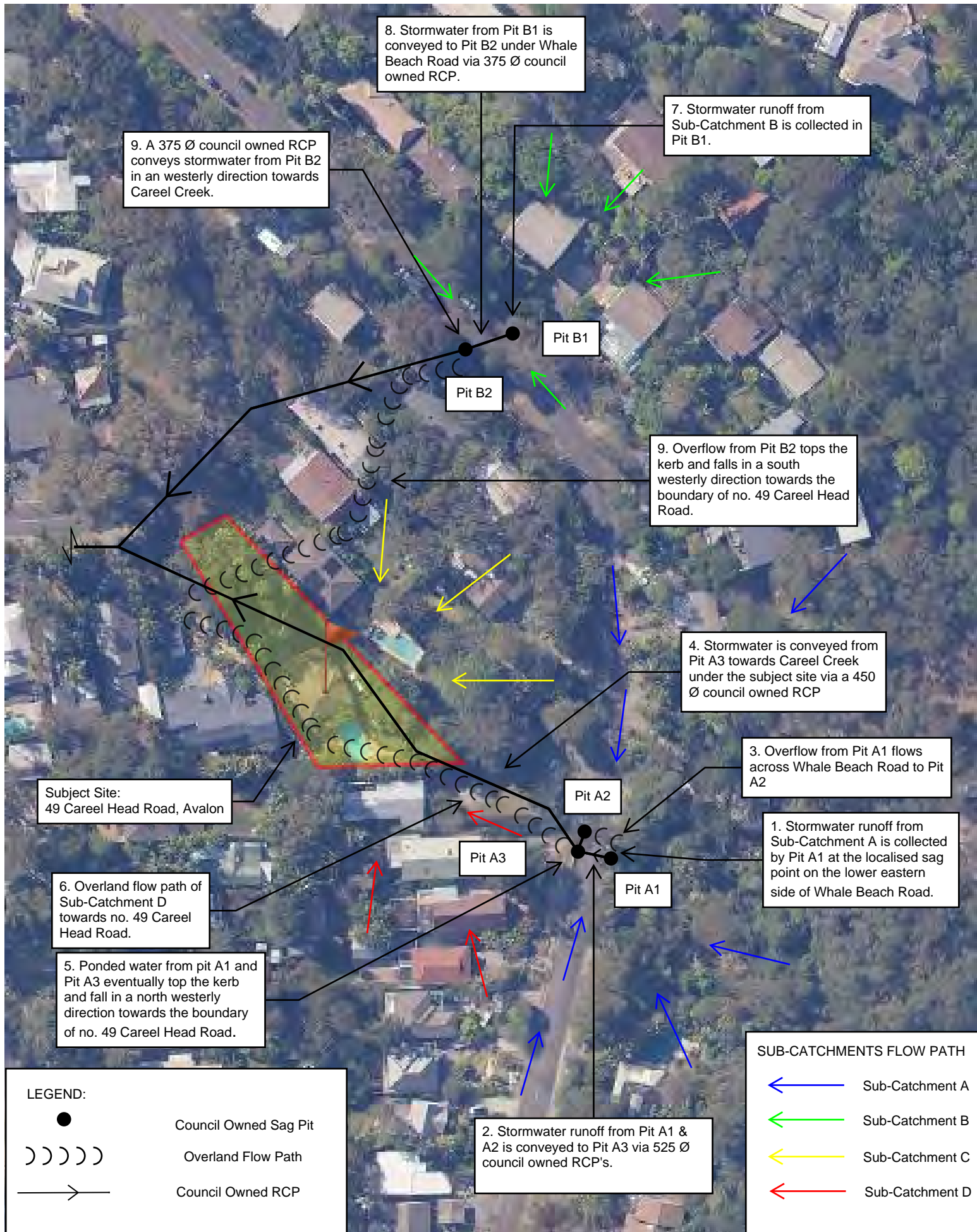
The total contributing catchment, comprised of four sub-catchments, affecting the subject site was measured in the computer program QGIS 3.4.7 using LiDAR data and is approximately 4.3 Ha. Refer figure 1. Refer figure 2 for Council's drainage infrastructure and overview of overland flow drainage regime.

Table 1 - Sub-Catchment summary

Sub-Catchment	Area	Description
A	3.4 Ha	Sub-Catchment A consists largely of densely vegetated bushland, some low-density residential sites and roadside pavement on Whale Beach Road.
B	0.26 Ha	Sub-Catchment B consists primarily of low-density residential sites, mildly vegetated bushland and roadside pavement on Whale Beach Road.
C	0.49 Ha	Sub-Catchment C consists of mildly vegetate bushland, low-density residential sites and roadside pavement on Whale Beach Road.
D	0.18 Ha	Sub-Catchment D Consists primarily of low-density residential sites, some mildly vegetated bushlands and roadside pavement on Whale Beach Road.



TITLE: <b>Contributing Sub-Catchments</b>		 <b>Consulting Engineers</b> STRUCTURAL - CIVIL - STORMWATER - REMEDIAL  ... STRUCTURALLY SOUND
FIGURE: <b>1</b>	ISSUE: <b>A</b>	



TITLE:

# Northern Beaches Council Stormwater Pit and Pipe Maps.

FIGURE:

2

ISSUE:

A



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## 2. Analysis & Results

### 2.1 Peak Flow Results

A DRAINS computation analysis was completed to determine the anticipated runoff within the contributing sub-catchments and to design a piped drainage system to convey any overland flows encroaching the subject site. The peak runoff rates for the 1% AEP storm event were computed, and are shown in Table 1 below:

Table 2 - Catchment Flow Rates for the 1% AEP Storm Event

AEP	Sub-Catchment	Area (Ha)	Piped Flow (m <sup>3</sup> /s)	Overflow (m <sup>3</sup> /s)
1%	A	3.412	0.008	1.530
1%	B	0.264	0.037	0.113
1%	C	0.496	0.000	0.231
1%	D	0.177	0.000	1.580

For further detail refer Appendix B.

Table 3 - Inlet Flow Conditions for the 1% AEP Storm Event

AEP	Inlet Condition	Inlet Flow (m <sup>3</sup> /s)
1%	A	1.580
1%	B	0.113
1%	C	0.231

For further detail refer Appendix B

#### 2.1.1 2D Hec-Ras Results

Modelled results from a DRAINS analysis have been used to estimate the peak flow hydrographs for the 1% AEP storm event. This flow hydrograph was used to estimate the flood levels in HEC-RAS 5.0.7.

Table 4 - Roughness Parameters used for HEC-RAS analysis

Surface Type	Manning's Roughness (n)
Road / Paving	0.015
Grass	0.05

The 2D HEC-RAS analysis demonstrated that the subject site is affected by overland flows in the 1% AEP storm event. Refer table 5 below for the predicted maximum 1% AEP level, depth and velocity within the subject site.

## 2.2 Site Flooding Extent

The 1% AEP water surface profile for the overland flow path has been estimated using HEC-RAS 5.0.7, consisting of a 2D hydrodynamic flow model.

### 2.2.1 Unsteady State Flow Analysis

An unsteady state flow analysis was used in the assessment to determine flow behaviour through the subject site. A mixed flow computation was used to simulate the effects of each storm event and account for variation of subcritical and supercritical flows through the site. The water surface elevation levels were computed at various cross sections through the channel for each storm event.

Utilising this type of model enabled a detailed analysis of the anticipated flood behaviour expected to occur through the subject site, accounting for varying flows throughout the modelled storm events. This generally provides greater accuracy in results as it simulates more realistic flow conditions.

### 2.2.2 2D Hydrodynamic Flow Analysis

2D hydrodynamic flood models provide numerical solutions based on depth-averaging equations. The model setup consisted of a 2D computational mesh or grid construction which represents the underlying topography using connected cells. In contrast to a 1D flood model, the 2D model requires continuous topographical data which covers the entire area being modelled in the 2D analysis. LiDAR (Light Detection and Ranging) survey information has been used to represent the underlying topography. Utilising this type of model enabled a detailed analysis of the anticipated flood behaviour expected to occur through the subject site, simulating complex flow patterns on the floodplain and calculating the resulting velocity and water surface elevations. This modelling approach combines both LiDAR data and surface roughness into a velocity field, generating results which are reflective of the anticipated flood behaviour through the site.

A Digital Elevation Model (DEM) was constructed using the software QGIS 3.4.7 to construct a surface roughness profile of the surrounding terrain. This required constructing elevated polygons to represent the surrounding buildings which have been modelled as impermeable blockages in the model. The modelled building polygons were created based on aerial imagery from online mapping services. The building polygons were superimposed onto the LiDAR data to create a complete Digital Surface Model (DSM). Furthermore, break lines have been used in the model to force an alignment of computational cell faces along elevation barriers, such as between the terrain and surrounding buildings.

The upstream boundary conditions used flow hydrographs representing the overland flow paths for the 1% AEP peak storm event which were computed in DRAINS. A normal depth condition has been assumed for the downstream boundary condition.





## 2.3 2D HEC-RAS Results

### 2.3.1 Critical Overland Flow Paths

A 2D HEC-RAS analysis demonstrated that the subject site is affected by overland flows in the 1% AEP storm event. There exists one primary overland flow path which emerges from the western boundary of the development site and forms two separate overland flow paths within the subject site. This overland flow path emerges when the capacity of the surrounding stormwater drainage infrastructure is exceeded. The predicted Maximum flood Depths and Velocities and critical points within the subject site can be found in table 5. Note: flood depths below 0.01m have been excluded from all diagrams as they are considered negligible. Refer figure 3 for Flood ID locations.

ID	Natural Ground Level (m AHD)	1% AEP Max WL (m AHD)	1% AEP Max Depth (m)	1% AEP Max Velocity (m/s)	Flood Hazard
1	24.60	25.15	0.43	3.00	High
2	25.35	25.54	0.19	0.70	Low
3	22.93	23.18	0.25	0.50	Low
4	22.87	23.10	0.20	3.50	High*
5	22.80	22.90	0.10	2.00	Low
6	22.20	22.40	0.30	3.80	High*

Table 5 - 1% AEP Flood Depths and Velocities

\*Model discrepancies may account for high flood hazard.



0 40 80 120m

Approx. Scale

TITLE: **1% AEP Flood ID Points**

FIGURE:

**3**

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## 2.4 Flood Velocity Hazard

The flood velocity hazard criterion considered for the development is in accordance with the *NSW Floodplain Development Manual (2005)* guidelines and is detailed in Figure 6 below.

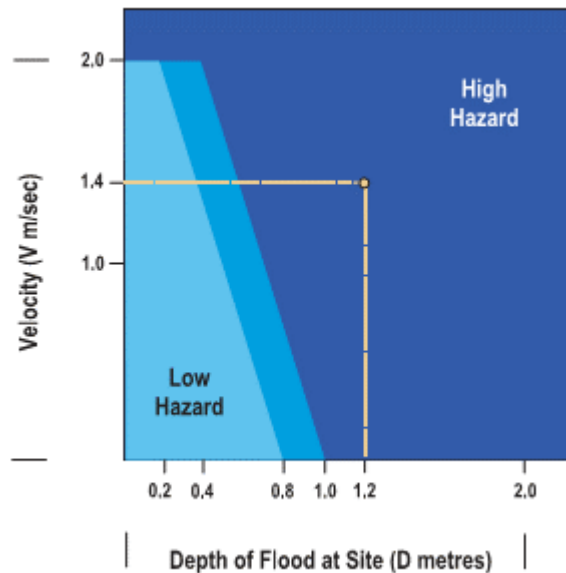


Figure 4 - Provisional Hydraulic Hazard Categories. Source: NSW Floodplain Development Manual (2005)

The substantial floodwaters encroaching upon the subject site enter through the western boundary of the property. Using the maximum velocities and depths from table 5, the provisional flood hazard can be found at each point. The flood hazard of the overland flows can be found in table 5. The critical flood hazard at point 5 has a maximum flow depth along of approximately 0.10m and a maximum velocity of 2.0 m/s which classifies the flood hazard adjacent to the proposed extension as Low, based on Figure 4. The flood hazard is not materially increased as a result of the development and therefore, no additional flood hazard measures are recommended beyond the recommendations in this report. The northern and southern overland flow pathways are low hazard and low-risk zones which do not have the potential to cause significant risk to life.



### 3. Recommendations

The critical flooding encroaches the site from the southern boundary when the capacity of the surrounding stormwater drainage infrastructure is exceeded. To safeguard the proposed development from flooding effects, the following is recommended and must be constructed (refer figure 5 for further detail):

1. **Flood Wall:** Extend proposed wall for the new patio (as per Figure 5 below) to protect internal and external areas from inundation. The new wall is to be designed by a suitably qualified structural engineer to withstand hydrostatic forces and debris loading up to the Flood Planning Level (FPL) at RL 23.40m AHD.
2. **Proposed Patio Extension:** The proposed patio extension is not envisaged to impact the 1% AEP flood extent. The proposed patio must have a minimum finished flood level (FFL) of 22.92m AHD to provide a minimum 50mm threshold between internal and external areas.
3. **Proposed Family Room Extension:** The extension is proposed within the existing building footprint and is not envisaged to impact the 1% AEP flood extent. Since a minimum freeboard cannot be achieved for the proposed extension, all new walls running parallel to the critical overland flow path are to be designed by a suitably qualified structural engineer to withstand hydrostatic forces and debris loading up to the Flood Planning Level (FPL) at RL 23.40m AHD.
4. **New entryway:** The entryway is proposed to move from the eastern side of the dwelling to the northern side of the dwelling (refer appendix C for further details). Since a minimum freeboard cannot be achieved at the new entryway, a minimum 300mm high wall is to be constructed to protect the entryway from potential inundation (refer Figure 5 below for location).

**Patio RL:**  
Lower patio level FFL 22.92 to provide minimum 50mm threshold between internal and external areas

**Flood Wall:**  
Extend the proposed wall for the new patio. The new wall is to be designed by a suitably qualified structural engineer to withstand hydrostatic forces and debris loading up to the Flood Planning Level (FPL) at RL 23.40.

**Proposed Extension:**  
The extension is proposed at an FFL of 22.97m AHD, which is approximately at the predicted 1% AEP flood level. As a minimum freeboard cannot be provided, no new openings are to be constructed in the wall running parallel to the overland flow path up to the Flood Planning Level (FPL) at RL 23.40. All new walls are to be designed by a suitably qualified structural engineer to withstand hydrostatic forces and debris loading up to the FPL.

**Critical overland flow path:**  
The overland flow path runs parallel to the proposed extension and is predicted to have a maximum depth of 0.10m during the 1% AEP peak flood event.

**Proposed entry:**  
Location of entry to be moved. Overland flow within vicinity is fringe flow (below 90mm) and is parallel to the flow direction, therefore the flow is considered negligible. The building footprint is to remain therefore, is not predicted to impact the 1% AEP flood extent.

**Legend:**

← Critical Overland Flow path

**Existing raised deck:**  
Inundation to be expected in the vicinity of the existing structure during the 1% AEP flood event. Water ingress is likely to occur and therefore it is recommended that this be adequately flood proofed to prevent inundation. (Recommendation only)



0 5 10 15 m

Approx. Scale

TITLE:

**1% AEP Flood Map with Recommendations**

FIGURE:

**5**

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**A**



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## 4. Conclusion

The overland flow profile surrounding the subject site has been modelled in HEC-RAS, with the results demonstrating the existing overland flow regime will generally remain unaffected as a result of the proposed development provided the recommendations of this report are adhered to.

In accordance with accepted engineering practice, NBCE has undertaken a flood study of the stormwater drainage system at 49 Careel Head Road in Avalon and can confirm the accuracy of the calculated results based on the HEC-RAS modelling. Please contact the author if further clarification is required.

**NORTHERN BEACHES CONSULTING ENGINEERS P/L**



**Michael Wachjo**

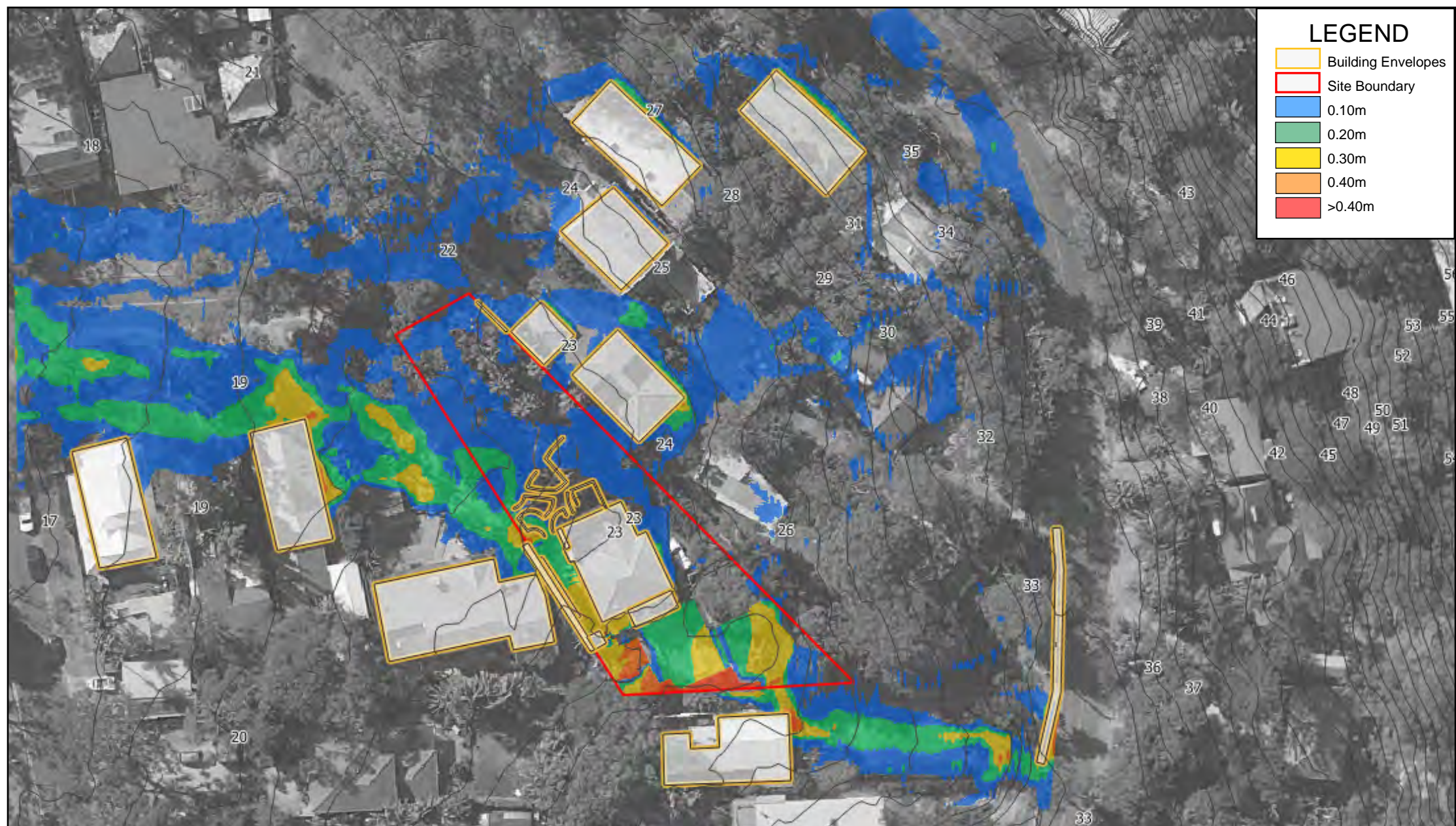
BE(Civil) MIEAust

\\NBADS\Company\Synergy\Projects\200808 49 CAREEL HEAD ROAD, AVALON\ENG Design\Flood Study\REPORT\Report\200808 Overland Flow Report 2022-04-07.docx

## APPENDIX A

### HEC-RAS Results





# LEGEND

- Building Envelopes
- Site Boundary
- 0.10m
- 0.20m
- 0.30m
- 0.40m
- >0.40m



0 10 20 30m

Approx. Scale

TITLE:

**Pre-Development Case: 1% AEP Depth Map**

FIGURE:

**6**

ISSUE:

**A**



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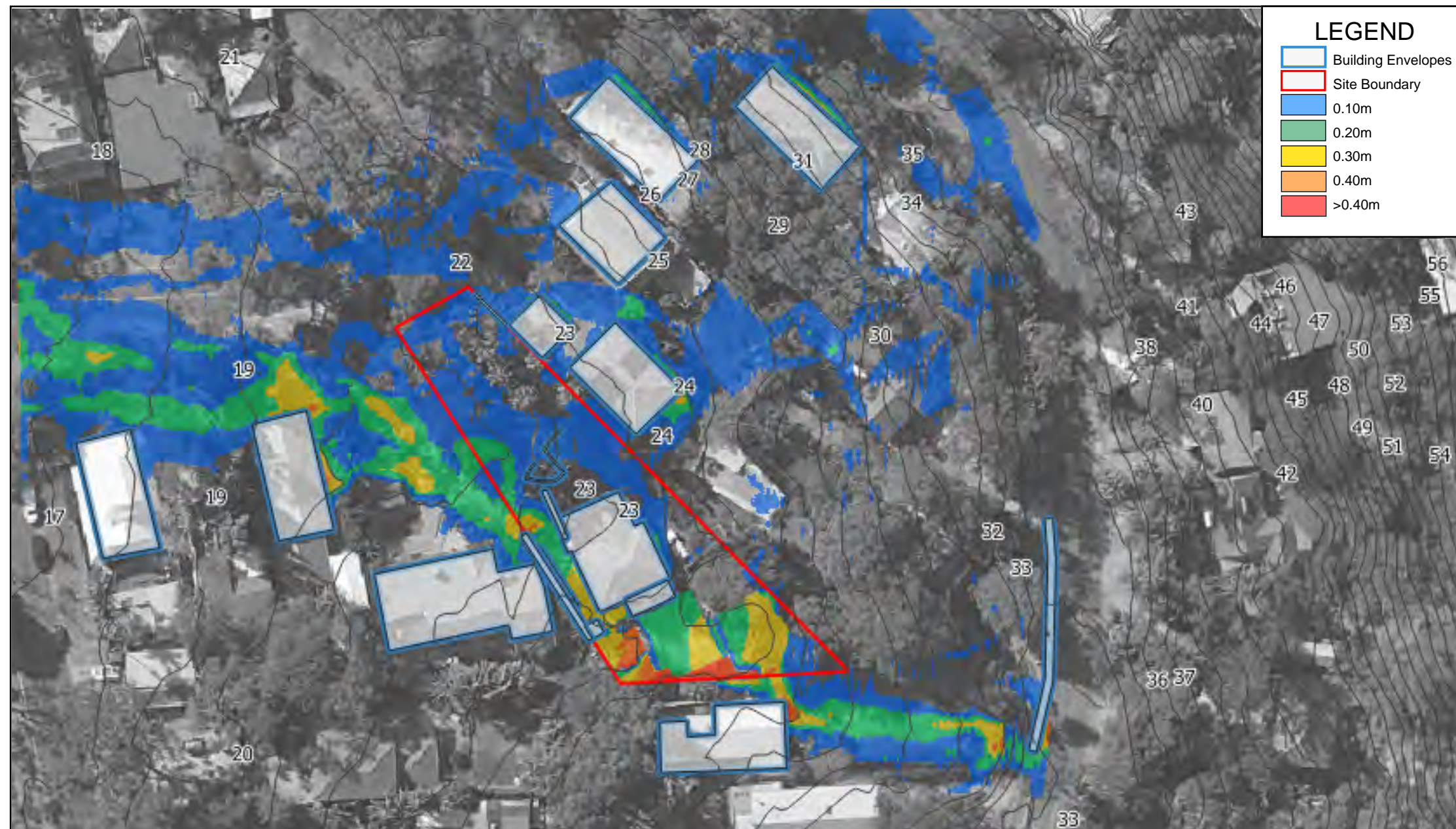
STRUCTURAL - CIVIL - STORMWATER - REMEDIAL

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# LEGEND

- Building Envelopes
- Site Boundary
- 0.10m
- 0.20m
- 0.30m
- 0.40m
- >0.40m



0 10 20 30m

Approx. Scale

TITLE:

**Post-Development Case: 1% AEP Depth Map**

FIGURE:

**7**

ISSUE:

**A**



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... STRUCTURALLY SOUND





# LEGEND

- Building Envelopes
- Site Boundary
- 0.02m
- 0.03m
- 0.04m
- 0.05m
- >0.05m



0 10 20 30m

Approx. Scale

TITLE:

**Difference Map: 1% AEP Depth**

FIGURE:

**8**

ISSUE:

**A**

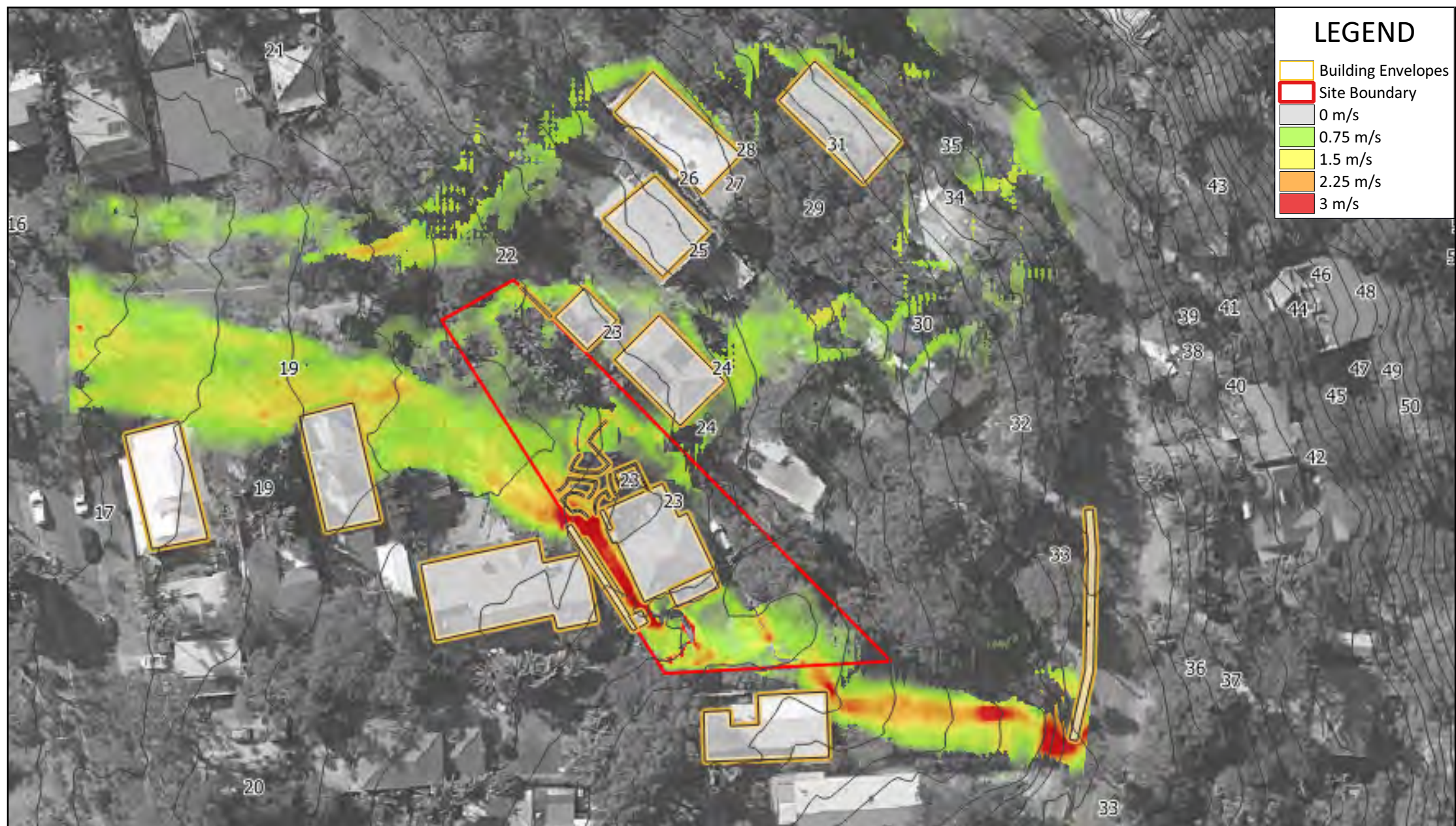


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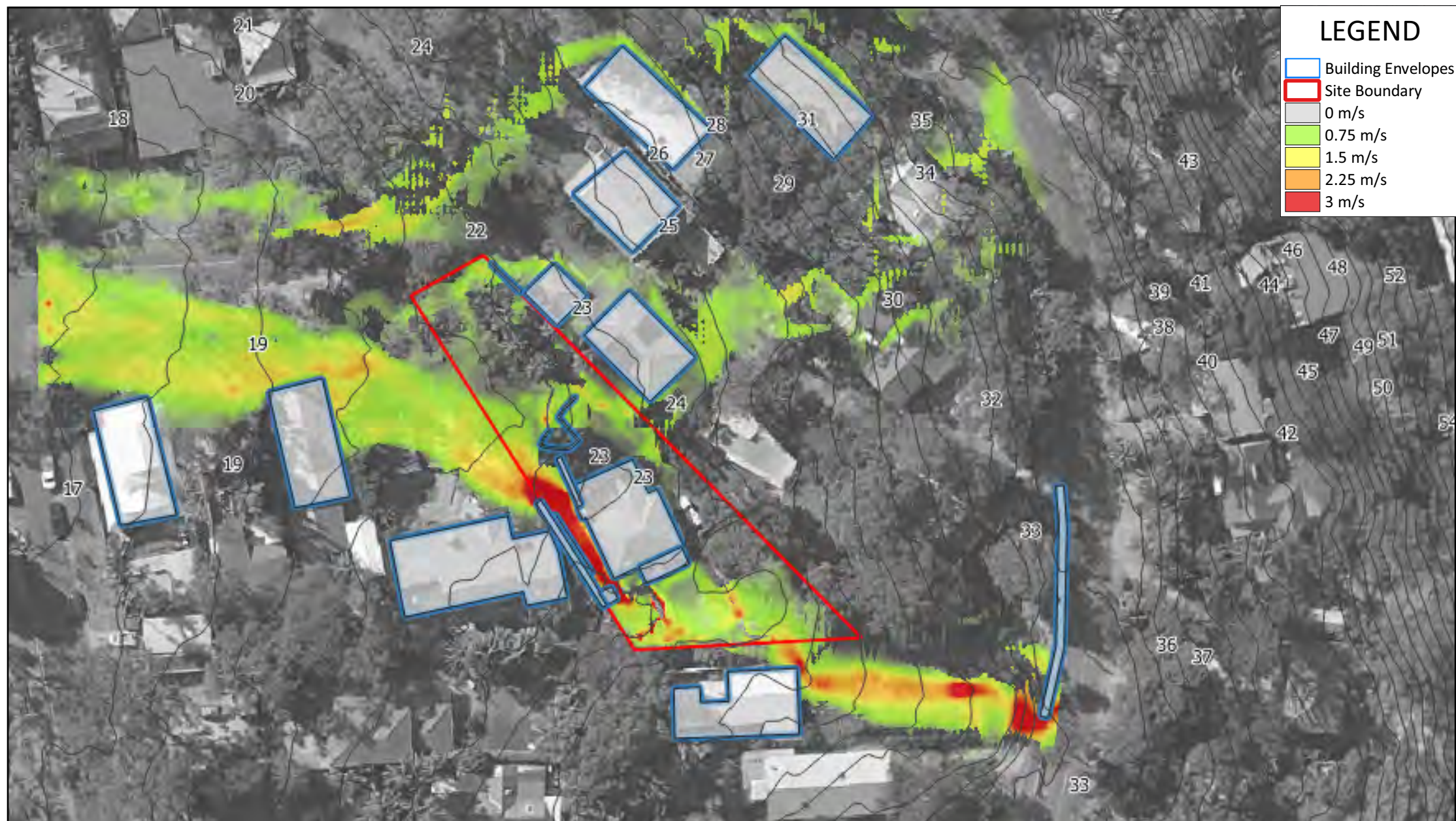


<p>0 10 20 30m</p> <p>Approx. Scale</p>	<p>TITLE:</p> <p><b>Pre-Development Case: 1% AEP Velocity Map</b></p>		<p><b>NB Consulting Engineers</b></p> <p>STRUCTURAL - CIVIL - STORMWATER - REMEDIAL</p> <p>... STRUCTURALLY SOUND</p>
	<p>FIGURE:</p> <p><b>9</b></p>	<p>ISSUE:</p> <p><b>A</b></p>	



# LEGEND

-  Building Envelopes
-  Site Boundary
-  0 m/s
-  0.75 m/s
-  1.5 m/s
-  2.25 m/s
-  3 m/s



0 10 20 30m

Approx. Scale

TITLE:

**Post-Development Case: 1% AEP Velocity Map**

FIGURE:

**10**

ISSUE:

**A**



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0 10 20 30m

Approx. Scale

TITLE:

**Difference Map: 1% AEP Velocity**

FIGURE:

**11**

ISSUE:

**A**



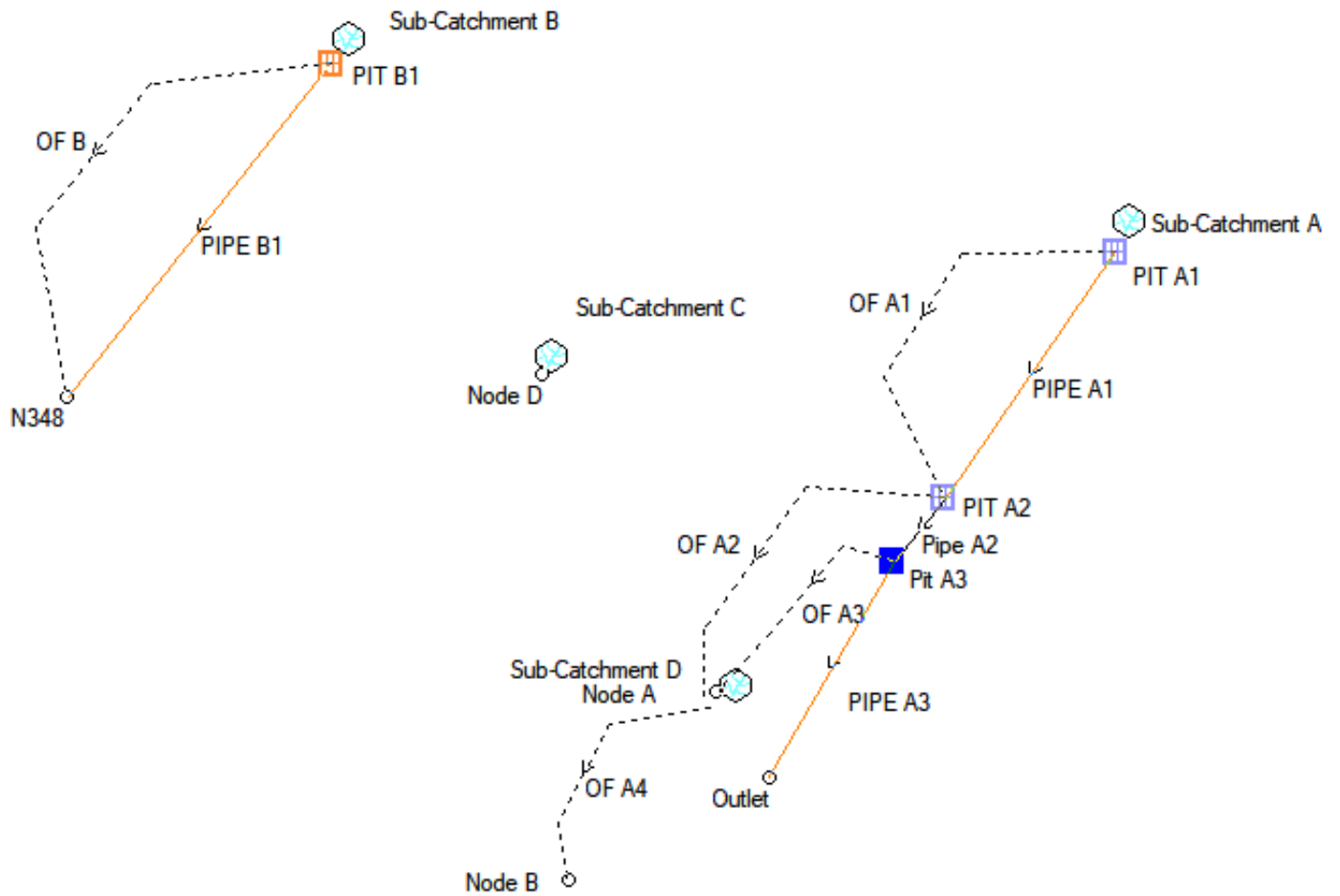
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## APPENDIX B

### DRAINS Results



TITLE: **DRAINS Catchment Model Layout**

FIGURE:

12

ISSUE:

A



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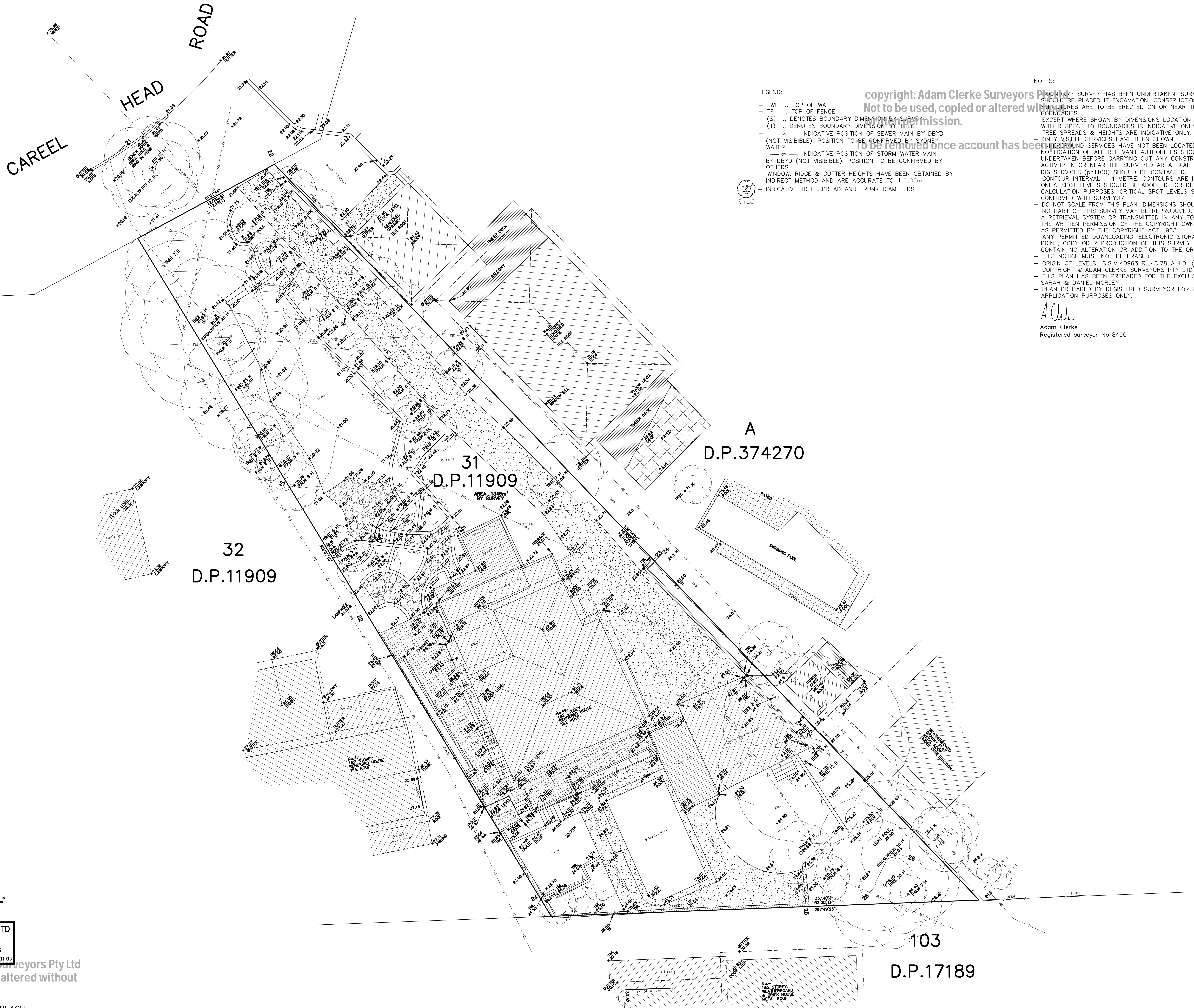
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## APPENDIX C

### Site Survey Plan & Architectural Plan



LEGEND:

- TWL .. TOP OF WALL
- TF .. TOP OF FENCE
- (S) .. DENOTES BOUNDARY DIMENSION BY SURVEY
- (T) .. DENOTES BOUNDARY DIMENSION BY TITLE
- SW .. INDICATIVE POSITION OF SEWER MAIN BY DBYD (NOT VISIBLE). POSITION TO BE CONFIRMED BY SYDNEY WATER.
- SW .. INDICATIVE POSITION OF STORM WATER MAIN BY DBYD (NOT VISIBLE). POSITION TO BE CONFIRMED BY OTHERS.
- WINDOW, RIDGE & CUTTER HEIGHTS HAVE BEEN OBTAINED BY INDIRECT METHOD AND ARE ACCURATE TO  $\pm 1/100$ mm.
- INDICATIVE TREE SPREAD AND TRUNK DIAMETERS



NOTES:

- A CADASTRAL SURVEY HAS BEEN UNDERTAKEN. SURVEY MARKS SHOULD BE PLACED IF EXCAVATION, CONSTRUCTION OR STRUCTURES ARE TO BE ERRECTED ON OR NEAR THE BOUNDARIES.
- EXCEPT WHERE SHOWN BY DIMENSIONS LOCATION OF DETAIL WITH RESPECT TO BOUNDARIES IS INDICATIVE ONLY.
- TREE SPREADS & HEIGHTS ARE INDICATIVE ONLY.
- ONLY VISIBLE SERVICES HAVE BEEN SHOWN.
- UNDERGROUND SERVICES HAVE NOT BEEN LOCATED. NOTIFICATION OF ALL RELEVANT AUTHORITIES SHOULD BE UNDERTAKEN BEFORE CARRYING OUT ANY CONSTRUCTION ACTIVITY IN OR NEAR THE SURVEYED AREA. DIAL BEFORE YOU DIG SERVICES (ph1100) SHOULD BE CONTACTED.
- CONTOUR INTERVAL - 1 METRE. CONTOURS ARE INDICATIVE ONLY. SPOT LEVELS SHOULD BE ADOPTED FOR DESIGN AND CALCULATION PURPOSES. CRITICAL SPOT LEVELS SHOULD BE CONFIRMED WITH SURVEYOR.
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- ORIGIN OF LEVELS: S.S.M.40963 R.148.78 A.H.D. (SCIMS)
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- PLAN PREPARED BY REGISTERED SURVEYOR FOR DEVELOPMENT APPLICATION PURPOSES ONLY.

A. Clerke

Adam Clerke  
Registered surveyor No:8490

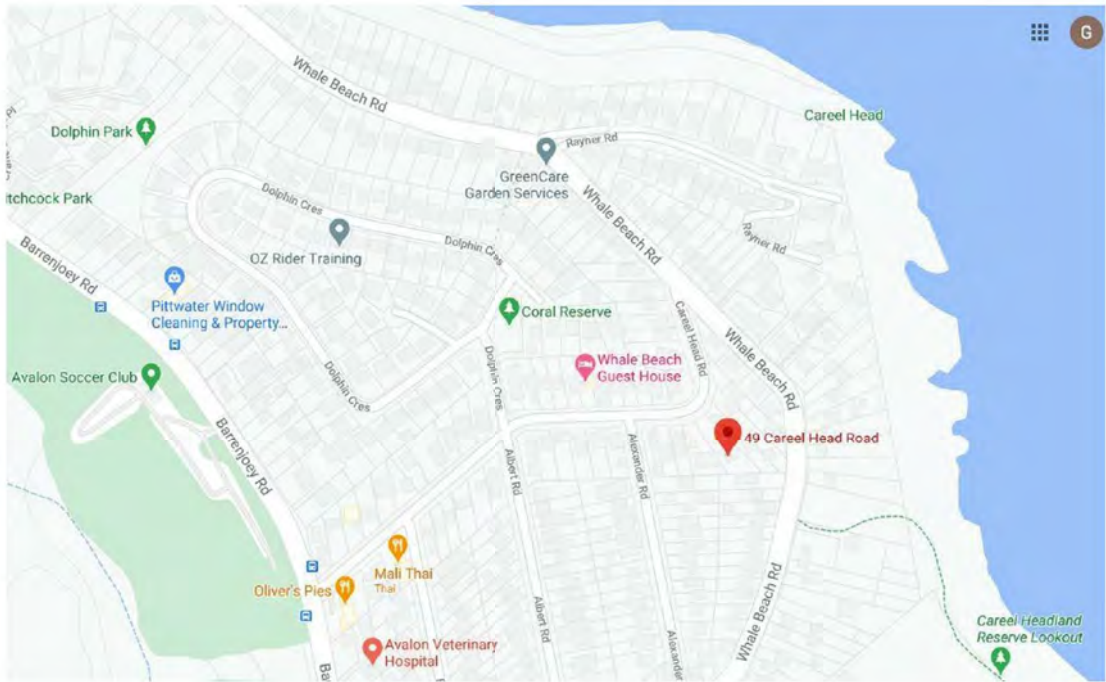
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LAND & ENGINEERING SURVEYORS  
P.O. BOX 175 NEWPORT NSW 2106  
TEL. 9918 4111. E. adam@adamclerkesurveyor.com.au

DETAILS AND LEVELS OVER  
LOT 31 IN D.P. 11909

49 CAREEL HEAD ROAD, AVALON BEACH

DATE... 17/12/20 REF... 11598  
SCALE...1:100(A0) DATUM...A.H.D





Sheet List			
Sheet Number	Current Revision	Sheet Name	Current Revision Date

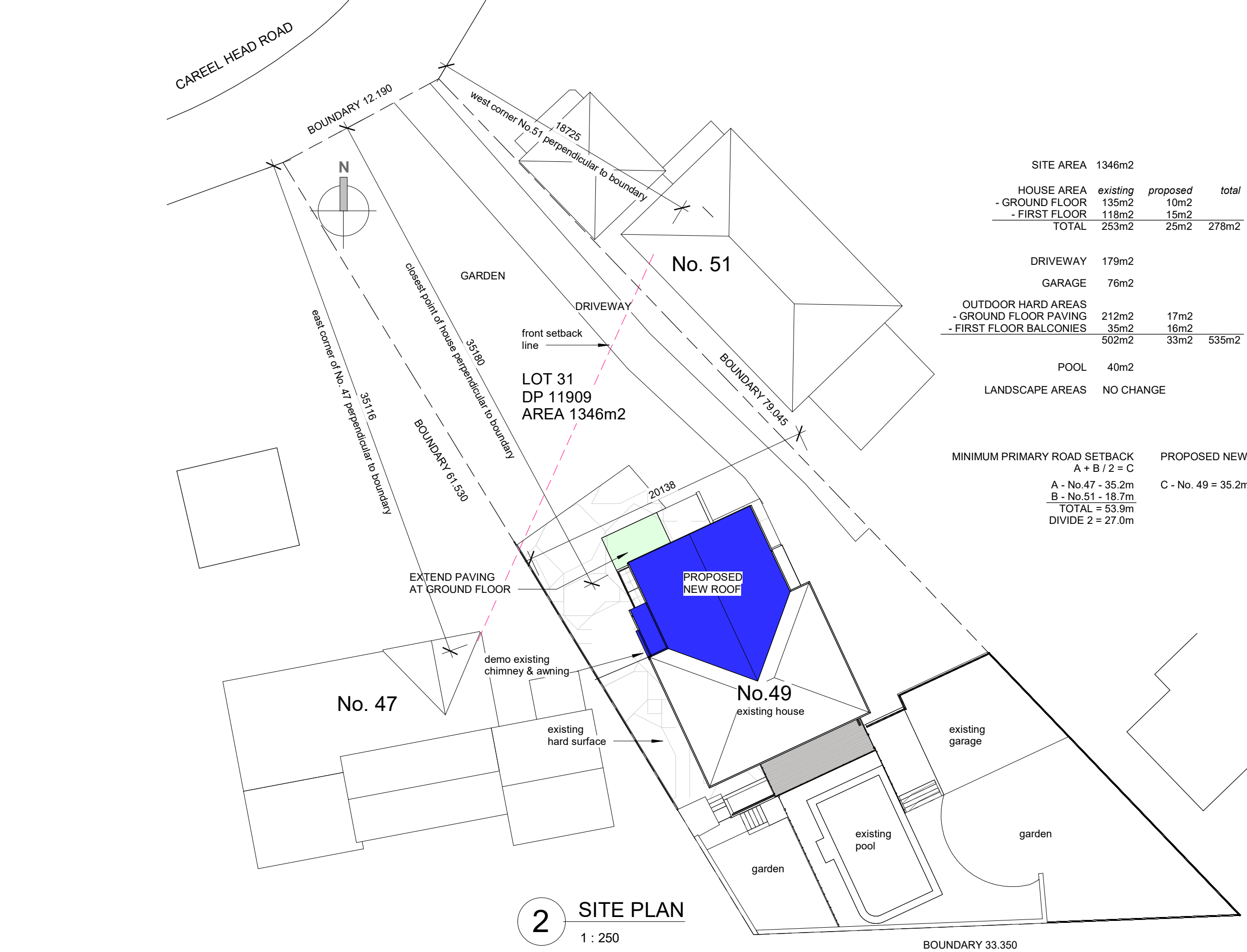
DA-001	A	COVER SHEET	20/09/2021
DA-002	A	SITE PLAN	20/09/2021
DA-003	A	SITE ANALYSIS PLAN	20/09/2021
DA-004	A	DEMOLITION PLANS	20/09/2021
DA-005	A	PLAN - LANDSCAPE AREAS	20/09/2021
DA-101	A	FIRST FLOOR	20/09/2021
DA-102	A	ROOF PLANS	20/09/2021
DA-201	A	ELEVATIONS - NORTH & SOUTH	20/09/2021
DA-202	A	ELEVATIONS - EAST & WEST	20/09/2021
DA-203	A	HEIGHT PLANES AND SETBACKS	20/09/2021
DA-301	A	TYPICAL SECTIONS	20/09/2021
DA-401	A	WINDOW SCHEDULE	20/09/2021
DA-501	A	SHADOWS - 9AM SEPT 22nd	20/09/2021
DA-502	A	SHADOWS - 12 NOON SEPT 22nd	20/09/2021
DA-503	A	SHADOWS - 3PM SEPT 22nd	20/09/2021
DA-601	A	FINISHES	20/09/2021

PROPOSED ALTERATIONS AND ADDITIONS  
49 CAREEL HEAD ROAD AVALON BEACH  
FOR DAN AND SARAH MORLEY



1 PROPOSED VIEW FROM DRIVEWAY

EXISTING HOUSE FROM DRIVEWAY



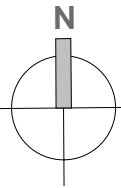
SITE AREA	1346m2		
HOUSE AREA	<i>existing</i>	<i>proposed</i>	<i>total</i>
- GROUND FLOOR	135m2	10m2	
- FIRST FLOOR	118m2	15m2	
TOTAL	253m2	25m2	278m2

DRIVEWAY	179m2		
GARAGE	76m2		
OUTDOOR HARD AREAS			
- GROUND FLOOR PAVING	212m2	17m2	
- FIRST FLOOR BALCONIES	35m2	16m2	
	502m2	33m2	535m2

POOL	40m2		
LANDSCAPE AREAS	NO CHANGE		

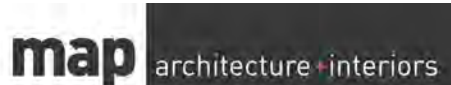
MINIMUM PRIMARY ROAD SETBACK	PROPOSED NEW SETBACK
$A + B / 2 = C$	
A - No.47 - 35.2m	C - No. 49 = 35.2m
B - No.51 - 18.7m	
TOTAL = 53.9m	
DIVIDE 2 = 27.0m	





# 1 SITE ANALYSIS PLAN

1 : 500



PO BOX 699 AVALON BEACH NSW 2107  
M +61 400 044 277 info@mapai.com.au

PROJECT  
**PROPOSED HOUSE ALTERATIONS  
AND ADDITIONS**  
**DAN AND SARAH MORLEY**

DWG NO.  
**DA-003**

DATE Issue Date DRAWN BY Author  
SCALE 1 : 500 CHECKED BY Checker

REVISION  
**A** **49 CAREEL HEAD ROAD  
AVALON BEACH**

**SITE ANALYSIS PLAN**

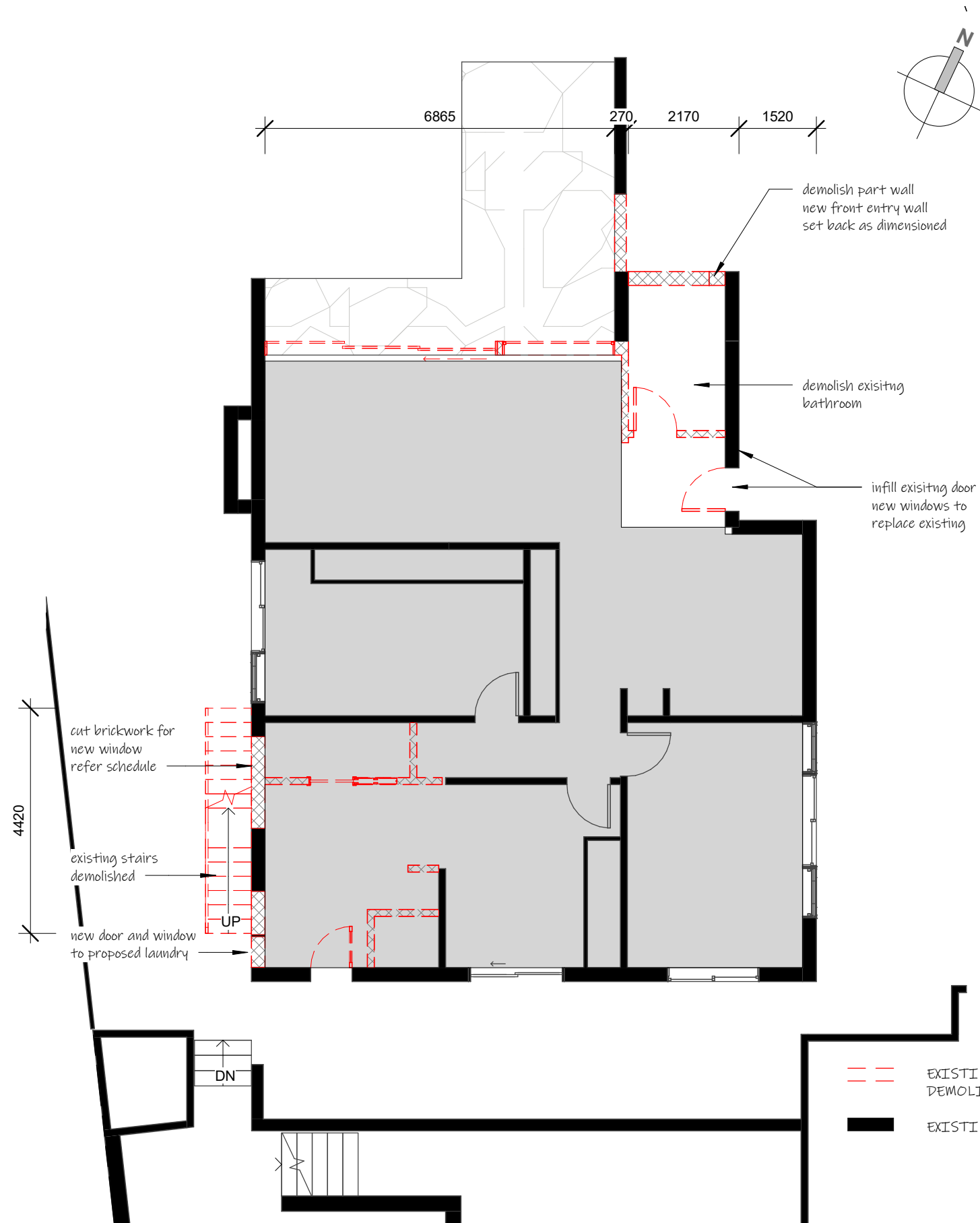
All work to be in accordance with local authorities building codes and standards. The BCA and all relevant current AUS / NZ standards. Do not scale from drawings use figured dimensions only verify all dimensions on site before ordering materials or commencing construction

Builder / Contractor to notify designer immediately of any discrepancies or errors in documentation. Workmanship shall be of the highest standard and to the satisfaction of the author. These plans remain the property of Map architecture + interiors and are subject to Copyright regulations

REVISION  
A 20/09/2021 ISSUE FOR DA

23/09/2021 2:10:23 PM





1

## Ground Floor - DEMOLITION PLAN

1 : 100

**map** architecture + interiors

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M +61 400 044 277 info@mapai.com.au

PROJECT

**PROPOSED HOUSE ALTERATIONS  
AND ADDITIONS**

**DAN AND SARAH MORLEY**

DWG NO.

**DA-004**

DATE

SCALE

Issue Date

1 : 100

DRAWN BY

CHECKED BY

Author

Checker

2

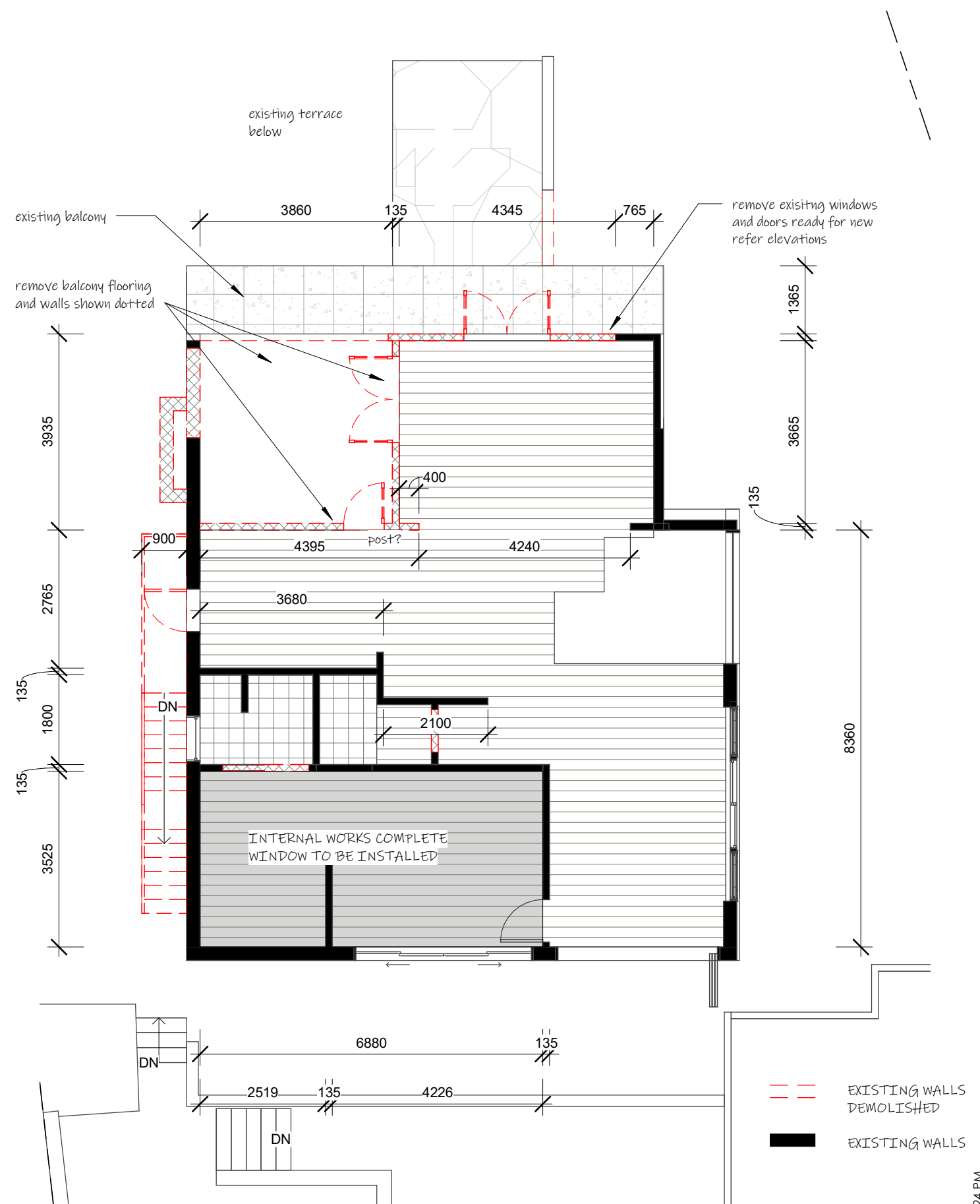
## Level 1 - DEMOLITION PLAN

1 : 100

**A**

**49 CAREEL HEAD ROAD  
AVALON BEACH**

**DEMOLITION PLANS**



All work to be in accordance with local authorities building codes and standards. The BCA and all relevant current AUS / NZ standards. Do not scale from drawings use figured dimensions only verify all dimensions on site before ordering materials or commencing construction

Builder / Contractor to notify designer immediately of any discrepancies or errors in documentation Workmanship shall be of the highest standard and to the satisfaction of the author These plans remain the property of Map architecture + interiors and are subject to Copyright regulations

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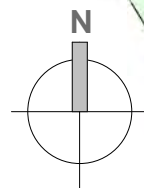
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**PRELIMINARY**

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CAREEL HEAD ROAD

No.47



No.49

No.51

BOUNDARY 12.190

GARDEN

PEEBLES

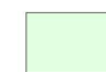
BOUNDARY 61.530

PAVERS

DRIVEWAY

BOUNDARY 79.045

SITE AREA 1346m2



EXISTING GARDEN 570m2



EXISTING PEEBLES 46m2

616m2 45.8%

DEEPSOIL PLANTING

500m2 37%



EXISTING DRIVEWAY 179m2



EXISTING PAVING 129m2



EXISTING POOL 40m2

NOTE:  
NO CHANGE TO LANDSCAPED AREAS

new balcony to back of house to join  
shed roof confirm levels  
pool gate from new balcony to roof top

close off with  
new pool fence

GARDEN

POOL

GARDEN

BOUNDARY 33.350

1

## LANDSCAPE PLAN

1 : 250

PROJECT

**PROPOSED HOUSE ALTERATIONS  
AND ADDITIONS**

**DAN AND SARAH MORLEY**

DWG NO.

**DA-005**

DATE  
SCALE

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**49 CAREEL HEAD ROAD  
AVALON BEACH**

**PLAN - LANDSCAPE AREAS**

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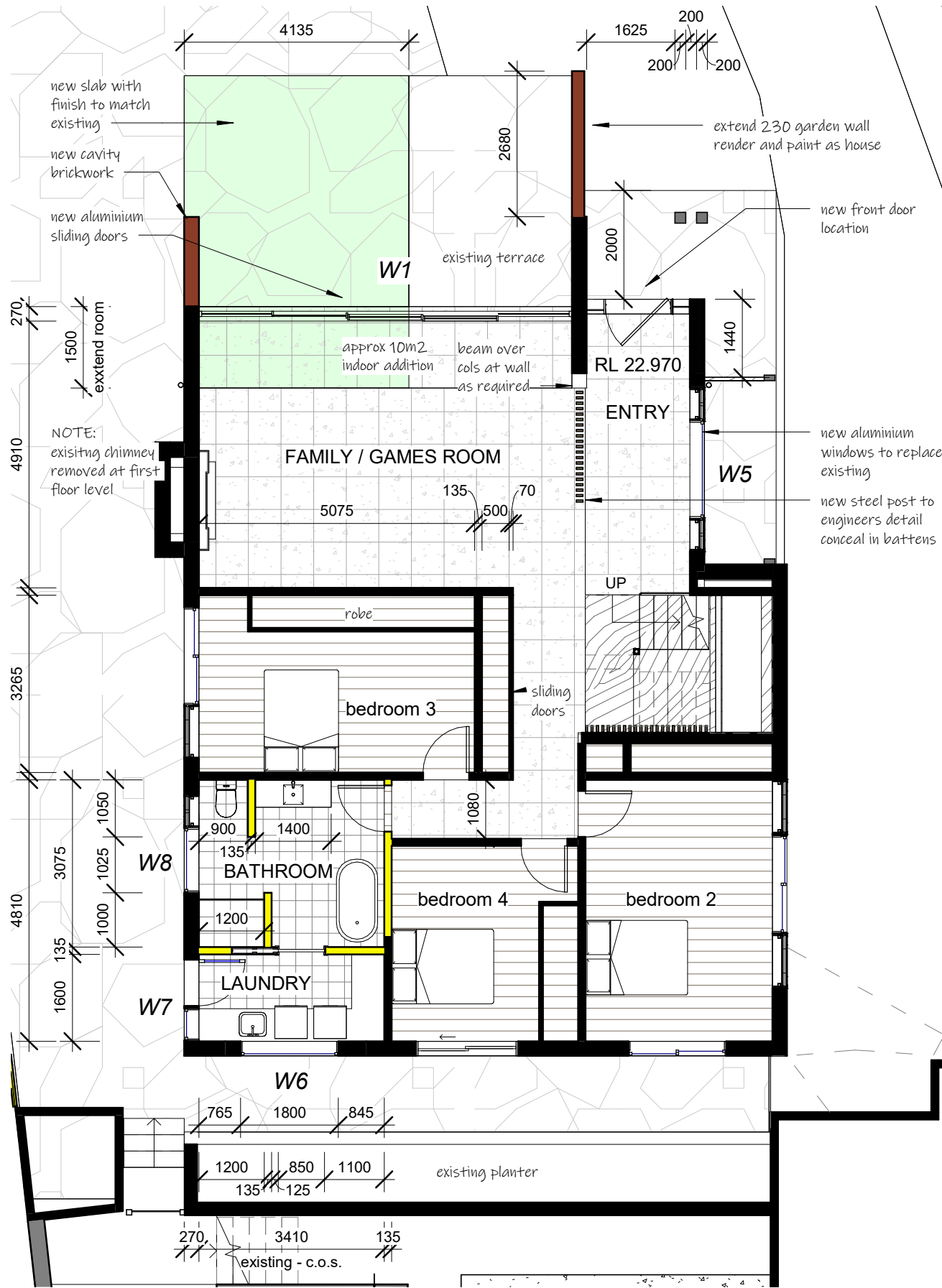
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1 ground floor plan

1 : 100

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NORTH SYDNEY NSW 2060

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PROJECT

**PROPOSED HOUSE ALTERATIONS  
AND ADDITIONS**

**DAN AND SARAH MORLEY**

DWG NO.

**DA-101**



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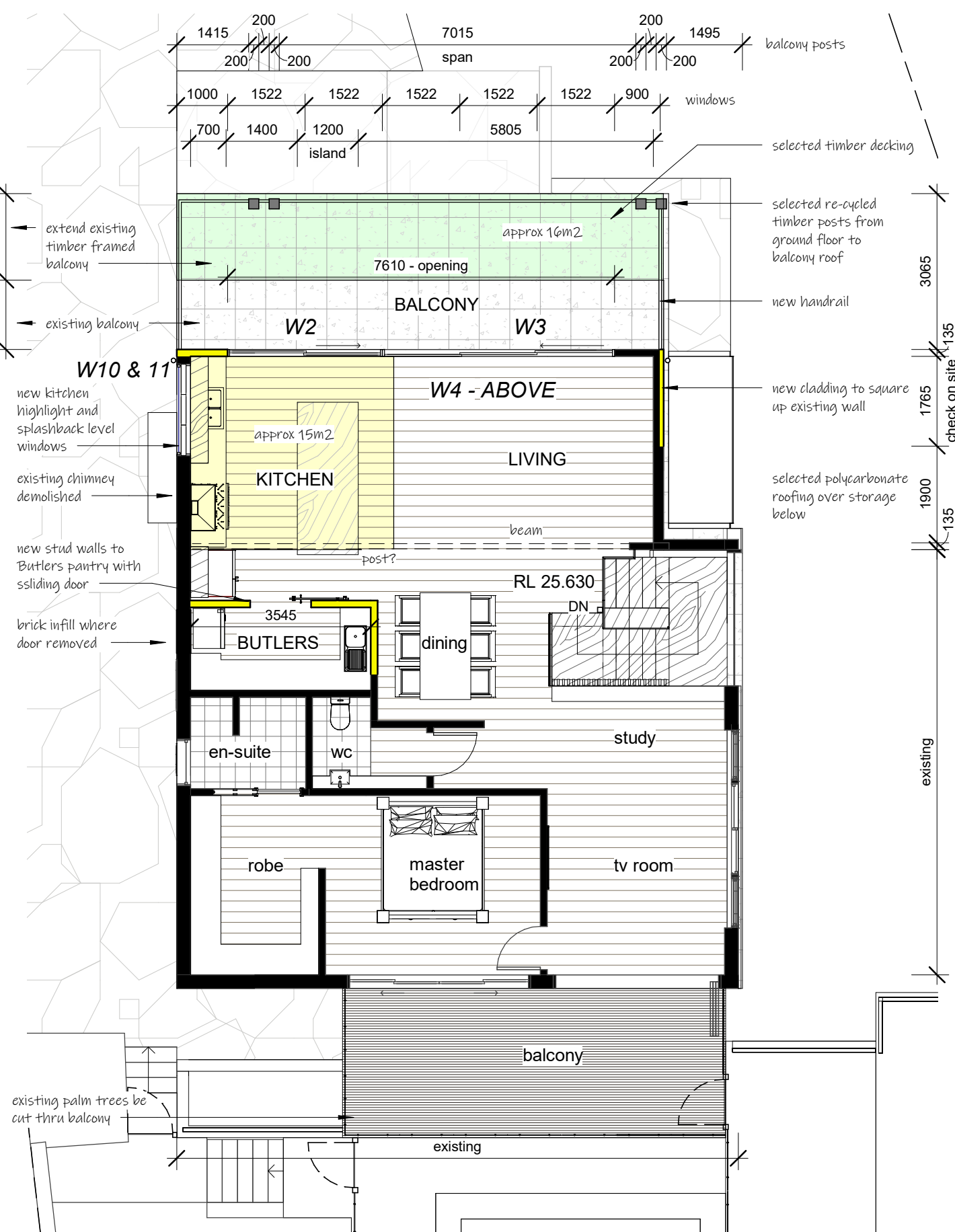
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**49 CAREEL HEAD ROAD  
AVALON BEACH**

**FIRST FLOOR**



2 first floor plan

1 : 100

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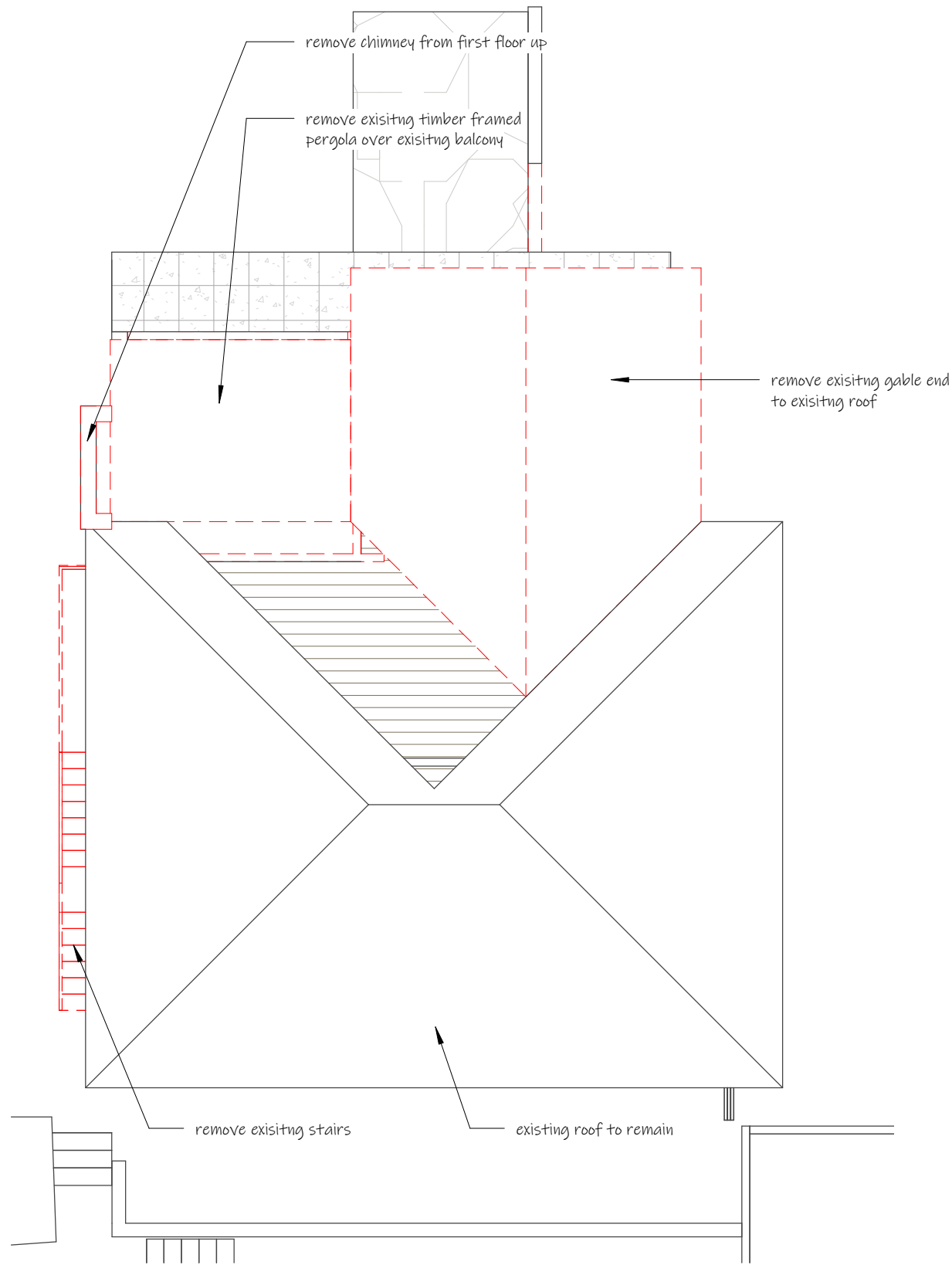
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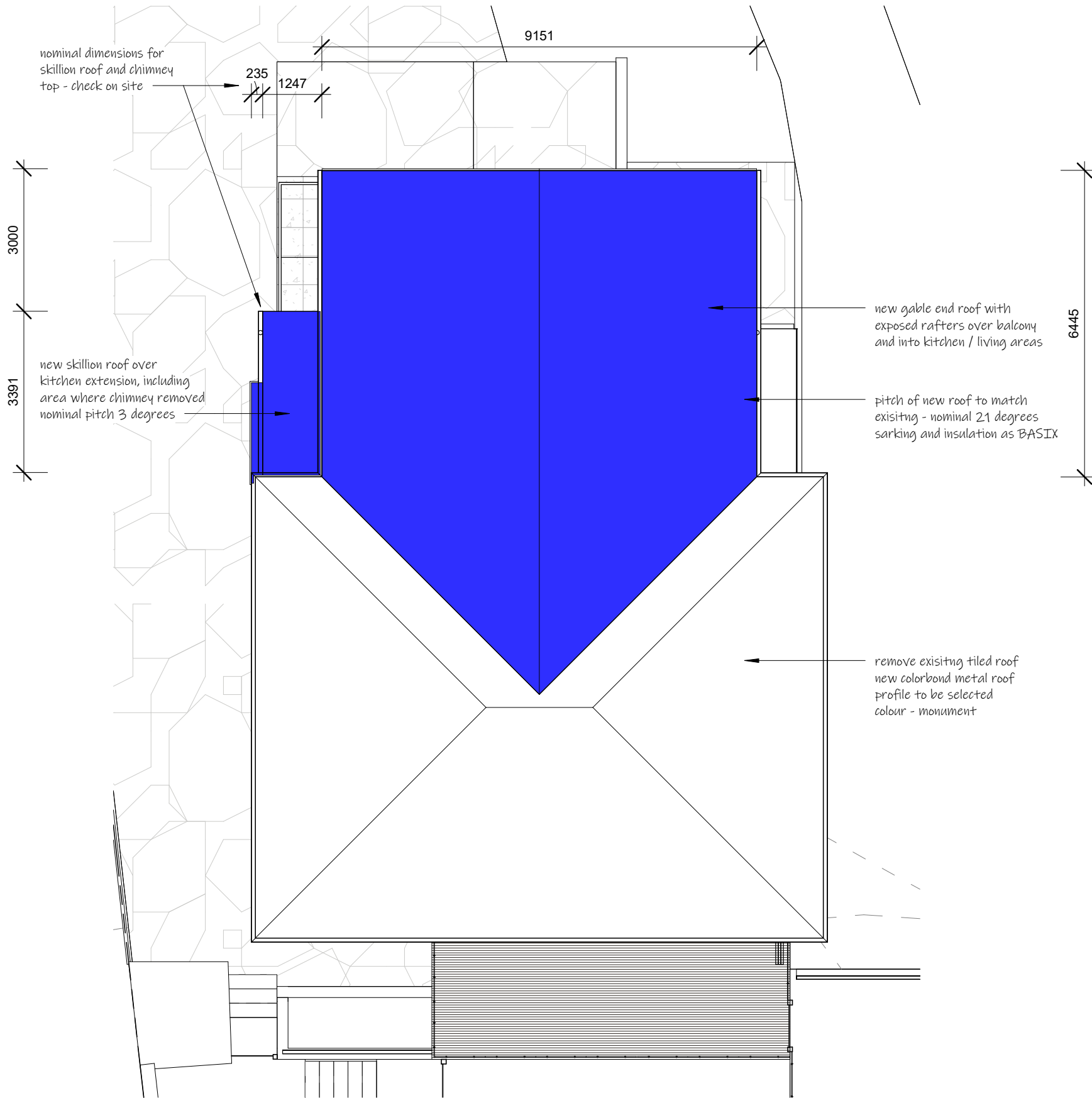




2

## Roof - DEMOLITION PLAN

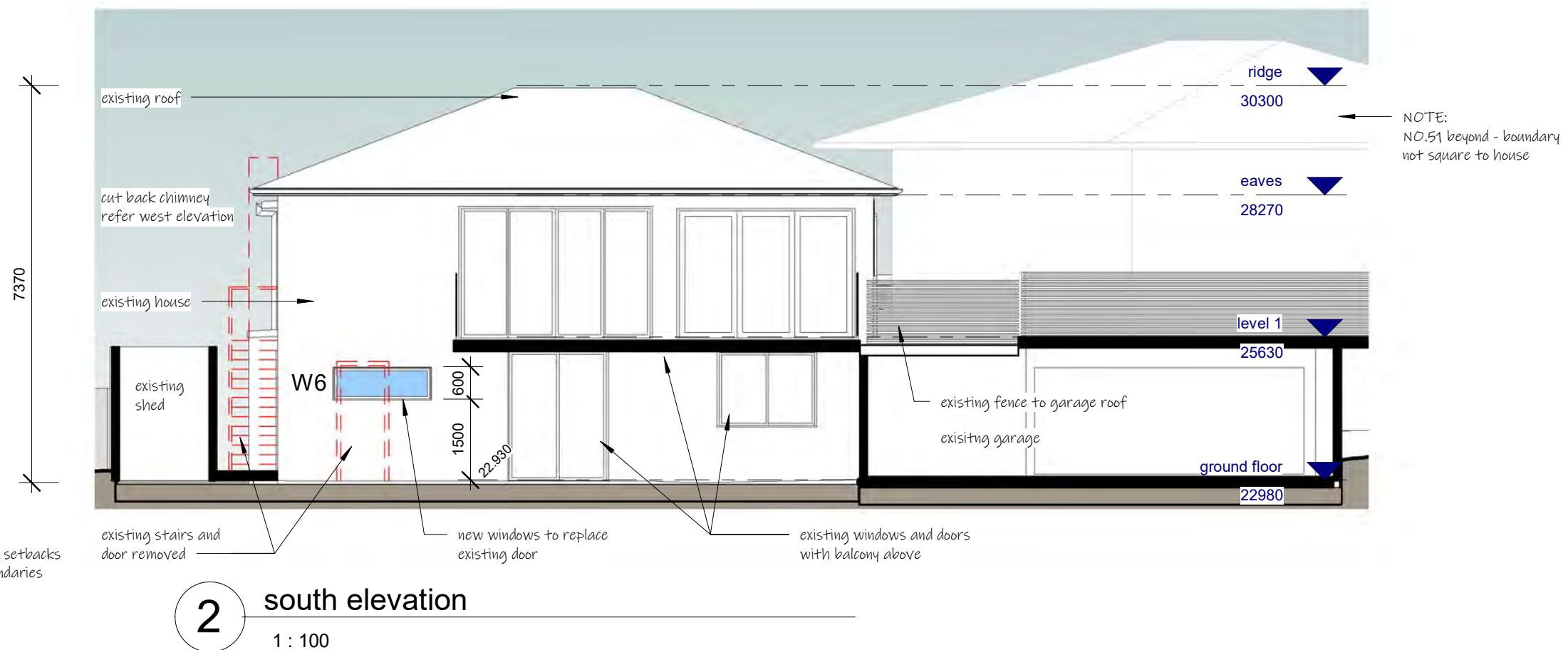
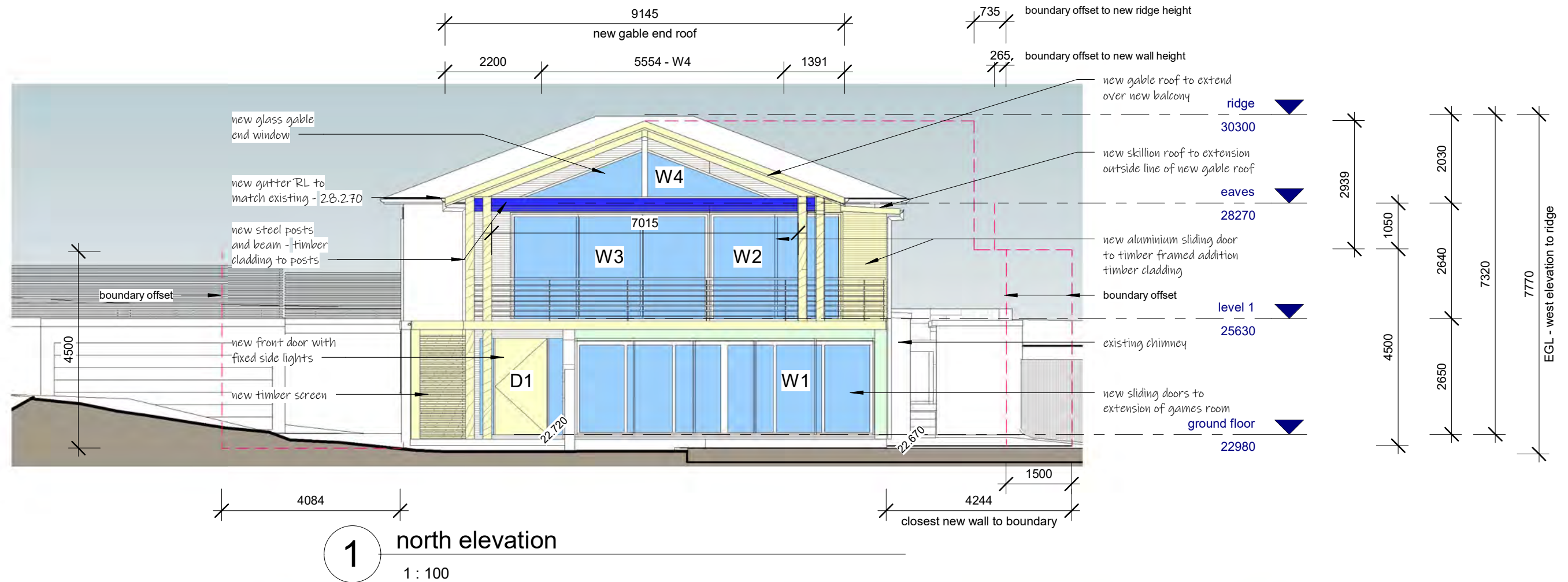
1 : 100

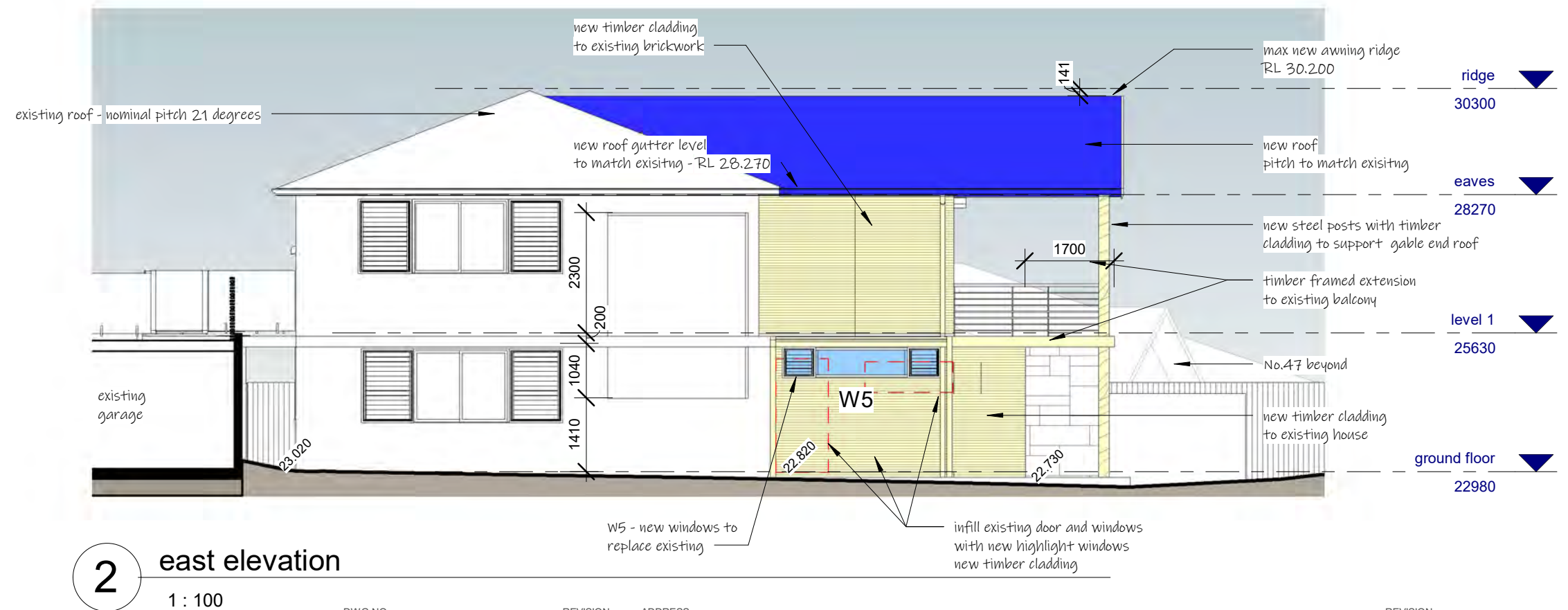
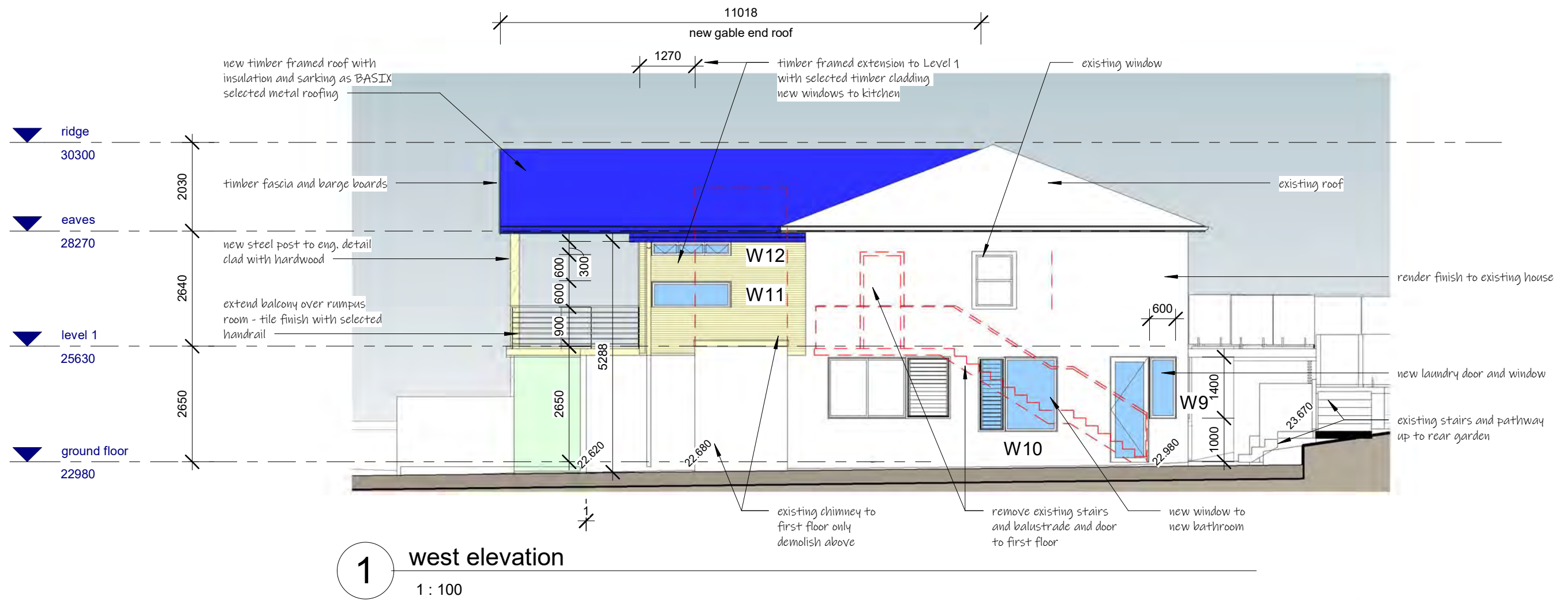


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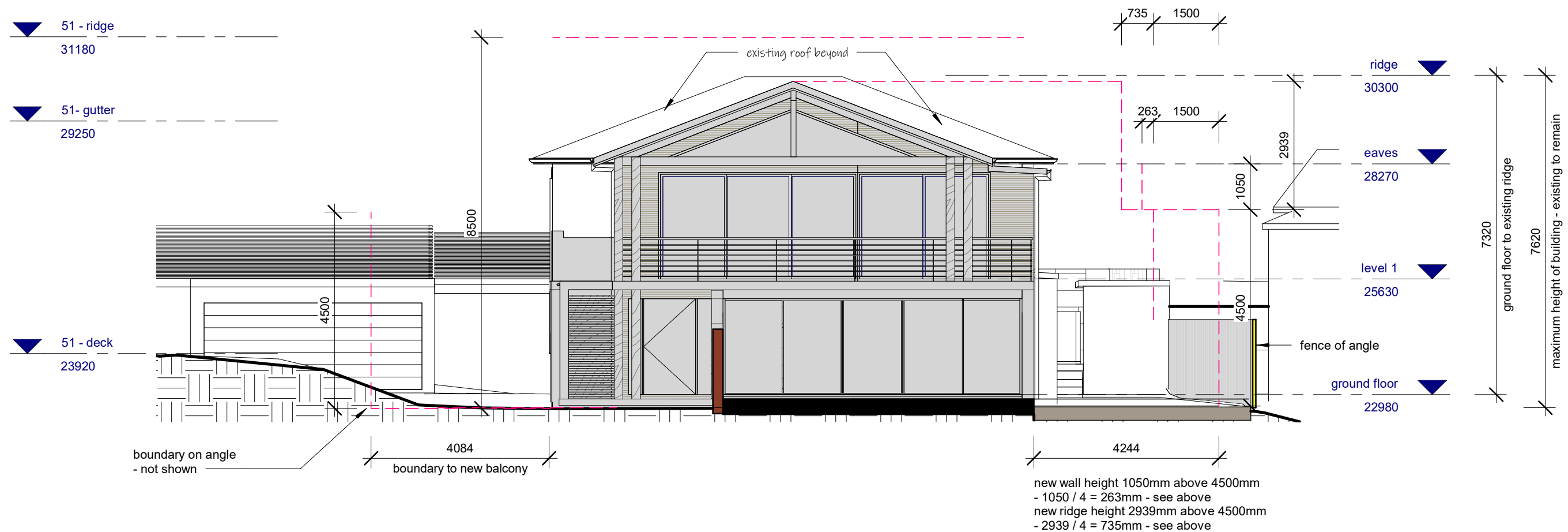
## Roof - CONCEPT PLAN

1 : 100



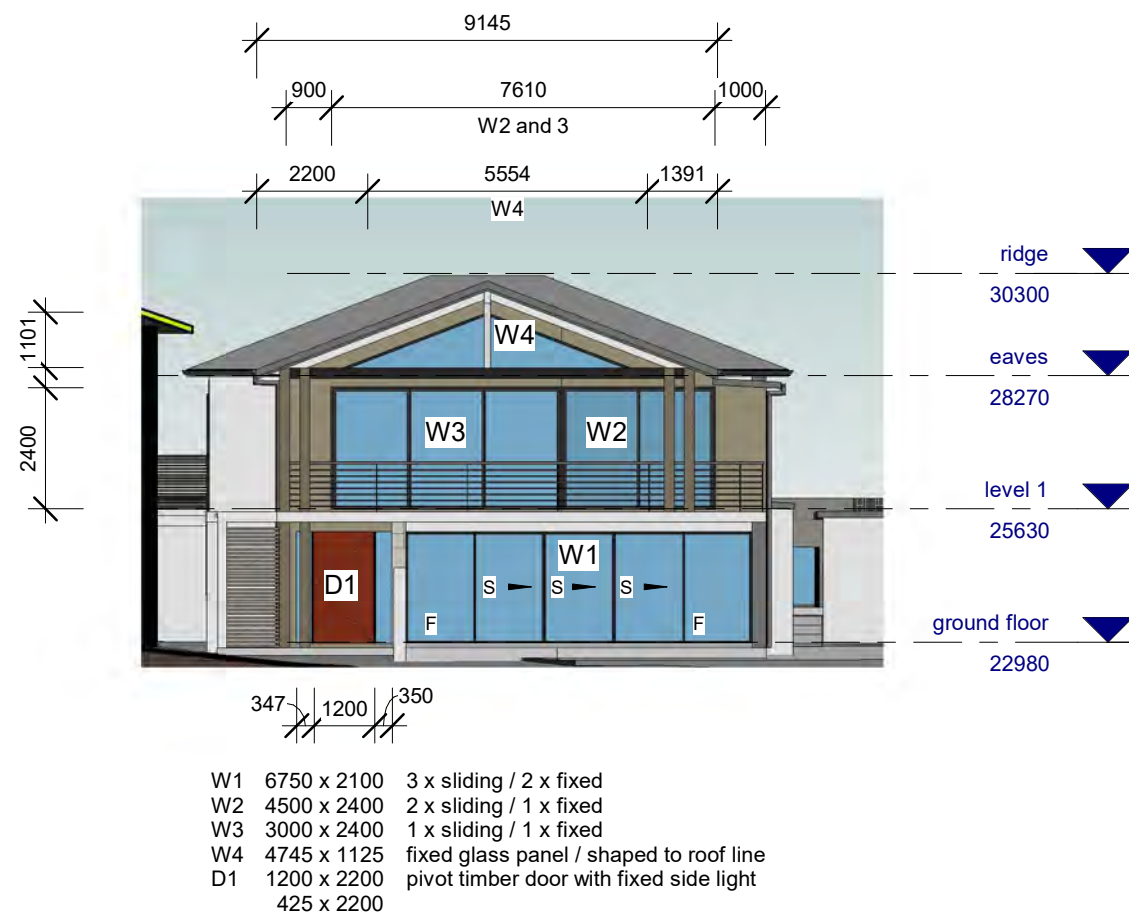




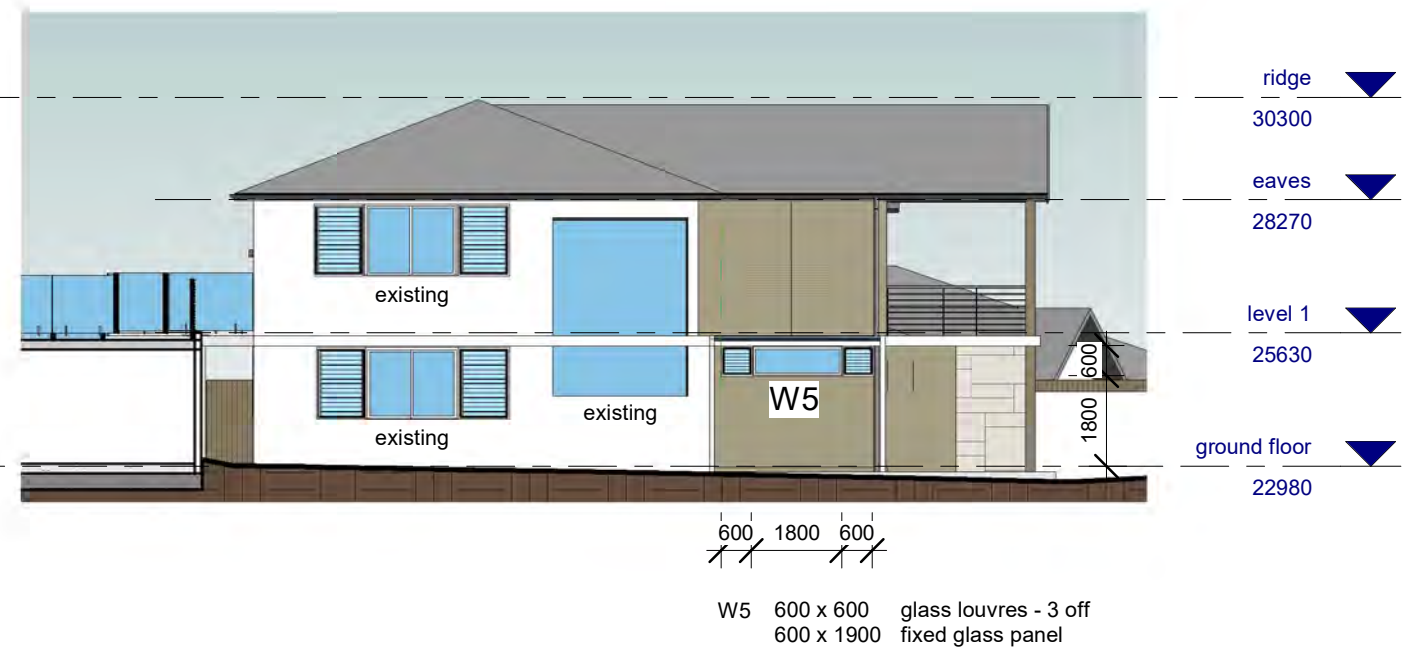


1 Section - SIDE SETBACK AND HEIGHT PLANES  
1 : 100

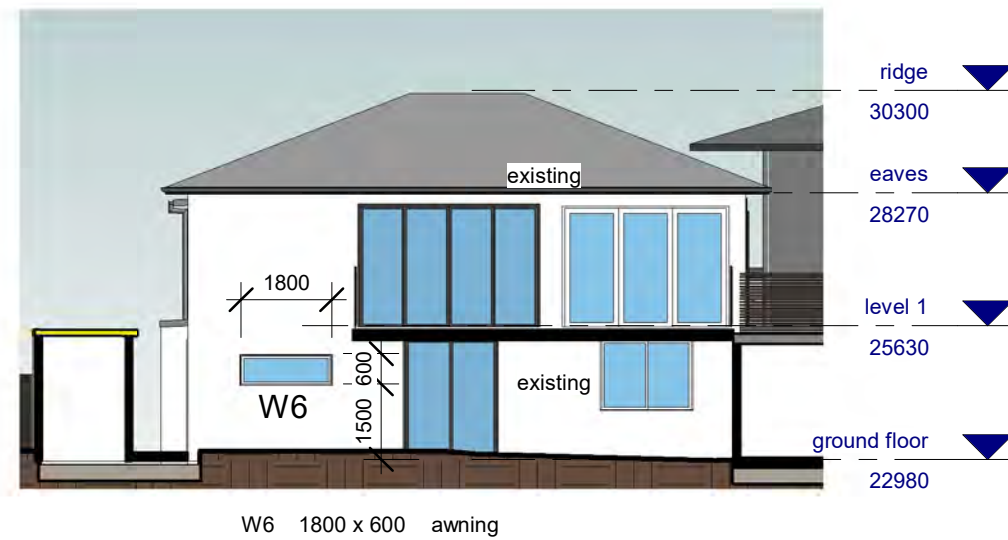




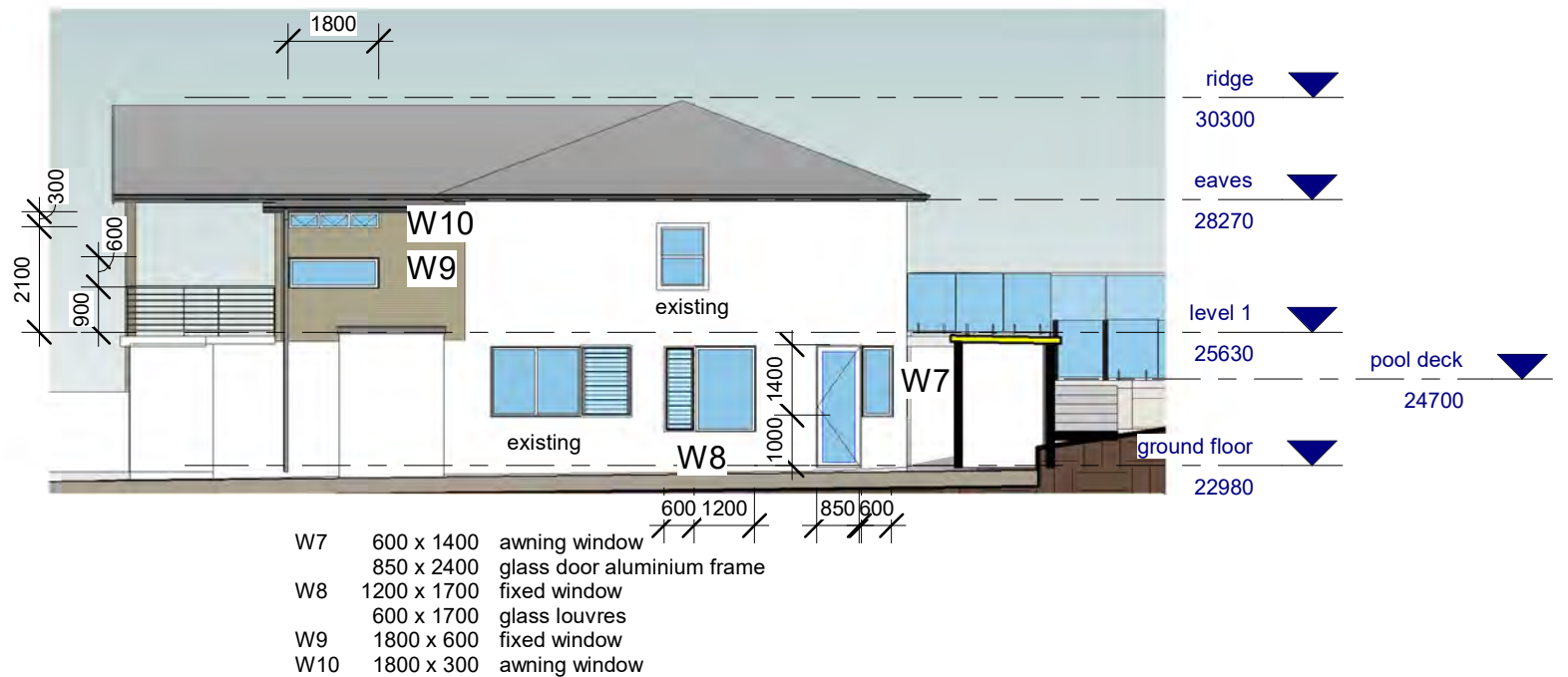
1 north elevation - windows  
1 : 150



2 east elevation - windows  
1 : 150

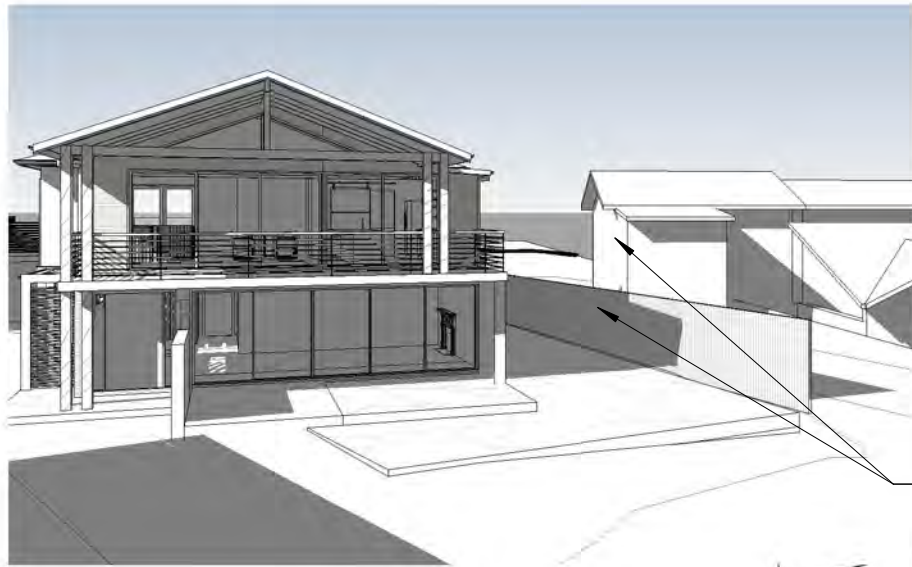


3 south elevation - windows  
1 : 150

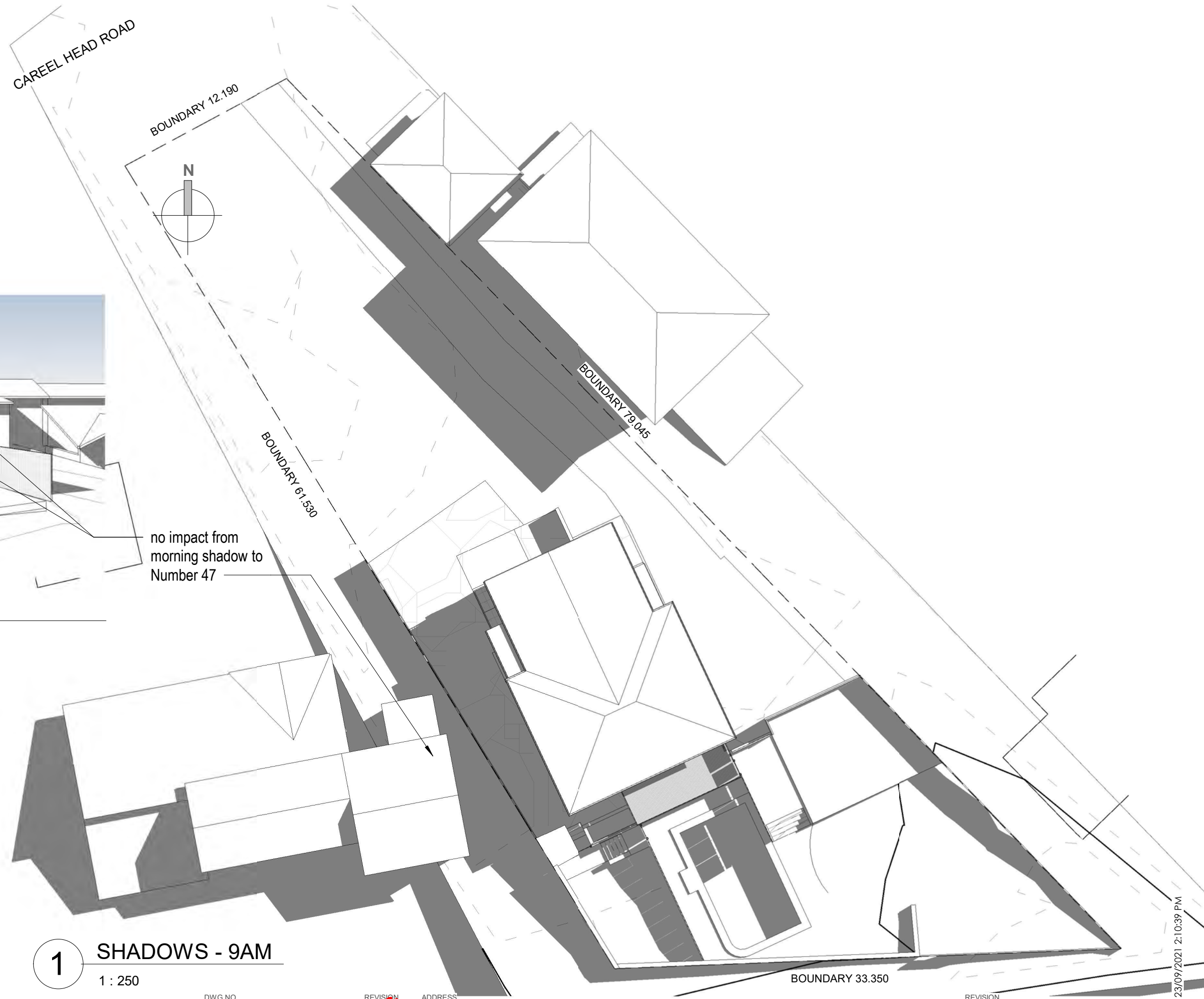


4 west elevation - windows  
1 : 150





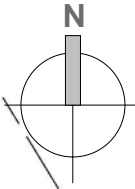
2 3D View - 9AM



1 SHADOWS - 9AM  
1 : 250

CAREEL HEAD ROAD

BOUNDARY 12.190



BOUNDARY 61.530

BOUNDARY 79.045

BOUNDARY 33.350

1 SHADOWS - 12 NOON  
1 : 250

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DWG NO.

DA-502

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49 CAREEL HEAD ROAD  
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SHADOWS - 12 NOON SEPT 22nd

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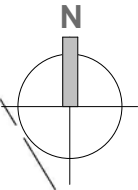
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CAREEL HEAD ROAD

BOUNDARY 12.190



BOUNDARY 61.530

BOUNDARY 79.045

BOUNDARY 33.350

1

## SHADOWS - 3PM

1 : 250

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PROJECT

**PROPOSED HOUSE ALTERATIONS  
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**DAN AND SARAH MORLEY**

DWG NO.

**DA-503**

DATE

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**49 CAREEL HEAD ROAD  
AVALON BEACH**

**SHADOWS - 3PM SEPT 22nd**

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roof colour



roof profile



black aluminium window frames



wall cladding



timber slat screen



feature door



stone clad entry wall



render colour



steel colour



balcony tile



hardwood timber posts



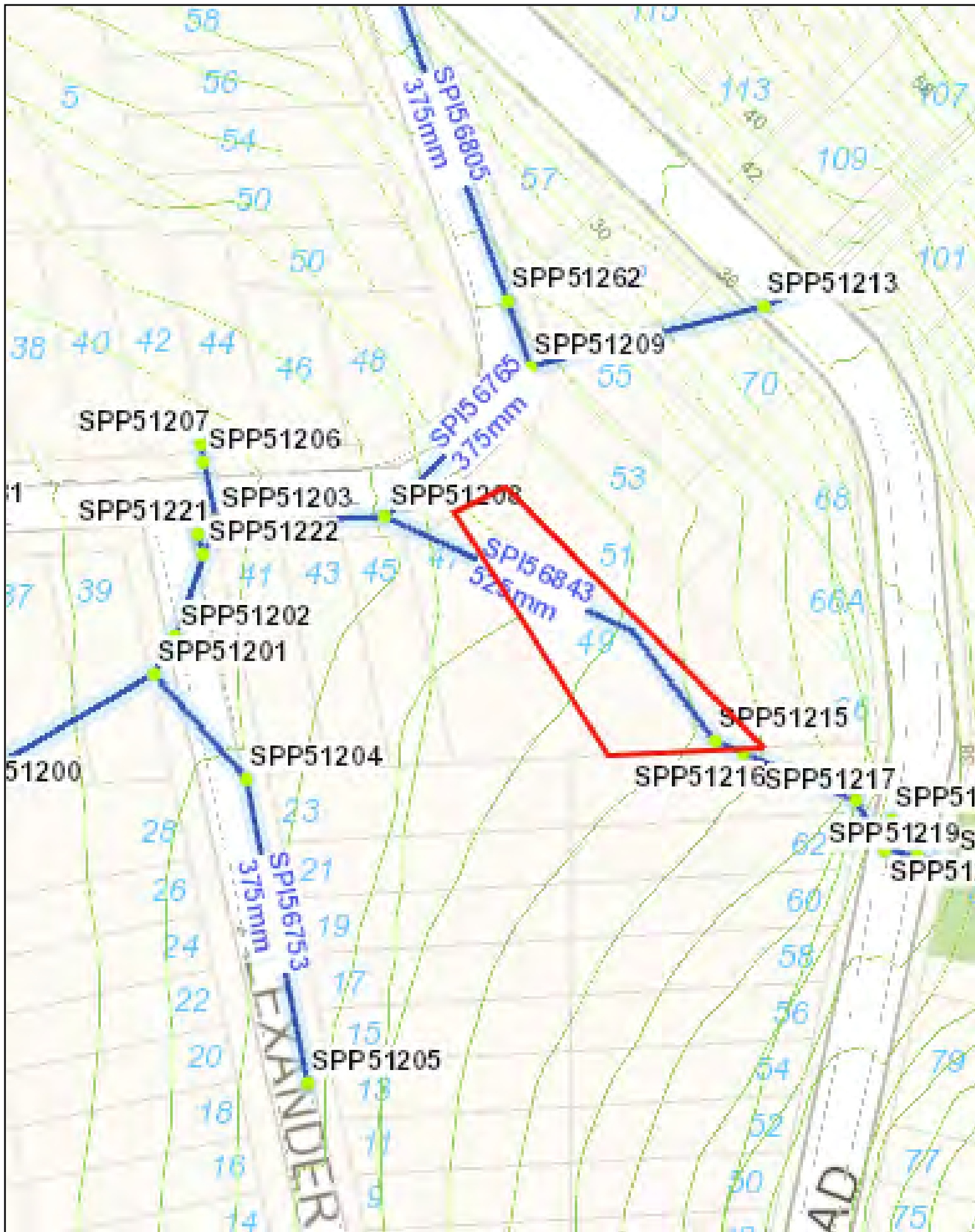
# 1 PROPOSED ALTERATION



## APPENDIX D

### Northern Beaches Council Stormwater Drainage Map





TITLE: <b>Northern Beaches Council Drainage Map</b>	<div data-bbox="922 1998 1024 2096" data-label="Image"> </div> <div data-bbox="1037 2056 1404 2119" data-label="Text"> <p><b>Consulting Engineers</b> STRUCTURAL - CIVIL - STORMWATER - REMEDIAL</p> </div>
FIGURE: <b>14</b>	ISSUE: <b>A</b>