GENERAL NOTES: - 09.08.2023 GENERAL

- G1. The drawings are to be read together with all Architects drawings and specifications. G2. Dimensions shall not be obtained by scaling from the drawings. All
- setting out dimensions shall be verified and discrepancies shall be referred to the Engineer and Architect / Designer prior to commencement of work. G3. Care is required during construction so that structural elements are not
- over stressed and that the works and excavations required therefore are kept stable at all times.
- G4. Design, materials and workmanship are to be in accordance with current S.A.A standards and statutory authority regulations except where varied by these documents.
- G5. Design live loads are in accordance with AS 1170.1-2002
- G6. Builder to ensure stability of existing structures in the vicinity of demolition and/or excavation works.

EXCAVATION

- EI. NOTICE PRIOR TO COMMENCEMENT. At least seven (7) days before commencement of any Excavation or Demolition works, the Builder or Client must give notice to the owner(s) of the adjoining property(s) and provide details of the proposed works, further, the adjoining owner must be made aware of the potential risk of cracking during construction. The Builder or Client is responsible to rectify any damage which may occur at their own cost.
- E2. UNLESS OTHER WISE APPROVED. Excavations near new or existing footings shall not be within the footing influence line.



FOOTINGS

- FI. FOUNDATION STRATA IS ASSUMED FOR DESIGN PURPOSES IN ACCORDANCE WITH AS 2870-2011 "RESIDENTIAL SLAB AND FOOTINGS-CONSTRUCTION". SEE FOOTNOTE. CLASSIFICATION TO BE VERIFIED BY A GEOTECHNICAL ENGINEER COMMISSIONED BY THE CLIENT FOR CERTIFICATION OF FOUNDATIONS.
- F2. Footings to be constructed and back filled as soon as possible following excavation to avoid softening by rain or drying out by exposure.
- F3. Footings must bear into undisturbed natural ground clear of organic material. Refer to details.
- F4. If rock or variable bearing strata is encountered during excavation of the footings all footings/piers are to be excavated to similar material of areater bearing capacity.
- The Engineer is to be contacted at that time for approval or review. F5. Footings to be cast in approved material having an allowable capacity

as follows: Sand Foundations:

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-02-2025

08-08-2024

Date

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Description

- SA1. Required bearing capacity 100 kPa.
- SA2. Trenches must be cleaned of all debris and hand compacted prior to placement of reinforcement. Clay Foundations:
- CLI. Required bearing capacity 200 kPa.
- CL2. Trenches must be cleaned of all debris. Soft spots must be cut out and filled as per compacted fill notes, prior to placement of reinforcement. Shale Foundations:
- SH1. Required bearing capacity 600 kPa.
- SH2. Excavation for footings into shale must be cast or capped with plain concrete on the same day as excavation.
- Sandstone Foundations:
- SSI. Required bearing capacity 1000 kPa.
- SS2. Scrape weathered surface to remove cleaved sandstone under footings. Refer adjacent for assumed Design bearing strata.
- F6. Future development of neighbouring properties may affect ground water conditions on this site. Consequently, reactivity in subgrade beneath footings may be locally altered therefore putting footing at risk of differential settlement. We recommend that, particularly in clay subarades, agricultural drainage is installed to the upstream perimeter of the building at a distance from the building which is outside the zone of influence of the footings. The agricultural drain must be installed below the fluctuating seasonal zone
- which should be identified by aeotechnical investigation. CONCRETE
- Φ Cl. All workmanship and materials shall be in accordance with AS 3600-2018.

REVISED PROJECT ADDRESS

ISSUED FOR CONSTRUCTION

- C2. Concrete quality shall be as follows and shall be verified by tests.
- C3. All concrete unless otherwise noted shall have a slump of 80mm at point of placement, a max. aggregate size of 20 mm. No water shall be added to the mix prior to or during placement of concrete. Strength as specified on plans.
- C4. Clear concrete cover to reinforcement shall be as follows unless otherwise shown-AЗ

ELEMENT	INTERIOR	EXTERIOR	EXTERIOR CAST AGAINST GROUND
FOOTINGS	-		50
COLUMNS/PEDESTALS	30 UNO	REFER TO PLAN	-
SLABS/WALLS	25	REFER TO PLAN	40 ON MEMBRANE
BEAMS	25 UNO	REFER TO PLAN	50
BLOCKWORK	55 FROM APPROPRIATE FACE		

- C5. Sizes of concrete elements do not include thickness of applied finishes.
- C6. All Construction Joints locations shall be approved by the Structural Engineer.
- C7. Beam depths are written first and include slab thickness, if any,
- C8. No holes or chases other than those shown on the structural drawings shall e made in concrete elements without the prior approval of the engineer.
- C9. Shrinkage reducing admixtures such as 'Eclipse' or approved equivalent, if specified, must be added to mix prior to pour.
- C10. Water reducing agents, if specified, must be added to mix prior to pour. No extra water is to be added to increase slump
- CII. Where vertical slab/beam surfaces are formed against a masonry (or other) wall, provide 10 mm styrene separation material
- C12. Water must not be added to concrete mix prior to placement of concrete.
- C13. Above covers may have to be adjusted if fire rating is a requirement
- C14. Any structural concrete slab that is designed in accordance with Australian Standards is susceptible to shrinkage cracking due to the nature of the hydration and curing process involved in setting. A certain level of cracking is therefor considered acceptable in the Standards and consequently if a crack free surface is necessary then a proprietary topping system should be considered. REINFORCEMENT
- RI. All reinforcement specified is Grade D500 unless noted otherwise.
- R2. Reinforcement is represented diagrammatically it is not necessarily shown in true projection.
- R3. Top reinforcement is to be continuous over supports. Bottom reinforcement to be lapped at supports.
- R4. Welding of reinforcement shall not be permitted unless shown on the structural drawings.
- R5. Pipes or conduits shall not be placed within the zone of concrete cover to the reinforcement without the approval of the engineer.
- R6. All reinforcing bars and fabric shall comply with AS 4671-2019.
- R7. Reinforcement symbols:
- N Grade 500N deformed bar (D500) Normal Ductility
- R Grade 250N plain round bar (R250) Normal Ductility SL - Grade 500L weided deformed ribbed mesh (D500)
- Square Low Ductility.
- RI Grade 5001 welded defarmed ribbed mesh (D500) Rectangular Low Ductility.

The number immediately following these symbols is the number of millimeters in the bar diameter

- 8 N12-250, denotes 8, Grade 500N deformed bars, 12 mm diameter at 250 cts.
- R8. Fabric reinforcement to be lapped 1 complete square + 25 mm unless noted otherwise
- R9 All reinforcement shall be firmly supported on bar chairs spaced at a maximum of 750 centres both ways under rod and fabric reinforcement. Reinforcement shall be tied at alternate intersections.
- FORMWORK
- FWI. Formwork must be cleaned of all debris prior to casting of concrete.
- FW2. Minimum stripping times for form work shall be as recommended in
- AS 3610.1-2018 or as directed by the engineer. FW3. The finished concrete shall be a dense homogeneous mass, completely filling the form work, thoroughly embedding the reinforcement and free of stone pockets. All concrete elements including slabs on ground and
- footings shall be compacted with mechanical vibrators. FW4. Curing of all concrete is to be achieved by keeping surfaces continuously wet for a period of 3 days, followed by prevention of loss of moisture for seven days followed by a gradual drying out . Approved sprayed on curing compounds may be used where no floor finishes are proposed. Polythene sheeting or wet hessign may be used if protected from wind and traffic.
- BRICKWORK

DOCUMENT CERTIFICATION

Date

NB

HENK. с.,

Bv:

Stewart McGeady

BE(Civil), MIEAust., PDP, PRE, DEP.

(Director NB Consulting Engineers)

18 Feb '25 /////

The copyright of this drawing remains with NB Consulting Engineers

- BRI. Brickwork is to be constructed to AS 3700-2018.
- BR2. Two layers of approved greased metal based slip material shall be used over all load bearing walls that support concrete slabs and placed on smooth brickwork or trowelled mortar finish. Non load-bearing walls shall have 10 mm compressible material and ties to the slab soffit. BR3. No brickwork shall be constructed on suspended slabs until
- all propping has been removed from the underside of the slab and the concrete has the specified 28 day cylinder strenath verified by tests.

Sydney: Ph: (02) 9984 7000

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 $\langle \Box \rangle$

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STRUCTURAL • CIVIL • STORMWATER • REMEDIAL

A.C.N. 076 121 616 A.B.N. 24 076 121 616

- BR4. Control joints to be placed at a maximum of 8m centres or in accordance with AS 3700-2018.
- BR5. Exposure grade bricks to be used below damp proof course.
- BR6. Vertical control joint material where specified on plan between slabs and brick walls shall be: 10 mm Spandex External UNO. Bitumastic fibreboard internal UNO
- BR7 Provide stainless steel wall ties below DPC to AS 3700-2018 Provide advanized wall ties above DPC to AS 3700-2018 and Local Council Specifications
- BR8. Extruded bricks are not suitable for standard Brick Retaining Walls detailed
- herein due to excessive brick growth potential, high porosity and insufficient mass. BR9. Excessive brick growth can cause cracking in walls and render. Ensure that manufacturers delivery certificates state that growth rate is within code limits.

BLOCKWORK

- BL1. Concrete blocks shall have a minimum compressive strength of 15 MPa and conform to AS 3700-2018. Masonry to be constructed to AS 3700-2018.
- BL2. Where cores of hollow blocks are to be filled, properly compacted 20MPa concrete with 10 mm aggregate and 230 mm slump shall be used. Clean out openings must be utilized for all cores.
- BL3. Location of actual starters is critical to suit block cores, allow 55 mm cover from the outside face of blockwork. All reinforcement lap lengths to conform to AS 3600-2018. BL4. Control joints to be placed at a maximum of 8 m centres
- or in accordance with AS 3700-2018.
- BL5. Vertical control joint material where specified on plan between slabs and brick walls shall be: 10 mm Spandex External UNO. Bitumastic fibreboard internal UNO
- BL6. Retaining walls or any reinforced and concrete core filled block walls to be of Double 'U' Block Construction.
- BL7. No blockwork shall be constructed on suspended slabs until all propping has been removed from the underside of the slab and the concrete has the specified 28 day cylinder strength verified by tests. unless approved by the Structural Engineer
- BL8. Max. pour height for unrestrained blockwork is 2000.
- BL9. Contractor to ensure all cores are filled with concrete where additional loads are to be applied to existing core filled blockwork walls.
- STEEL
- S1. All Structural steelwork to be Grade 300 or greater.
- Design, fabrication and erection to be in accordance with AS 4100-2020. 52. Materials and workmanship shall comply with AS 1250-1981, SAA Steel
- Structures Code and the specification for Structural Steel 53. Rolled steel sections including steel plates shall comply with AS 3678-2016.
- 54. Cold formed steel sections shall be Grade 450 Zinc coated in accordance with AS 1538-1988.
- 55. Welded and seamless steel hollow sections shall comply with AS 1163-2016. Grade 350. S6. Bolt Designation:
- 4.65 Commercial bolts Grade 4.6, snug tightened.

contractor using High Strength, Non-Shrink grout.

SI3. Purlin bolts shall be M12 - 4.65 galvanised.

Structural Steel Welding Code.

accordance with AS 4100-2020.

- 8.85 High Strength structural bolts Grade 8.8, snug tightened.
- 8.8TB High Strength structural bolts Grade 8.8, fully tightened to
- AS 1511-1984 and acting as a Bearing Joint. 8.8TF - High Strength structural bolts Grade 8.8, fully tensioned to
- AS 1511-1984 and acting as a Friction Joir

59. All welding shall be carried out in accordance with AS 1554.2-2021 SAA

All butt welds shall be complete penetration butt welds category SF

S12. Fabrication and erection tolerances for Structural Steelwork shall be in

SI4. All specification, inspection and maintenance of steel coating systems

the NCC BCA Table 3.4.4.2 and AS/NZS 2312-2014 PART 1 and 2

for any specification, inspection and maintenance of steel coating

INTERNAL "LOW" GREATER THAN IKM FOR THE COAST AND ENCLOSED

Galvanised(HDG500) to AS/NZS 4680-2006 (after fabrication)

Galvanised in accordance with AS/NZS 2699.3-2020 and Epoxy primer

(two pack) to AS/NZ 3750.13-1997 plus Epoxy micaceous iron oxide

Galvanised(HDG500) to AS/NZS 4680-2006 (after fabrication)

(two pack) to AS/NZ 3750.14-1997 (after fabrication)

EXTERNAL "MEDIUM" GREATER THAN 1km FROM COAST

Galvanised in accordance with AS/NZS 2699.3-2020 (after fabrication)

Drawing Title

BAYVIEW

GENERAL NOTES

INTERNAL "MEDIUM" & TO Ikm FROM THE COAST AND ENCLOSED

Epoxy primer (two pack) to AS/NZ 3750.13-1997

CAVITY "R3" GREATER THAN 5000 FROM COAST

CAVITY "R4" 0 TO 500m FROM COAST

NIELS & LENORE WALTER

rchitect

Client:

systems. At a minimum NB Consulting recommends the following:

to be the responsibility of the project manager and in accordance with

NB Consulting Engineers does not warrant certify or take responsibility

SII. Grouting of anchor bolt sleeves and base plates shall be completed by the

- Unless noted otherwise, all bolts will be 8.85.
- 57. Unless shown otherwise, minimum connection shall be 21/16 bolts, 10 thick gusset plates, 6mm continuous fillet welds. Load indicating washers shall be used in all fully tensioned joints (8.8TF \$ 8.8TB).

SIO. Unless noted otherwise all welds shall be category SP using E41xx Electrodes.

EXTERNAL "HIGH" 500m TO 1km FROM COAST Galvanised(HDG900) to AS/NZS 4680-2006 (after fabrication) EXTERNAL "VERY HIGH" O TO 500m FROM COAST Galvanised(HDG900) to AS/NZS 4680-2006 (after fabrication) and Epoxy primer (two pack) to AS/NZ 3750.13-1997 plus Epoxy micaceousiron oxide (two pack) to AS/NZ 3750.14-1997 All protective coatings to be maintained in accordance with the required Durability - Years to first maintenance specification from AS/NZS 2312-2014 PART 1 and 2 S15. Workshop drawinas shall be prepared and two copies submitted to the engineer for review prior to fabrication commencement. TIMBER TI, All workmanship and materials to be in accordance with AS 1684.2-2021. AS 1720 1-2010 and AS 3959-2018. All soft wood to be Grade E7 unless noted otherwise. All hardwood to be minimum Grade F14 unless otherwise noted. Exposed timber to be CCA treated (to AS 1604-2021) redried after full impregnation, or durability class 1, 2 or 3. ALL SOFTWOOD TIMBER FRAMING TO HAVE A MINIMUM TREATMENT PROTECTION OF H2 or T2 TREATED FOR TERMITE PROTECTION UNLESS NOTED OTHERWISE. ALL TIMBER SHALL BE KILN DRIED / SEASONED. DO NOT USE GREEN TIMBER T2. All joists deeper than 150 to have blocking over support bearers and at a maximum 3000 centres T3. Roof trusses to be designed by the manufacturer to the relevant standards. Pre camber to be an amount equal to dead load deflection u.n.o. T4. All holes for bolts to be exact size. Washers to be used under all heads and nuts and to be at least 2.5 times the bolt diameter. Bolts to be MI6 grade 4.6 unless noted otherwise. T5. Treat all exposed cut ends with Reseal by Protim to manufacturers specification to achieve required Hazard Level Exposure Classification. T6. Battens for T \$ G to be Kiln Dried to 12 %. 38mm minimum deep treated pine or as recommended by supplier. Flooring to be installed no sooner than 28 days after slab pour T7 Hot dip aalvanized nails/clouts/screws to be used with M all timber connections. T8. Continuous nailing must not be used for any timber connections. Ð T9. All exposed CCA treated pine to have an application of penetrating sealer to reduce warping and twist of the Ū timber due to varying moisture content in service. T10. All Stud walls to be 90x45 F7 Kiln Dried T2 Treated at 450 Cts and noggings to AS 1684.2-2021. U.N.O. Č COMPACTED FILL _ CFI. Only to be used with approval by Engineer \$ to be certified by 60 a Geotechnical Engineer CF2. Clear organic material, topsoil and any uncontrolled existing fill under proposed slabs/footings. CF3. Filling shall be granular material compacted in not more than 200 mm layers to a minimum dry density ratio (AS 1289.6.3.3-1997) of 98 percent standard maximum dry density subject to Geotechnical Engineer's Approval. CF4. During clearing and excavation for slabs and footings cut out soft **b**0 spots and fill as above. INSPECTIONS BY ENGINEER C 48 HOURS NOTICE IS REQUIRED BEFORE ANY SITE INSPECTION ulti ANY STRUCTURAL ELEMENT NOT INSPECTED BY NBC WILL NOT BE CERTIFIED BY NBC 1. Bearing strata of all footings prior to concrete pour by Geotechnical Engineer. 2. Any reinforcement prior to concrete pour. 3. Timber and Steel framing prior to cladding or lining. 4. Steel lintels after installation. N 5. CONTACT YOUR PCA (Principal Certifying Authority) AS TO REQUIREMENTS FOR MANDATORY CRITICAL STAGE INSPECTIONS IN ACCORDANCE WITH Ċ REVISED EP#A ACT REGULATIONS EFFECTIVE JULY 1, 2004. 6. Inspection by Geotechnical Engineer over 1.5m of vertical cut through Sandstone 0 bed rock to permit identification of defects and remedial measures initiated 7. ALTERATIONS AND ADDITIONS, Engineer to Inspect Existing Structure following completion of soft demolition. IF IN DOUBT ASK ASSUMED FOUNDATION CLASSIFICATION FOR DESIGN PURPOSES -ASSUMED BEARING STRATA FOR DESIGN PURPOSES - CLAY, 200 kPa. CONTRACTOR TO ENGAGE GEOTECHNICAL CONSULTANT TO VERIFY FOUNDATION CLASSIFICATION FOUNDATION CLASSIFICATION AND BEARING CAPACITY VERIFICATION TO BE OBTAINED IF CERTIFICATION OF THESE ITEMS IS REQUIRED. Project: PROPOSED RETAINING WALL Date Design Drawn:

9 \$ 10 TAMINGA STREET JULY 2024 L.B./ S.S. HENK. Job No Drawing No: Issue: 501 В



