

TRAFFIC AND PARKING IMPACT ASSESSMENT OF MIXED-USE DEVELOPMENT AT 28 LOCKWOOD AVENUE, BELROSE



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1 INTRODUCTION

M^cLaren Traffic Engineering (MTE) was commissioned by *Avenue Property* to provide a Traffic and Parking Impact Assessment of the proposed Mixed-Use Development at 28 Lockwood Avenue, Belrose, as depicted in **Annexure A**.

1.1 Description and Scale of Development

The proposed mixed-use development has the following scale relevant to traffic and parking:

- Residential component consisting of:
 - Three (3) x one-bedroom apartments;
 - 27 x two-bedroom apartments;
 - 21 x three-bedroom apartments;
- Retail with a total of 3,767m² Gross Floor Area (GFA) consisting of:
 - 1,756m² GFA Slow Trade;
 - 346m² GFA Fast Trade;
 - 1,665m² GFA Specialty Shops;
- Gymnasium with a total of 997m² GFA;

The site layout includes a basement car park with a total of **193** car parking spaces including 116 retail/gymnasium spaces (including 4 disabled) and 77 residential spaces (including 5 disabled and 10 visitor). Vehicular access to the car park is proposed via separated oneway driveways from Glenrose Place, the lower order road.

1.2 State Environmental Planning Policy (Infrastructure) 2007

The proposed development includes over 2,000m² of shop area and therefore does qualify as a development with relevant size and/or capacity under Clause 104 of the SEPP (Infrastructure) 2007. Accordingly, formal referral to the Roads and Maritime Services (RMS) is necessary and Northern Beaches Council officers can determine this proposal with input from the RMS, should RMS provide any comments or conditions.

1.3 Site Description

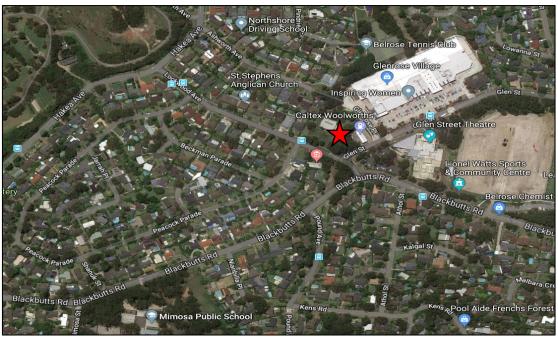
The subject site is currently occupied by a vacant building, formerly known as the Belrose Library, and is zone B2 - Local Centre by the Warringah Local Environmental Plan 2011 (LEP). The site fronts Glenrose Place to the north, Glen Street to the east and Lockwood Avenue to the west. Vehicular access to the basement carpark is provided via separated one-way entry and exit driveways onto Glenrose Place. Access for loading vehicles is shared with the one-way passenger vehicle exit driveway.



The site is generally surrounded by low to medium-density residential dwellings. Glenrose Village is located directly north-east from the subject site, Glen Street Theatre is located directly east from the site, Mimosa Public School and Davidson High School are located south of the site and a Caltex Woolworth service station adjoins the site to the south.

1.4 Site Context

The location of the site is shown in **Figure 1** & **Figure 2** below.



Site Location

FIGURE 1: SITE CONTEXT - AERIAL PHOTO

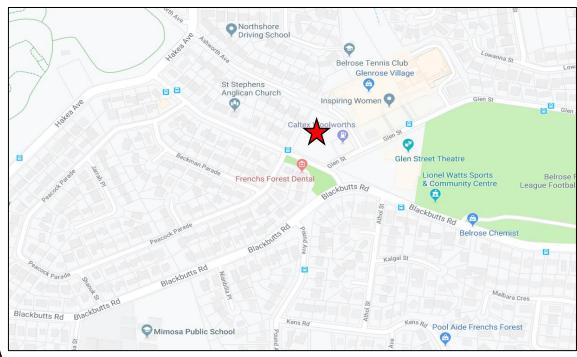




FIGURE 2: SITE CONTEXT - STREET MAP



2 EXISTING TRAFFIC AND PARKING CONDITIONS

2.1 Road Hierarchy

The road network servicing the site has the following characteristics:

2.1.1 Lockwood Avenue

- Unclassified LOCAL Road;
- Approximately 13m in width facilitating two traffic flow lanes (one in each direction) and kerbside parking;
- No speed limit signposted, default 50km/h applies;
- Unrestricted kerbside parking permitted on both sides of the road with areas of "No-Stopping" along the site frontage.

2.1.2 Glen Street

- Unclassified LOCAL Road;
- Approximately 11m in width facilitating two traffic flow lanes (one in each direction) and kerbside parking;
- Signposted 50km/h speed limit;
- "No-Stopping" restriction throughout the street;
- Unrestricted kerbside parking permitted along both sides of the street to the north of Glenrose Village.

2.1.3 Glenrose Place

- Unclassified LOCAL Road;
- Approximately 12m in width facilitating three traffic flow lanes (one northbound; 2 southbound);
- No speed limit signposted, default 50km/h applies;
- Two (2) disabled kerbside parking spaces available at the end of the street;
- No other kerbside parking permitted along both sides of the street.

2.2 Existing Traffic Management

- Priority controlled intersection of Glen Street / Glenrose Place;
- 'GIVE-WAY' sign-controlled intersection of Lockwood Avenue / Glen Street;



- Pedestrian crossing across Lockwood Avenue adjacent to the intersection of Lockwood Avenue / Glen Street;
- Pedestrian crossing across Glen Street adjacent to the intersection of Glen Street / Glenrose Place.

2.3 Existing Traffic and Parking Environment

Traffic counts were completed at the intersections Glen Street / Glenwood Place, Glen Street / Blackbutts Road, and Glen Street / Lockwood Avenue on Thursday 25th July 2019 between 7-9am and 4-7pm, and Saturday 27th July 2019 between 10am-2pm representing a typical weekday and weekend peak traffic flow periods. The results of the surveys are reproduced in **Annexure B** for reference.

2.3.1 <u>Intersection Performances</u>

The traffic volumes recorded in the surveys have been used to assess the existing intersection performance using SIDRA INTERSECTION 8.0. The results of the analysis are summarised in **Table 1** below, with detailed SIDRA outputs reproduced in **Annexure C** for reference.



TABLE 1: EXISTING INTERSECTION PERFORMANCES (SIDRA INTERSECTION 8.0)

Intersection	Peak Hour	Degree of Saturation ⁽¹⁾	Average Delay ⁽²⁾ (sec/veh)	Level of Service ⁽³⁾⁽⁴⁾	Control Type	Worst Movement	95th Percentile Queue
		EX	ISTING PERFOR	MANCE			
	AM	0.22	4.1 (Worst: 8.6)	NA (Worst: A)		RT from st: Glen Street	1 veh (7.1m) st: Glen Street
Glen St / Lockwood Av	PM	0.30	4.4 (Worst: 9.4)	NA (Worst: A)	Give Way	RT from st: Glen Street	1.6 veh (11.1m) st: Glen Street
	SAT	0.34	4.7 (Worst: 10.2)	NA (Worst: A)		RT from st: Glen Street	1.9 veh (13.2m) st: Glen Street
	АМ	0.24	2.8 (Worst: 10.5)	NA (Worst: A)	Give Way	RT from Glen Street	0.6 veh (4.2m) Glen Street
Glen Street / Blackbutts Road	PM	0.25	3.8 (Worst: 10.1)	NA (Worst: A)		RT from Glen Street	1 veh (7m) Blackbutts Road
	SAT	0.23	4.1 (Worst: 9.6)	NA (Worst: A)		RT from Glen Street	1.1 veh (7.8m) Blackbutts Road
	AM	0.23	3.5 (Worst: 7.3)	NA (Worst: A)		RT from Glenrose Place	1.2 veh (8.4m) Glen Street
Glen Street / Glenwood Place	PM	0.25	4.4 (Worst: 8.1)	NA (Worst: A)	Give Way	RT from Glenrose Place	1.3 veh (9.3m) Glen Street
	SAT	0.29	5.1 (Worst: 9.7)	NA (Worst: A)		RT from Glenrose Place	1.6 veh (11.2m) Glen Street

NOTES:

As shown above, the nearby intersections operate at Level of Service (LoS) A during the weekday morning, weekday afternoon and Saturday midday peak periods. This represents minimal delays and additional capacity.

 ⁽¹⁾ Degree of Saturation is the ratio of demand to capacity for the most disadvantaged movement.
 (2) Average delay is the delay experienced on average by all vehicles. The value in brackets represents the delay to the most disadvantaged movement.

⁽³⁾ Level of Service is a qualitative measure of performance describing operational conditions. There are six levels of service, designated from A to F, with A representing the best operational condition and level of service F the worst. The LoS of the intersection is shown in bold, and the LoS of the most disadvantaged movement is shown in brackets.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.



2.4 Public Transport

The subject site is within 200m walking distance of existing bus stops (ID: 208668, ID:2086104, ID:208687, ID:208558, ID 208668, ID: 2086104, ID 208687, ID: 208558) servicing bus routes 141 (Austlink to Manly via Frenchs Forest & Seaforth), 271 (Belrose to City QVB), 274 (City QVB to Davidson via Frenchs Forest), 281 (Davidson to Chatswood), 282 (Davidson & Belrose to Chatswood) and 283 (Belrose to Chatswood) provided by Forest Coach Lines. The location of the site is shown on a local public transport network map in **Figure 3** below, indicating that the site is very well located with respect to public bus services.

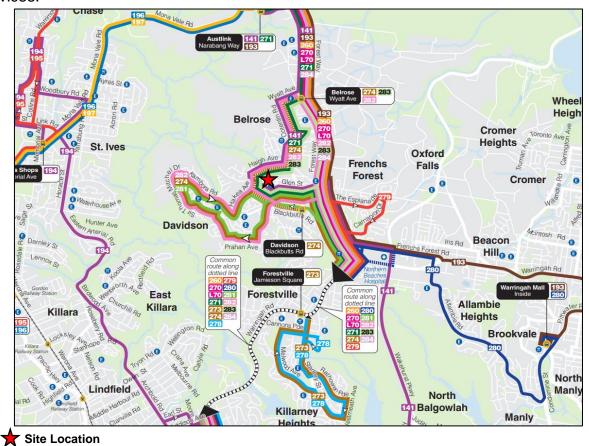


FIGURE 3: PUBLIC TRANSPORT MAP

2.5 Future Road and Infrastructure Upgrades

From *Northern Beaches Council* Development Application tracker and RMS Projects website, it appears that there are no future planned road or public transport changes that will affect traffic conditions within the immediate vicinity of the subject site.



3 PARKING ASSESSMENT

3.1 Council Car Parking Requirement

Reference is made to Warringah Development Control Plan – Part H: Appendices – Appendix 1: Car Parking Requirements which outlines the following car parking requirements for mixed use developments:

Residential

Multi-dwelling housing, Residential flat buildings, serviced apartments...

1 space per 1-bedroom dwelling

1.2 spaces per 2-bedroom dwelling

1.5 spaces per 3-bedroom dwelling

1 visitor space per 5 units or part of dwellings

Retail and Commercial

Shop (includes retail / business component of shop top housing, retail premises and neighbourhood shop)

1 space per 16.4m² GLFA (6.1 spaces per 100m² GLFA)

The above rate may be varied in shopping centre complexes, such as shopping malls, where multi-purpose trips predominate, in accordance with the following:

For 0-10,000m² GLFA - 6.1 spaces per 100m² GLFA

For 10,000-20,000m² GLFA – 5.6 spaces per 100m² GLFA

For 20,000-30,000m² GLFA – 4.3 spaces per 100m² GLFA

For more than 20,000m² GLFA – 4.1 spaces per 100m² GLFA

Recreational and tourist facilities

Gymnasium

4.5 spaces per 100m² GFA

The car parking requirement based upon the Council car parking rates is summarised in **Table 2**.



TABLE 2: COUNCIL DCP CAR PARKING REQUIREMENT

Land Use	Туре	Scale ⁽¹⁾	Rate	Spaces Required
	One-bedroom	3	1 per dwelling	3
Danidantial	Two-bedroom	27	1.2 per dwelling	32.4
Residential	Three- bedroom	21	1.5 per dwelling	31.5
	Visitor	51	1 per 5 dwellings	10
Residential Subtotal				77
Retail	0-10,000m ² GLFA	2,825.25m ² GLFA	6.1 per 100m ² GLFA	172
Gym	Gymnasium	997m² GFA	4.5 per 100m ²	45
Retail/Gym Subtotal				217
Total				294

Note: (1) The GLFA is taken to be 75% of the GFA as per Section 5.7.1 of the RMS Guide

As shown, the car parking requirement based on the Council DCP is **294** spaces.

3.2 RMS Car Parking Requirement

Reference is made to the *RMS Guide to Traffic Generating Developments* which outlines the following car parking requirement for the shopping centre portion of the development.

Peak Parking Demand per 1,000m²

$$PPD = 24 A(S) + 40 A(F) + 42 A(SM) + 45 A(SS) + 9 A(OM)$$

Where:

A(S): Slow Trade GLFA, include major Department stores such as David Jones and Grace Brothers, furniture, electrical and utility goods stores.

A(F): Faster Trade GLFA, includes discount department stores such as K-Mart and Target, together with larger specialist stores such Fosseys.

A(SM): Supermarket GLFA, includes stores such as Franklins and large fruit markets.



A(SS): Specialty Shops and Secondary retail GLFA, includes specialty shops and take-away stores such as McDonalds. These stores are grouped since they tend not to be primary attractors to the centre.

A(OM): Offices, medical GLFA

A(C): Cinemas

Gymnasium

Minimum - 4.5 spaces per 100m2 GFA

The resulting RMS car parking requirements for the subject mixed-use development are summarised in **Table 3** below. It is noted that the site is not located within 800m of a train station or within 400m of a B3 or B4 zone. Therefore, the Council's DCP car parking rates apply to the development in accordance with the Apartment Design Guide.

TABLE 3: RELEVANT RMS AND DCP CAR PARKING REQUIREMENTS

Land Use	Туре	Scale ⁽¹⁾	Rate ⁽²⁾	Authority	Spaces Required
	One-bedroom	3	1 per dwelling		3
Residential	Two-bedroom	27	1.2 per dwelling	Council DCP	32.4
Residential	Three-bedroom	21	1.5 per dwelling	Council DCP	31.5
	Visitor	51	1 per 5 dwellings		10
Residential Subtotal					77
	A(S): Slow Trade	1,317m ² GLFA	0.024 per m² GLFA		31.6
Retail	A(F): Faster Trade	259.5m ² GLFA	0.040 per m² GLFA	RMS Guide	10.38
	A(SS): Specialty Shops	1248.25m² GLFA	0.045 per m² GLFA		56.17
Gym	Gymnasium	997m² GFA	4.5 per 100m²	Council DCP	45
Retail/Gym Subtotal					143
Total	-	-	-		220

Note: (1) The GLFA is taken to be 75% of the GFA as per Section 5.7.1 of the RMS Guide

⁽²⁾ The peak shopping centre parking demand occurs on Thursday



As shown, the development requires a total of **220** car parking spaces based upon the RMS car parking requirements. This requirement is 75 spaces fewer than the Council DCP car parking requirement summarised in **Table 2**.

It is relevant to note that the Council's DCP car parking requirement for shopping centres is taken directly from *Table 5.2* of the *RMS Guide to Traffic Generating Developments*. The rates in this table are general and do not consider the specific type of retail uses within the shopping centre. The subject development does not include a supermarket, which is the highest parking generator in shopping centres. Therefore, the RMS parking requirement which provides consideration for types of retail uses is expected to provide a more accurate parking requirement for the subject site.

The site provides a total of **193** car parking spaces, representing a numerical shortfall of **27** parking spaces compared to the parking requirement summarised in **Table 3**. Justification for the parking shortfall is discussed in the following subsection.

3.3 Parking Shortfall Justification Strategy

Reference is made to the RMS Guide which states the following about complementary uses within retail precincts:

When it can be demonstrated that the time of peak demand for parking associated with the proposed shopping centre and the adjacent land uses do not coincide, or where common usage reduces total demand, a lower level of parking provision may be acceptable.

The development contains a gymnasium and a shopping centre precinct, which are expected to experience peak usage at different times of the day and week. Based on an examination of the profile of "peak times" facility (available on website searches) which provides both real-time and historical usage data for commercial premises, retail in Belrose generally peaks during business hours whilst gyms peak before 8am and after 6pm. As such, it is reasonable that the car parking demands of the retail and gym can be accommodated within a shared pool of parking, as their peaks will not coincide. Comparison is made to Glenrose Village shopping precinct located across Glenrose Place from the subject development. Glenrose Village contains an all-hours gym, along with various specialty shops, supermarkets and other retail/commercial premises. MTE has utilised usage profile data from Glenrose Village to chart the expected peak car parking demands for the proposed gym and retail components, as shown in **Figure 4** through to **Figure 7**.

The working tables showing the Glenrose Village and gym peak usage profiles are reproduced in **Annexure D** for reference.



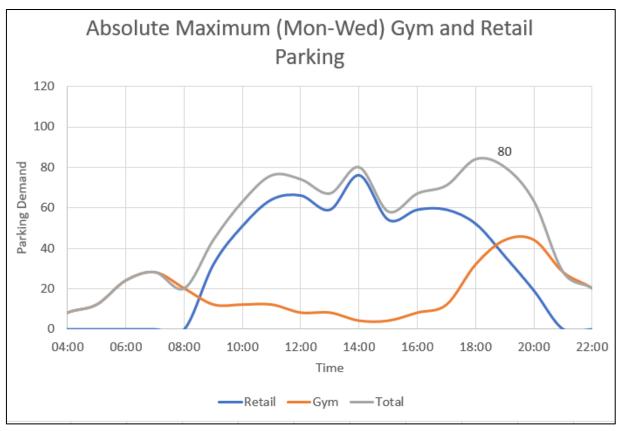


FIGURE 4: MONDAY-WEDNESDAY SHARED CAR PARKING DEMAND

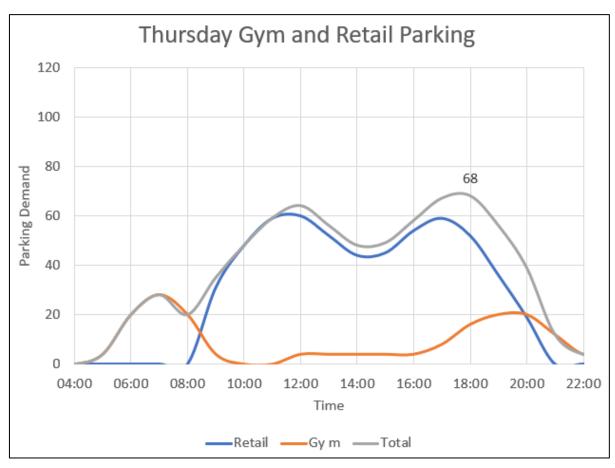


FIGURE 5: THURSDAY SHARED CAR PARKING DEMAND



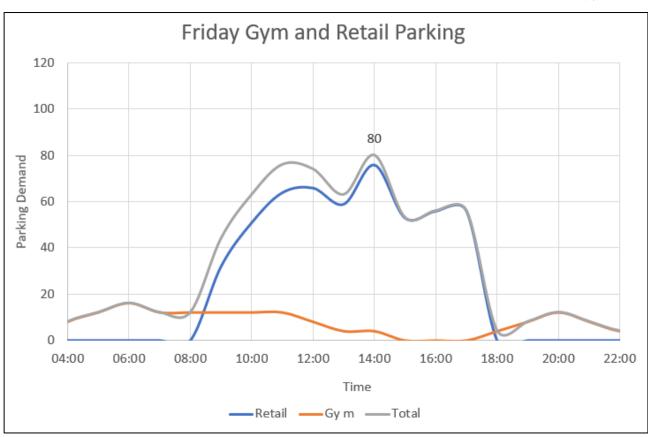


FIGURE 6: FRIDAY SHARED CAR PARKING DEMAND



FIGURE 7: SATURDAY SHARED CAR PARKING DEMAND



As shown, the peak shared car parking demand for the gym and retail components is **102** parking spaces, which occurs on Saturday at 11:00am. The proposed plans provide **116** parking spaces for retail and gym shared parking, which satisfies the practical peak car parking demand of 102 spaces with a surplus of 14 spaces for sensitivity purposes.

The assessed car parking demand is summarised in **Table 4**.

TABLE 4: ASSESSED CAR PARKING DEMAND

Land Use	Туре	Scale ⁽¹⁾	Rate	Spaces Required	Spaces Provided
	One-bedroom	3	1 per dwelling	3	
Desidential	Two-bedroom	27	1.2 per dwelling	32.4	67
Residential	Three- bedroom	21	1.5 per dwelling	31.5	
	Visitor	51	1 per 5 dwellings	10	10
Residential Subtotal				77	77
Retail/Gym	Retail	2,825.25m ² GLFA	Soo Figure 7	102 ⁽²⁾	116
Saturday AM	Gym	997m² GFA	See Figure 7	10217	110
Total	-	-	-	179	193

Note:

(1) The GLFA is taken to be 75% of the GFA as per Section 5.7.1 of the RMS Guide

(2) Demand

As shown, the assessed car parking demand is **179** spaces based upon data from Glenrose Village for shopping centre and gym demand. The development provides **193** spaces, which represents a 14-space surplus compared to the assessed car parking demand.

The above assessment is considered to be conservative based upon the following factors:

- The development provides 15 car wash spaces which have not been included in the parking provision. Carwash spaces in shopping centres operate as dual use spaces for uses within the development. For example, some portion of retail/gym patrons will utilise the car wash service facility and associated car parking space and have their car washed while they patronise the retail or gym uses. In this case, a retail/gym space will remain vacant, which reduces the gym/retail parking demand within the main and general car parking area.
- The development provides 12 motorcycle parking spaces where none are required by the DCP, which represents an oversupply of motorcycle parking. The provision of



motorbike spaces where none are required will theoretically reduce car parking requirements, given that some portion of shopping centre staff and retail patrons are expected to ride motorcycles.

3.4 Bicycle Storage Requirements

Warringah DCP 2011 outlines the following bicycle parking requirement for the proposed development:

Residential Accommodation containing 3 or more dwellings (excluding group homes; boarding houses; hostels; seniors housing):

Column 1 (High-Medium Security Level*)

1 per dwelling

Column 2 (High-Low Security Level**)

Visitor: 1 per 12 dwellings

Business and Retail Premises:

Column 1 (High-Medium Security Level*)

1 per 200m² GFA

Column 2 (High-Low Security Level**)

Visitors: 1 per 600m² GFA

Recreation Facility (indoor, outdoor, or major):

Column 1 (High-Medium Security Level*)

1 per 4 employees PLUS

1 per 1500 spectator places

Column 2 (High-Low Security Level**)

1 per 200m² GFA

1 per 250 spectator places

The resulting bicycle parking requirements for the subject mixed-use development are summarised in **Table 5** below.

^{*}Bicycles are stored in individual or locked to rails within a secure room / enclosure (Refer to Part 7.6 of the NSW Planning Guidelines to Walking and Cycling for more detail.)

^{**}Bicycle frames and wheel are locked to high quality rails. (Refer to Part 7.6 of the NSW Planning Guidelines to Walking and Cycling for more detail.)



TABLE 5: BICYCLE PARKING REQUIREMENTS

Land Use	Туре	Scale ⁽¹⁾	Rate	Spaces Required
Residential	Column 1	51	1 per dwelling	51
Residential	Column 2	31	1 per 12 dwelling	4
Business and Batail	Column 1	3,767m ²	1 per 200m ² GFA	19
Business and Retail	Column 2	GFA	1 per 600m ² GFA	6
Degraption	Column 1	997m²	1 per 4 employees	3 ⁽²⁾
Recreation	Column 2	GFA	1 per 200m ² GFA	5
Total	-	-	-	88 (73 tenant; 15 visitors)

Note:

The development therefore requires the provision of **88** bicycle spaces. A provision of **90** bicycle spaces are provided, satisfying Council requirements.

3.5 Motorcycle Parking Requirements

Council's DCP does not provide any motorcycle parking requirement for the proposed site. The development proposes 12 motorcycle spaces, which will theoretically reduce the car parking demand and should be looked upon favourably by Council.

3.6 Disabled Parking

Council's DCP does not outline provisions for disabled parking. According to the *Building Code of Australia*, the proposed development uses are classified as the following building classes:

Class 6

A shop or other building for the of goods by retail or the supply to services direct to the public.

The disabled car parking requirement for a Class 6 building is 1 space for every 50 carparking spaces or part thereof. The proposed development provides 116 car parking spaces for retail and gym uses but requires 142 car parking spaces in accordance with the RMS and DCP requirements. Nonetheless, applying the BCA rate of 1 space per 50 carparking spaces results in a requirement for three (3) disabled retail parking spaces. The site provides three (3) retail disabled spaces, satisfying the BCA requirements.

The proposed development includes five (5) adaptable units. It is best practice to provide a disabled space for each adaptable unit. The plans provide five (5) disabled spaces, satisfying the requirements of AS4299 for adaptable housing.

⁽¹⁾ Scale measured in GFA

⁽²⁾ The gymnasium is expected to operate with a maximum 12 staff at any one time



3.7 Servicing & Loading

Council's DCP does not outline provision of loading facilities for uses within the proposed development. However, a loading area has been provided which can facilitate up to a 12.5m length Heavy Rigid Vehicle under a forward entry / forward out manoeuvre. This is considered adequate for the scale of the development.

The loading facility driveway is shared with the passenger vehicle exit driveway, and therefore must operate under traffic signal control. A concept of the traffic signal system is provided in **Annexure E** for reference, whilst the operation is summarised as follows:

- Signals are provided at the top and bottom of the exit ramp for passenger vehicles, and within the loading area for loading vehicles.
- Red lights for passenger vehicles are activated within the basement carpark when a loading vehicle enters the site.
- Loading vehicles are required to stop, as directed by an externally mounted "red" signal that faces approaching truck drivers along Glenrose Place upon entry into the site to allow any remaining passenger vehicles on the ramp to exit the site.
- Once the exit ramp is cleared, the loading vehicle enters the loading bay, and the exit ramp signal changes from red to green, allowing passenger vehicles to exit the site freely.
- Once loading is completed, the loading vehicle will wait within the loading area, activating the red light for the basement exit.
- Once the basement exit is clear of exiting vehicles, the loading signal turns from red to green and the loading vehicle exits the site.
- When sufficient time has passed, the basement exit signals turn from red to green, restoring the signal's default position when no loading vehicles are within the system.

The loading facility shall be managed amongst the tenancies under a Loading Dock Management Plan that includes time of the week schedule, given that the area can accommodate a single HRV at a time.

3.7.1 Passenger Vehicle Egress Queueing Analysis

MTE has completed a queueing analysis to demonstrate the potential queueing of passenger vehicles leaving the basement carpark level during the peak traffic generation scenarios when a service vehicle enters the site. The queueing calculations are summarised in **Table 6**.



TABLE 6: TRAFFIC SIGNAL INTERNAL CAR QUEUEING ANALYSIS

Peak Hour	Peak Hourly Exiting Volume	Service Time	98 th Percentile Queue
Thursday - Friday AM	70 vehicles	eles 4 vehicles	
Thursday – Friday PM	103 vehicles	20 seconds	7 vehicles
Saturday Midday	146 vehicles		18 vehicles

It is considered that a queue over four (4) vehicles is undesirable given the geometry of the basement. The internal 98th percentile vehicle queue is not expected to be more than 4 cars outside of the peak times included in **Table 6**. Therefore, loading should be prohibited after 3pm on weekdays, after 10am on Saturdays and all day on Sundays.

Truck frequencies will also be restricted within the Loading Dock Management Plan so that no two trucks will enter the site within an hour of each other.

3.8 Proposed Roundabout Treatment on Glenrose Place

It is noted that the proposed access is on a cul-de-sac at the end of Glenrose Place, just opposite the entry and exit driveway to Glenrose Village. MTE proposes that this cul-de-sac be redesigned to operate as a roundabout, with a mountable island in the centre for service vehicles. Roundabout signage would be required within the subject site and the driveway for Glenrose Village. The new road treatment is subject to approval by Council's Local Traffic Committee. An image of the proposed roundabout treatment is shown in **Figure 8**, whilst a more detailed concept plan reproduced in **Annexure F**. It is noted that the roundabout concept is subject to detailed design.



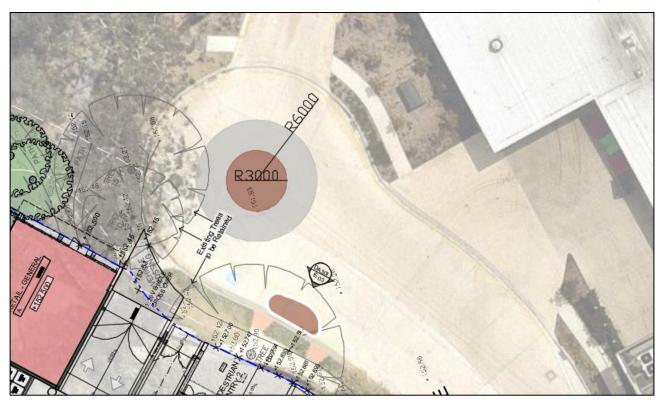


FIGURE 8: GLENROSE PLACE ROUNDABOUT CONCEPT

3.9 Car Park Design & Compliance

The car parking layouts of the basement level have been assessed and found to be generally compliant with the relevant clauses of AS2890.1:2004, AS2890.2:2002 and AS2890.6:2009. The design achieves the following:

- Car parking spaces of minimum 2.6m x 5.4m for retail and gym visitors;
- Disabled car parking spaces with minimum dimensions of 2.4m in width by 5.4m in length (AS2890.6) with equivalent shared zone area, or 5.4m length x 3.8m width (AS4299);
- Tandem car parking spaces for use by staff or residents of the same unit only.
- Maximum driveway ramp grade of 1:5 (20%);
- Loading facilities suitable for vehicles up to and including a 12.5m length HRV;
- Minimum headroom areas as follows:
 - 4.5m within loading areas and accesses thereto;
 - 2.5m above disabled spaces and shared spaces;
 - 2.2m above all vehicle manoeuvring areas.



Any required changes for compliance (including the signal system) are shown in **Annexure E** for reference.

It should be noted that while we have assessed the plans to be compliant with the relevant standards or to function acceptably, it is usual that a design certification is required at the Construction Certificate Stage to account for any design changes during the Development Application process.



4 TRAFFIC ASSESSMENT

The impact of the expected traffic generation levels associated with the subject proposal is discussed in the following sub-sections.

4.1 Traffic Generation

The estimated traffic generation level for the mixed-use development is based upon the RMS *Guide to Traffic Generating Developments October 2002* and more recent supplements (*TDT 2013 4a*). The traffic generation is summarised in **Table 7** below.

According to the *TDT 2013 4a*, the Sydney average traffic generation rates for residential uses are 0.19 and 0.15 trips per unit in the AM and PM peak hour periods respectively. As a conservative assessment, the rate provided in the 2002 RMS Guide (0.29 trips per unit) has been utilised for the subject development, given that the site is not located within walking distance of a train station.



TABLE 7: ESTIMATED TRAFFIC GENERATION

Land Use	Time	Scale	Rate	Traffic Generation	Direction
Lliab donaitu	AM Peak		0.29 per unit	15	3 in, 12 out
High density residential	PM Peak	51 dwellings	0.29 per unit	15	12 in, 3 out
flat building	Saturday		0.29 per unit ⁽²⁾	15	7 in, 8 out
	AM Peak		0.010 per m² GLFA ⁽³⁾	13	7 in, 6 out
Retail – Slow Trade	PM Peak	1,317m² GLFA	0.020 per m² GLFA	26	13 in, 13 out
	Saturday		0.038 per m² GLFA	50	25 in, 25 out
	AM Peak		0.026 per m² GLFA ⁽²⁾	7	3 in, 4 out
Retail – Faster Trade	PM Peak	259.5m² GLFA	0.051 per m² GLFA	13	6 in, 7 out
	Saturday		0.013 per m² GLFA	3	1 in, 2 out
	AM Peak		0.028 per m² GLFA ⁽⁴⁾	7	4 in, 3 out
Retail – Specialty Shops	PM Peak	1,238.25m² GLFA	0.056 per m² GLFA	70	35 in, 35 out
Chiopo	Saturday		0.107 per m² GLFA	133	67 in, 66 out
	AM Peak				
Gymnasiums	PM Peak	997m² GFA	9 per 100m ²⁽³⁾	90	45 in, 45 out
	Saturday				
	AM Peak			132	62 in, 70 out
Total	PM Peak			214	111 in, 103 out
	Saturday			291	145 in, 146 out

NOTE:

- (1) GLFA is taken to be 75% of GFA in accordance with Section 5.7.1 of the RMS Guide
- (2) Traffic generation for residential developments taken as 80% out, 20% during AM peak. Vice versa for PM peak.
- (3) AM peak traffic generation rate for shopping centres is conservatively assumed to be 50% of the PM peak. Friday and Thursday rates differ, so the larger of the two was chosen for the weekday PM peak.
- (4) Gymnasium rates are the evening peak hour vehicle trips for metropolitan sub-regional areas

As shown above, the peak traffic generation of the site has been estimated to be **132** (62 in; 70 out) trips during the AM peak period, **214** (111 in; 103 out) trips in the PM peak period and **291** (145 in, 146 out) in the Saturday midday period. As a conservative assessment,



the existing pedestrian volumes on the zebra crossings have been doubled in the future condition.

4.2 Traffic Assignment

Given the surrounding road network, the available routes to/from the site, and the existing traffic flows into Glenrose Village, the following trip assignment is assumed:

- 20% arriving from / departing toward the east along Glen Street;
- 30% arriving from / departing toward the west along Blackbutts Road;
- 30% arriving from / departing toward the east along Blackbutts Road;
- 20% arriving from / departing toward the north along Lockwood Avenue.

4.3 Traffic Impact

The traffic generation outlined in **Section 4.1 & 4.2** above has been added to the existing traffic volumes recorded and SIDRA INTERSECTION 8.0 used to assess the resulting performance of each intersection. The purpose of this assessment is to compare the existing intersection operations to the future scenario under the increased traffic load. The results of this assessment are shown in **Table 9** below, with detailed SIDRA results reproduced in **Annexure C** for reference. The existing intersections are reproduced in **Table 8** for comparison.



TABLE 8: EXISTING INTERSECTION PERFORMANCES (SIDRA INTERSECTION 8.0)

Intersection	Peak Hour	Degree of Saturation ⁽¹⁾	Average Delay ⁽²⁾ (sec/veh)	Level of Service ⁽³⁾⁽⁴⁾	Control Type	Worst Movement	95th Percentile Queue		
		EX	ISTING PERFOR	MANCE					
	AM	0.22	4.1	NA		RT from st: Glen Street	1 veh (7.1m) st: Glen		
			(Worst: 8.6)	(Worst: A)		Gleff Street	Street		
Glen St /	PM	0.30	4.4	NA	Give Way	RT from st:	1.6 veh (11.1m)		
Lockwood Av	1 101	0.00	(Worst: 9.4)	(Worst: A)	One may	Glen Street	st: Glen Street		
	SAT	0.34	4.7	NA		RT from st:	1.9 veh (13.2m)		
	OAT	0.54	(Worst: 10.2)	(Worst: A)		Glen Street	st: Glen Street		
	АМ	A.N.4	M 0.24	2.8	NA		RT from	0.6 veh (4.2m)	
		0.24	(Worst: 10.5)	(Worst: A)	Give Way	Glen Street	Glen Street		
Glen Street / Blackbutts	PM	0.25	3.8	NA		RT from	1 veh (7m) Blackbutts		
Road		0.20	(Worst: 10.1)	(Worst: A)		Glen Street	Road		
	0.4	CAT	SAT	0.23	4.1	NA		RT from	1.1 veh (7.8m)
	SAI	0.23	(Worst: 9.6)	(Worst: A)		Glen Street	Blackbutts Road		
	0.04	0.00	3.5	NA		RT from	1.2 veh (8.4m)		
	AM	0.23	(Worst: 7.3)	(Worst: A)		Glenrose Place	Glen Street		
Glen Street /	DM	0.05	4.4	NA	0: 14	RT from	1.3 veh (9.3m)		
Glenwood Place	PM	0.25	(Worst: 8.1)	(Worst: A)	Give Way	Glenrose Place	Glen Street		
	CAT	0.00	5.1	NA		RT from	1.6 veh (11.2m)		
	SAT	0.29	(Worst: 9.7)	(Worst: A)		Glenrose Place	Glen Street		

NOTES:

- (1) (2) Degree of Saturation is the ratio of demand to capacity for the most disadvantaged movement.
- Average delay is the delay experienced on average by all vehicles. The value in brackets represents the delay to the most disadvantaged movement.
- Level of Service is a qualitative measure of performance describing operational conditions. There are six levels of service, designated from A to F, with A representing the best operational condition and level of service F the worst. The LoS of the intersection is shown in bold, and the LoS of the most disadvantaged movement is shown in brackets.
- (4) NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.



TABLE 9: FUTURE INTERSECTION PERFORMANCES (SIDRA INTERSECTION 8.0)

Intersection	Peak Hour	Degree of Saturation ⁽¹⁾	Average Delay ⁽²⁾ (sec/veh)	Level of Service ⁽³⁾⁽⁴⁾	Control Type	Worst Movement	95th Percentile Queue
		FUTUTE PE	RFORMANCE (Po	ost Developme	nt)		
	AM	0.25	4.4 (Worst: 9.5)	NA (Worst: A)		RT from st: Glen Street	1.2 veh (8.3m) st: Glen
Glen St / Lockwood Av	PM	0.35	4.7 (Worst: 10.5)	NA (Worst: A)	Give Way	RT from st: Glen Street	Street 1.9 veh (13.5m) st: Glen
	SAT	0.42	5.4 (Worst: 13.2)	NA (Worst: A)		RT from st: Glen Street	Street 2.5 veh (17.2m) st: Glen
	AM	0.25	3.1 (Worst: 11)	NA (Worst: A)	Give Way	RT from Glen Street	Street 0.7 veh (4.8m) Glen
Glen Street / Blackbutts Road	PM	0.32	4.3 (Worst: 11.6)	NA (Worst: A)		RT from Glen Street	Street 1.4 veh (10m) Glen Street
	SAT	0.33	4.7 (Worst: 11.7)	NA (Worst: A)		RT from Glen Street	1.4 veh (10.1m) Glen Street
	AM	0.27	4.4 (Worst: 7.9)	NA (Worst: A)		RT from Car Park	1.5 veh (10.2m) Glen Street
Glen Street / Glenwood Place	PM	0.32	5.5 (Worst: 9.4)	NA (Worst: A)	Give Way	RT from Glenrose Place	1.7 veh (12.1m) Glen Street
NOTES	SAT	0.50	6.5 (Worst: 12.9)	NA (Worst: A)		RT from Glenrose Place	2.7 veh (18.7m) Glenrose Place

NOTES:

- (5) Degree of Saturation is the ratio of demand to capacity for the most disadvantaged movement.
- (6) Average delay is the delay experienced on average by all vehicles. The value in brackets represents the delay to the most disadvantaged movement.
- (7) Level of Service is a qualitative measure of performance describing operational conditions. There are six levels of service, designated from A to F, with A representing the best operational condition and level of service F the worst. The LoS of the intersection is shown in bold, and the LoS of the most disadvantaged movement is shown in brackets.
- (8) NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

As shown the surrounding intersections remain unaltered under the future scenario. The existing Level of Service (LoS) for each intersection has been retained, indicating minimal impact under the future scenario.



5 CONCLUSION

The traffic and parking impacts of the proposed Mixed-Use Development at 28 Lockwood Avenue, Belrose, as depicted in **Annexure A** for reference, have been assessed.

The car parking layout has been assessed to generally comply with the relevant dimensional requirements and objectives of AS2890.1, AS2890.2 and AS2890.6, subject to the detailed design of a traffic signal system and Loading Dock Management Plan.

A roundabout is proposed at the end of the Glenrose Place cul-de-sac to manage traffic entering and exiting both the subject development and the adjacent Glenrose Village shopping centre, as shown in **Annexure E**. The proposal is concept only and is subject to detailed design and approval from Council's Local Traffic Committee.

The site proposes a total of **193** car parking spaces for residents, visitors and retail staff, falling short of the requirements of Council's DCP, but satisfying the assessed peak car parking demand. The provision is therefore considered appropriate and acceptable. In addition, the respective BCA and DCP requirements for adaptable, disabled, bicycle and motorcycle parking are met or exceeded.

The peak traffic generation of the proposed development has been estimated to be **132** (62 in; 70 out) trips during the AM peak period, **214** (111 in; 103 out) trips in the PM peak period and **291** (145 in, 146 out) in the Saturday midday period. The additional traffic generation has been assessed to have no noticeable impact on the surrounding network in terms of level of service or delays.

Waste collection for the development will occur within the loading area along with deliveries for the retail portion of the development. The largest vehicle which can utilise the on-site loading area is a 12.5m length HRV. A Loading Dock Management Plan will be required for the efficient operation of the loading facilities, given that the loading access is shared with the carpark exit driveway.

In view of the foregoing, the proposed Mixed-Use Development is fully supported in terms of its traffic and parking impacts, subject to the required changes and detailed design of the internal traffic signal system associated with the loading operation, provided in **Annexure E**.



ANNEXURE A: PROPOSED PLAN



ABN: 81956706590 NSW: Nominated Architects Koos de Keijzer 5767 | David Randerson 8542

DA SUBMISSION 21/11/2019 DA SUBMISSION 26/11/2019 28/11/2019 DA SUBMISSION DA SUBMISSION 29/11/2019

Belrose, NSW 2085

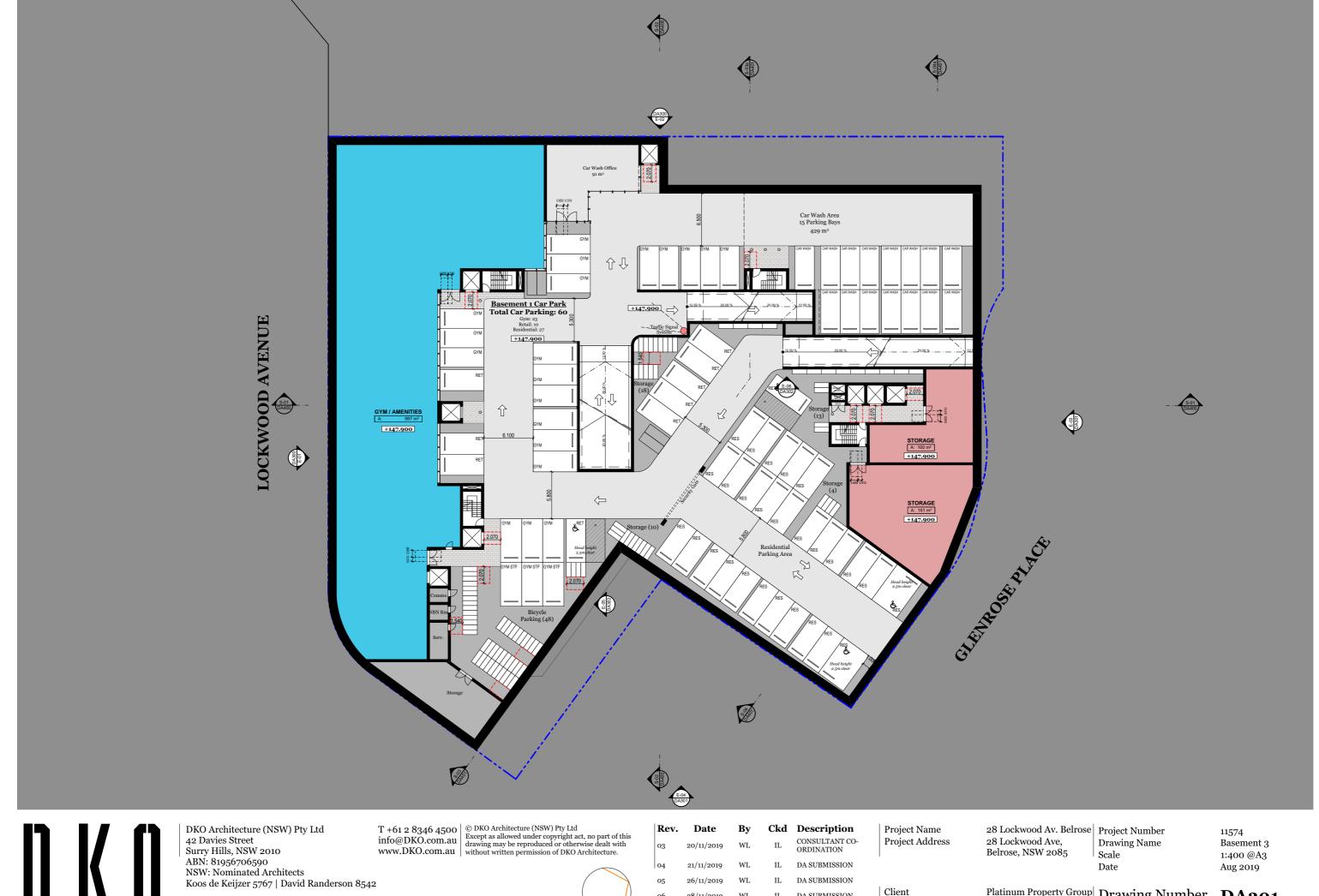
Scale Date

1:400 @A3 Aug 2019

Client

Platinum Property Group | Drawing Number **DA200** Revision

A



Rev.	Date	By	Ckd	Description
03	20/11/2019	WL	IL	CONSULTANT CO ORDINATION
04	21/11/2019	WL	IL	DA SUBMISSION
05	26/11/2019	WL	IL	DA SUBMISSION
06	28/11/2019	WL	IL	DA SUBMISSION
A	29/11/2019	WL	IL	DA SUBMISSION

Client

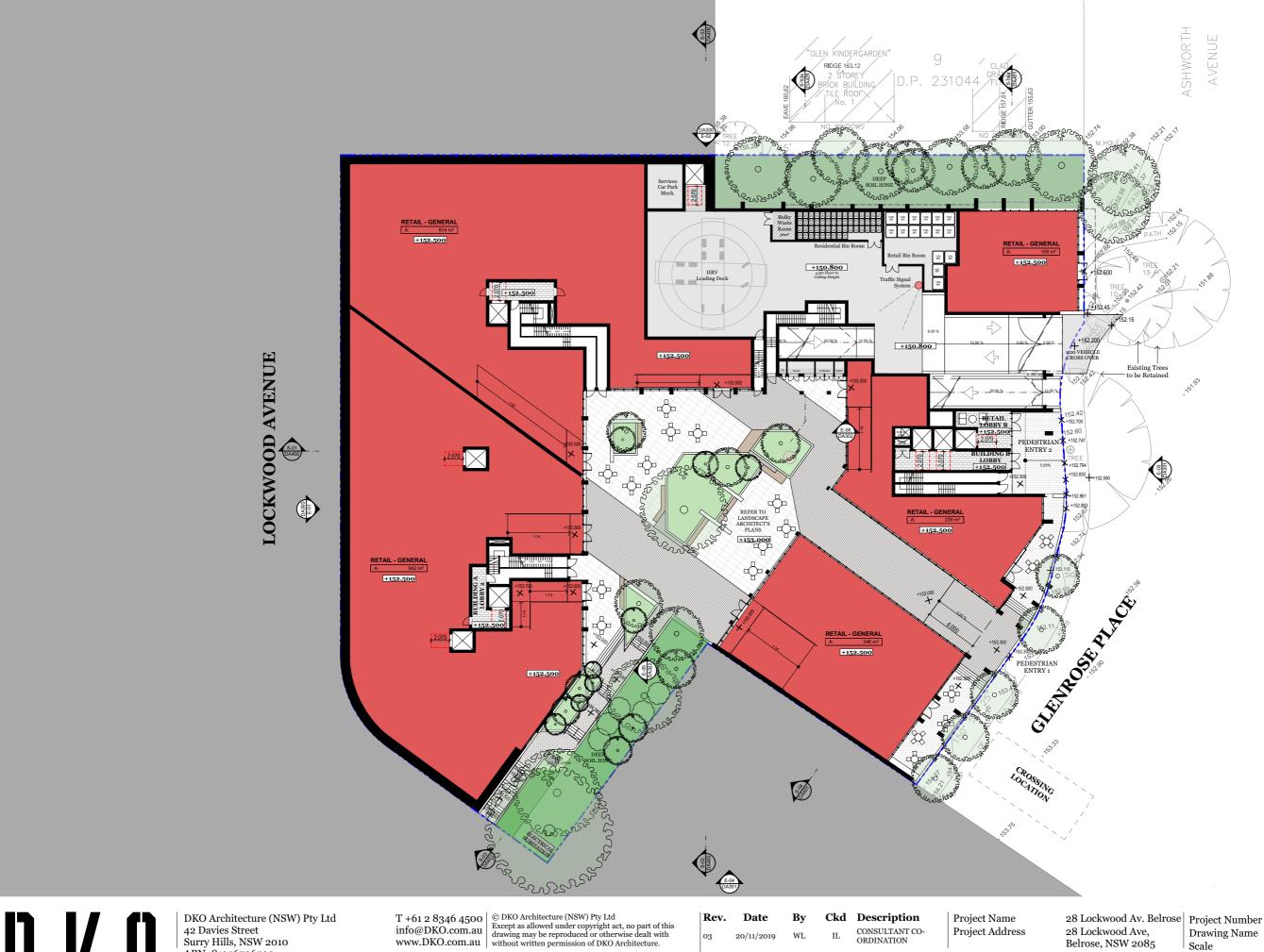
Belrose, NSW 2085

Scale Date

1:400 @A3 Aug 2019

Platinum Property Group | Drawing Number **DA201**

Revision \mathbf{A}



DKO Architecture (NSW) Pty Ltd 42 Davies Street Surry Hills, NSW 2010 ABN: 81956706590 NSW: Nominated Architects Koos de Keijzer 5767 | David Randerson 8542

20/11/2019 ORDINATION DA SUBMISSION 21/11/2019 DA SUBMISSION 26/11/2019 28/11/2019 DA SUBMISSION DA SUBMISSION 29/11/2019

Project Address

Client

Belrose, NSW 2085

Drawing Name Scale Date

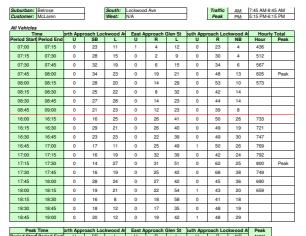
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Platinum Property Group | Drawing Number **DA202**

Revision A

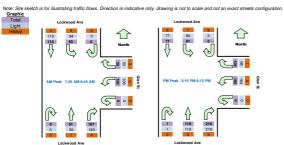


ANNEXURE B: TRAFFIC SURVEY DATA



Peak	Time	orth Appr	oach Loc	kwood A	East A	pproach (Glen St	outh Appr	kwood A	Peak	
Period Start	Period End	U	SB	L	U	R	L	U	R	NB	total
07:45	08:45	0	114	93	0	55	105	0	187	51	605





	me			kwood A		pproach (Glen St			
	Period End		SB	L	U	R	L	U	R	NB
07:00	07:15	0	23	9	1	3	11	0	23	4
07:15	07:30	0	28	15	0	2	8	0	29	4
07:30	07:45	0	31	15	0	6	15	0	34	6
07:45	08:00	0	34	21	0	18	21	0	47	12
08:00	08:15	0	28	18	0	12	29	0	51	10
08:15	08:30	0	25	21	0	8	32	0	41	14
08:30	08:45	0	25	24	0	14	23	0	44	14
08:45	09:00	0	21	19	0	11	22	0	36	8
16:00	16:15	0	15	24	0	22	41	0	49	25
16:15	16:30	0	29	18	0	24	40	0	49	19
16:30	16:45	0	23	22	0	20	39	0	49	30
16:45	17:00	0	17	10	0	23	49	1	50	25
17:00	17:15	0	16	17	0	30	38	0	42	24
17:15	17:30	0	14	25	0	30	50	0	62	25
17:30	17:45	0	16	17	0	23	41	0	68	38
17:45	18:00	0	28	23	0	26	41	0	45	36
18:00	18:15	0	19	19	0	19	52	- 1	43	20
18:15	18:30	0	16	8	0	17	56	0	41	18
18:30	18:45	0	18	11	0	15	34	0	48	19
18:45	19:00	0	20	11	0	16	40	- 1	48	29

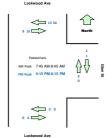
Peak	Time	orth Appr	oach Loc	kwood A	East A	pproach (Glen St	outh Appr	kwood A	Peak	
Period Start	Period End	U	SB	Т	U	R	L	U	R	NB	total
07:45	08:45	0	112	84	0	52	105	0	183	50	586
17:15	18:15	0	77	84	0	98	184	1	218	119	781

				kwood A		pproach (Glen St	outh App		
	Period End		SB	L	U	R	L	U	R	NB
07:00	07:15	0	0	2	0	1	1	0	0	0
07:15	07:30	0	0	0	0	0	1	0	1	0
07:30	07:45	0	1	4	0	0	0	0	0	0
07:45	08:00	0	0	2	0	1	0	0	1	- 1
08:00	08:15	0	0	2	0	2	0	0	2	0
08:15	08:30	0	0	1	0	0	0	0	1	0
08:30	08:45	0	2	4	0	0	0	0	0	0
08:45	09:00	0	0	4	0	1	1	0	3	0
16:00	16:15	0	1	1	0	4	0	0	1	- 1
16:15	16:30	0	0	3	0	2	0	0	0	0
16:30	16:45	0	0	1	0	2	0	0	0	0
16:45	17:00	0	0	1	0	2	0	0	0	1
17:00	17:15	0	0	2	0	2	- 1	0	0	0
17:15	17:30	0	0	2	0	1	- 1	0	0	0
17:30	17:45	0	0	2	0	2	- 1	0	0	0
17:45	18:00	0	0	1	0	1	1	0	0	0
18:00	18:15	0	0	2	0	3	2	0	0	0
18:15	18:30	0	0	0	0	1	2	0	0	0
18:30	18:45	0	0	1	0	2	- 1	0	0	0
18:45	19:00	0	0	1	0	3	2	0	0	0

Peak	Time	prτn Appr	oacn Loc	KWOOD A	East A	pproacn (alen St	putn Appi	oacn Loc	KWOOD A	Peak
Period Start	Period End	U	SB	Т	U	R	L	U	R	NB	total
07:45	08:45	0	2	9	0	3	0	0	4	- 1	19
17:15	18:15	0	0	7	0	7	5	0	0	0	19

Time North Approach Lockwood Ave East Approach Glen St South Approach Lockwood Ave Period End Westbound Eastbound Southbound Northbound Westbound Eastbound Hourly Total 55 52 07:45 08:00 53 45 08:15 08:00 08:15 08:30 08:45 16:00 08:30 08:45 09:00 16:15 16:15 16:30 16:45 16:30 51 16:45 28 34 17:00 17:00 17:15 27 24 17:30 17:45 17:45 18:00 18:00 18:15 13

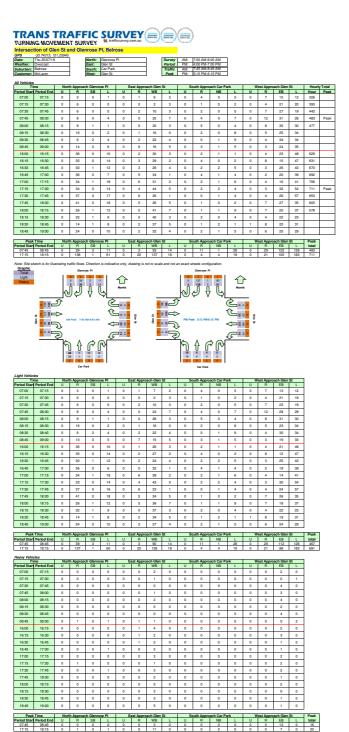
Peak	Time	North Approach	Lockwood Ave	East Approx	ach Glen St	Westbound Eastbound		Peak total
Period Start	Period End	Westbound	Eastbound	Southbound	Northbound	Westbound	Eastbound	Peak total
07:45	08:45	34	9	2	0	0	0	45
17:15	18:15	13	10	1	0	0	0	24



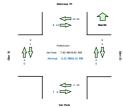
18:15

18:45

18:30



		ch Glenrose PI		ach Glen St	South Appro			ach Glen St	Hourly Tota
Period End	Westbound	Eastbound	Southbound	Northbound	Westbound	Eastbound	Southbound	Northbound	Hourry I of
07:15	1	0	0	0	0	0	1	0	30
07:30	3	6	0	0	2	1	0	0	40
07:45	- 1	5	0	0	0	0	0	0	39
08:00	5	1	0	0	3	1	0	0	39
08:15	6	2	0	0	3	1	0	0	31
08:30	7	3	0	0	1	0	0	0	
08:45	- 1	2	0	0	3	0	0	0	
09:00	1	1	0	0	0	0	0	0	
16:15	6	2	0	0	1	0	0	0	27
16:30	2	6	0	0	1	0	0	0	26
16:45	3	0	0	0	2	0	0	0	24
17:00	2	2	0	0	0	0	0	0	28
17:15	4	4	0	0	0	0	0	0	33
17:30	3	4	0	0	0	0	0	0	33
17:45	2	2	0	0	2	2	1	0	31
18:00	3	5	0	0	0	1	0	0	29
18:15	2	1	0	0	0	5	0	0	23
18:30	0	0	0	0	0	5	0	0	
18:45	0	1	0	0	1	3	0	2	
19:00	2	1	0	0	0	0	0	0	
Time	North Approa	ch Glenrose PI	East Appro	ach Glen St	South Appro	ach Car Park	West Appro	ach Glen St	Peak hou
	Westbound	Eastbound	Southbound	Northbound	Westbound	Eastbound	Southbound	Northbound	total
08:45	19	8	0	0	10	2	0	0	39 33
	07:30 07:45 08:00 08:15 08:30 08:45 09:00 16:15 16:30 16:45 17:00 17:15 18:00 18:15 18:30 18:45 19:00	0730 3 3 0745 1 07050 3 0 0745 1 0 0750 1 5 0 0750 1 0 0750 1 0 0750 1 0 0750 1 0 0750 1 0 0750 1 0 0750 1 0 0750 1 0 0750 1 0 0750 1 0 0750 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	8790 3 6 6 6795 1 5 6 6795 1 1 5 6 6795 1 1 5 7 7 3 6815 6 2 7 3 6845 1 2 6 7 3 6845 1 2 7 3 6845 1 2 7 3 6845 1 2 7 3 6845 1 2 7 3 6845 1 1 2 6850 2 6 7 3 6850 3 6 7 3 6850 3 5 7 3 6850 3 7 3 6	8790 3 6 0 0 6795 1 5 0 0 6895 1 0 6895 1 0 0 6895 1 0 0 6895 1 0 0 6895 1 0 0 6895 1 0 0 6895 1 0	0750 3	0750 3	0750 3	07950 3	0750



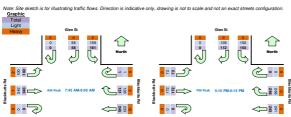
TRANS TRAFFIC SURVEY TURNING MOVEMENT SURVEY TURNING MOVEMENT SURVEY Intersection of Blackbutts Rd and Glen St. Belrose

GPS	-33.74081, 151.20905		
	Thu 25/07/19		Glen St
	Overcast		Blackbutts Rd
Suburban:	Belrose		N/A
Customer:	McLaren	West:	Blackbutts Rd

Survey	AM:	7:00 AM-9:00 AM
Period	PM:	4:00 PM-7:00 PM
Traffic	AM:	7:45 AM-8:45 AM
Peak	PM-	5:15 PM-6:15 PM

All Vehicles												
	ne									ckbutts R	Hourly	
Period Start	Period End	U	R	L	U	R	WB	U	EB	L	Hour	Peak
07:00	07:15	0	7	28	2	18	16	0	59	9	757	
07:15	07:30	0	9	28	1	15	27	0	58	19	885	
07:30	07:45	0	8	39	0	23	50	0	80	17	992	
07:45	08:00	0	4	51	1	37	52	0	80	19	1041	Peak
08:00	08:15	0	20	37	0	42	57	0	90	21	1013	
08:15	08:30	0	19	38	0	25	72	0	79	31		
08:30	08:45	0	15	35	0	27	64	0	94	31		
08:45	09:00	0	12	32	0	22	45	0	80	25		
16:00	16:15	0	15	42	0	58	58	0	47	18	976	
16:15	16:30	0	22	47	1	46	53	0	64	22	968	
16:30	16:45	0	24	38	0	62	50	0	46	17	977	
16:45	17:00	0	22	44	0	60	60	0	43	17	1010	
17:00	17:15	0	24	31	0	52	62	0	47	14	1012	
17:15	17:30	0	27	38	0	67	69	0	43	20	1019	Peak
17:30	17:45	0	30	28	0	84	63	0	43	22	977	
17:45	18:00	0	28	42	0	68	57	0	40	13	918	
18:00	18:15	0	27	46	0	47	54	0	46	17	892	
18:15	18:30	0	29	45	- 1	47	50	0	38	12		
18:30	18:45	0	21	32	0	55	61	0	30	12		
18:45	19:00	0	20	42	1	56	51	0	30	22		

Peak	Time	North A	pproach	Glen St	ast Appr	oach Blac	kbutts Re	lest Appr	oach Blad	kbutts R	Peak
Period Start	Period End	U	R	L	U	R	WB	U	EB	Т	total
07:45	08:45	0	58	161	1	131	245	0	343	102	1041
17:15	18:15	0	112	154	0	266	243	0	172	72	1019



Peak			ach Glen St		Blackbutts Rd		Blackbutts Rd	Peak total
Period Start	Period End	Westbound	Eastbound	Southbound	Northbound	Southbound	Northbound	r cun tota
07:45	08:45	0	3	0	0	1	3	7
17:15	18:15	3	0	1	0	1	4	9
		3	Glen St	North				
	Slackbutts Rd	AM Peak	7:45 AM-8:45 AM 5:15 PM-6:15 PM	\$	Bla ckbutts Re			

07:45 08:00 08:15

08:30 08:45 09:00 16:15 16:30 16:45 17:00 17:15

17:30 17:45

18:00 18:15

18:30 19:00

07:30 07:45 08:00 08:15 08:30 08:45 16:00 16:15 16:30 16:45 17:00 17:15

17:45 18:00

18:15 18:30 18:45

12

Light Vehicles											
	me		Approach					st Approach Blackbutts I			
Period Start	Period End	U	R	L	U	R	WB	U	EB	L	
07:00	07:15	0	7	27	2	18	13	0	58	9	
07:15	07:30	0	9	27	0	15	25	0	58	18	
07:30	07:45	0	8	38	0	23	48	0	79	17	
07:45	08:00	0	4	51	1	35	51	0	80	19	
08:00	08:15	0	20	37	0	42	52	0	90	19	
08:15	08:30	0	19	38	0	24	69	0	79	31	
08:30	08:45	0	15	33	0	27	61	0	93	31	
08:45	09:00	0	12	31	0	20	44	0	78	24	
16:00	16:15	0	14	42	0	56	53	0	46	18	
16:15	16:30	0	22	47	0	46	53	0	64	22	
16:30	16:45	0	24	38	0	62	50	0	45	17	
16:45	17:00	0	22	44	0	59	59	0	43	17	
17:00	17:15	0	23	31	0	52	62	0	47	14	
17:15	17:30	0	27	37	0	67	68	0	43	20	
17:30	17:45	0	30	27	0	84	63	0	43	22	
17:45	18:00	0	27	42	0	68	56	0	40	13	
18:00	18:15	0	25	46	0	47	54	0	46	17	
18:15	18:30	0	27	45	1	47	50	0	38	12	
18:30	18:45	0	21	31	0	55	61	0	30	12	
18:45	19:00	0	18	42	1	56	49	0	30	22	

Peak Time		North /	Approach	Glen St	ast Appr	oach Blad	kbutts R	est App	Peak		
Period Start	Period End	U	R	L	U	R	WB	U	EB	Т	total
07:45	08:45	0	58	159	1	128	233	0	342	100	1021
17:16	10-15	٥	100	152	0	200	241	0	172	72	1012

Tir	me	North A	Approach	Glen St	ast Approach Blackbutts Rest Approach Blackbutts							
Period Start	Period End	U	R	L	U	R	WB	U	EB	L		
07:00	07:15	0	0	1	0	0	3	0	1	0		
07:15	07:30	0	0	- 1	- 1	0	2	0	0	1		
07:30	07:45	0	0	- 1	0	0	2	0	1	0		
07:45	08:00	0	0	0	0	2	1	0	0	0		
08:00	08:15	0	0	0	0	0	5	0	0	2		
08:15	08:30	0	0	0	0	1	3	0	0	0		
08:30	08:45	0	0	2	0	0	3	0	1	0		
08:45	09:00	0	0	- 1	0	2	1	0	2	1		
16:00	16:15	0	1	0	0	2	5	0	1	0		
16:15	16:30	0	0	0	- 1	0	0	0	0	0		
16:30	16:45	0	0	0	0	0	0	0	1	0		
16:45	17:00	0	0	0	0	1	1	0	0	0		
17:00	17:15	0	1	0	0	0	0	0	0	0		
17:15	17:30	0	0	- 1	0	0	1	0	0	0		
17:30	17:45	0	0	- 1	0	0	0	0	0	0		
17:45	18:00	0	1	0	0	0	1	0	0	0		
18:00	18:15	0	2	0	0	0	0	0	0	0		
18:15	18:30	0	2	0	0	0	0	0	0	0		
18:30	18:45	0	0	- 1	0	0	0	0	0	0		
18:45	19:00	0	2	0	0	0	2	0	0	0		

Peak Time					ast Appr	oach Blac	kbutts R	Vest Approach Blackbutts R			Peak
Period Start	Period End	U	R	L	U	R	WB	U	EB	٦	total
07:45	08:45	0	0	2	0	3	12	0	1	2	20
17:15	18:15	0	9	2	0	0	2	0	0)	7



TURNING MOVEMENT SURVEY

Intersection of Glen St and Glenrose PI, Belrose

-33.7401, 151.20961 GPS

Date:	Thu 25/07/19
Weather:	Overcast
Suburban:	Belrose
Customer:	McLaren

North:	Glenrose Pl
East:	Glen St
South:	Glenrose Pl
West:	Glen St

Survey Period Traffic Peak

Peds crossing

Tir	ne	Gle	n St	Glenr	ose Pl
Period Start		SB	NB	WB	EB
07:00	07:15	3	5	3	0
07:15	07:30	1	6	0	2
07:30	07:45	1	5	1	0
07:45	08:00	7	4	4	0
08:00	08:15	4	22	7	2
08:15	08:30	2	8	10	1
08:30	08:45	4	2	2	2
08:45	09:00	2	3	3	0
16:00	16:15	4	7	6	6
16:15	16:30	8	5	7	13
16:30	16:45	2	10	3	4
16:45	17:00	1	6	6	2
17:00	17:15	3	10	2	4
17:15	17:30	8	4	2	2
17:30	17:45	3	10	8	1
17:45	18:00	5	5	3	2
18:00	18:15	4	6	5	1
18:15	18:30	1	17	5	2
18:30	18:45	2	11	4	1
18:45	19:00	4	8	0	3

TRANS TRAFFIC SURVEY TURNING MOVEMENT SURVEY W trafficurvey.com.au Intersection of Glen St and Lockwood Ave, Belrose

IIIICI 3CC	tion of dich of	una
GPS	-33.7405, 151.20879	
	Thu 25/07/19	T
	Overcast	T
Suburban:	Belrose	Ī
Cuctomore	Mol area	Т

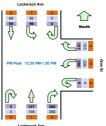
	Lockwood Ave
	Glen St
South:	Lockwood Ave
West:	N/A

Survey	AM:	10:00 AM-12:00 PM
Period	PM:	12:00 PM-2:00 PM
Traffic	AM:	11:45 AM-12:45 PM
Peak	PM:	12:30 PM-1:30 PM

				kwood A		pproach	Glen St	outh App		kwood A		y Total
Period Start	Period End	U	SB	L	U	R	L	U	R	NB	Hour	Peak
10:00	10:15	0	14	18	0	25	40	0	59	11	751	
10:15	10:30	0	27	20	0	18	53	0	64	15	751	
10:30	10:45	0	23	18	0	28	43	0	58	24	740	
10:45	11:00	0	18	26	0	14	45	0	69	21	767	
11:00	11:15	0	15	19	0	21	41	0	53	18	784	
11:15	11:30	0	16	20	0	17	50	0	60	23	828	
11:30	11:45	0	23	21	0	23	56	0	69	29	847	
11:45	12:00	0	18	23	0	21	54	0	68	26	865	
12:00	12:15	0	20	22	0	30	54	0	66	19	894	
12:15	12:30	0	22	25	0	22	59	0	62	15	899	
12:30	12:45	0	19	30	0	28	63	2	72	25	927	Peak
12:45	13:00	0	21	29	0	28	56	0	73	32	889	
13:00	13:15	0	18	15	0	34	47	1	75	26	867	
13:15	13:30	0	11	25	0	37	70	0	66	24		
13:30	13:45	0	14	13	0	26	62	0	65	21		
13:45	14:00	0	19	21	0	28	66	0	62	21		

		orth Appr	oach Loc	kwood A	East A	pproach (Glen St	outh Appr	Peak		
Period Start	Period End	U	SB	Г	U	R	Г	U	R	NB	total
11:45	12:45	0	79	100	0	101	230	2	268	85	865





Light Vehic										
Tir	me	orth Appr		kwood A		pproach (Glen St	outh App		
Period Start	Period End	U	SB	L	U	R	L	U	R	NB
10:00	10:15	0	14	18	0	22	40	0	59	11
10:15	10:30	0	26	19	0	18	52	0	64	15
10:30	10:45	0	23	17	0	27	43	0	58	24
10:45	11:00	0	18	24	0	14	45	0	69	20
11:00	11:15	0	15	19	0	18	41	0	53	18
11:15	11:30	0	16	19	0	17	49	0	60	23
11:30	11:45	0	23	20	0	22	56	0	69	29
11:45	12:00	0	18	21	0	21	54	0	68	26
12:00	12:15	0	20	22	0	27	54	0	65	19
12:15	12:30	0	22	23	0	22	59	0	62	15
12:30	12:45	0	19	30	0	27	63	2	72	25
12:45	13:00	0	21	26	0	28	56	0	73	32
13:00	13:15	0	18	15	0	31	47	1	75	25
13:15	13:30	0	11	21	0	37	70	0	66	24
13:30	13:45	0	14	12	0	24	62	0	65	21
13:45	14:00	0	19	19	0	28	64	0	62	21

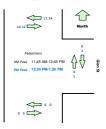
			oach Loc	kwood A	East A	East Approach Glen St			outh Approach Lockwood A		
Period Start	Period End	U	SB	Г	U	R	L	U	R	NB	total
11:45	12:45	0	79	96	0	97	230	2	267	85	856
12:30	13:30	0	69	92	0	123	236	3	286	106	915

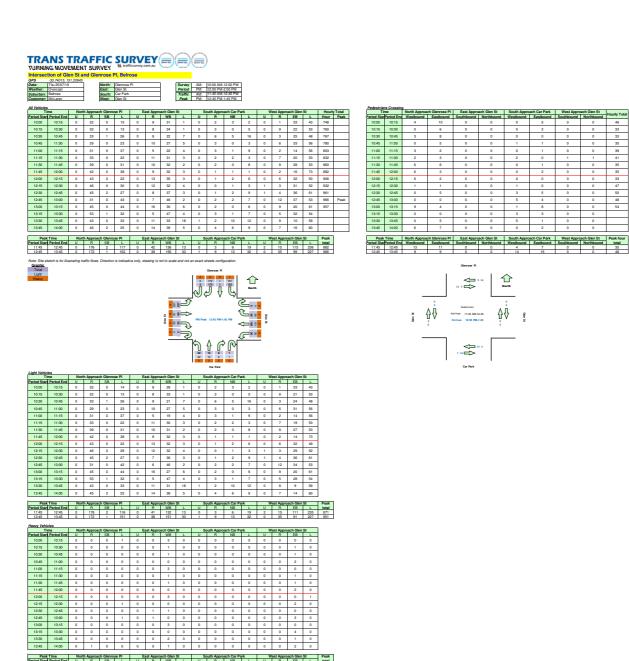
Tir				kwood A		pproach (outh App		
	Period End		SB	L	U	R	L	U	R	NB
10:00	10:15	0	0	0	0	3	0	0	0	0
10:15	10:30	0	1	1	0	0	1	0	0	0
10:30	10:45	0	0	1	0	1	0	0	0	0
10:45	11:00	0	0	2	0	0	0	0	0	- 1
11:00	11:15	0	0	0	0	3	0	0	0	0
11:15	11:30	0	0	1	0	0	1	0	0	0
11:30	11:45	0	0	1	0	1	0	0	0	0
11:45	12:00	0	0	2	0	0	0	0	0	0
12:00	12:15	0	0	0	0	3	0	0	1	0
12:15	12:30	0	0	2	0	0	0	0	0	0
12:30	12:45	0	0	0	0	1	0	0	0	0
12:45	13:00	0	0	3	0	0	0	0	0	0
13:00	13:15	0	0	0	0	3	0	0	0	- 1
13:15	13:30	0	0	4	0	0	0	0	0	0
13:30	13:45	0	0	- 1	0	2	0	0	0	0
13:45	14:00	0	0	2	0	0	2	0	0	0

Peak	Time	orth Approach Lockwood Av			East A	East Approach Glen St			outh Approach Lockwood A			
Period Start	Period End	U	SB	L	U	R	L	U	R	NB	total	
11:45	12:45	0	0	4	0	4	0	0	- 1	0	9	
12:30	13:30	0	0	7	0	4	0	0	0	1	12	

	me		Lockwood Ave		ach Glen St		Lockwood Ave	Hourly Total
Period Start	Period End	Westbound	Eastbound	Southbound	Northbound	Westbound	Eastbound	nourly Total
10:00	10:15	0	4	0	0	0	0	42
10:15	10:30	9	3	0	0	0	0	48
10:30	10:45	6	11	0	1	0	0	46
10:45	11:00	3	5	0	0	0	0	34
11:00	11:15	2	7	1	0	0	0	47
11:15	11:30	6	4	0	0	0	0	46
11:30	11:45	3	3	0	0	0	0	40
11:45	12:00	13	7	0	1	0	0	39
12:00	12:15	7	2	0	0	0	0	24
12:15	12:30	4	0	0	0	0	0	20
12:30	12:45	0	5	0	0	0	0	25
12:45	13:00	1	3	2	0	0	0	25
13:00	13:15	4	1	0	0	0	0	23
13:15	13:30	6	3	0	0	0	0	
13:30	13:45	1	4	0	0	0	0	
13:45	14:00	3	0	0	1	0	0	

Γ	Peak	Time	North Approach	Lockwood Ave	East Approx	East Approach Glen St South Approach Lockwood Ave					
Г	Period Start	Period End	Westbound	Eastbound	Southbound	Northbound	Westbound	Eastbound	reak total		
Г	11:45	12:45	24	14	0	1	0	0	39		
г	12-20	12-20	11	12	2	0	0	0	26		





 Fast Tax
 Morth Agrossis Glienose PI
 East Agrossis Glan SI
 South Agrossis Gar Pet
 West Agrossis Glan SI
 Peak

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	-33.74081, 151.20905		
	Sat 08/10/16		Glen St
Weather:			Blackbutts Rd
Suburban:			N/A
Customer:	McLaren	West:	Blackbutts Rd

Survey	AM:	10:00 AM-12:00 PM
Period	PM:	12:00 PM-2:00 PM
Traffic	AM:	11:30 AM-12:30 PM
Peak	PM:	12:30 PM-1:30 PM

Tir	ne	North A	Approach	Glen St	ast Appr	oach Blad	kbutts R	est App	oach Bla	ckbutts R	Hourl	y Total
Period Start	Period End	J	R	L	U	R	WB	U	EB	L	Hour	Peak
10:00	10:15	0	18	36	0	54	27	0	46	16	908	
10:15	10:30	0	24	56	0	54	34	0	45	25	932	
10:30	10:45	0	23	43	1	63	31	0	50	19	925	
10:45	11:00	0	14	49	1	67	31	0	58	23	970	
11:00	11:15	0	16	40	0	55	43	0	51	16	1008	
11:15	11:30	0	14	52	1	63	40	0	41	20	1041	
11:30	11:45	0	22	57	1	75	44	0	53	23	1060	Peak
11:45	12:00	0	26	46	0	70	57	0	58	24	1045	
12:00	12:15	0	24	50	0	63	35	0	60	22	1032	
12:15	12:30	0	24	57	2	49	38	0	52	28	1039	
12:30	12:45	0	35	47	3	76	31	0	45	23	1054	
12:45	13:00	0	22	55	3	73	48	0	35	32	1041	
13:00	13:15	0	18	47	0	71	46	0	48	31	1036	
13:15	13:30	0	27	54	0	67	53	0	41	23		
13:30	13:45	0	28	48	0	64	43	0	42	22		
13:45	14:00	0	23	62	0	58	43	0	52	25		

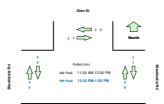
Peak	Time	North A	pproach	Glen St	ast Appr	oach Blad	kbutts R	lest Appr	oach Blad	ckbutts R	Peak
Period Start	Period End	U	R	L	U	R	WB	U	EB	Т	total
11:30	12:30	0	96	210	3	257	174	0	223	97	1060
12:30	13:30	0	102	203	6	287	178	0	169	109	1054

Note: Site ske Graphic Total Light Heavy



Pedestrians Cro	Pedestrians Crossing Time North Ap							
			ach Glen St		Blackbutts Rd		Blackbutts Rd	Hourly Total
Period Start	Period End	Westbound	Eastbound	Southbound	Northbound	Southbound	Northbound	mouny rotal
10:00	10:15	0	0	0	2	0	1	11
10:15	10:30	0	0	0	0	1	1	14
10:30	10:45	0	0	0	0	0	1	17
10:45	11:00	3	0	0	0	0	2	20
11:00	11:15	2	1	0	0	2	1	19
11:15	11:30	2	1	0	0	0	2	15
11:30	11:45	0	1	2	0	0	1	14
11:45	12:00	0	1	0	0	1	2	13
12:00	12:15	0	0	0	0	2	0	13
12:15	12:30	0	1	0	0	1	2	14
12:30	12:45	1	1	0	0	0	1	10
12:45	13:00	0	2	0	0	0	2	7
13:00	13:15	1	0	1	0	0	1	9
13:15	13:30	0	0	0	0	0	0	
13:30	13:45	0	0	0	0	0	0	
13:45	14:00	0	1	0	0	2	3	

Pea	k Time	North Appro	oach Glen St	East Approach	Blackbutts Rd	West Approach	Blackbutts Rd	Peak total
Period Start	Period End	Westbound Eastbound		Southbound Northbound		Southbound Northbound		Peak total
11:30	12:30	0	3	2	0	4	5	14
12:30	13:30	2	3	1	0	0	4	10



Tir				Glen St						proach Blackbutts F		
Period Start	Period End	U	R	L	U	R	WB	U	EB	L		
10:00	10:15	0	18	36	0	54	27	0	46	16		
10:15	10:30	0	24	54	0	54	32	0	44	25		
10:30	10:45	0	23	43	- 1	63	31	0	50	19		
10:45	11:00	0	14	49	1	66	31	0	58	23		
11:00	11:15	0	16	40	0	55	43	0	51	16		
11:15	11:30	0	14	51	1	63	39	0	41	20		
11:30	11:45	0	22	57	1	75	44	0	52	23		
11:45	12:00	0	26	46	0	70	57	0	58	24		
12:00	12:15	0	24	50	0	62	35	0	60	22		
12:15	12:30	0	24	57	2	49	38	0	52	28		
12:30	12:45	0	35	47	3	76	29	0	45	23		
12:45	13:00	0	22	55	3	73	48	0	35	32		
13:00	13:15	0	18	47	0	70	46	0	48	31		
13:15	13:30	0	27	54	0	67	52	0	41	23		
13:30	13:45	0	28	48	0	64	43	0	42	22		
13:45	14:00	0	23	60	0	58	43	0	52	25		

ſ	Peak			pproach	Glen St	ast Appr	oach Blac	kbutts Re	tts Reest Approach Blackbutts R			
ĺ	Period Start	Period End	U	R	_	U	R	WB	U	EB	L	total
Γ	11:30	12:30	0	96	210	3	256	174	0	222	97	1058
Γ	12:30	13:30	0	102	203	6	286	175	0	169	109	1050

	me	North /	Approach	Glen St	ast Appr	oach Blac	kbutts R	Vest Appr	oach Bla	ckbutts
Period Start	Period End	U	R	L	U	R	WB	U	EB	L
10:00	10:15	0	0	0	0	0	0	0	0	0
10:15	10:30	0	0	2	0	0	2	0	1	0
10:30	10:45	0	0	0	0	0	0	0	0	0
10:45	11:00	0	0	0	0	1	0	0	0	0
11:00	11:15	0	0	0	0	0	0	0	0	0
11:15	11:30	0	0	1	0	0	1	0	0	0
11:30	11:45	0	0	0	0	0	0	0	1	0
11:45	12:00	0	0	0	0	0	0	0	0	0
12:00	12:15	0	0	0	0	- 1	0	0	0	0
12:15	12:30	0	0	0	0	0	0	0	0	0
12:30	12:45	0	0	0	0	0	2	0	0	0
12:45	13:00	0	0	0	0	0	0	0	0	0
13:00	13:15	0	0	0	0	1	0	0	0	0
13:15	13:30	0	0	0	0	0	1	0	0	0
13:30	13:45	0	0	0	0	0	0	0	0	0
13:45	14:00	0	0	2	0	0	0	0	0	0

Peak	Time	North A	pproach	Glen St	East Appr	oach Blac	kbutts R	Vest Appr	oach Blac	kbutts R	Peak
Period Start	Period End	U	R	L	U	R	WB	U	EB	L	total
11:30	12:30	0	0	0	0	1	0	0	- 1	0	2
12:30	13:30	0	0	0	0	1	3	0	0	0	4



Intersection of Glen St and Glenrose PI, Belrose

-33.7401, 151.20961 GPS

Date:	Thu 25/07/19
Weather:	Overcast
Suburban:	Belrose
Customer:	McLaren

North:	Glenrose Pl
East:	Glen St
South:	Glenrose Pl
West:	Glen St

Survey Period
Traffic
Peak

All Vehicles

Tir	me	Gle	n St	Glenr	ose PI
Period Start	Period End	SB	NB	WB	EB
10:00	10:15	8	9	7	4
10:15	10:30	11	7	4	1
10:30	10:45	10	11	3	6
10:45	11:00	9	9	4	4
11:00	11:15	10	12	9	4
11:15	11:30	6	12	1	2
11:30	11:45	17	14	2	4
11:45	12:00	1	15	9	3
12:00	12:15	12	13	7	1
12:15	12:30	6	11	2	2
12:30	12:45	5	9	5	7
12:45	13:00	4	25	5	1
13:00	13:15	11	11	6	3
13:15	13:30	9	16	9	2
13:30	13:45	23	2	5	9
13:45	14:00	86	6	3	4



ANNEXURE C: SIDRA INTERSECTION 8 DETAILED MOVEMENT SUMMARIES

V Site: 101 [(AM Existing) Lockwood Avenue / Glen Street]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement P	erformand	e - Ve	hicles								
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South	East: Gl	en Street										
5	T1	54	0.0	0.216	0.3	LOSA	1.0	7.1	0.24	0.47	0.24	54.3
6	R2	197	0.0	0.216	4.3	LOSA	1.0	7.1	0.24	0.47	0.24	51.8
Appro	ach	251	0.0	0.216	3.5	NA	1.0	7.1	0.24	0.47	0.24	52.3
North	East: Gle	en Street										
7	L2	111	0.0	0.068	5.6	LOSA	0.3	2.0	0.08	0.55	0.08	48.4
9	R2	58	0.0	0.084	8.6	LOSA	0.3	2.2	0.51	0.72	0.51	51.1
Appro	ach	168	0.0	0.084	6.6	LOSA	0.3	2.2	0.22	0.61	0.22	49.7
North ¹	West: Lo	ckwood Ave	nue									
10	L2	98	0.0	0.088	5.7	LOSA	0.4	2.7	0.14	0.46	0.14	53.9
11	T1	120	0.0	0.088	0.3	LOSA	0.4	3.0	0.16	0.13	0.16	57.4
Appro	ach	218	0.0	0.088	2.7	NA	0.4	3.0	0.15	0.28	0.15	55.3
All Ve	hicles	637	0.0	0.216	4.1	NA	1.0	7.1	0.21	0.44	0.21	52.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [(AM Existing) Glen Street / Blackbutts Road]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement F	erformanc	e - Ve	hicles								
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East:	East: Blackbutts Road											
5	T1	258	0.0	0.133	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	60.0
6	R2	139	0.0	0.124	7.4	LOSA	0.5	3.8	0.50	0.69	0.50	46.4
Appro	ach	397	0.0	0.133	2.6	NA	0.5	3.8	0.18	0.24	0.18	56.3
North	: Glen St	treet										
7	L2	169	0.0	0.145	4.7	LOSA	0.6	4.2	0.42	0.63	0.42	50.3
9	R2	61	0.0	0.144	10.5	LOSA	0.5	3.6	0.68	0.86	0.68	43.1
Appro	ach	231	0.0	0.145	6.2	LOSA	0.6	4.2	0.49	0.69	0.49	48.2
West:	Blackbu	ıtts Road										
10	L2	107	0.0	0.243	5.6	LOSA	0.0	0.0	0.00	0.14	0.00	29.8
11	T1	361	0.0	0.243	0.0	LOSA	0.0	0.0	0.00	0.14	0.00	58.7
Appro	ach	468	0.0	0.243	1.3	NA	0.0	0.0	0.00	0.14	0.00	51.8
All Ve	hicles	1096	0.0	0.243	2.8	NA	0.6	4.2	0.17	0.29	0.17	52.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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▽ Site: 101 [(AM Existing) Glen Street / Genrose Place]

Site Category: (None) Giveway / Yield (Two-Way)

Move	ement P	erformano	e - Ve	hicles								
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	
South	n: Car Pa	rk										
1	L2	29	0.0	0.035	5.8	LOSA	0.1	0.9	0.20	0.56	0.20	53.1
2	T1	1	0.0	0.035	5.9	LOSA	0.1	0.9	0.20	0.56	0.20	53.2
3	R2	12	0.0	0.035	7.2	LOSA	0.1	0.9	0.20	0.56	0.20	47.5
Appro	oach	42	0.0	0.035	6.2	LOSA	0.1	0.9	0.20	0.56	0.20	52.0
East:	Glen Str	eet										
4	L2	15	0.0	0.100	4.7	LOS A	0.5	3.4	0.18	0.14	0.18	55.2
5	T1	98	0.0	0.100	0.4	LOSA	0.5	3.4	0.18	0.14	0.18	57.8
6	R2	3	0.0	0.100	5.6	LOSA	0.5	3.4	0.18	0.14	0.18	54.8
Appro	oach	116	0.0	0.100	1.1	NA	0.5	3.4	0.18	0.14	0.18	57.4
North	: Glenros	se Place										
7	L2	12	0.0	0.008	6.1	LOSA	0.0	0.2	0.26	0.54	0.26	47.5
8	T1	3	0.0	0.056	5.6	LOSA	0.2	1.3	0.39	0.65	0.39	52.5
9	R2	41	0.0	0.056	7.3	LOSA	0.2	1.3	0.39	0.65	0.39	51.8
Appro	oach	56	0.0	0.056	6.9	LOSA	0.2	1.3	0.36	0.63	0.36	51.3
West	: Glen St	reet										
10	L2	133	0.0	0.231	5.8	LOSA	1.2	8.4	0.19	0.35	0.19	55.0
11	T1	129	0.0	0.231	0.4	LOS A	1.2	8.4	0.19	0.35	0.19	54.0
12	R2	31	0.0	0.231	6.1	LOSA	1.2	8.4	0.19	0.35	0.19	54.4
Appro	oach	293	0.0	0.231	3.4	NA	1.2	8.4	0.19	0.35	0.19	54.6
All Ve	hicles	506	0.0	0.231	3.5	NA	1.2	8.4	0.20	0.35	0.20	54.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [(PM Existing) Lockwood Avenue / Glen Street]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement P	erformanc	e - Vel	hicles								
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	9
South	East: Gl	en Street										
5	T1	126	0.0	0.297	0.2	LOS A	1.6	11.1	0.15	0.38	0.15	55.3
6	R2	229	0.0	0.297	4.2	LOSA	1.6	11.1	0.15	0.38	0.15	52.8
Appro	ach	356	0.0	0.297	2.8	NA	1.6	11.1	0.15	0.38	0.15	53.6
North	East: Gle	en Street										
7	L2	199	0.0	0.122	5.5	LOS A	0.5	3.7	0.01	0.57	0.01	48.7
9	R2	112	0.0	0.173	9.4	LOS A	0.7	4.8	0.56	0.79	0.56	50.5
Appro	ach	311	0.0	0.173	6.9	LOSA	0.7	4.8	0.21	0.65	0.21	49.6
North ¹	West: Lo	ckwood Ave	nue									
10	L2	96	0.0	0.069	5.6	LOSA	0.3	2.0	0.09	0.54	0.09	53.4
11	T1	81	0.0	0.069	0.2	LOS A	0.3	2.3	0.11	0.04	0.11	58.9
Appro	ach	177	0.0	0.069	3.1	NA	0.3	2.3	0.10	0.31	0.10	55.0
All Ve	hicles	843	0.0	0.297	4.4	NA	1.6	11.1	0.16	0.46	0.16	52.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 101 [(PM Existing) Glen Street / Blackbutts Road]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement F	erformanc	e - Vel	nicles								
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	9
East:	Blackbu	tts Road										
5	T1	256	0.0	0.132	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	60.0
6	R2	280	0.0	0.197	6.5	LOSA	1.0	6.8	0.39	0.61	0.39	46.9
Appro	ach	536	0.0	0.197	3.4	NA	1.0	6.8	0.20	0.32	0.20	54.4
North	: Glen S	treet										
7	L2	162	0.0	0.116	3.9	LOS A	0.5	3.4	0.28	0.54	0.28	51.0
9	R2	118	0.0	0.248	10.1	LOSA	1.0	7.0	0.67	0.87	0.73	43.4
Appro	ach	280	0.0	0.248	6.5	LOSA	1.0	7.0	0.45	0.68	0.47	47.5
West:	Blackbu	ıtts Road										
10	L2	76	0.0	0.134	5.6	LOS A	0.0	0.0	0.00	0.18	0.00	29.7
11	T1	181	0.0	0.134	0.0	LOS A	0.0	0.0	0.00	0.18	0.00	58.4
Appro	ach	257	0.0	0.134	1.6	NA	0.0	0.0	0.00	0.18	0.00	49.5
All Ve	hicles	1073	0.0	0.248	3.8	NA	1.0	7.0	0.22	0.38	0.22	51.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [(SAT Existing) Lockwood Avenue / Glen Street]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ment P	erformanc	e - Vel	hicles								
Mov ID	Turn	Demand I Total	HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed
0 11	E . OI	veh/h	%	v/c	sec		veh	m				km/h
South	_	en Street										
5	T1	113	0.0	0.340	0.4	LOS A	1.9	13.2	0.23	0.43	0.23	54.9
6	R2	304	0.0	0.340	4.1	LOS A	1.9	13.2	0.23	0.43	0.23	52.4
Appro	ach	417	0.0	0.340	3.1	NA	1.9	13.2	0.23	0.43	0.23	53.0
Northl	East: Gle	en Street										
7	L2	248	0.0	0.152	5.6	LOS A	0.7	4.8	0.05	0.56	0.05	48.6
9	R2	134	0.0	0.227	10.2	LOSA	0.9	6.3	0.59	0.83	0.60	49.9
Appro	ach	382	0.0	0.227	7.2	LOSA	0.9	6.3	0.24	0.65	0.24	49.2
North\	Nest: Lo	ckwood Ave	nue									
10	L2	69	0.0	0.057	5.7	LOSA	0.2	1.7	0.12	0.49	0.12	53.7
11	T1	73	0.0	0.057	0.3	LOS A	0.3	1.9	0.15	0.10	0.15	57.8
Appro	ach	142	0.0	0.057	2.9	NA	0.3	1.9	0.14	0.29	0.14	55.1
All Ve	hicles	941	0.0	0.340	4.7	NA	1.9	13.2	0.22	0.50	0.22	51.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [(SAT Existing) Glen Street / Blackbutts Road]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement F	erformand	e - Ve	hicles								
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East:	East: Blackbutts Road											
5	T1	187	0.0	0.097	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	60.0
6	R2	308	0.0	0.225	6.7	LOSA	1.1	7.8	0.42	0.63	0.42	46.8
Appro	ach	496	0.0	0.225	4.2	NA	1.1	7.8	0.26	0.39	0.26	52.9
North	: Glen St	reet										
7	L2	214	0.0	0.152	3.9	LOSA	0.7	4.6	0.29	0.54	0.29	51.0
9	R2	107	0.0	0.220	9.6	LOSA	0.9	6.0	0.65	0.85	0.67	43.9
Appro	ach	321	0.0	0.220	5.8	LOSA	0.9	6.0	0.41	0.65	0.42	48.4
West:	Blackbu	itts Road										
10	L2	115	0.0	0.153	5.6	LOSA	0.0	0.0	0.00	0.23	0.00	29.4
11	T1	178	0.0	0.153	0.0	LOSA	0.0	0.0	0.00	0.23	0.00	57.9
Appro	ach	293	0.0	0.153	2.2	NA	0.0	0.0	0.00	0.23	0.00	46.3
All Ve	hicles	1109	0.0	0.225	4.1	NA	1.1	7.8	0.24	0.42	0.24	49.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [(PM Existing) Glen Street / Genrose Place]

Site Category: (None) Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles												
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles		
South	: Car Pa	ırk											
1	L2	20	0.0	0.025	6.0	LOSA	0.1	0.6	0.25	0.56	0.25	53.0	
2	T1	4	0.0	0.025	6.5	LOSA	0.1	0.6	0.25	0.56	0.25	53.2	
3	R2	4	0.0	0.025	7.7	LOSA	0.1	0.6	0.25	0.56	0.25	47.4	
Appro	ach	28	0.0	0.025	6.3	LOSA	0.1	0.6	0.25	0.56	0.25	52.5	
East:	Glen Str	eet											
4	L2	19	0.0	0.159	4.7	LOS A	0.8	5.6	0.18	0.18	0.18	54.8	
5	T1	144	0.0	0.159	0.4	LOS A	8.0	5.6	0.18	0.18	0.18	57.4	
6	R2	21	0.0	0.159	5.9	LOS A	0.8	5.6	0.18	0.18	0.18	54.4	
Appro	ach	184	0.0	0.159	1.5	NA	0.8	5.6	0.18	0.18	0.18	56.7	
North	: Glenros	se Place											
7	L2	64	0.0	0.045	6.0	LOS A	0.2	1.3	0.25	0.55	0.25	47.6	
8	T1	1	0.0	0.202	6.2	LOS A	0.7	5.1	0.47	0.75	0.47	51.8	
9	R2	145	0.0	0.202	8.1	LOS A	0.7	5.1	0.47	0.75	0.47	51.2	
Appro	ach	211	0.0	0.202	7.5	LOSA	0.7	5.1	0.40	0.69	0.40	50.4	
West:	Glen St	reet											
10	L2	193	0.0	0.254	5.8	LOSA	1.3	9.3	0.18	0.39	0.18	54.5	
11	T1	111	0.0	0.254	0.4	LOS A	1.3	9.3	0.18	0.39	0.18	53.3	
12	R2	22	0.0	0.254	6.3	LOS A	1.3	9.3	0.18	0.39	0.18	54.0	
Appro	ach	325	0.0	0.254	4.0	NA	1.3	9.3	0.18	0.39	0.18	54.2	
All Ve	hicles	748	0.0	0.254	4.4	NA	1.3	9.3	0.25	0.43	0.25	53.3	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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▽ Site: 101 [(SAT Existing) Glen Street / Genrose Place]

Site Category: (None) Giveway / Yield (Two-Way)

Move	ement F	Performanc	ce - Vel	nicles			_					
Mov ID	Turn	Demand I Total veh/h		Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	
South	: Car Pa		70	V/C	300		VCII	- '''				KITI/TI
1	L2	34	0.0	0.063	6.1	LOSA	0.2	1.6	0.34	0.61	0.34	52.5
2	T1	14	0.0	0.063	7.5	LOSA	0.2	1.6	0.34	0.61	0.34	52.6
3	R2	11	0.0	0.063	9.3	LOSA	0.2	1.6	0.34	0.61	0.34	46.6
Appro	ach	58	0.0	0.063	7.0	LOSA	0.2	1.6	0.34	0.61	0.34	51.8
East:	Glen St	reet										
4	L2	32	0.0	0.231	4.8	LOSA	1.2	8.6	0.18	0.21	0.18	54.4
5	T1	196	0.0	0.231	0.4	LOSA	1.2	8.6	0.18	0.21	0.18	56.9
6	R2	41	0.0	0.231	6.3	LOSA	1.2	8.6	0.18	0.21	0.18	54.0
Appro	ach	268	0.0	0.231	1.8	NA	1.2	8.6	0.18	0.21	0.18	56.2
North	: Glenro	se Place										
7	L2	160	0.0	0.112	6.1	LOSA	0.5	3.3	0.26	0.56	0.26	47.5
8	T1	1	0.0	0.292	7.5	LOSA	1.2	8.3	0.55	0.85	0.63	50.6
9	R2	181	0.0	0.292	9.7	LOSA	1.2	8.3	0.55	0.85	0.63	50.0
Appro	ach	342	0.0	0.292	8.0	LOSA	1.2	8.3	0.42	0.71	0.46	49.1
West:	Glen St	treet										
10	L2	239	0.0	0.294	5.8	LOSA	1.6	11.2	0.18	0.43	0.18	54.2
11	T1	104	0.0	0.294	0.5	LOSA	1.6	11.2	0.18	0.43	0.18	52.8
12	R2	38	0.0	0.294	6.7	LOSA	1.6	11.2	0.18	0.43	0.18	53.7
Appro	ach	381	0.0	0.294	4.4	NA	1.6	11.2	0.18	0.43	0.18	53.9
All Ve	hicles	1049	0.0	0.294	5.1	NA	1.6	11.2	0.27	0.48	0.28	52.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [(AM Future) Lockwood Avenue / Glen Street]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ment F	erformand	e - Ve	hicles								
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	9
South	East: Gl	en Street										
5	T1	54	0.0	0.247	0.7	LOSA	1.2	8.3	0.33	0.51	0.33	53.9
6	R2	228	0.0	0.247	4.4	LOSA	1.2	8.3	0.33	0.51	0.33	51.5
Appro	ach	282	0.0	0.247	3.7	NA	1.2	8.3	0.33	0.51	0.33	52.0
North	East: Gle	en Street										
7	L2	145	0.0	0.090	5.6	LOSA	0.4	2.7	0.07	0.55	0.07	48.5
9	R2	65	0.0	0.106	9.5	LOSA	0.4	2.8	0.55	0.77	0.55	50.5
Appro	ach	211	0.0	0.106	6.8	LOSA	0.4	2.8	0.22	0.62	0.22	49.3
North ¹	West: Lo	ckwood Ave	nue									
10	L2	104	0.0	0.095	5.9	LOSA	0.4	2.9	0.20	0.48	0.20	53.6
11	T1	120	0.0	0.095	0.6	LOSA	0.5	3.2	0.24	0.17	0.24	56.8
Appro	ach	224	0.0	0.095	3.1	NA	0.5	3.2	0.22	0.31	0.22	54.8
All Ve	hicles	717	0.0	0.247	4.4	NA	1.2	8.3	0.27	0.48	0.27	52.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 101 [(AM Future) Glen Street / Blackbutts Road]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ment P	erformand	e - Vel	nicles								
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East:	Blackbut	ts Road										
5	T1	258	0.0	0.133	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	60.0
6	R2	155	0.0	0.140	7.5	LOSA	0.6	4.3	0.52	0.71	0.52	46.4
Appro	ach	413	0.0	0.140	2.8	NA	0.6	4.3	0.19	0.27	0.19	56.0
North: Glen Street												
7	L2	186	0.0	0.159	4.7	LOSA	0.7	4.6	0.43	0.63	0.43	50.3
9	R2	78	0.0	0.190	11.0	LOSA	0.7	4.8	0.70	0.87	0.70	42.6
Appro	ach	264	0.0	0.190	6.6	LOSA	0.7	4.8	0.51	0.70	0.51	47.7
West:	Blackbu	tts Road										
10	L2	123	0.0	0.251	5.6	LOSA	0.0	0.0	0.00	0.15	0.00	29.7
11	T1	361	0.0	0.251	0.0	LOSA	0.0	0.0	0.00	0.15	0.00	58.6
Appro	ach	484	0.0	0.251	1.4	NA	0.0	0.0	0.00	0.15	0.00	50.9
All Ve	hicles	1161	0.0	0.251	3.1	NA	0.7	4.8	0.18	0.32	0.18	52.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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▽ Site: 101 [(AM Future) Glen Street / Genrose Place]

Site Category: (None) Giveway / Yield (Two-Way)

Mov	ement F	Performanc	e - Vel	hicles								
Mov ID	Turn	Demand I Total veh/h		Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	
South	n: Car Pa			1,0								101711
1	L2	29	0.0	0.037	5.8	LOS A	0.1	1.0	0.20	0.57	0.20	53.0
2	T1	1	0.0	0.037	6.3	LOSA	0.1	1.0	0.20	0.57	0.20	53.1
3	R2	12	0.0	0.037	7.9	LOSA	0.1	1.0	0.20	0.57	0.20	47.3
Appro	oach	42	0.0	0.037	6.4	LOSA	0.1	1.0	0.20	0.57	0.20	51.9
East:	Glen Str	reet										
4	L2	15	0.0	0.128	4.9	LOS A	0.6	4.3	0.27	0.28	0.27	53.8
5	T1	98	0.0	0.128	0.7	LOS A	0.6	4.3	0.27	0.28	0.27	56.3
6	R2	28	0.0	0.128	6.1	LOS A	0.6	4.3	0.27	0.28	0.27	53.4
Appro	oach	141	0.0	0.128	2.3	NA	0.6	4.3	0.27	0.28	0.27	55.4
North	: Glenro:	se Place										
7	L2	39	0.0	0.029	6.3	LOSA	0.1	0.8	0.31	0.56	0.31	47.3
8	T1	3	0.0	0.113	5.9	LOSA	0.4	2.7	0.43	0.70	0.43	52.1
9	R2	82	0.0	0.113	7.7	LOSA	0.4	2.7	0.43	0.70	0.43	51.5
Appro	oach	124	0.0	0.113	7.2	LOSA	0.4	2.7	0.39	0.66	0.39	50.6
West	: Glen St	reet										
10	L2	171	0.0	0.274	6.0	LOSA	1.5	10.2	0.28	0.41	0.28	54.4
11	T1	129	0.0	0.274	0.9	LOS A	1.5	10.2	0.28	0.41	0.28	53.1
12	R2	31	0.0	0.274	6.4	LOSA	1.5	10.2	0.28	0.41	0.28	53.9
Appro	oach	331	0.0	0.274	4.1	NA	1.5	10.2	0.28	0.41	0.28	54.0
All Ve	ehicles	638	0.0	0.274	4.4	NA	1.5	10.2	0.30	0.44	0.30	53.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [(PM Future) Lockwood Avenue / Glen Street]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles													
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h		
South	East: Gl	en Street												
5	T1	126	0.0	0.348	0.4	LOSA	1.9	13.5	0.26	0.43	0.26	54.8		
6	R2	284	0.0	0.348	4.3	LOSA	1.9	13.5	0.26	0.43	0.26	52.3		
Appro	ach	411	0.0	0.348	3.1	NA	1.9	13.5	0.26	0.43	0.26	53.0		
North	East: Gle	en Street												
7	L2	253	0.0	0.154	5.5	LOSA	0.0	0.0	0.00	0.58	0.00	48.8		
9	R2	122	0.0	0.215	10.5	LOSA	0.8	5.9	0.60	0.84	0.60	49.8		
Appro	ach	375	0.0	0.215	7.2	LOSA	0.8	5.9	0.20	0.66	0.20	49.3		
North ¹	West: Lo	ckwood Ave	enue											
10	L2	106	0.0	0.077	5.7	LOSA	0.3	2.2	0.13	0.54	0.13	53.2		
11	T1	81	0.0	0.072	0.3	LOSA	0.3	2.4	0.17	0.06	0.17	58.6		
Appro	ach	187	0.0	0.077	3.4	NA	0.3	2.4	0.15	0.33	0.15	54.7		
All Ve	hicles	973	0.0	0.348	4.7	NA	1.9	13.5	0.21	0.50	0.21	51.8		

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [(PM Future) Glen Street / Blackbutts Road]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement F	erformanc	e - Vel	nicles								
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	9
East:	Blackbu	tts Road										
5	T1	256	0.0	0.132	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	60.0
6	R2	307	0.0	0.222	6.6	LOSA	1.1	7.7	0.42	0.63	0.42	46.8
Appro	ach	563	0.0	0.222	3.6	NA	1.1	7.7	0.23	0.34	0.23	54.0
North	: Glen S	treet										
7	L2	188	0.0	0.135	3.9	LOS A	0.6	4.0	0.29	0.54	0.29	51.0
9	R2	144	0.0	0.322	11.6	LOS A	1.4	10.0	0.71	0.92	0.87	42.0
Appro	ach	333	0.0	0.322	7.3	LOSA	1.4	10.0	0.47	0.71	0.54	46.7
West:	Blackbu	ıtts Road										
10	L2	103	0.0	0.148	5.6	LOS A	0.0	0.0	0.00	0.22	0.00	29.5
11	T1	181	0.0	0.148	0.0	LOS A	0.0	0.0	0.00	0.22	0.00	58.1
Appro	ach	284	0.0	0.148	2.0	NA	0.0	0.0	0.00	0.22	0.00	47.3
All Ve	hicles	1180	0.0	0.322	4.3	NA	1.4	10.0	0.24	0.41	0.26	50.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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▽ Site: 101 [(PM Future) Glen Street / Genrose Place]

Site Category: (None) Giveway / Yield (Two-Way)

Move	ement F	Performanc	e - Ve	hicles								
Mov ID	Turn	Demand F Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	
		veh/h	%	v/c	sec		veh	m				km/h
South	: Car Pa	ırk										
1	L2	20	0.0	0.027	6.0	LOS A	0.1	0.7	0.26	0.57	0.26	52.8
2	T1	4	0.0	0.027	7.2	LOS A	0.1	0.7	0.26	0.57	0.26	53.0
3	R2	4	0.0	0.027	8.8	LOS A	0.1	0.7	0.26	0.57	0.26	47.2
Appro		28	0.0	0.027	6.6	LOSA	0.1	0.7	0.26	0.57	0.26	52.3
East:	Glen Str	eet										
4	L2	19	0.0	0.210	5.0	LOS A	1.1	7.4	0.29	0.33	0.29	53.1
5	T1	144	0.0	0.210	0.8	LOSA	1.1	7.4	0.29	0.33	0.29	55.5
6	R2	65	0.0	0.210	6.7	LOSA	1.1	7.4	0.29	0.33	0.29	52.7
Appro	ach	228	0.0	0.210	2.8	NA	1.1	7.4	0.29	0.33	0.29	54.4
North	: Glenros	se Place										
7	L2	106	0.0	0.078	6.2	LOS A	0.3	2.2	0.30	0.58	0.30	47.3
8	T1	1	0.0	0.319	7.4	LOS A	1.4	9.6	0.54	0.85	0.63	50.8
9	R2	209	0.0	0.319	9.4	LOSA	1.4	9.6	0.54	0.85	0.63	50.2
Appro	ach	317	0.0	0.319	8.4	LOSA	1.4	9.6	0.46	0.76	0.52	49.5
West	Glen St	reet										
10	L2	258	0.0	0.318	6.1	LOSA	1.7	12.1	0.29	0.45	0.29	53.9
11	T1	111	0.0	0.318	1.0	LOS A	1.7	12.1	0.29	0.45	0.29	52.3
12	R2	22	0.0	0.318	6.7	LOS A	1.7	12.1	0.29	0.45	0.29	53.4
Appro	ach	391	0.0	0.318	4.7	NA	1.7	12.1	0.29	0.45	0.29	53.6
All Ve	hicles	964	0.0	0.319	5.5	NA	1.7	12.1	0.34	0.53	0.36	52.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [(SAT Future) Lockwood Avenue / Glen Street]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement P	erformand	e - Ve	hicles								
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South	East: Gl	en Street										
5	T1	113	0.0	0.420	0.8	LOSA	2.5	17.2	0.36	0.50	0.36	54.0
6	R2	379	0.0	0.420	4.5	LOSA	2.5	17.2	0.36	0.50	0.36	51.6
Appro	ach	492	0.0	0.420	3.6	NA	2.5	17.2	0.36	0.50	0.36	52.2
North	East: Gle	en Street										
7	L2	323	0.0	0.197	5.5	LOSA	0.0	0.0	0.00	0.58	0.00	48.8
9	R2	148	0.0	0.312	13.2	LOSA	1.4	9.6	0.69	0.91	0.83	48.0
Appro	ach	472	0.0	0.312	8.0	LOSA	1.4	9.6	0.22	0.68	0.26	48.4
North ¹	West: Lo	ckwood Ave	nue									
10	L2	119	0.0	0.089	5.8	LOSA	0.4	2.6	0.18	0.54	0.18	53.1
11	T1	73	0.0	0.067	0.5	LOSA	0.3	2.2	0.23	0.10	0.23	58.1
Appro	ach	192	0.0	0.089	3.8	NA	0.4	2.6	0.20	0.37	0.20	54.2
All Ve	hicles	1155	0.0	0.420	5.4	NA	2.5	17.2	0.27	0.55	0.29	50.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [(SAT Future) Glen Street / Blackbutts Road]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement F	Performanc	e - Ve	hicles								
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	
East:	Blackbu	tts Road										
5	T1	187	0.0	0.097	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	60.0
6	R2	346	0.0	0.263	6.9	LOSA	1.3	9.2	0.46	0.66	0.46	46.6
Appro	ach	534	0.0	0.263	4.5	NA	1.3	9.2	0.30	0.43	0.30	52.5
North:	: Glen S	treet										
7	L2	252	0.0	0.179	3.9	LOS A	0.8	5.6	0.30	0.55	0.30	51.0
9	R2	145	0.0	0.325	11.7	LOSA	1.4	10.1	0.71	0.92	0.88	42.0
Appro	ach	397	0.0	0.325	6.8	LOSA	1.4	10.1	0.45	0.68	0.51	47.3
West:	Blackbu	utts Road										
10	L2	153	0.0	0.173	5.6	LOS A	0.0	0.0	0.00	0.27	0.00	29.3
11	T1	178	0.0	0.173	0.0	LOSA	0.0	0.0	0.00	0.27	0.00	57.5
Appro	ach	331	0.0	0.173	2.6	NA	0.0	0.0	0.00	0.27	0.00	44.0
All Ve	hicles	1261	0.0	0.325	4.7	NA	1.4	10.1	0.27	0.47	0.29	48.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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▽ Site: 101 [(SAT Future) Glen Street / Genrose Place]

Site Category: (None) Giveway / Yield (Two-Way)

Mari	Т	Damand	-laura	Dan	A.,	l aval af	OFO/ Dool	of O	Duan	T#aatii.ca	Aven Ne	A
Mov ID	Turn	Demand I Total	HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop.	Stop Rate	Aver. No.	Speed
טו		veh/h	пv %	V/C	sec	Service	vernicies	Distance	Queueu	Stop Nate	Cycles	km/h
South	: Car Pa		70	,,,			7011					1011/11
1	L2	34	0.0	0.072	6.1	LOSA	0.3	1.8	0.36	0.63	0.36	51.9
2	T1	14	0.0	0.072	9.0	LOSA	0.3	1.8	0.36	0.63	0.36	52.1
3	R2	11	0.0	0.072	11.2	LOSA	0.3	1.8	0.36	0.63	0.36	45.9
Appro	ach	58	0.0	0.072	7.7	LOS A	0.3	1.8	0.36	0.63	0.36	51.2
East:	Glen Str	eet										
4	L2	32	0.0	0.308	5.0	LOSA	1.7	11.7	0.31	0.37	0.31	52.4
5	T1	196	0.0	0.308	0.9	LOSA	1.7	11.7	0.31	0.37	0.31	54.8
6	R2	101	0.0	0.308	7.4	LOSA	1.7	11.7	0.31	0.37	0.31	52.0
Appro	ach	328	0.0	0.308	3.3	NA	1.7	11.7	0.31	0.37	0.31	53.7
North	: Glenros	se Place										
7	L2	220	0.0	0.161	6.3	LOSA	0.7	4.9	0.32	0.59	0.32	47.3
8	T1	1	0.0	0.499	10.3	LOSA	2.7	18.7	0.68	0.98	1.04	48.5
9	R2	271	0.0	0.499	12.9	LOSA	2.7	18.7	0.68	0.98	1.04	47.9
Appro	ach	492	0.0	0.499	10.0	LOS A	2.7	18.7	0.52	0.81	0.72	47.7
West:	Glen St	reet										
10	L2	328	0.0	0.379	6.1	LOSA	2.2	15.3	0.30	0.48	0.30	53.6
11	T1	104	0.0	0.379	1.0	LOSA	2.2	15.3	0.30	0.48	0.30	51.8
12	R2	38	0.0	0.379	7.2	LOSA	2.2	15.3	0.30	0.48	0.30	53.1
Appro	ach	471	0.0	0.379	5.1	NA	2.2	15.3	0.30	0.48	0.30	53.3
All Ve	hicles	1348	0.0	0.499	6.5	NA	2.7	18.7	0.38	0.58	0.46	51.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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ANNEXURE D: PEAK USAGE PROFILES

(Values within each cell are percentages of peak demand, with peak being 100%)

				Glenros	e Village		
Time	Monday	Tuesday	Wednesd	Thursday	Friday	Saturday	Sunday
00:00	0	0	0	0	0	0	0
01:00	0	0	0	0	0	0	0
02:00	0	0	0	0	0	0	0
03:00	0	0	0	0	0	0	0
04:00	0	0	0	0	0	0	0
05:00	0	0	0	0	0	0	0
06:00	0	0	0	0	0	0	0
07:00	0	0	0	0	0	0	0
08:00	0	0	0	0	0	0	0
09:00	32	27	31	32	33	51	0
10:00	45	41	48	49	52	82	70
11:00	54	50	60	60	65	100	89
12:00	57	51	62	61	67	98	96
1:00 PM	56	48	56	53	60	86	94
2:00 PM	54	47	53	45	78	88	88
3:00 PM	55	53	55	46	54	76	82
4:00 PM	56	57	60	55	57	73	0
5:00 PM	52	54	58	60	57	0	0
6:00 PM	0	0	0	53	0	0	0
7:00 PM	0	0	0	37	0	0	0
8:00 PM	0	0	0	19	0	0	0
9:00 PM	0	0	0	0	0	0	0
10:00 PM	0	0	0	0	0	0	0
11:00 PM	0	0	0	0	0	0	0

	Gym						
Time	Monday	Tuesday	Wednesd	Thursday	Friday	Saturday	Sunday
0	0	0	9	0	0	0	0
1	0	0	9	0	0	0	0
2	0	0	0	0	0	9	0
3	0	0	0	0	0	0	0
4	9	9	0	0	18	0	0
5	27	27	9	9	27	9	0
6	36	54	18	45	36	9	0
7	36	54	27	63	27	9	0
8	18	36	18	45	27	18	0
9	9	18	9	9	27	18	0
10	0	9	9	0	27	9	0
11	0	9	9	0	27	9	0
12	9	9	18	9	18	9	0
13	18	9	18	9	9	18	0
14	9	9	9	9	9	27	0
15	9	9	9	9	0	27	0
16	9	18	9	9	0	36	0
17	27	27	9	18	0	36	0
18	72	45	36	36	9	36	0
19	100	63	81	45	18	27	0
20	100	72	72	45	27	18	0
21	63	63	18	27	18	9	0
22	27	45	9	9	9	27	0
23	9	18	0	9	9	9	0

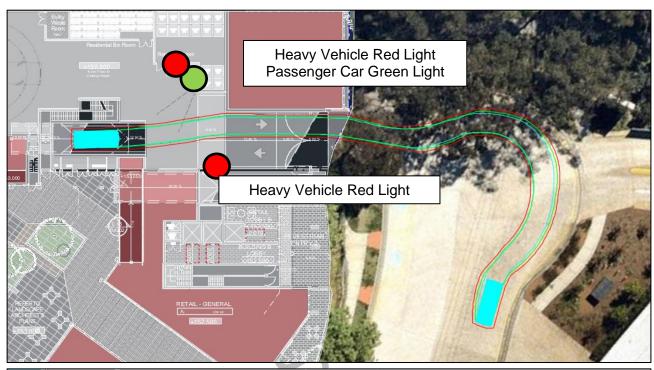


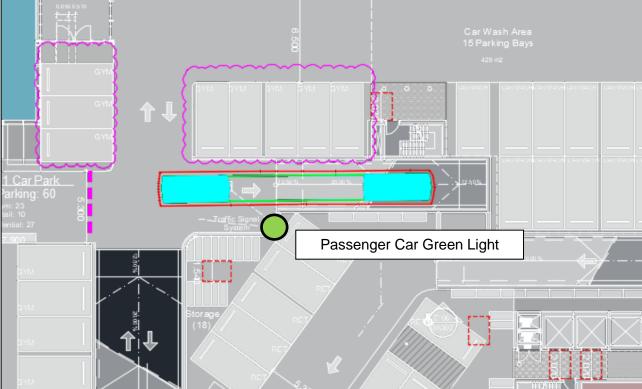
ANNEXURE E: REQUIRED CHANGES AND TRAFFIC SIGNAL CONCEPT



SIGNAL DESIGN CONCEPT FOR THE MIXED USE DEVELOPMENT AT 28 LOCKWOOD AVENUE, BELROSE

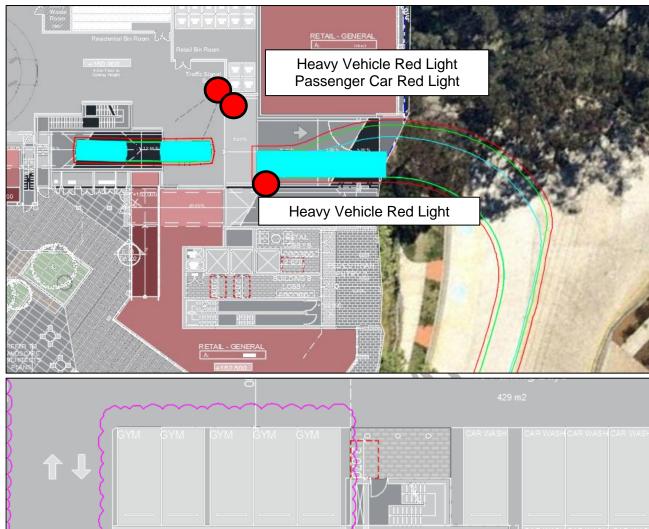
The default position is shown below, followed by the sequence of events which occurs when trucks arrive.

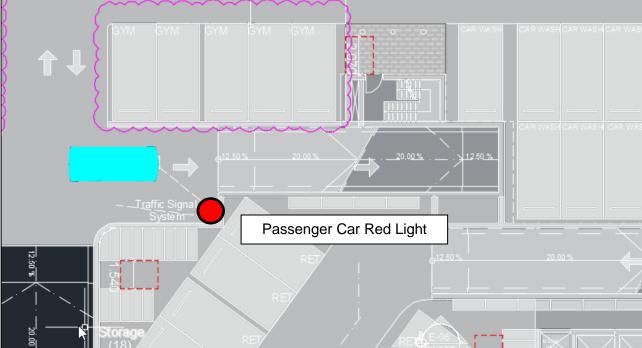




Default Position: Green lights for passenger cars at the top and bottom of the ramp. No trucks present.

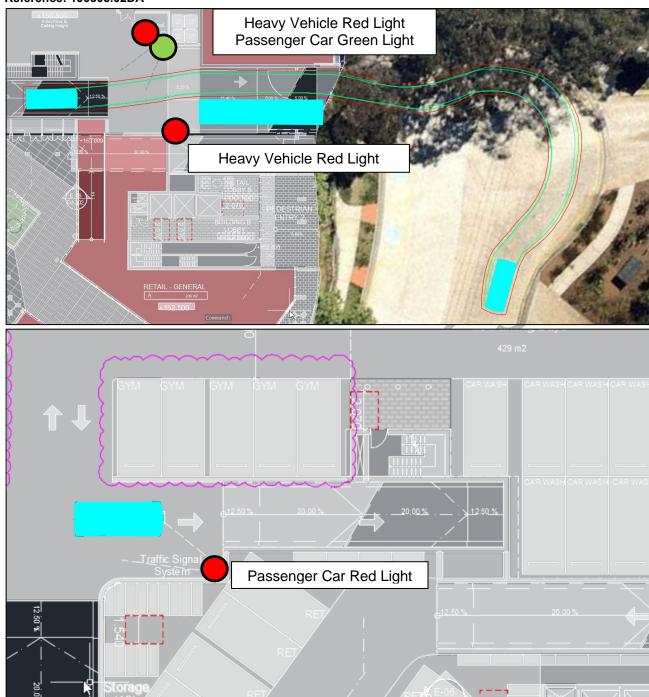






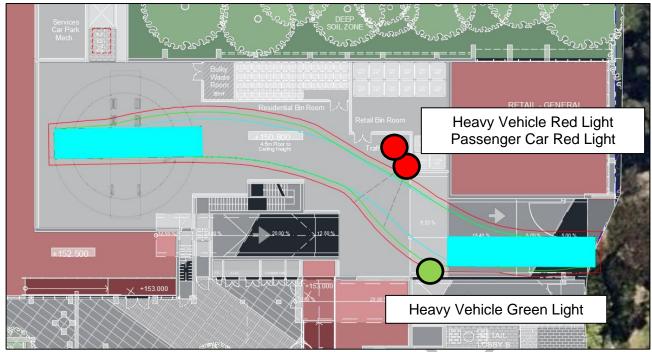
Stage 1: HRV enters the site, passenger cars are required to stop to allow HRV to stand in the position shown. All trucks must stop in this location upon entry. Passenger cars are stopped at the bottom of the ramp to avoid queueing on a steep gradient, but also at the top of the ramp in case any vehicles bypassed the first light before the red was triggered by the entering truck.





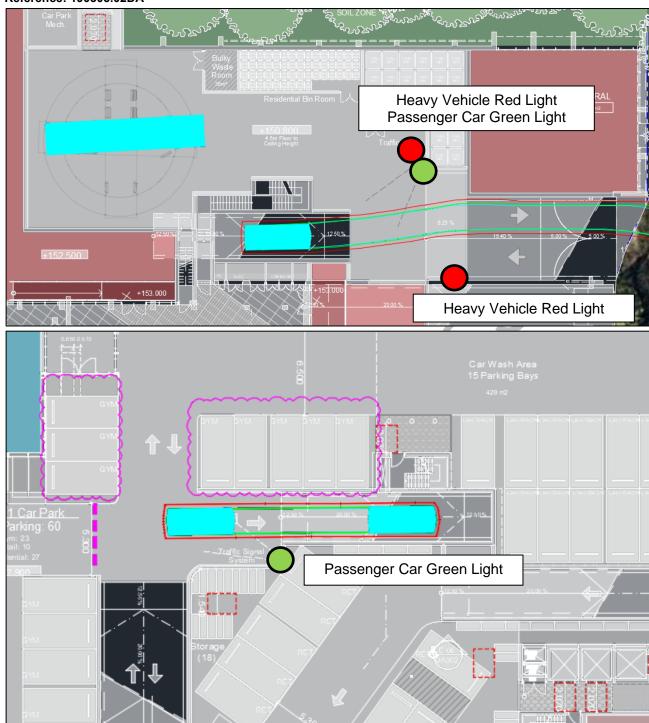
Stage 2: Once HRV stops in above location (which will happen every time a truck enters the site), the upper car light system turns green to allow the car to exit the site. The car light system at the bottom of the ramp remains red. The point of this stage is to clear the ramp queue of passenger car vehicles.





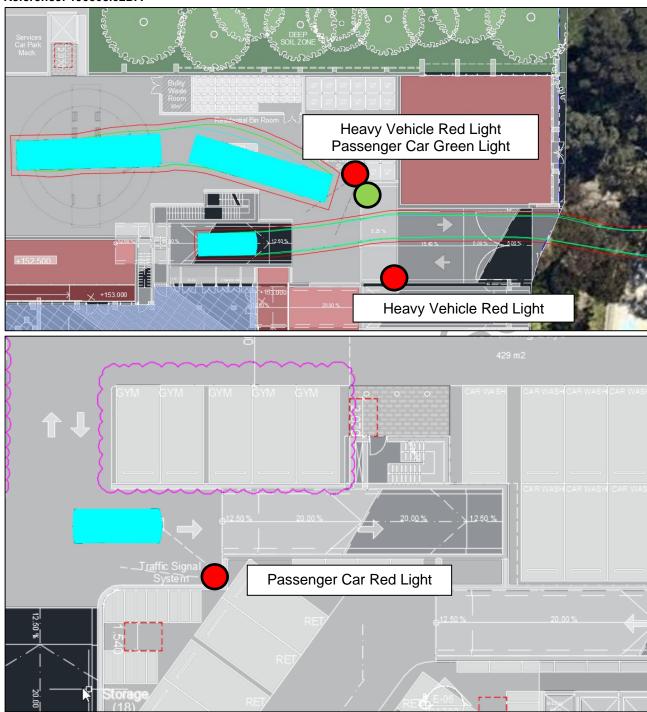
Stage 3: HRV enters loading area once ramp queue is cleared





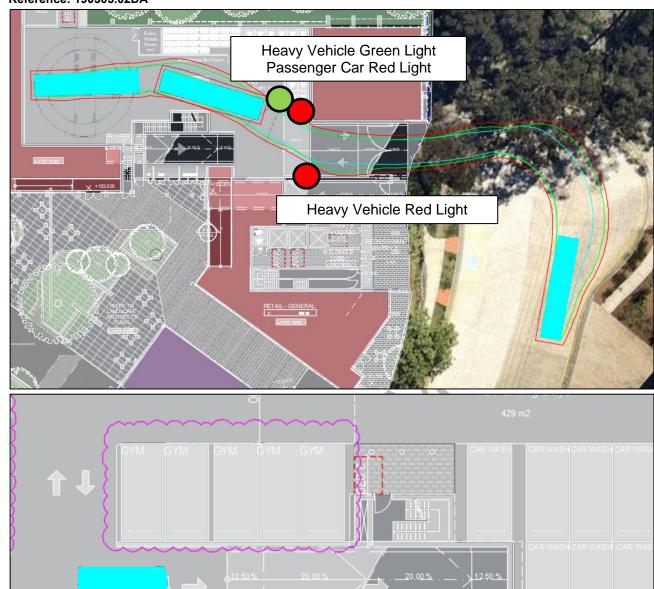
Stage 4: Passenger car lights both green while loading is taking place





Stage 5: The HRV must stop at the light when leaving the site. This triggers a red light in the basement. The ground floor passenger car light stays green to allow any queue in the ramp to clear.

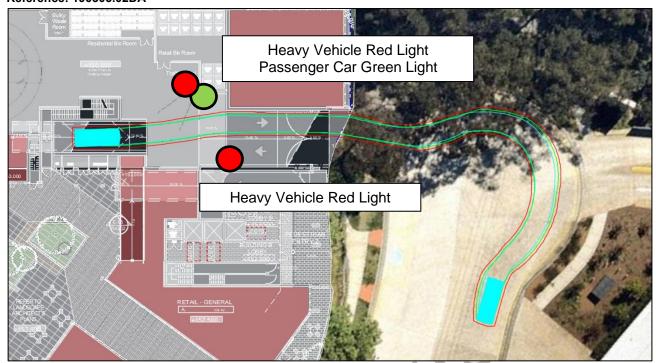


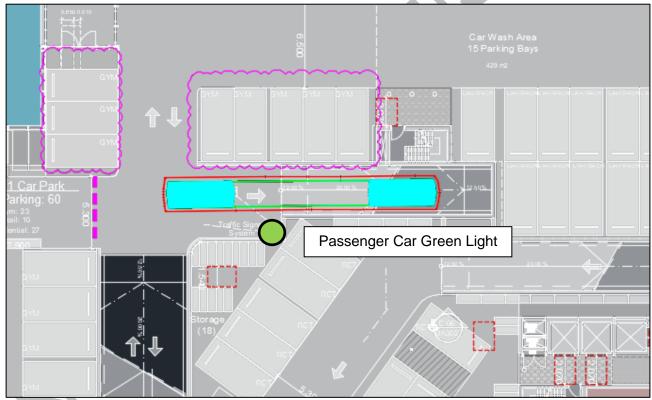


Stage 6: HRV free and clear to leave the site while passenger cars are queued at the bottom of the basement ramp.

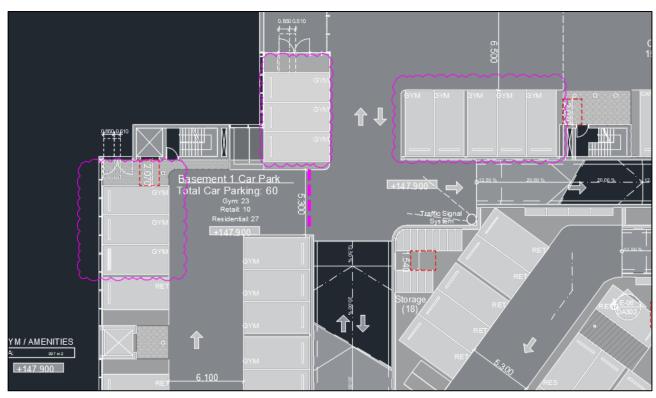
Passenger Car Red Light



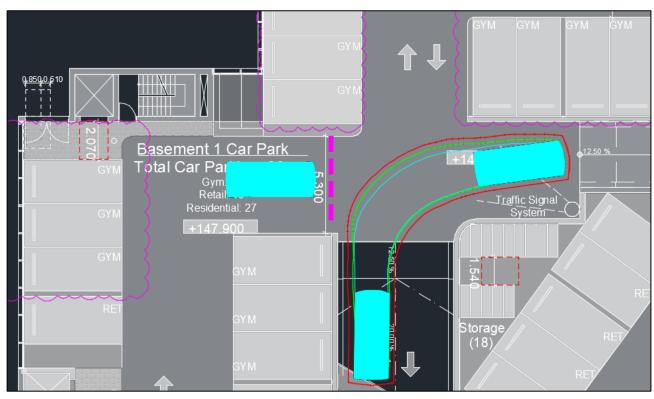




Default Position Restored: Green lights for passenger cars at the top and bottom of the ramp. No trucks present.



Gym spaces must be 2.6m in width. Currently they are shown as 2.4m wide



A give way line must be installed to give priority to vehicles exiting from the basement



Aisle width is shown as 5.3m where 6m is available. The minimum allowable aisle width is 5.8m. The plans should be amended to show a minimum 5.8m aisle.



ANNEXURE F: PROPOSED GLENROSE PLACE ROUNDABOUT CONCEPT

