

Our Ref: NA49913132-012:BCP/bcp  
Contact: Dr Brett C. Phillips



5<sup>th</sup> November 2014

The Manager  
Scentre Group Design  
GPO Box 4004  
**SYDNEY NSW 2001**

Attention: Mr Jade Hudson

Dear Jade,

**STORMWATER MANAGEMENT DEFERRED COMMENCEMENT CONDITION NO. 1  
DA2008/1742 AT WARRINGAH MALL – SECTION 96 APPLICATION**

One of the General Conditions of DA2008/1742 relates to a requirement to undertake a physical model study. This condition is worded as follows.

*17. Physical Model Studies*

*Physical model studies of the two large junction boxes, (B4 and B6 on drawing W4548- 101 by Cardno) to confirm the hydraulic losses which were assumed in the computer model studies are to be submitted to the Certifying Authority for approval. The studies are to provide advice on how the hydraulic losses in each junction box can be minimised.*

*The details are to be prepared by a suitably qualified Civil Engineer, who has membership to the Institution of Engineers Australia, National Professional Engineers Register (NPER-3) and is an Accredited Certifier (Category C4).*

*Details demonstrating compliance are to be submitted to the Certifying Authority for approval prior to the issue of the Construction Certificate.*

To assist in the scoping of the physical model study further modelling assessments were undertaken to optimise the layout of Chambers B4 and B6 on drawing W4548-101 taking into account planned future development and the likely location of columns and existing and/or relocated infrastructure. These modelling assessment were described in our letter report titled “Stormwater Management Deferred Commencement Condition No. 1 DA2008/1742 at Warringah Mall – Section 96 Application” and dated 28 October 2014.

The physical model study of the preferred configuration of Chambers B4 and B6 is described in the attached WRL Technical Report 13/30 dated July 2014.

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The observed losses at Chambers B4 and B6 were compared with the losses calculated from the 1D/2D model. It was concluded from this comparison that:

- The culvert exit / entry losses at Chamber B6 are currently over-estimated and could be reduced by say 50%;
- The culvert exit / entry losses associated with the entry and exit of the existing culvert into/from Chamber B4 could be doubled;
- These changes are unlikely to change the level of agreement between the predicted and observed water levels/heads upstream of Chamber B4.

It was also noted that the observed heads just upstream of Chamber B4 are substantially lower than calculated by the 1D/2D model at upstream locations. Adjustments to adopted culvert exit / entry losses will not substantially reduce these differences. This suggests that the roughness value for the various culverts in the numerical model is higher than represented in the physical model.

It is concluded that the current numerical 1D/2D model results appear to be conservative in comparison with the physical model results (which may be due to differences in adopted conduit roughness values in the numerical model and the roughness of the conduits in the physical model) and that the flood levels estimated by the numerical model are likely to be conservative.

### **Re-configuration of the Stormwater DA Layout**

Subsequent to the modelling assessments to optimise the layout of Chambers B4 and B6 further consideration was given to the interaction of the proposed works with other services, the feasibility of relocating services at the southern end of Green Street, traffic management during construction, construction sequencing and potential hydraulic impacts during construction.

This led to further consideration of the proposed construction of two additional culverts under Condamine Street and the merit of separating the alignment of the new stormwater culverts from the existing culverts realigning the additional stormwater culverts through the Bing Lee site. These are discussed as follows.

#### *Green Street Inlet*

The structural design of the Green Street inlet incorporates “360 UB 50” transverse beams at 960 mm centres to support the grated inlet. On the side facing Green Street this has created a 9.6 m wide and 0.36 m high side entry inlet with an invert level set at 8.86 m AHD. The underside of the grates are set at 9.22 m AHD while the top of the trafficable grate is set at 9.32 m AHD.

This configuration substantially lowers the level at which water ponded in the Green Street low point can spill into the new culverts. It also increases the complexity of inflows into the inlet. Consequently a rating curve was developed for the inlet which accounts for the initial weir flow through the side inlet which changes to orifice flow when the water level in Green Street rises above around 9.22 m AHD in combination with initial weir flow spilling across the grates which in turn changes regime if the water level rises to a level which covers the grated inlet.

The latest configuration has a substantial reserve capacity to deal with any blockage of the side inlet and/or the grates.

### *Re-alignment of Additional Culverts*

In our letter of 21 February 2014 we concluded that the temporary closure of a main culvert to allow the reconstruction of Chamber B4 would have a significant adverse impact on flood levels in a number of locations and that in events greater than a 10 yr ARI event the construction impacts are sufficient to initiate flows through entries into Warringah Mall sooner than would otherwise would be the case (and subject to the to the degree of debris blockage of key inlets).

Further consideration was also given to the interaction of the proposed works with other services and the feasibility of relocating services at the southern end of Green Street and traffic management during construction. This led to a decision to realign the additional stormwater culverts through the Bing Lee site.

The ramifications of this decision include:

- The hydraulic separation of the existing culverts and the additional culverts;
- The simplification of the proposed works due to the removal of the need to adjust culverts sizes to match existing culvert sizes and the removal of a limitation on pit invert levels due to the need to match existing pit invert levels;
- No need to reconstruct Chamber B4 thereby avoiding the adverse impacts on flood levels during construction;

At the same time it has been possible to retain the same inlet configuration at Chamber B6 as was assessed during the physical model study.

The re-aligned layout of the proposed stormwater culverts is given in Drawing W4548-401(3).

The modified Stormwater DA scheme which has been assessed is based on the re-alignment of the proposed stormwater culverts and the modified layouts of Chamber C6 described above.

### *Chamber C6*

Currently there are 4 x 2.8 m (W) x 1.8 m (H) culverts which convey stormwater from the Warringah Mall under Condamine Street and into the Brookvale Creek GPT. The GPT in turn discharges into a rehabilitated reach of Brookvale Creek. Currently stormwater discharges from the culverts and across an apron before discharging into the pool created by the GPT and its trash rack. The primary features of the outfall of the existing culverts and of the downstream GPT are displayed in **Plates 1 – 6**.

Upstream of Condamine Street stormwater conveyed by 1 x 4.2 m (W) x 1.8 m (H) + 2 x 1350 mm diameter conduits + 1 x 1500 mm diameter conduit + 3 x 1.8 m diameter conduits + 1 x 900 mm diameter conduit discharges into a large chamber which in turn discharges into the Condamine Street culverts (refer **Attachment A**).

The approval of the Warringah Mall Stormwater DA (with Deferred Commencement Conditions) was based on a scheme which included the construction of two additional 3.3 m (W) x 1.8 m (H) culverts beside existing the 4 x 2.8 m (W) x 1.8 m (H) culverts.

The proposed construction of two additional culverts under Condamine Street poses major challenges both in terms of traffic management and construction sequencing.

Consequently consideration has been given to a modification of the proposed works in the vicinity of Condamine Street to allow the hydraulic capacity of the Condamine Street culverts to be increased without disrupting Condamine Street.

Numerical model has indicated that connecting the two new 3.3 m (W) x 1.8 m (H) culverts into the existing chamber (Chamber C6) and lowering the two central culverts under Condamine Street by 1.0 m gives comparable hydraulic performance to the approved scheme.

The concept works are sketched in **Attachments A and B**.

#### *Further Refinements*

Based on a review of the number of flood barriers required to achieve the required freeboard during Stage 1 which would not be required under Stage 2 further refinement of the Stage 1 works was undertaken. These refinements included:

- Raising the level of the parapet wall beside the access road on the upstream side of Warringah Mall by 0.4 m to confine overflows over the parapet wall to the east of the access road flood barrier;
- Including the access road flood barrier in Stage 1 rather than Stage 2 as previously proposed;
- Including the transfer beams which span the open chamber southeast of the Cross Street roundabout in the hydraulic model;
- Connecting the new Green Street culverts temporarily to the existing culvert at the southern end of Green Street by installing a temporary 1350 mm diameter RCP connection;
- Upgrading conduits which convey Myer roof runoff to the drainage system, namely replacing 525 mm diameter pipes with 750 mm diameter pipes, and replacing 450 mm diameter or smaller pipes with 525 mm diameter pipes.

These refinements are also reflected in the Stage 2 scheme.

#### **DEFERRED COMMENCEMENT CONDITION NO. 1**

In Council's letter dated 22 May 2012 in relation to Modification Application No: DA2008/1742, the proposed works were described as follows:

*Construction of stormwater upgrade works for Warringah Mall and watercourse bank stabilisation works*

Council's letter advised that the request to modify the above-mentioned Development Consent has been approved on 16 May 2012 as follows:

##### **1. Stormwater Management**

*Evidence that the stormwater works will meet the objectives and requirements of Section 6.1 'Stormwater Management' of the Warringah Development Control Plan - Warringah Mall Shopping Centre (the DCP) is to be submitted to Council for written approval.*

*The following information is to be submitted as a minimum:*

- A ground floor plan of Warringah Mall (on land at Lot 100, DP 1015283, No 145 Old Pittwater Road) showing extent of works approved as part of DA1996/137 (Development Consent DA96/335) and DA1998/17 (Development Consent DA98/229). The plan must show critical*

*finished floor levels for DA19961137 and DA1998117 and critical peak 100 year ARI flood levels as a result of the augmentation works.*

This plan is given in the attached Drawing W4548-420 (5).pdf. The approach is outlined in W4548-401 (3).pdf and detailed in

- W4548-422 (3).pdf,
- W4548-423 (3).pdf,
- W4548-424 (3).pdf
- W4548-426 (3).pdf,
- W4548-428 (3).pdf,
- W4548-429 (3).pdf

*ii) Details demonstrating how the flood protection measures will provide the required minimum 300mm freeboard to the 100 year ARI flood event to the finished floor levels of the development approved under DA1996/137 and DA1998/17.*

The details are given in Drawings W4548-422 (3).pdf, W4548-423 (3).pdf, W4548-424 (3).pdf, W4548-426 (3).pdf, W4548-428 (3).pdf and W4548-429 (3).pdf

*iii) An update of Table A.5 in the 'Flood Impact Assessment Addendum Report' dated January 2010 prepared by Cardno showing the results of the latest flood modelling and as a result of complying with the above points.*

Table A.5 from the 2010 Warringah Mall Flood Impact Assessment Addendum has been updated to include:

- The estimated 100 yr ARI flood levels for both 50% and 0% blockage of key stormwater inlets under Existing Conditions,
- Updated estimated 100 yr ARI flood levels under the Stormwater DA for both 50% and 0% blockage of key stormwater inlets;
- Updated estimated 100 yr ARI flood levels under the reconfigured Stormwater DA (Stage 2) for both 50% and 0% blockage of key stormwater inlets;
- Additional reference locations as follows:
  - Reference locations 221, 222 – DJ Loading Dock
  - Reference Locations 300 – 307 – Green St, Cross St and Dale St
  - Reference Locations 401 -404 – “Bus Depot” subcatchment east of Pittwater Road
  - Reference Locations 501-505 – Anchor Car Park / Coles Loading Dock

The updated Table A.5 is attached.

*iv) A set of figures showing the impact the proposed development will have on peak 100 year ARI flood levels. The figures are to show the difference in peak flood levels between pre- and post developed conditions as a graded set of coloured regions. A separate figure is to be prepared for each of the development scenarios contained in the updated Table A.5. It is anticipated that the figures will be centred mainly on Warringah Mall (on land at Lot 100, DP 1015283, No 145 Old Pittwater Road) since it is understood that the impact of the proposed development is limited to the site and areas immediately adjacent to it. The figures are to be presented at a scale which renders the information legible to the naked eye. A legend clearly identifying the range of levels comprising the graded set of coloured regions is to be included on each figure.*

Two sets of drawings each comprising two sheets are attached for the reconfigured Stormwater DA (Stage 2) 0% and 50% blocked scenarios.

## POTENTIAL CONSTRUCTION IMPACTS ON FLOODING

The potential impacts of the reconstructing Chamber C6 and lowering the two central culvert under Condamine St were previously assessed by analysing the impact of temporarily blocking one (Scenario C6A) or two (Scenario C6B) of the existing 2.8 m (W) x 1.8 m (H) culverts under Condamine Street.

The construction scenario was based on the following assumptions:

- The Condamine Street culverts are lowered and Chamber C6 is reconstructed in the initial stages of construction ie. the upstream drainage system is equivalent to Existing Conditions;
- The best case scenario is that only one of the existing culverts under Condamine St is temporarily de-commissioned (Scenario A);
- The worst case scenario is that two of the existing culverts under Condamine St are temporarily de-commissioned (Scenario B);
- The reconstruction of Chamber C6 will be managed such that incoming flows in a 5 yr ARI or 10 yr ARI flood can be conveyed through the chamber to the active culverts under Condamine St.

The 5 yr ARI and 10 yr ARI 90 minute storms have been assessed under Existing Conditions and under the Chamber C6 construction scenarios A and B.

The impacts of the construction scenario are assessed against the same flood event under Existing Conditions. The construction scenario flood levels are also compared with the 100 yr ARI flood levels under Existing Conditions to assess if the construction scenario locally raises flood levels anywhere to around the 100 yr ARI flood levels.

The following conclusions are drawn from the tabulated results which are attached in separate tables:

- In a 5 yr ARI event the temporary de-commissioning one culvert under Condamine St would:
  - Locally increase the 5 yr ARI flood levels in Brookvale Creek upstream of Warringah Mall by up to 3 cm only;
  - Locally increase the 5 yr ARI flood levels in Green St by up to 2 cm but still be around 17 cm lower than the 100 yr ARI flood level under Existing Conditions;
  - Locally increase the 5 yr ARI flood level in the vicinity of the DJ Loading Dock by around 3 cm but still be around 90 cm lower than the 100 yr ARI flood level under Existing Conditions;
- In a 10 yr ARI event the temporary de-commissioning one culvert under Condamine St would:
  - Locally increase the 10 yr ARI flood levels in Brookvale Creek upstream of Warringah Mall by up to 3 cm but still be around 55 cm lower than the 100 yr ARI flood level under Existing Conditions;
  - Locally increase the 10 yr ARI flood levels in Green St by up to 2 cm (and to within 17 cm of the 100 yr ARI flood level under Existing Conditions);
  - Locally increase the 10 yr ARI flood level in the vicinity of the DJ Loading Dock by around 8 cm but still be around 80 cm lower than the 100 yr ARI flood level under Existing Conditions;
- In a 5 yr ARI event the temporary de-commissioning two culverts under Condamine St would:

- Locally increase the 5 yr ARI flood levels in Brookvale Creek upstream of Warringah Mall by up to 15 cm but still be around 60 cm lower than the 100 yr ARI flood level under Existing Conditions;
  - Locally increase the 5 yr ARI flood levels in Green St by up to 4 cm but still be around 15 cm lower than the 100 yr ARI flood level under Existing Conditions;
  - Locally increase the 5 yr ARI flood level in the vicinity of the DJ Loading Dock by around 8 cm but still be around 90 cm lower than the 100 yr ARI flood level under Existing Conditions; and
  - Slightly reduce the 5 yr ARI flood levels downstream of Condamine St by up to 2 cm.
- In a 10 yr ARI event the temporary de-commissioning two culverts under Condamine St would:
    - Locally increase the 10 yr ARI flood levels in Brookvale Creek upstream of Warringah Mall by up to 14 cm but still be around 45 cm lower than the 100 yr ARI flood level under Existing Conditions;
    - Locally increase the 10 yr ARI flood levels in Green St by up to 4 cm but still be around 15 cm lower than the 100 yr ARI flood level under Existing Conditions;
    - Locally increase the 10 yr ARI flood level in the vicinity of the DJ Loading Dock by around 34 cm but still be around 55 cm lower than the 100 yr ARI flood level under Existing Conditions; and
    - Slightly reduce the 10 yr ARI flood levels downstream of Condamine St by up to 2 cm.

We would be pleased to respond to any queries regarding this information upon request.

Yours faithfully



.....  
*Dr Brett C. Phillips*  
*Director, Water Engineering*  
for **Cardno**



**Plate 1 Condamine Street Culverts**

This plate shows the existing 4 x 2.8 m (W) x 1.8 m (H) culverts located under Condamine Street. Also note the downstream apron.



**Plate 2 Condamine Street Culverts**

This is an oblique view of the existing culverts and apron. Note the step change in the level of the apron. The change in level is around 1.0 m.



**Plate 3 Downstream Apron**

This view shows the extent of the apron downstream of the culverts.





**Plate 4 Downstream Apron /GPT Pool**

This plate provides a view from above the culverts of the apron and the upper section of the pool created by the downstream GPT.



**Plate 5 Brookvale Creek GPT**

This is a view of the existing GPT including the trash rack. Note the pool of water impounded behind the wall of the GPT.



**Plate 6 Brookvale Creek GPT**

This is a closer view of the existing trash rack. If this trash was to fully block and to create a weir then the resulting backwater may impact on the discharge characteristics of the Condamine Street culverts.

# Water Research Laboratory

## **Warringah Mall Box Culvert Physical Model Study**

WRL Technical Report 2013/30  
July 2014

by  
N S Guerry and B M Miller



**UNSW**  
THE UNIVERSITY OF NEW SOUTH WALES

Water Research Laboratory  
University of New South Wales  
School of Civil and Environmental Engineering

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### Project Details

Report Title	Warringah Mall Box Culvert Physical Model Study
Report Author(s)	N S Guerry and B M Miller
Report No.	2013/30
Report Status	Final
Date of Issue	14 July 2014
WRL Project No.	2013062
Project Manager	Nathan Guerry
Client Name	Cardno (NSW/ACT) Pty Ltd
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Client Reference	89914004

### Document Status

Version	Reviewed By	Approved By	Date Issued
Draft	G P Smith	G P Smith	10/12/2013
Final	G P Smith	G P Smith	14/07/2014

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## 1. Introduction

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The Water Research Laboratory (WRL) of the School of Civil and Environmental Engineering at The University of New South Wales was commissioned by Cardno (NSW/ACT) Pty Ltd to undertake a physical model study of two proposed stormwater junction boxes at Warringah Mall in Brookvale, Sydney. In particular hydraulic losses across the junction boxes were investigated.

The stormwater upgrade works are proposed as part of broader re-development and refurbishment works at the existing shopping complex at Warringah Mall. This study was undertaken in order to satisfy a condition of DA2008/1742, which states:

*Physical model studies of the two large junction boxes, (B4 and B6 on drawing W4548-101 by Cardno) to confirm the hydraulic losses which were assumed in the computer model studies are to be submitted to the Certifying Authority for approval. The studies are to provide advice on how the hydraulic losses in each junction box can be minimised.*

Physical modelling was utilised to understand and optimise the complex hydraulic flows that may occur in these culverts. Physical modelling is an established and proven method for studying complex hydraulics, which are often difficult to assess using numerical modelling.

The two junction boxes were tested in a single physical model which extended from 12 m (prototype) upstream of junction box B4, to 12 m (prototype) downstream of junction Boxes B6. The model was constructed at a scale of 15:1. Testing was carried out under six different steady state boundary conditions, based on hydrograph and water level data provided by Cardno. These sets of tests were carried out for three different design configurations of junction box B4. This report presents the design of the physical model, the scaling laws, and the measurements and observations taken and a summary of model test results. Additional photographs and videos of model performance have been provided to Cardno.

The physical model study has provided essential information on the optimal layout of the junction boxes to ensure efficient hydraulic flow.

## 2. Physical Model

---

### 2.1 Model Scaling

A Froude scale of 15:1 was used for this model and Table 1 gives the ratios implied by this. The scale provided flow depths that could be accurately measured and ensured that head losses had suitable resolution. The culverts have a relatively flat grade and the scale ensured that frictional and surface tension effects did not become dominant. At this scale, the form losses and turbulence were accurately represented.

This scale ensured that maximum prototype flow rates were achievable, with total prototype flow of 33 m<sup>3</sup>/s being represented by 39 L/s in the model. It also ensured that lower flow rates were resolvable, with, for example, flow through B6-1 of 3.5 m<sup>3</sup>/s being represented by model flows of 4 L/s.

**Table 1 - Model Scaling Ratios**

Ratio	Symbol	Value
Length ratio	$L_R$	15
Velocity ratio	$V_R$	$(L_R)^{\frac{1}{2}}$
Flow ratio	$Q_R$	$(L_R)^{\frac{3}{2}}$

### 2.2 Model Construction

The model was constructed with a marine plywood base and acrylic walls and roofs. The model was constructed at a scale of 15:1 to the designs provided by Cardno as represented in Figure 5. The final design drawings are reproduced in Appendix B, and these include:

- 89914004-SK100, Rev 2, 19/08/2013
- 89914004-SK101, Rev 2, 16/08/2013
- 89914004-SK103, Rev 1, 19/08/2013
- 89914004-SK104, Rev 1, 19/08/2013
- 89914004-SK105, Rev 1 19/08/2013
- 89914004-SK106, Rev 1, 19/08/2013
- 89914004-SK2001, Rev 2, 21/08/2013
- 89914004-SK2002, Rev 2, 21/08/2013, amended 19/09/2013
- 89914004-SK2003, Rev 2, 21/08/2013, amended 19/09/2013

Six separate inflows were applied to the model as shown in Figure 5. Two head boxes were constructed to control the flow into the existing single culvert and into the proposed twin culverts upstream of junction box B4. The inflow pipe at the Green Street inlet was constructed using the closest available acrylic pipe diameter, having an internal diameter of 98 mm (a 2% variation from the precise model scale). Three separate screened inlets were constructed to model inflows at the grated inlets above each culvert in boxes B6-1 and B6-2.

The model invert levels were interpolated from invert level and grade data from the drawings supplied by Cardno. The model was installed on supports, which were surveyed with a dumpy level accurate to +/- 0.5 mm at the model scale (+/- 7.5 mm prototype).

Tailwater conditions were controlled by three separate, adjustable overshot weirs located 12 m (prototype) downstream of the B6 junction boxes.

Figures 1 to 4 present photographs of the physical model showing the key features.





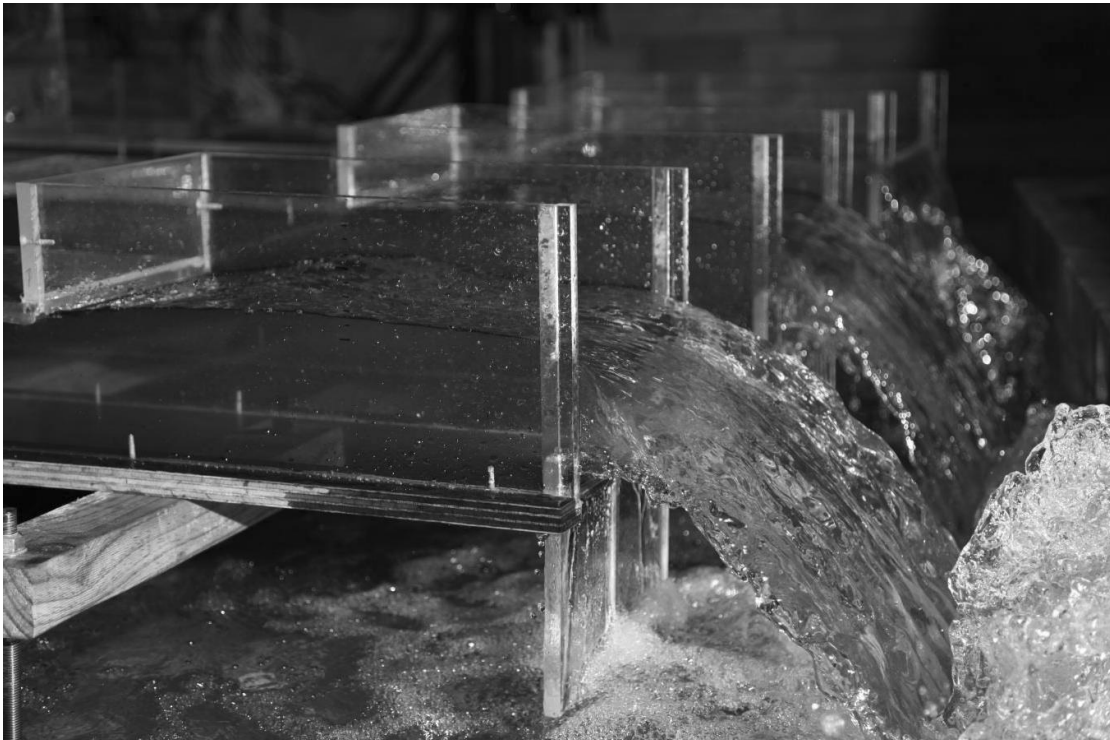
**Figure 1 - Box B4 and Inlets for Q1 and Q2**



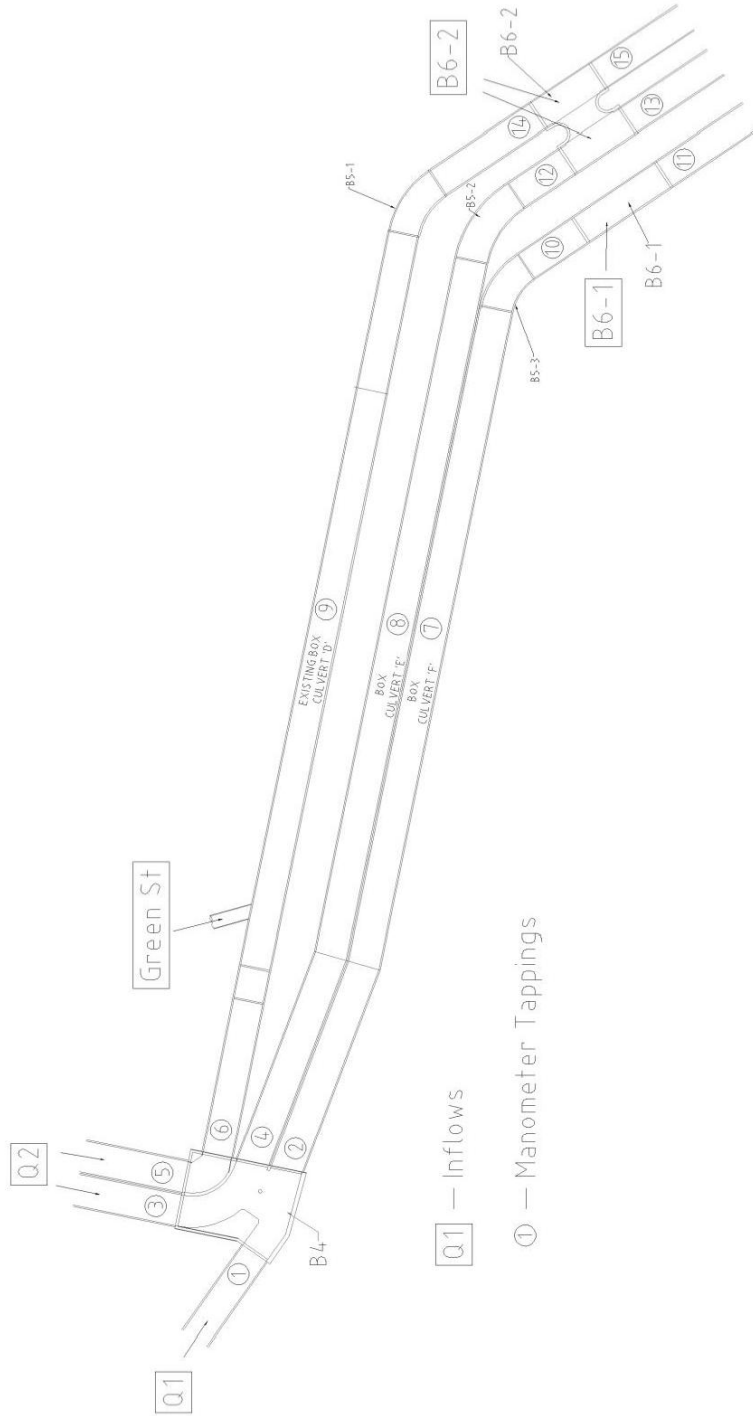
**Figure 2 - Green Street Inlet**



**Figure 3 - Boxes B6 and Grated Inlets**



**Figure 4 - Tailwater Weirs**



**Figure 5 - Model Plan View**

## 2.3 Model Testing

A testing regime of six steady state boundary conditions was undertaken for four different model configurations. An overview of the four design configurations is provided in Table 2.

**Table 2 – Summary of Model Configurations Tested**

Case	Description
Base	Base case for proposed design
1	Junction box B4 was modified by the addition of a turning vane
2	As per case 1 with the exception of subcritical flow at Q1
3	Major modifications were made to junction box B4

The six steady state tests were modelled on conditions at 15 minute intervals from the 1:100 Annual Exceedance Probability (AEP) hydrograph, as supplied by Cardno. Prototype and model inlet flow rates and tailwater levels are summarised in Table 3 and Table 4, respectively.

**Table 3 - Prototype Boundary Conditions**

Time	Minutes	T=15	T=30	T=45	T=60	T=75	T=90
<b>Inflows</b>							
Q1	m <sup>3</sup> /s	9.64	14.82	15.60	16.37	15.93	14.39
Q2	m <sup>3</sup> /s	1.67	7.51	9.52	12.66	15.42	15.45
Green St	m <sup>3</sup> /s	2.47	1.53	0.68	-0.65	-1.42	-1.04
B6-1	m <sup>3</sup> /s	0.68	3.54	0.50	0.97	1.13	1.58
B6-2	m <sup>3</sup> /s	0.39	1.32	4.05	2.08	1.49	1.79
<b>Tailwater</b>							
WL1	m R.L.	7.222	8.081	7.997	8.089	8.142	8.117
WL2	m R.L.	7.548	8.244	8.415	8.433	8.467	8.423
WL3	m R.L.	7.695	8.37	8.534	8.55	8.582	8.533

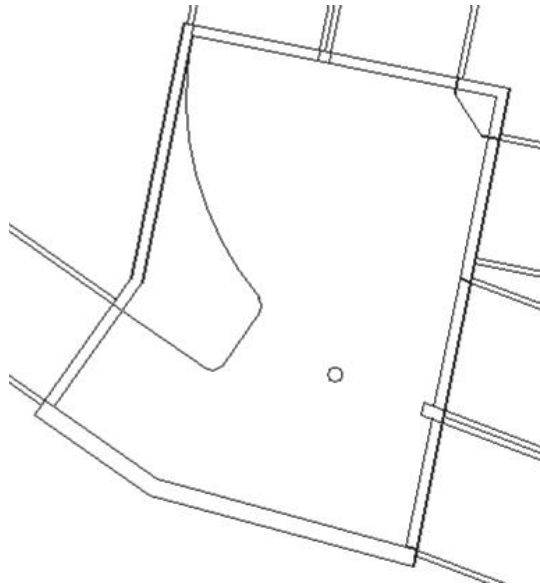
**Table 4 - Model Boundary Conditions**

Time	Minutes	T=15	T=30	T=45	T=60	T=75	T=90
<b>Inflows</b>							
Q1	L/s	11.1	17.0	17.9	18.8	18.3	16.5
Q2	L/s	1.9	8.6	10.9	14.5	17.7	17.7
Green St	L/s	2.8	1.8	0.8	0.0	0.0	0.0
B6-1	L/s	0.8	4.1	0.6	1.1	1.3	1.8
B6-2	L/s	0.4	1.5	4.6	2.4	1.7	2.1
<b>Tailwater</b>							
WL1	mm above invert	62	119	114	120	123	122
WL2	mm above invert	84	130	142	143	145	142
WL3	mm above invert	94	139	150	151	153	150

Inflows from Q1 had larger velocities than inflows from Q2. As such, this inlet could run either supercritical or subcritical depending on the applied combination of flow boundary conditions. During testing of the Base Case and Case 1, the inflow was supercritical. Following some further consideration of inlet flow conditions later in the testing program, a method was determined to force this culvert to flow full (hydraulic grade line at approximately 8.94 m RL at maximum flow), creating subcritical flow. This was achieved in the model by increasing the head at the inlet head box and temporarily raising the tailwater until the inlet switched to subcritical flow. Except at the low flow condition for this inlet, at T=15 min, the inlet remained at subcritical flow once the tailwater was returned to the correct level. Case 2 is a repetition of the model design and boundary conditions of Case 1, with the exception of subcritical flow at Q1, thus allowing direct comparison of these two flow conditions. After discussions with Cardno, Case 3, being the final design, was tested with subcritical flow at Q1.

### **2.3.1 Base case**

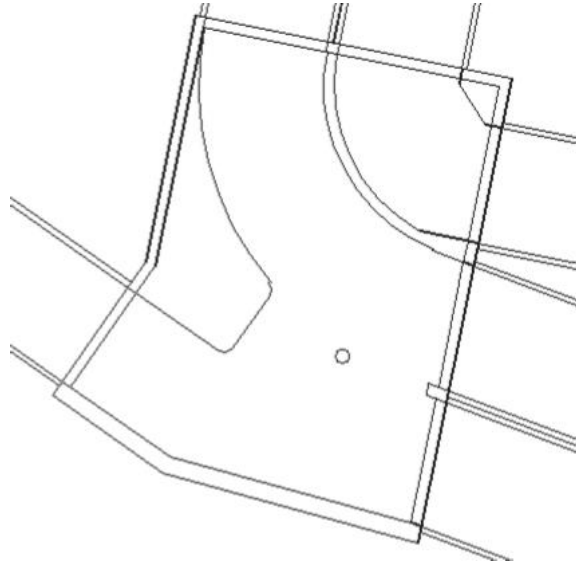
The Base Case model was constructed as per the initial designs provided by Cardno. Junction box B4 design can be seen in Figure 6.



**Figure 6 - Junction Box B4 - Base Case Design**

### **2.3.2 Case 1**

For Case 1 the design of junction box B4 was modified by the addition of a turning vane, separating flow from the proposed twin culverts. The modified design of B4 can be seen in Figure 7. No alterations were made to the B6 junction boxes.



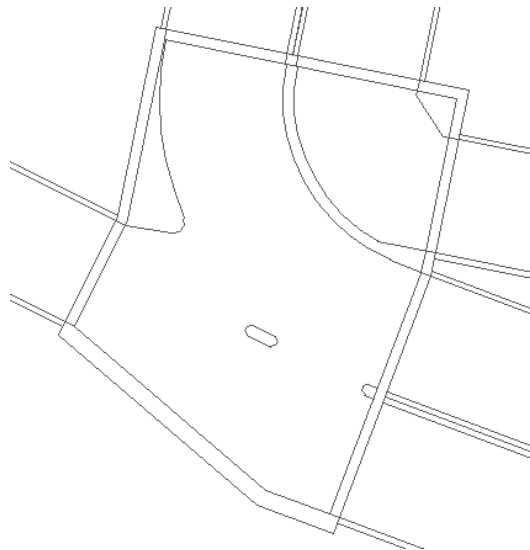
**Figure 7 - Junction Box B4 - Case 1 and 2 Design**

### **2.3.3 Case 2**

Case 2 model design was identical to Case 1, except the inflow from the existing single culvert was flowing subcritically (as opposed to supercritical flow from this inlet as in Case 1).

### **2.3.4 Case 3**

For Case 3, major modifications were made to junction box B4. These included realignment of the existing single culvert to more evenly partition the outflow from B4, moving the box walls to accommodate this realignment, and the addition of a blade column. The modified design of B4 can be seen in Figure 8. No alterations were made to the B6 junction boxes.



**Figure 8 - Junction Box B4 - Case 3 Design**

## 2.4 Measurements and Observations Recorded

Inflow rates were measured using a combination of electro-magnetic flow meters and rotameters. A 150 mm and a 100 mm electro-magnetic flow meter were used to measure the inflow to the existing single culvert and the proposed twin culverts, respectively. Inflows to the Green Street inlet and the grated inlets at B6 were measured using rotameters.

The tailwater weirs were adjusted to achieve the specified tailwater level. The height of the water level above the weirs was measured for the purpose of estimating the proportion of flow in each channel. Discharge proportions were estimated using the broad crested weir equation:

$$Q \approx C \cdot W \cdot H^{\frac{3}{2}}$$

Where:

C is a weir coefficient [ $\text{m}^{\frac{1}{2}}/\text{s}$ ]

W is the width of the weir [m]

H is the height of the water level above the weir [m].

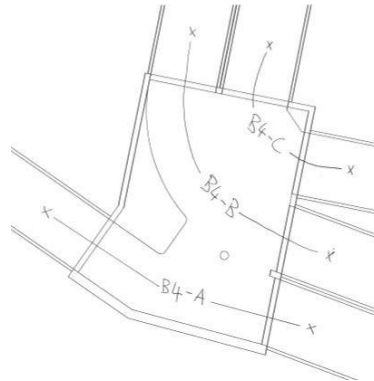
It was not deemed necessary to determine the weir coefficient in this case. Instead, the relation above was used to determine the proportion of flow over each weir, by comparing the height of the water levels above the weir.

Piezometric heads were measured with manometer tubes. Locations of manometer tappings can be seen in Figure 5.

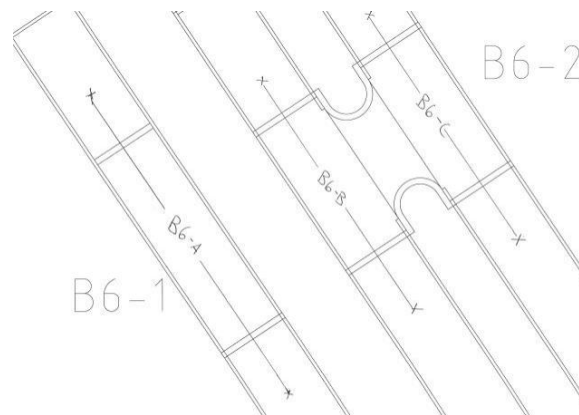
Flow observations were recorded with photos and high definition videos. Visual observations were aided by the use of dye.

### 3. Model Results

Hydraulic losses across the junction boxes were measured as piezometric head upstream and downstream of the boxes. Summaries of the losses across the junction boxes are reported here, see Figure 9 and Figure 10 for locations. Full model results are presented in Appendix A.



**Figure 9 - Location of Losses Reported at B4**



**Figure 10 - Location of Losses Reported at B6**

#### 3.1 Base Case

Table 5 provides a summary of losses recorded in the Base Case.

**Table 5 - Hydraulic Losses - Base Case**

<b>Location</b>	<b>T=15</b>	<b>T=30</b>	<b>T=45</b>	<b>T=60</b>	<b>T=75</b>	<b>T=90</b>
	m	m	m	m	m	m
B4-A	-0.02	-1.13	-0.41	-0.48	-0.50	-0.47
B4-B	0.26	0.33	0.20	0.30	0.36	0.30
B4-C	0.02	0.05	0.09	0.09	0.15	0.15
B6-A	0.27	0.95	0.33	0.63	0.65	0.80
B6-B	0.09	0.12	0.18	0.17	0.18	0.18
B6-C	-0.03	-0.06	-0.02	-0.02	-0.02	0.02

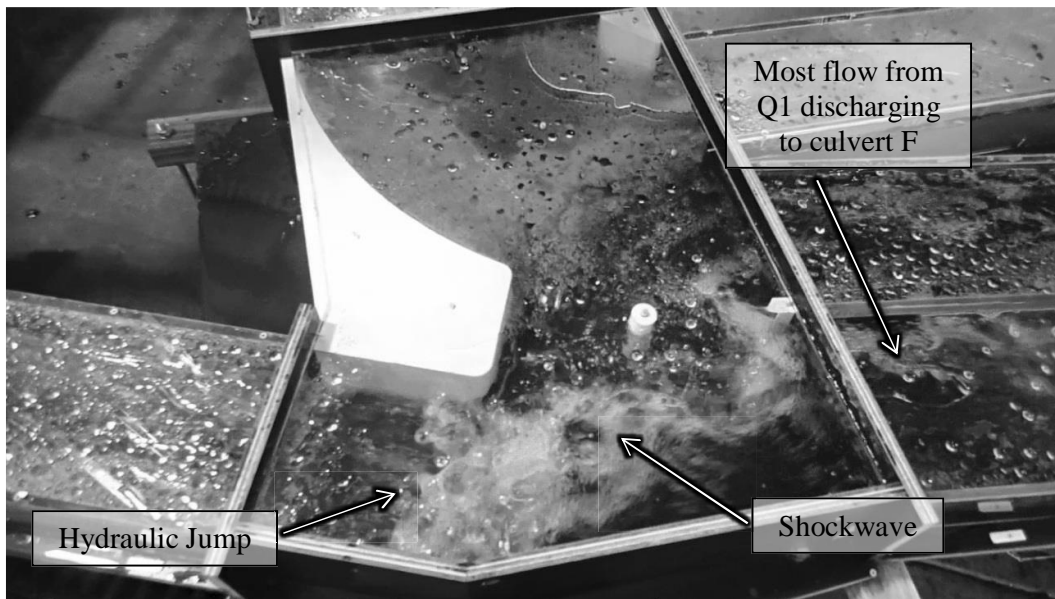


Figure 13 shows the major features of the flow conditions for the Base Case. These features were generally consistent over the higher flow conditions (T=45 and later). Figure 11 shows a hydraulic jump and shockwave in B4. The addition of blue dye from Q1 illustrates that almost all of the flow from Q1 discharged from B4 into culvert F.

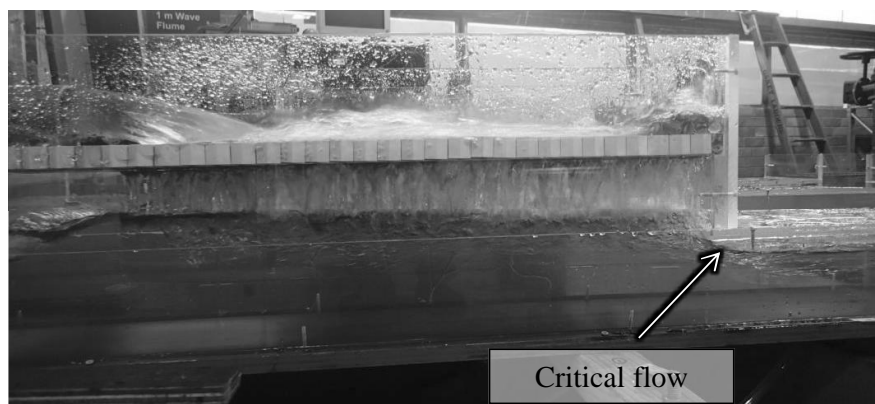
Negative losses observed at B4-A can be attributed to the hydraulic jump and the addition of extra flows coming from Q2. All culverts ran full except for:

- Culverts D and E for 15 – 30 m downstream of B4; and
- Culvert F running supercritical with hydraulic jump 0 – 30 m upstream of bend B5-3.

Figure 12 shows B6-1 behaving as an undershot weir under maximum flow conditions at this inlet at T=30. Figure 14 shows the generalised flow conditions around B6 junction boxes.



**Figure 11 – Base Case Flow Conditions in B4**



**Figure 12 - B6-1 Behaving as an Undershot Weir**

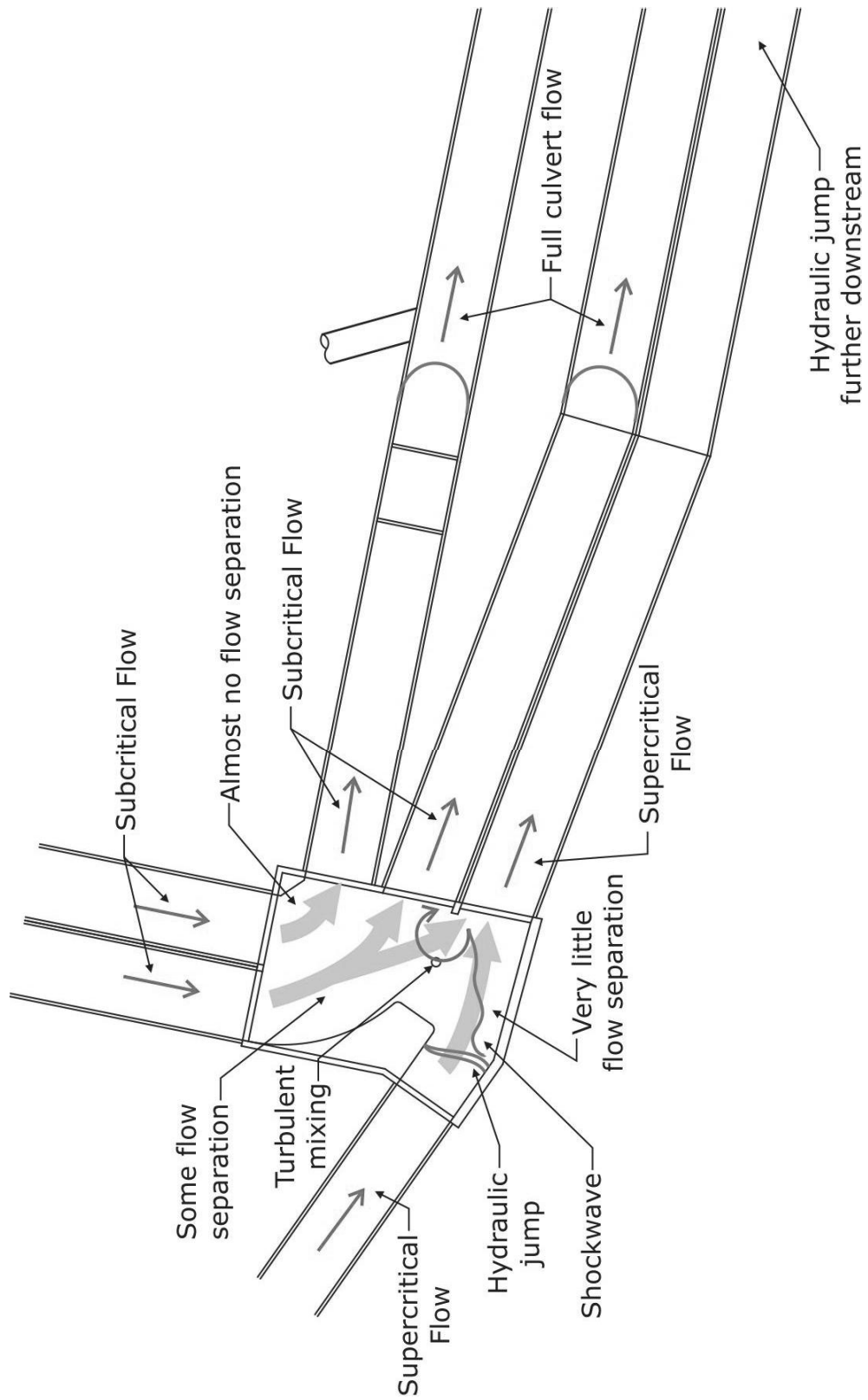


Figure 13 – Base Case Flow Conditions Sketch

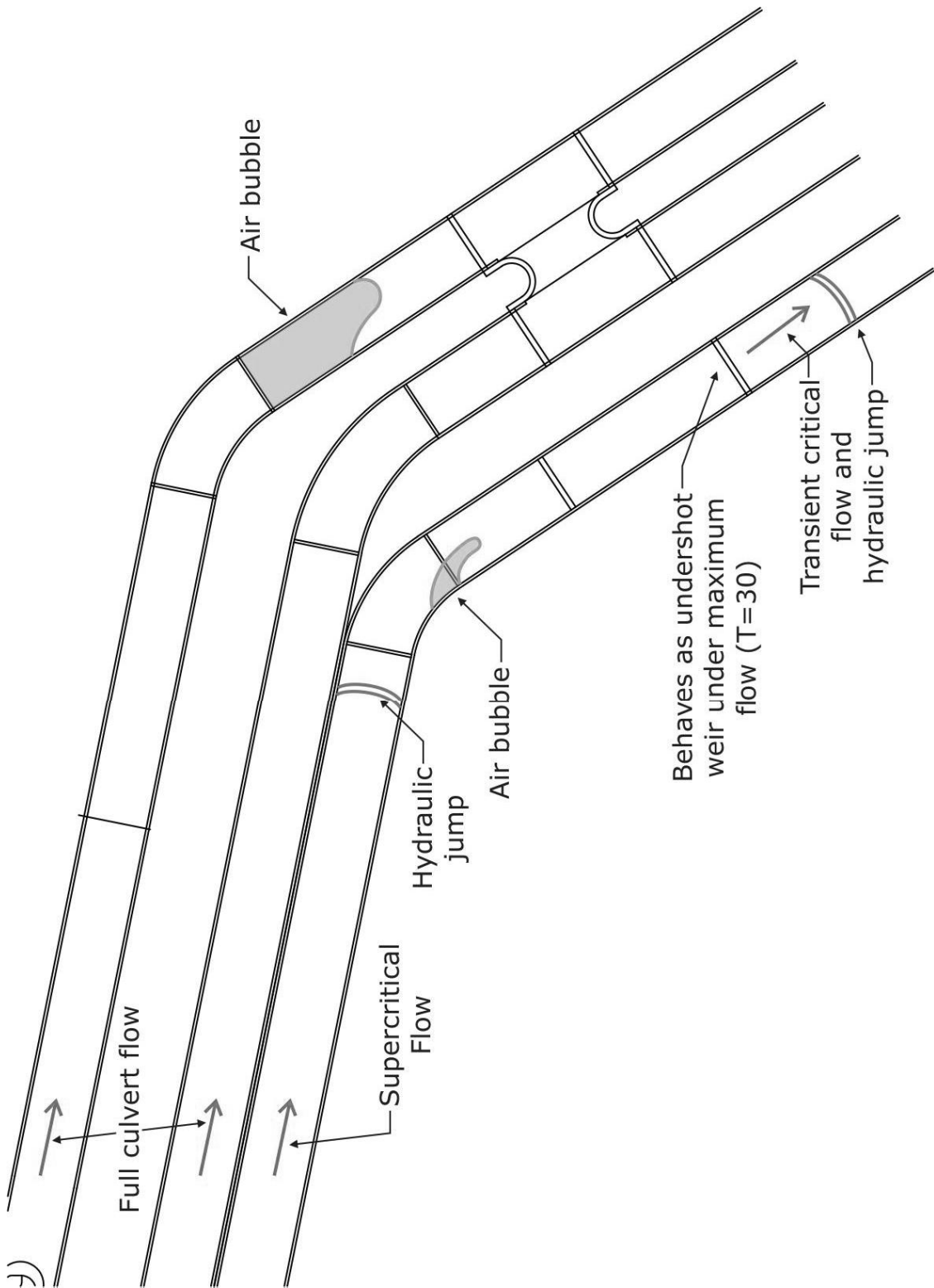


Figure 14 - Base Case Flow Conditions at B6

### 3.2 Case 1

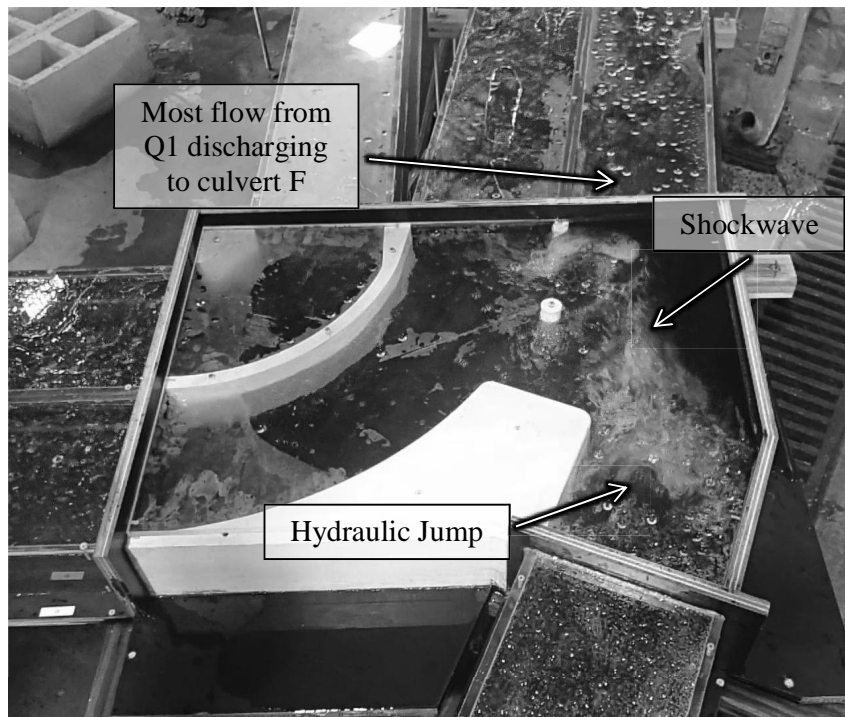
Flow observations for Case 1 were similar to those observed in the Base Case, since most of the flow separated by the addition of the turning vane was already following that flow path in the base case.

Table 6 provides a summary of losses recorded in Case 1. Figure 16 shows the major features of the flow conditions for Case 1. These features were generally consistent over the higher flow conditions (T=45 and later). Figure 15 shows the hydraulic jump and shockwave in B4 and the lack of flow separation from Q1.

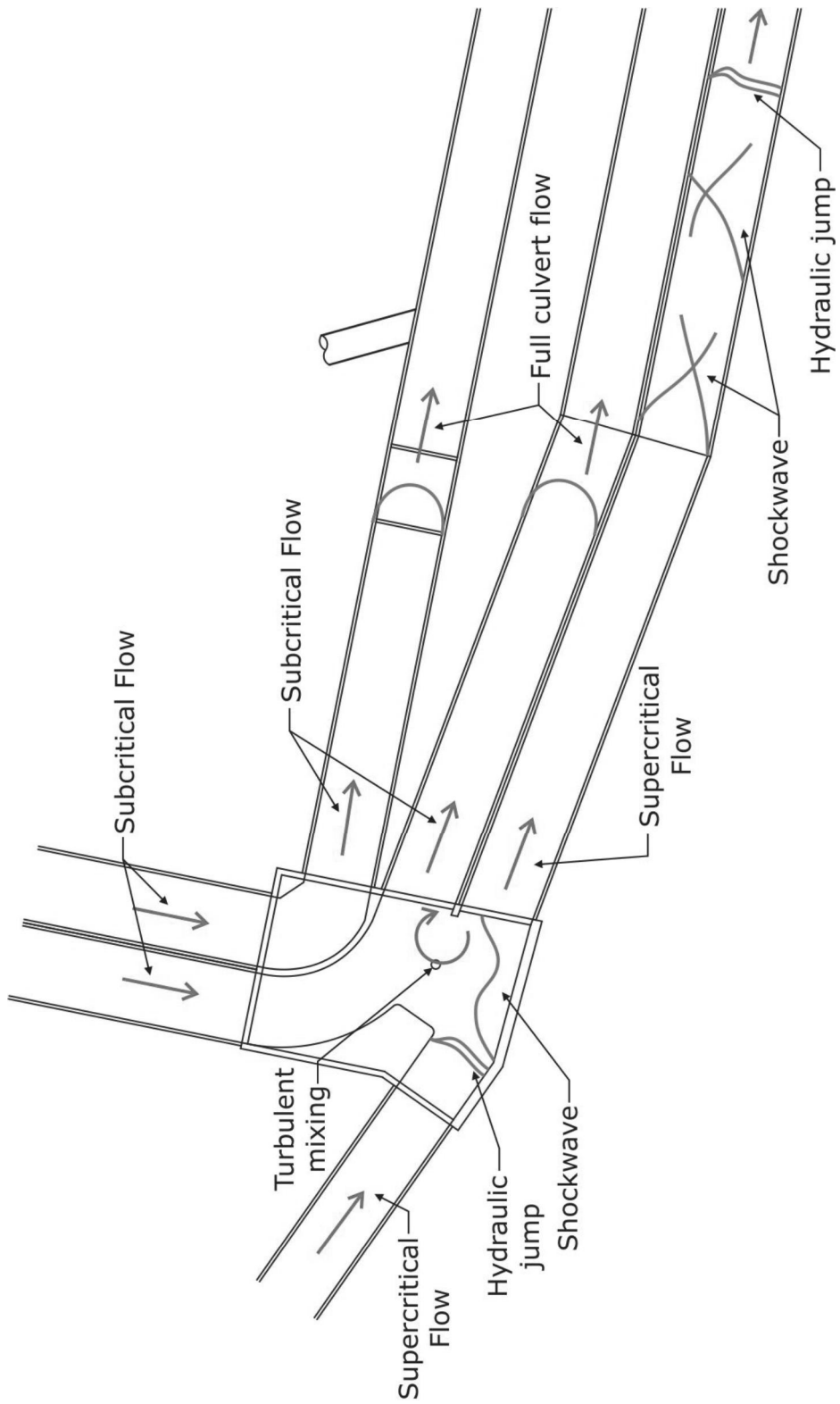
Again, negative losses observed at B4-A can be attributed to the hydraulic jump and the addition of extra flows coming from Q2.

**Table 6 - Hydraulic Losses - Case 1**

Location	T=15	T=30	T=45	T=60	T=75	T=90
	m	m	m	m	m	m
B4-A	0.02	-0.90	-0.35	-0.33	-0.14	-0.84
B4-B	0.27	0.20	0.18	0.21	0.12	0.11
B4-C	0.00	0.06	0.08	0.12	0.14	0.12
B6-A	0.30	0.98	0.20	0.30	0.33	0.35
B6-B	0.06	0.02	0.15	0.18	0.18	0.18
B6-C	-0.03	0.03	0.03	0.00	-0.03	-0.03



**Figure 15 – Case 1 Flow Conditions in B4**



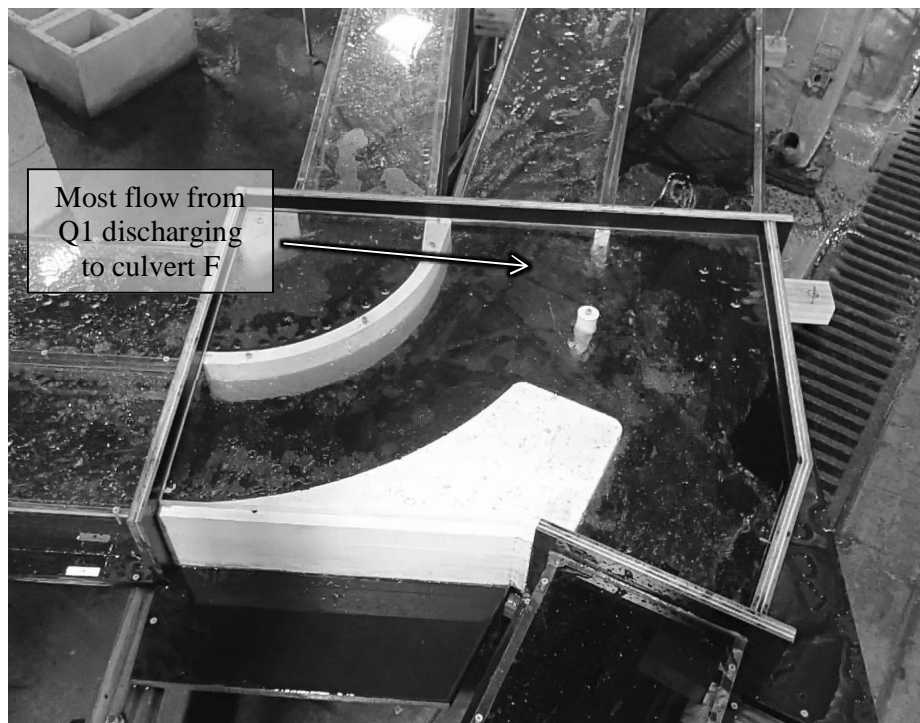
**Figure 16 – Case 1 Flow Conditions Sketch**

### 3.3 Case 2

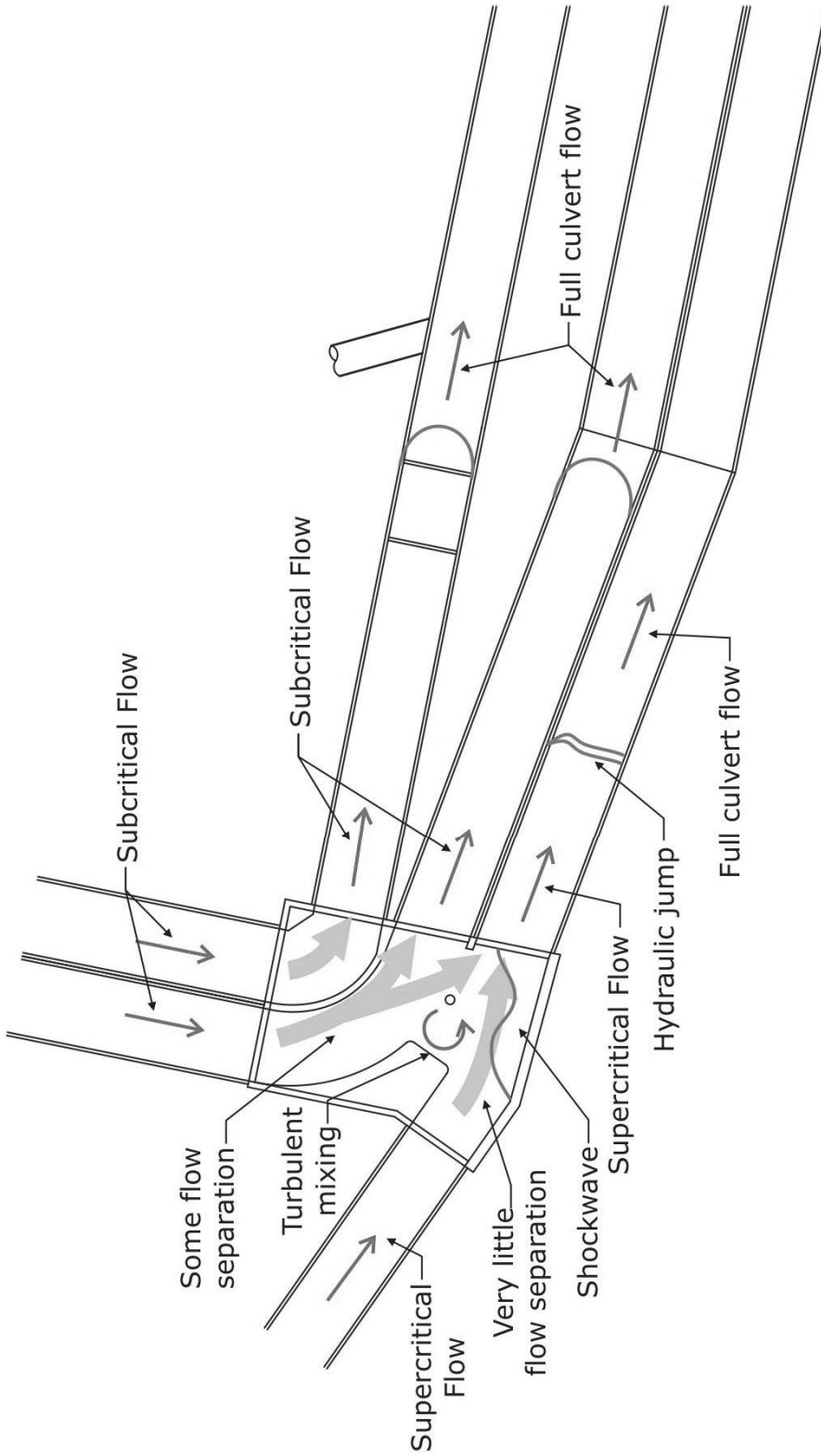
Table 7 gives a summary of hydraulic losses for Case 2. Figure 17 shows the subcritical flow from Q1, smaller shockwave in B4, but still very little flow separation from Q1. Generalised flow conditions are shown in Figure 18. Flow conditions at B6 were similar to those observed in the Base Case.

**Table 7 - Hydraulic Losses - Case 2**

Location	T=15	T=30	T=45	T=60	T=75	T=90
	m	m	m	m	m	m
B4-A		0.17	0.62	0.27	0.38	0.59
B4-B		0.15	0.02	0.03	0.05	0.02
B4-C		0.06	0.05	0.09	0.18	0.17
B6-A		0.62	0.23	0.38	0.42	0.33
B6-B		0.12	0.17	0.17	0.15	0.15
B6-C		-0.03	-0.02	-0.03	-0.05	-0.03



**Figure 17 – Case 2 Flow Conditions in B4**



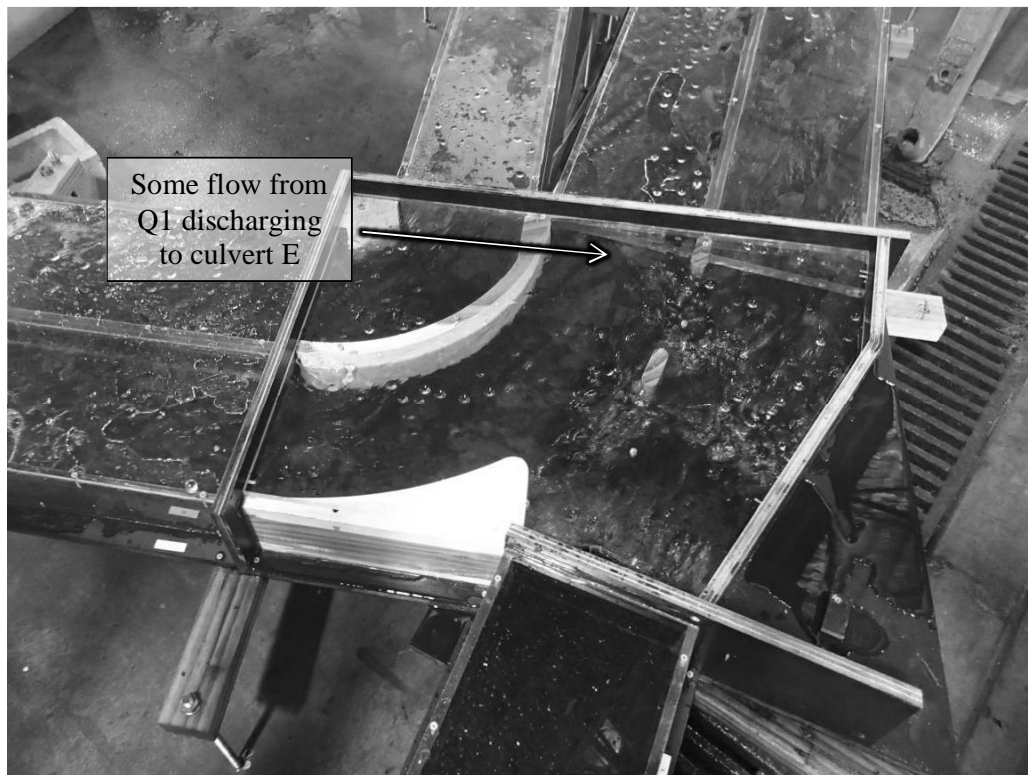
**Figure 18 – Case 2 Flow Conditions Sketch**

### 3.4 Case 3

Table 8 gives a summary of hydraulic losses for Case 3. Figure 19 shows that the realignment of the culvert carrying Q1 has successfully produced better separation of flow into culvert E and F. Generalised flow conditions are shown in Figure 20. Flow conditions at B6 were similar to those observed in the Base Case.

**Table 8 - Hydraulic Losses - Case 3**

<b>Location</b>	<b>T=15</b>	<b>T=30</b>	<b>T=45</b>	<b>T=60</b>	<b>T=75</b>	<b>T=90</b>
	m	m	m	m	m	m
B4-A	0.23	0.27	0.60	0.62	0.59	0.63
B4-B	0.2	0.03	0.00	0.06	0.12	0.06
B4-C	0.03	0.06	0.05	0.08	0.30	0.26
B6-A	0.18	0.75	0.15	0.26	0.24	0.29
B6-B	0.06	0.14	0.20	0.21	0.18	0.17
B6-C	-0.02	-0.02	0.00	-0.03	-0.02	-0.03



**Figure 19 – Case 3 Flow Conditions in B4**



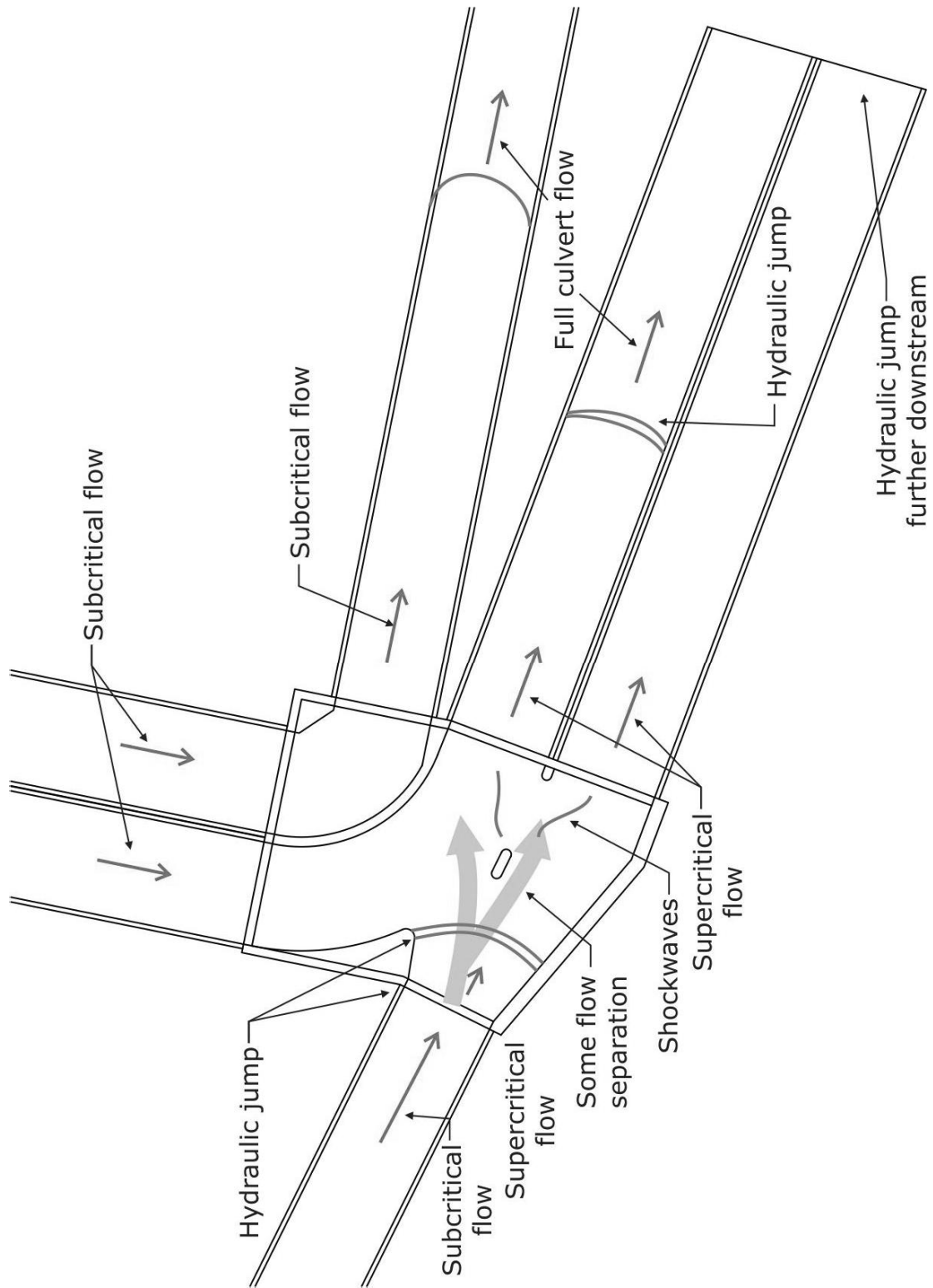


Figure 20 – Case 3 Flow Conditions Sketch

## **4. Conclusions**

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A physical model was constructed at a scale of 15:1 to undertake investigations of proposed stormwater culverts and junction boxes at Warringah Mall, NSW. The purpose of the testing was to determine hydraulic losses across the junction boxes.

Five steady state tests were carried out with flow rates and tailwater levels derived from 15 minute intervals in the 1:100 AEP hydrograph. One boundary condition modification and two design modifications to junction box B4 were made, and the steady state tests were repeated for each modification.

Physical modelling demonstrated that the flow conditions through culvert B4 are complex. The physical modelling has allowed for these flow conditions to be improved through changes to the culvert design. Physical modelling is a well-established engineering method for assessing complex hydraulics in an efficient manner. During model demonstrations with the client, options could be quickly trialled.

The original design of B4 did not allow inflows from Q1 to be distributed into the culverts D and E. Cardno advised that achieving this distribution was important to ensure that discharge out of B4 was similar for all three culverts, as assumed by the numerical model. The addition of the turning vane (Case 1) did not significantly change the flow conditions as most of the flow separated by this vane was already flowing in the desired manner. The realignment of the culvert carrying Q1 into B4 (Case 3) successfully achieved a better distribution of flow from B4.

## 5. Appendix A

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### 5.1 Base Case

#### 5.1.1 Base Case T = 15

<b>BOUNDARY CONDITIONS</b>				
<b>Inflows</b>	<b>Model</b>	<b>Prototype</b>		
	L/s	m <sup>3</sup> /s		
Existing single	11.1	9.64		
Proposed twin	1.9	1.67		
Green St	2.8	2.47		
B6-1	0.8	0.68		
B6-2	0.4	0.39		
<b>Total</b>	17.0	14.86		
<b>Tailwater</b>	<b>Model</b>	<b>Prototype</b>	<b>Height above weir</b>	<b>Estimated proportion of Q</b>
	mm above invert	R.L.	m	
WL1	62	7.220	0.062	0.56
WL2	83	7.535	0.053	0.44
WL3	83	7.535	0	0.00
<b>HEADS</b>				
	<b>Reading</b>	<b>R.L.</b>		
	mm	m		
1	178	7.755		
2	179	7.770		
3	172	7.665		
4	155	7.410		
5	171	7.650		
6	170	7.635		
7	151	7.350		
8	156	7.425		
9	148	7.305		
10	172	7.665		
11	154	7.395		
12	156	7.425		
13	150	7.335		
14	157	7.440		
15	159	7.470		

**5.1.2 Base Case T = 30**

<b>BOUNDARY CONDITIONS</b>				
<b>Inflows</b>	<b>Model</b>	<b>Prototype</b>		
	L/s	m <sup>3</sup> /s		
Existing single	17.0	14.82		
Proposed twin	8.6	7.51		
Green St	1.8	1.53		
B6-1	4.1	3.54		
B6-2	1.5	1.32		
<b>Total</b>	<b>33.0</b>	<b>28.72</b>		
<b>Tailwater</b>	<b>Model</b>	<b>Prototype</b>	<b>Height above weir</b>	<b>Estimated proportion of Q</b>
	mm above invert	R.L.	m	
WL1	119	8.075	0.094	0.57
WL2	130	8.240	0.070	0.37
WL3	139	8.375	0.021	0.06
<b>HEADS</b>				
	<b>Reading</b>	<b>R.L.</b>		
	mm	m		
1	185	7.860		
2	260 +/- 7	8.985		
3	221	8.400		
4	199 +/- 6	8.070		
5	217	8.340		
6	214	8.295		
7	213 +/- 5	8.280		
8	212 +/- 2	8.265		
9	214 +/- 1	8.295		
10	217 +/- 1	8.340		
11	154 +/- 1	7.395		
12	211	8.250		
13	203	8.130		
14	212	8.265		
15	216	8.325		

\* Values reported with +/- value are due to oscillations about the reported mean.

### 5.1.3 Base Case T = 45

<b>BOUNDARY CONDITIONS</b>				
<b>Inflows</b>	<b>Model</b>	<b>Prototype</b>		
	L/s	m <sup>3</sup> /s		
Existing single	17.9	15.60		
Proposed twin	10.9	9.52		
Green St	0.8	0.68		
B6-1	0.6	0.50		
B6-2	4.6	4.05		
<b>Total</b>	<b>34.8</b>	<b>30.35</b>		
<b>Tailwater</b>	<b>Model</b>	<b>Prototype</b>	<b>Height above weir</b>	<b>Estimated proportion of Q</b>
	mm above invert	R.L.	m	
WL1	114.0	8.00	0.089	0.47
WL2	142.0	8.42	0.082	0.42
WL3	150.0	8.54	0.035	0.12
<b>HEADS</b>				
	<b>Reading</b>	<b>R.L</b>		
	mm	m		
1	190 +/- 1	7.935		
2	217 +/- 4	8.340		
3	230 +/- 1	8.535		
4	217 +/- 3	8.340		
5	226 +/- 1	8.475		
6	220	8.385		
7	182 +/- 5	7.815		
8	223 +/- 1	8.430		
9	224 +/- 1	8.445		
10	192 +/- 3	7.965		
11	170 +/- 10	7.635		
12	222	8.415		
13	210	8.235		
14	223	8.430		
15	224	8.445		

**5.1.4 Base Case T = 60**

<b>BOUNDARY CONDITIONS</b>				
<b>Inflows</b>	<b>Model</b>	<b>Prototype</b>		
	L/s	m <sup>3</sup> /s		
Existing single	18.8	16.37		
Proposed twin	14.5	12.66		
Green St	0.0	0.00		
B6-1	1.1	0.97		
B6-2	2.4	2.08		
<b>Total</b>	<b>36.8</b>	<b>32.07</b>		
<b>Tailwater</b>	<b>Model</b>	<b>Prototype</b>	<b>Height above weir</b>	<b>Estimated proportion of Q</b>
	mm above invert	R.L.	m	
WL1	120.0	8.090	0.095	0.49
WL2	143.0	8.435	0.083	0.40
WL3	151.0	8.555	0.036	0.11
<b>HEADS</b>				
	<b>Reading</b>	<b>R.L</b>		
	mm	m		
1	193 +/- 3	7.980		
2	225 +/- 6	8.460		
3	236 +/- 1	8.625		
4	216 +/- 4	8.325		
5	228 +/- 2	8.505		
6	222 +/- 2	8.415		
7	191 +/- 6	7.950		
8	225 +/- 3	8.460		
9	226 +/- 2	8.475		
10	200 +/- 2	8.085		
11	158 +/- 6	7.455		
12	224	8.445		
13	213	8.280		
14	225	8.460		
15	226	8.475		

**5.1.5 Base Case T = 75**

<b>BOUNDARY CONDITIONS</b>				
<b>Inflows</b>	<b>Model</b>	<b>Prototype</b>		
	L/s	m <sup>3</sup> /s		
Existing single	18.3	15.93		
Proposed twin	17.7	15.42		
Green St	0.0	0.00		
B6-1	1.3	1.13		
B6-2	1.7	1.49		
<b>Total</b>	<b>39.0</b>	<b>33.97</b>		
<b>Tailwater</b>	<b>Model</b>	<b>Prototype</b>	<b>Height above weir</b>	<b>Estimated proportion of Q</b>
	mm above invert	R.L.	m	
WL1	124.0	8.150	0.099	0.46
WL2	145.0	8.465	0.087	0.38
WL3	153.0	8.585	0.048	0.16
<b>HEADS</b>				
	<b>Reading</b>	<b>R.L</b>		
	mm	m		
1	197 +/- 3	8.040		
2	230 +/- 8	8.535		
3	244 +/- 2	8.745		
4	220 +/- 7	8.385		
5	233 +/- 2	8.580		
6	223 +/- 3	8.430		
7	194 +/- 3	7.995		
8	230 +/- 1	8.535		
9	230 +/- 1	8.535		
10	202 +/- 2	8.115		
11	159 +/- 4	7.470		
12	226	8.475		
13	214	8.295		
14	228	8.505		
15	229	8.520		

**5.1.6 Base Case T = 90**

<b>BOUNDARY CONDITIONS</b>				
<b>Inflows</b>	<b>Model</b>	<b>Prototype</b>		
	L/s	m <sup>3</sup> /s		
Existing single	16.5	14.39		
Proposed twin	17.7	15.45		
Green St	0.0	0.00		
B6-1	1.8	1.58		
B6-2	2.1	1.79		
<b>Total</b>	<b>38.1</b>	<b>33.20</b>		
<b>Tailwater</b>	<b>Model</b>	<b>Prototype</b>	<b>Height above weir</b>	<b>Estimated proportion of Q</b>
	mm above invert	R.L.	m	
WL1	122	8.12	0.097	0.46
WL2	142	8.42	0.084	0.37
WL3	150	8.54	0.050	0.17
<b>HEADS</b>				
	<b>Reading</b>	<b>R.L</b>		
	mm	m		
1	192 +/- 3	7.965		
2	223 +/- 7	8.430		
3	240 +/- 2	8.685		
4	220 +/- 5	8.385		
5	230 +/- 1	8.535		
6	220 +/- 2	8.385		
7	187 +/- 4	7.890		
8	226 +/- 4	8.475		
9	228 +/- 2	8.505		
10	205 +/- 3	8.160		
11	152 +/- 3	7.365		
12	224	8.445		
13	212	8.265		
14	226	8.475		
15	225	8.460		



## 5.2 Case 1

### 5.2.1 Case 1 T = 15

<b>BOUNDARY CONDITIONS</b>				
<b>Inflows</b>	<b>Model</b>	<b>Prototype</b>		
	L/s	m <sup>3</sup> /s		
Existing single	11.1	9.64		
Proposed twin	1.9	1.67		
Green St	2.8	2.47		
B6-1	0.8	0.68		
B6-2	0.4	0.39		
<b>Total</b>	17.0	14.86		
<b>Tailwater</b>	<b>Model</b>	<b>Prototype</b>	<b>Height above weir</b>	<b>Estimated proportion of Q</b>
	mm above invert	R.L.	m	
WL1	62	7.220	0.047	0.49
WL2	83	7.535	0.048	0.51
WL3	85	7.565	0.000	0.00
<b>HEADS</b>				
	<b>Reading</b>	<b>R.L</b>		
	mm	m		
1	181	7.80		
2	180	7.785		
3	172	7.665		
4	154	7.395		
5	173	7.680		
6	173	7.680		
7	150	7.335		
8	160	7.485		
9	156	7.425		
10	166 +/- 1	7.575		
11	146	7.275		
12	160	7.485		
13	156	7.425		
14	162	7.515		
15	164	7.545		

**5.2.2 Case 1 T = 30**

<b>BOUNDARY CONDITIONS</b>				
<b>Inflows</b>	Model	Prototype		
	L/s	m <sup>3</sup> /s		
Existing single	17.0	14.82		
Proposed twin	8.6	7.51		
Green St	1.8	1.53		
B6-1	4.1	3.54		
B6-2	1.5	1.32		
<b>Total</b>	33.0	28.72		
<b>Tailwater</b>	<b>Model</b>	<b>Prototype</b>	<b>Height above weir</b>	<b>Estimated proportion of Q</b>
	mm above invert	R.L.	m	
WL1	119	8.075	0.089	0.50
WL2	130	8.240	0.052	0.22
WL3	139	8.375	0.061	0.28
<b>HEADS</b>				
	<b>Reading</b>	<b>R.L.</b>		
	mm	m		
1	184	7.845		
2	244 +/- 7	8.745		
3	212 +/- 1	8.265		
4	199 +/- 4	8.070		
5	211	8.250		
6	207	8.190		
7	205 +/- 4	8.160		
8	207	8.190		
9	207	8.190		
10	223	8.430		
11	158	7.455		
12	205	8.160		
13	204	8.145		
14	207	8.190		
15	205	8.160		

### 5.2.3 Case 1 T = 45

<b>BOUNDARY CONDITIONS</b>				
<b>Inflows</b>	<b>Model</b>	<b>Prototype</b>		
	L/s	m <sup>3</sup> /s		
Existing single	17.9	15.60		
Proposed twin	10.9	9.52		
Green St	0.8	0.68		
B6-1	0.6	0.50		
B6-2	4.6	4.05		
<b>Total</b>	<b>34.8</b>	<b>30.35</b>		
<b>Tailwater</b>	<b>Model</b>	<b>Prototype</b>	<b>Height above weir</b>	<b>Estimated proportion of Q</b>
	mm above invert	R.L.	m	
WL1	114.0	8.00	0.089	0.45
WL2	142.0	8.42	0.077	0.36
WL3	150.0	8.54	0.050	0.19
<b>HEADS</b>				
	<b>Reading</b>	<b>R.L</b>		
	mm	m		
1	187 +/- 3	7.890		
2	210 +/- 5	8.235		
3	224	8.445		
4	212 +/- 2	8.265		
5	225	8.460		
6	220	8.385		
7	177 +/- 5	7.740		
8	220 +/- 1	8.385		
9	220	8.385		
10	194 +/- 1	7.995		
11	181	7.800		
12	217	8.340		
13	207	8.190		
14	220	8.385		
15	218	8.355		

**5.2.4 Case 1 T = 60**

<b>BOUNDARY CONDITIONS</b>				
<b>Inflows</b>	<b>Model</b>	<b>Prototype</b>		
	L/s	m <sup>3</sup> /s		
Existing single	18.8	16.37		
Proposed twin	14.5	12.66		
Green St	0.0	0.00		
B6-1	1.1	0.97		
B6-2	2.4	2.08		
<b>Total</b>	<b>36.8</b>	<b>32.07</b>		
<b>Tailwater</b>	<b>Model</b>	<b>Prototype</b>	<b>Height above weir</b>	<b>Estimated proportion of Q</b>
	mm above invert	R.L.	m	
WL1	120.0	8.090	0.095	0.46
WL2	143.0	8.435	0.083	0.38
WL3	151.0	8.555	0.046	0.16
<b>HEADS</b>				
	<b>Reading</b>	<b>R.L</b>		
	mm	m		
1	198 +/- 4	8.055		
2	220 +/- 4	8.385		
3	235 +/- 1	8.610		
4	221 +/- 3	8.400		
5	233	8.580		
6	225	8.460		
7	187 +/- 4	7.890		
8	227 +/- 1	8.490		
9	228 +/- 1	8.505		
10	205 +/- 1	8.160		
11	185	7.860		
12	225	8.460		
13	213	8.280		
14	226	8.475		
15	226	8.475		

### 5.2.5 Case 1 T = 75

<b>BOUNDARY CONDITIONS</b>				
<b>Inflows</b>	<b>Model</b>	<b>Prototype</b>		
	L/s	m <sup>3</sup> /s		
Existing single	18.3	15.93		
Proposed twin	17.7	15.42		
Green St	0.0	0.00		
B6-1	1.3	1.13		
B6-2	1.7	1.49		
<b>Total</b>	<b>39.0</b>	<b>33.97</b>		
<b>Tailwater</b>	<b>Model</b>	<b>Prototype</b>	<b>Height above weir</b>	<b>Estimated proportion of Q</b>
	mm above invert	R.L.	m	
WL1	124.0	8.150	0.094	0.45
WL2	145.0	8.465	0.085	0.39
WL3	153.0	8.585	0.048	0.16
<b>HEADS</b>				
	<b>Reading</b>	<b>R.L</b>		
	mm	m		
1	197 +/- 3	8.040		
2	206 +/- 2	8.175		
3	235 +/- 2	8.610		
4	227 +/- 2	8.490		
5	236 +/- 1	8.625		
6	227 +/- 1	8.490		
7	232 +/- 2	8.565		
8	229 +/- 1	8.520		
9	229 +/- 1	8.520		
10	211 +/-1	8.250		
11	189	7.920		
12	225	8.460		
13	213	8.280		
14	228	8.505		
15	230	8.535		

### 5.2.6 Case 1 T = 90

<b>BOUNDARY CONDITIONS</b>				
<b>Inflows</b>	<b>Model</b>	<b>Prototype</b>		
	L/s	m <sup>3</sup> /s		
Existing single	16.5	14.39		
Proposed twin	17.7	15.45		
Green St	0.0	0.00		
B6-1	1.8	1.58		
B6-2	2.1	1.79		
<b>Total</b>	<b>38.1</b>	<b>33.20</b>		
<b>Tailwater</b>	<b>Model</b>	<b>Prototype</b>	<b>Height above weir</b>	<b>Estimated proportion of Q</b>
	mm above invert	R.L.	m	
WL1	122	8.12	0.092	0.46
WL2	142	8.42	0.082	0.39
WL3	150	8.54	0.045	0.16
<b>HEADS</b>				
	<b>Reading</b>	<b>R.L</b>		
	mm	m		
1	194 +/- 1	7.995		
2	250 +/- 2	8.835		
3	236 +/- 1	8.625		
4	229 +/- 1	8.520		
5	236	8.625		
6	228	8.505		
7	207 +/- 2	8.190		
8	230 +/- 1	8.535		
9	231 +/- 1	8.550		
10	211 +/- 1	8.250		
11	188	7.905		
12	226	8.475		
13	214	8.295		
14	227	8.490		
15	229	8.520		

### 5.3 Case 2

#### 5.3.1 Case 2 T = 30

<b>BOUNDARY CONDITIONS</b>				
<b>Inflows</b>	<b>Model</b>	<b>Prototype</b>		
	L/s	m <sup>3</sup> /s		
Existing single	17.0	14.82		
Proposed twin	8.6	7.51		
Green St	1.8	1.53		
B6-1	4.1	3.54		
B6-2	1.5	1.32		
<b>Total</b>	33.0	28.72		
<b>Tailwater</b>	<b>Model</b>	<b>Prototype</b>	<b>Height above weir</b>	<b>Estimated proportion of Q</b>
	mm above invert	R.L.	m	
WL1	119	8.075	0.099	0.60
WL2	130	8.240	0.067	0.33
WL3	139	8.375	0.024	0.07
<b>HEADS</b>				
	<b>Reading</b>	<b>R.L.</b>		
	mm	m		
1	253	8.880		
2	242	8.715		
3	216	8.325		
4	206	8.175		
5	220	8.385		
6	216	8.325		
7	234	8.595		
8	215	8.310		
9	216	8.325		
10	219	8.370		
11	178	7.755		
12	213	8.280		
13	205	8.160		
14	213	8.280		
15	215	8.310		

### 5.3.2 Case 2 T = 45

<b>BOUNDARY CONDITIONS</b>				
<b>Inflows</b>	<b>Model</b>	<b>Prototype</b>		
	L/s	m <sup>3</sup> /s		
Existing single	17.9	15.60		
Proposed twin	10.9	9.52		
Green St	0.8	0.68		
B6-1	0.6	0.50		
B6-2	4.6	4.05		
<b>Total</b>	<b>34.8</b>	<b>30.35</b>		
<b>Tailwater</b>	<b>Model</b>	<b>Prototype</b>	<b>Height above weir</b>	<b>Estimated proportion of Q</b>
	mm above invert	R.L.	m	
WL1	114.0	8.00	0.094	0.51
WL2	142.0	8.42	0.079	0.39
WL3	150.0	8.54	0.032	0.10
<b>HEADS</b>				
	<b>Reading</b>	<b>R.L</b>		
	mm	m		
1	255	8.910		
2	214	8.295		
3	226	8.475		
4	225	8.460		
5	232	8.565		
6	229	8.520		
7	219	8.370		
8	226	8.475		
9	228	8.505		
10	197	8.040		
11	182	7.815		
12	225	8.460		
13	214	8.295		
14	226	8.475		
15	227	8.490		



### 5.3.3 Case 2 T = 60

<b>BOUNDARY CONDITIONS</b>				
<b>Inflows</b>	<b>Model</b>	<b>Prototype</b>		
	L/s	m <sup>3</sup> /s		
Existing single	18.8	16.37		
Proposed twin	14.5	12.66		
Green St	0.0	0.00		
B6-1	1.1	0.97		
B6-2	2.4	2.08		
<b>Total</b>	36.8	32.07		
<b>Tailwater</b>	<b>Model</b>	<b>Prototype</b>	<b>Height above weir</b>	<b>Estimated proportion of Q</b>
	mm above invert	R.L.	m	
WL1	120.0	8.090	0.100	0.52
WL2	143.0	8.435	0.080	0.38
WL3	151.0	8.555	0.033	0.10
<b>HEADS</b>				
	<b>Reading</b>	<b>R.L</b>		
	mm	m		
1	257	8.940		
2	239	8.670		
3	230	8.535		
4	228	8.505		
5	234	8.595		
6	228	8.505		
7	228	8.505		
8	227	8.490		
9	229	8.520		
10	209 +/- 1	8.220		
11	184	7.845		
12	226	8.475		
13	215	8.310		
14	226	8.475		
15	228	8.505		

**5.3.4 Case 2 T = 75**

<b>BOUNDARY CONDITIONS</b>				
<b>Inflows</b>	<b>Model</b>	<b>Prototype</b>		
	L/s	m <sup>3</sup> /s		
Existing single	18.3	15.93		
Proposed twin	17.7	15.42		
Green St	0.0	0.00		
B6-1	1.3	1.13		
B6-2	1.7	1.49		
<b>Total</b>	<b>39.0</b>	<b>33.97</b>		
<b>TAILWATER</b>				
<b>Tailwater</b>	<b>Model</b>	<b>Prototype</b>	<b>Height above weir</b>	<b>Estimated proportion of Q</b>
	mm above invert	R.L.	m	
WL1	124.0	8.150	0.104	0.49
WL2	145.0	8.465	0.082	0.34
WL3	153.0	8.585	0.050	0.16
<b>HEADS</b>				
	<b>Reading</b>	<b>R.L</b>		
	mm	m		
1	258	8.955		
2	233 +/- 1	8.580		
3	230	8.535		
4	227	8.490		
5	238	8.655		
6	226	8.475		
7	227	8.490		
8	227	8.490		
9	228	8.505		
10	209 +/- 1	8.220		
11	181	7.800		
12	225	8.460		
13	215	8.310		
14	226	8.475		
15	229	8.520		

### 5.3.5 Case 2 T = 90

<b>BOUNDARY CONDITIONS</b>				
<b>Inflows</b>	<b>Model</b>	<b>Prototype</b>		
	L/s	m <sup>3</sup> /s		
Existing single	16.5	14.39		
Proposed twin	17.7	15.45		
Green St	0.0	0.00		
B6-1	1.8	1.58		
B6-2	2.1	1.79		
<b>Total</b>	<b>38.1</b>	<b>33.20</b>		
<b>Tailwater</b>	<b>Model</b>	<b>Prototype</b>	<b>Height above weir</b>	<b>Estimated proportion of Q</b>
	mm above invert	R.L.	m	
WL1	122	8.12	0.102	0.49
WL2	142	8.42	0.079	0.34
WL3	150	8.54	0.050	0.17
<b>HEADS</b>				
	<b>Reading</b>	<b>R.L</b>		
	mm	m		
1	256	8.925		
2	217	8.340		
3	226 +/- 1	8.475		
4	225	8.460		
5	235	8.610		
6	224	8.445		
7	224	8.445		
8	226	8.475		
9	227	8.490		
10	203	8.130		
11	181	7.800		
12	224	8.445		
13	214	8.295		
14	224	8.445		
15	226	8.475		

## 5.4 Case 3

### 5.4.1 Case 3 T = 15

<b>BOUNDARY CONDITIONS</b>				
<b>Inflows</b>	<b>Model</b>	<b>Prototype</b>		
	L/s	m <sup>3</sup> /s		
Existing single	11.1	9.64		
Proposed twin	1.9	1.67		
Green St	2.8	2.47		
B6-1	0.8	0.68		
B6-2	0.4	0.39		
<b>Total</b>	17.0	14.86		
<b>Tailwater</b>	<b>Model</b>	<b>Prototype</b>	<b>Height above weir</b>	<b>Estimated proportion of Q</b>
	mm above invert	R.L.	m	
WL1	68	7.310	0.048	0.38
WL2	90	7.640	0.060	0.53
WL3	93	7.685	0.018	0.09
<b>HEADS</b>				
	<b>Reading</b>	<b>R.L.</b>		
	mm	m		
1	187	7.890		
2	172	7.665		
3	177	7.740		
4	164	7.545		
5	175	7.710		
6	173	7.680		
7	143	7.230		
8	155	7.410		
9	162	7.515		
10	153	7.380		
11	141	7.200		
12	162	7.515		
13	158	7.455		
14	167	7.590		
15	168	7.605		

**5.4.2 Case 3 T = 30**

<b>BOUNDARY CONDITIONS</b>				
<b>Inflows</b>	<b>Model</b>	<b>Prototype</b>		
	L/s	m <sup>3</sup> /s		
Existing single	17.0	14.82		
Proposed twin	8.6	7.51		
Green St	1.8	1.53		
B6-1	4.1	3.54		
B6-2	1.5	1.32		
<b>Total</b>	33.0	28.72		
<b>Tailwater</b>	<b>Model</b>	<b>Prototype</b>	<b>Height above weir</b>	<b>Estimated proportion of Q</b>
	mm above invert	R.L.	m	
WL1	119	8.075	0.082	0.47
WL2	130	8.240	0.078	0.43
WL3	139	8.375	0.029	0.10
<b>HEADS</b>				
	<b>Reading</b>	<b>R.L</b>		
	mm	m		
1	245	8.760		
2	227	8.490		
3	213	8.280		
4	211	8.250		
5	222	8.415		
6	218	8.355		
7	225	8.460		
8	219	8.370		
9	219	8.370		
10	212	8.265		
11	162	7.515		
12	215	8.310		
13	206	8.175		
14	217	8.340		
15	218	8.355		

### 5.4.3 Case 3 T = 45

<b>BOUNDARY CONDITIONS</b>				
<b>Inflows</b>	<b>Model</b>	<b>Prototype</b>		
	L/s	m <sup>3</sup> /s		
Existing single	17.9	15.60		
Proposed twin	10.9	9.52		
Green St	0.8	0.68		
B6-1	0.6	0.50		
B6-2	4.6	4.05		
<b>Total</b>	<b>34.8</b>	<b>30.35</b>		
<b>Tailwater</b>	<b>Model</b>	<b>Prototype</b>	<b>Height above weir</b>	<b>Estimated proportion of Q</b>
	mm above invert	R.L.	m	
WL1	114.0	8.00	0.082	0.41
WL2	142.0	8.42	0.087	0.45
WL3	150.0	8.54	0.040	0.14
<b>HEADS</b>				
	<b>Reading</b>	<b>R.L</b>		
	mm	m		
1	244 +/- 3	8.745		
2	204 +/- 5	8.145		
3	228	8.505		
4	228 +/- 2	8.505		
5	234	8.595		
6	231	8.550		
7	200 +/- 5	8.085		
8	230 +/- 1	8.535		
9	230	8.535		
10	194 +/- 1	7.995		
11	184	7.845		
12	227	8.490		
13	214	8.295		
14	228	8.505		
15	228	8.505		

**5.4.4 Case 3 T = 60**

<b>BOUNDARY CONDITIONS</b>				
<b>Inflows</b>	<b>Model</b>	<b>Prototype</b>		
	L/s	m <sup>3</sup> /s		
Existing single	18.8	16.37		
Proposed twin	14.5	12.66		
Green St	0.0	0.00		
B6-1	1.1	0.97		
B6-2	2.4	2.08		
<b>Total</b>	<b>36.8</b>	<b>32.07</b>		
<b>Tailwater</b>	<b>Model</b>	<b>Prototype</b>	<b>Height above weir</b>	<b>Estimated proportion of Q</b>
	mm above invert	R.L.	m	
WL1	120.0	8.090	0.085	0.42
WL2	143.0	8.435	0.091	0.46
WL3	151.0	8.555	0.037	0.12
<b>HEADS</b>				
	<b>Reading</b>	<b>R.L</b>		
	mm	m		
1	247	8.790		
2	206	8.175		
3	233	8.580		
4	229	8.520		
5	232	8.565		
6	227	8.490		
7	219	8.370		
8	230	8.535		
9	229	8.520		
10	204	8.145		
11	187	7.890		
12	227	8.490		
13	213	8.280		
14	227	8.490		
15	229	8.520		

**5.4.5 Case 3 T = 75**

<b>BOUNDARY CONDITIONS</b>				
<b>Inflows</b>	<b>Model</b>	<b>Prototype</b>		
	L/s	m <sup>3</sup> /s		
Existing single	18.3	15.93		
Proposed twin	17.7	15.42		
Green St	0.0	0.00		
B6-1	1.3	1.13		
B6-2	1.7	1.49		
<b>Total</b>	<b>39.0</b>	<b>33.97</b>		
<b>Tailwater</b>	<b>Model</b>	<b>Prototype</b>	<b>Height above weir</b>	<b>Estimated proportion of Q</b>
	mm above invert	R.L.	m	
WL1	124.0	8.150	0.087	0.38
WL2	145.0	8.465	0.093	0.42
WL3	153.0	8.585	0.058	0.21
<b>HEADS</b>				
	<b>Reading</b>	<b>R.L</b>		
	mm	m		
1	251	8.850		
2	212	8.265		
3	236	8.625		
4	228	8.505		
5	239	8.670		
6	219	8.370		
7	220	8.385		
8	230	8.535		
9	230	8.535		
10	206 +/- 1	8.175		
11	190	7.935		
12	226	8.475		
13	214	8.295		
14	226	8.475		
15	227	8.490		



#### 5.4.6 Case 3 T = 90

<b>BOUNDARY CONDITIONS</b>				
<b>Inflows</b>	<b>Model</b>	<b>Prototype</b>		
	L/s	m <sup>3</sup> /s		
Existing single	16.5	14.39		
Proposed twin	17.7	15.45		
Green St	0.0	0.00		
B6-1	1.8	1.58		
B6-2	2.1	1.79		
<b>Total</b>	<b>38.1</b>	<b>33.20</b>		
<b>Tailwater</b>	<b>Model</b>	<b>Prototype</b>	<b>Height above weir</b>	<b>Estimated proportion of Q</b>
	mm above invert	R.L.	m	
WL1	122	8.12	0.085	0.38
WL2	142	8.42	0.090	0.42
WL3	150	8.54	0.055	0.20
<b>HEADS</b>				
	<b>Reading</b>	<b>R.L</b>		
	mm	m		
1	248	8.805		
2	206	8.175		
3	231	8.550		
4	227	8.490		
5	236	8.625		
6	219	8.370		
7	220	8.385		
8	229	8.520		
9	229	8.520		
10	209	8.220		
11	190	7.935		
12	224	8.445		
13	213	8.280		
14	225	8.460		
15	227	8.490		

## 6. Appendix B

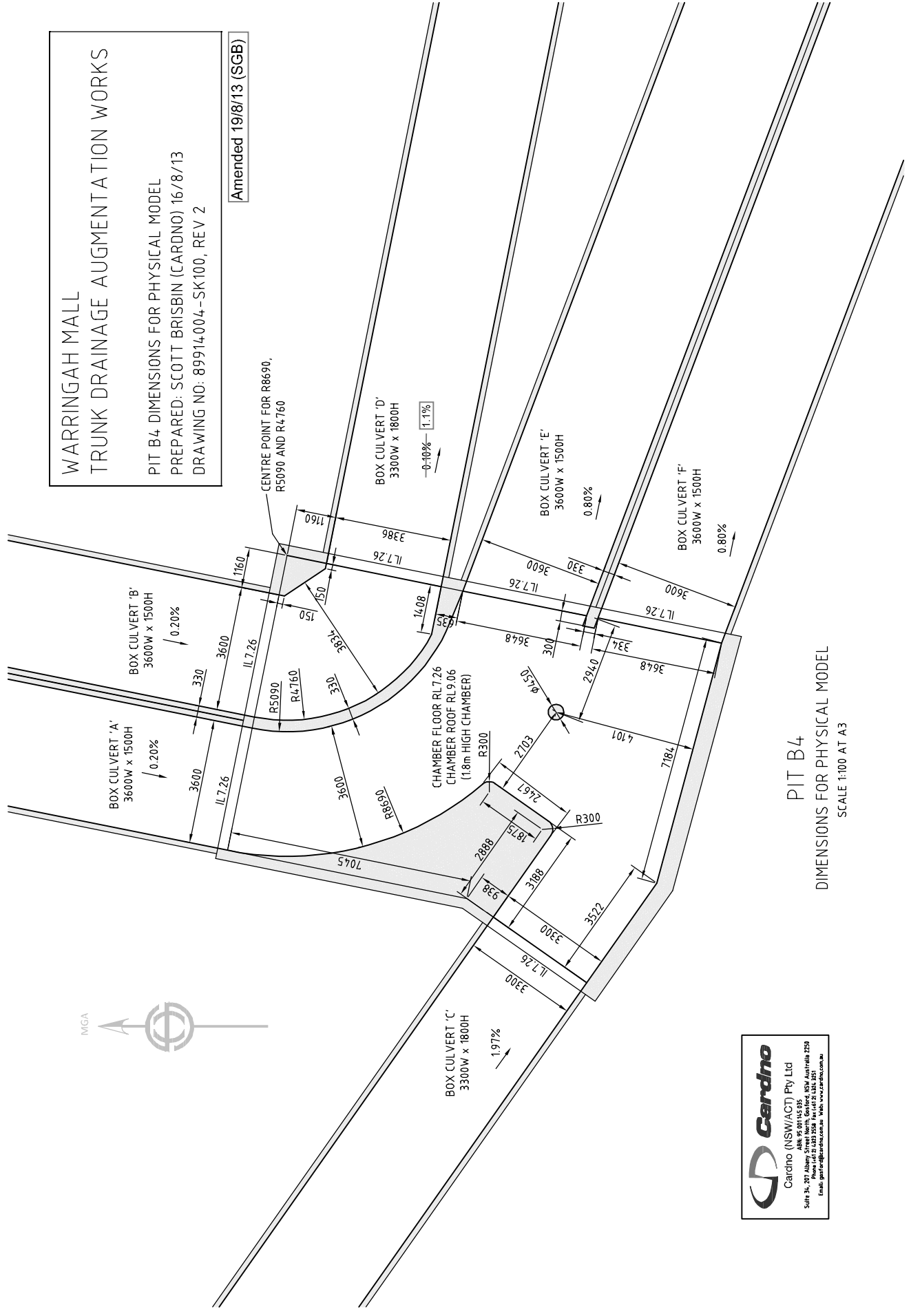
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WARRINGAH MALL  
TRUNK DRAINAGE AUGMENTATION WORKS

PIT B4 DIMENSIONS FOR PHYSICAL MODEL  
PREPARED: SCOTT BRISBIN (CARDNO) 16/8/13  
DRAWING NO: 89914004-SK100, REV 2

Amended 19/8/13 (SGB)



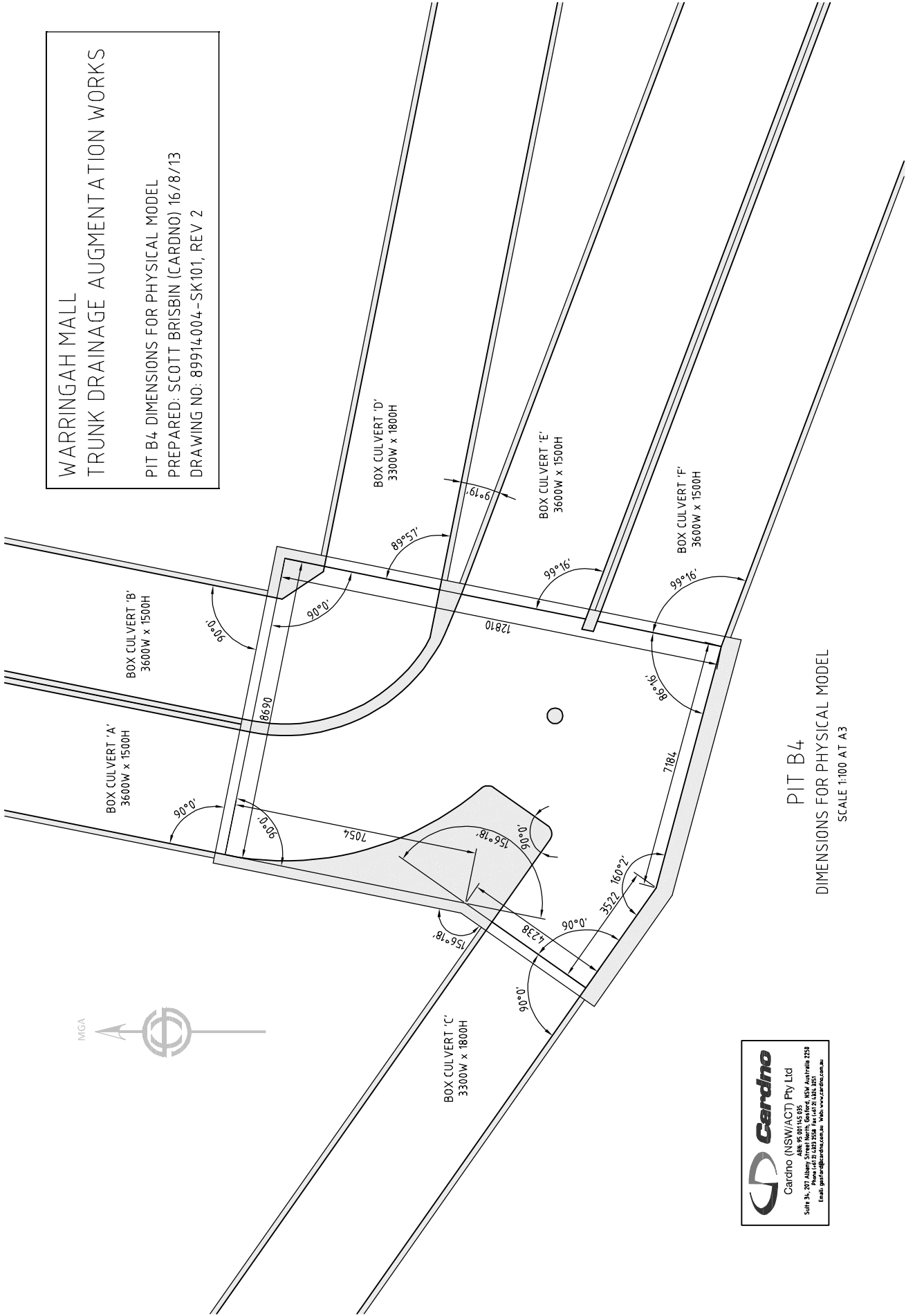
PIT B4  
DIMENSIONS FOR PHYSICAL MODEL  
SCALE 1:100 AT A3



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# WARRINGAH MALL TRUNK DRAINAGE AUGMENTATION WORKS

PIT B4 DIMENSIONS FOR PHYSICAL MODEL  
 PREPARED: SCOTT BRISBIN (CARDNO) 16/8/13  
 DRAWING NO: 89914004-SK101, REV 2

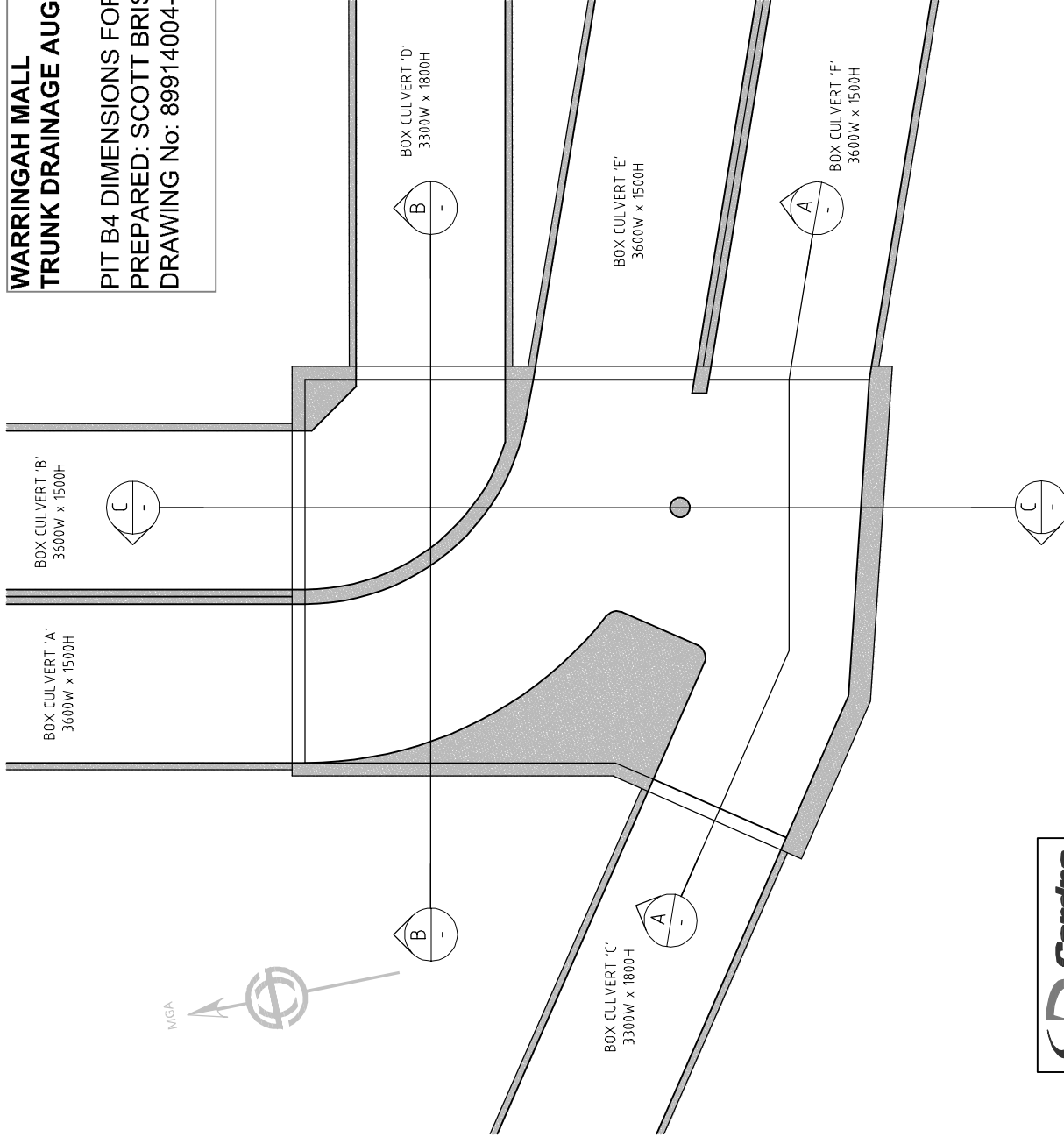


PIT B4  
 DIMENSIONS FOR PHYSICAL MODEL  
 SCALE 1:100 AT A3

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**WARRINGAH MALL  
TRUNK DRAINAGE AUGMENTATION WORKS**

PIT B4 DIMENSIONS FOR PHYSICAL MODEL  
 PREPARED: SCOTT BRISBIN (CARDNO) 19/8/13  
 DRAWING No: 89914004-SK103, REV 1



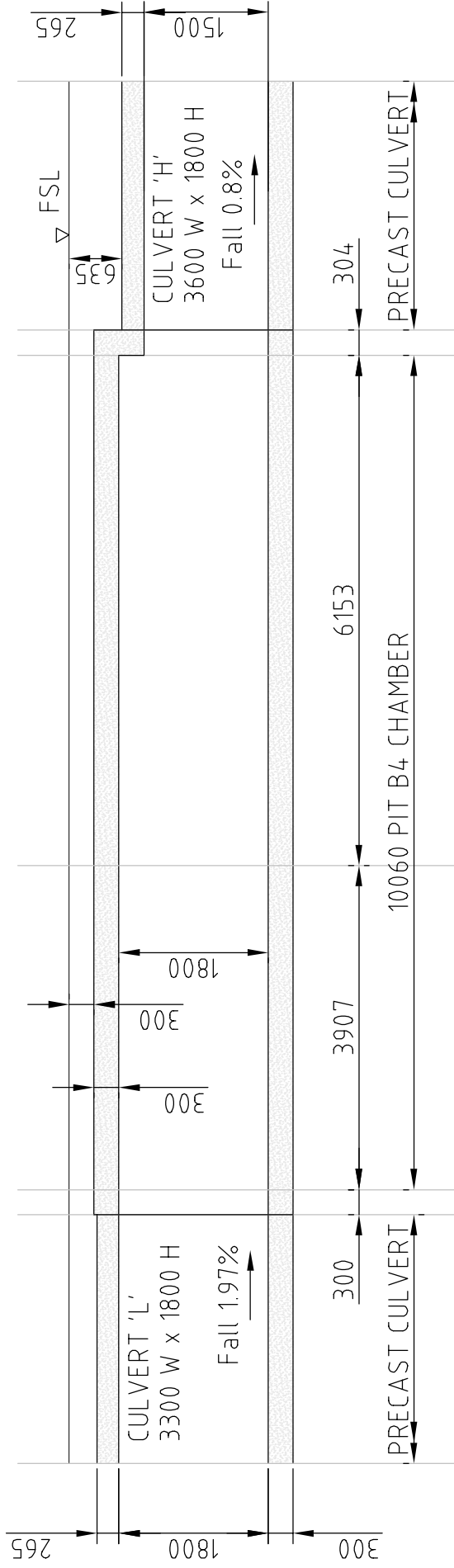
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 DIMENSIONS FOR PHYSICAL MODEL  
 SCALE 1:100 AT A3



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**WARRINGAH MALL  
TRUNK DRAINAGE AUGMENTATION WORKS**

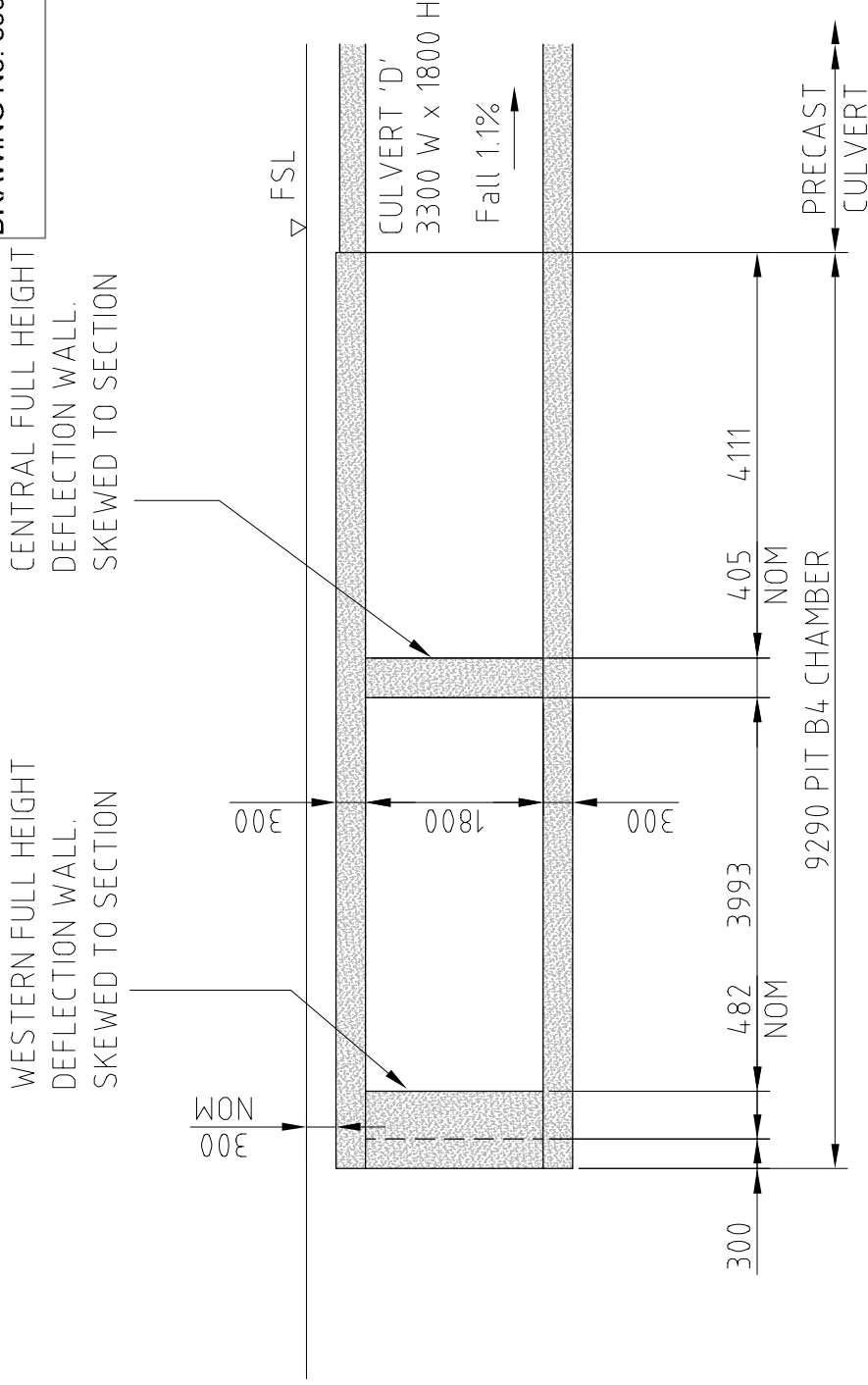
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 PREPARED: SCOTT BRISBIN (CARDNO) 19/8/13  
 DRAWING No: 89914004-SK104, REV 1



SECTION A -  
NTS

**WARRINGAH MALL  
TRUNK DRAINAGE AUGMENTATION WORKS**

PIT B4 DIMENSIONS FOR PHYSICAL MODEL  
PREPARED: SCOTT BRISBIN (CARDNO) 19/8/13  
DRAWING No: 89914004-SK105, REV 1

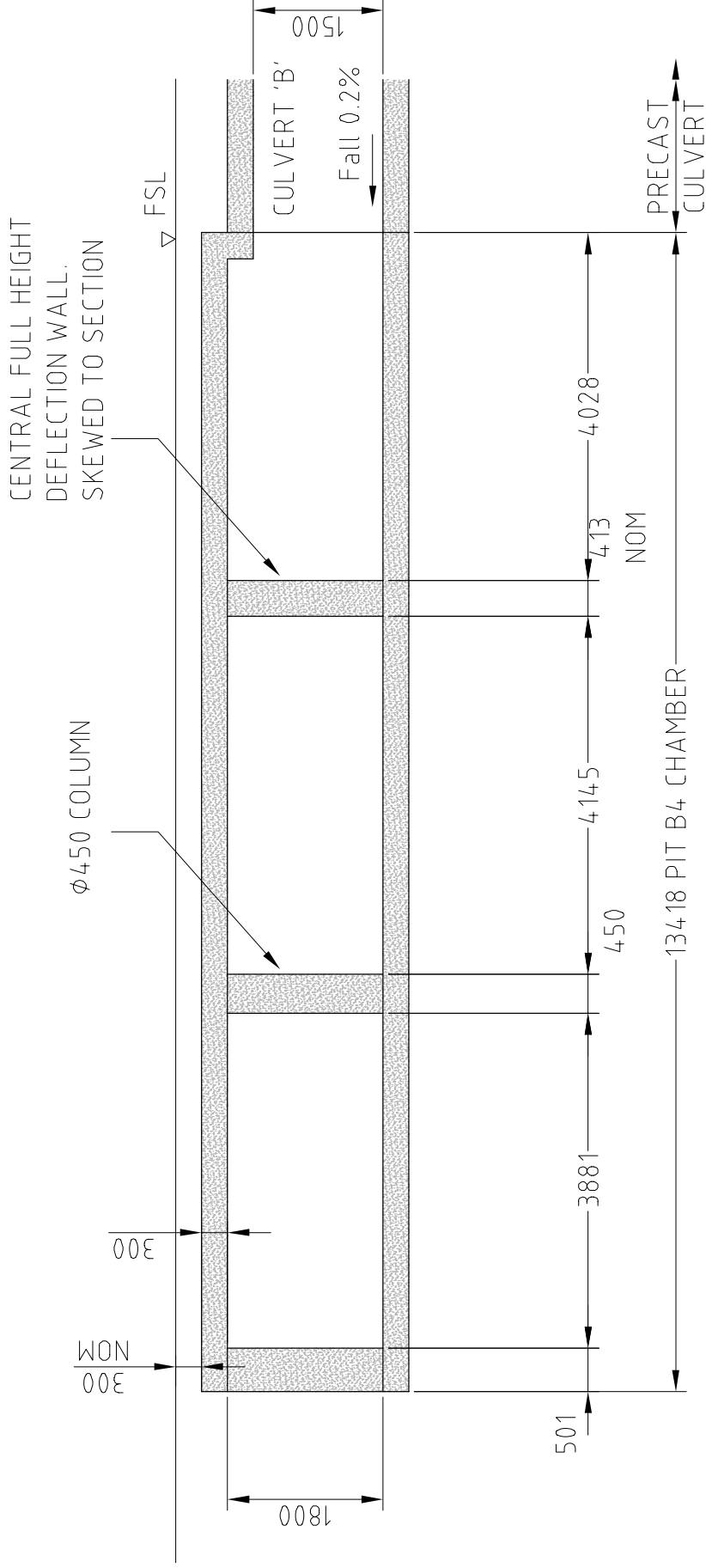


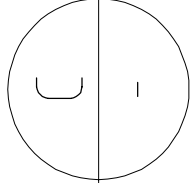
SECTION B  
NTS

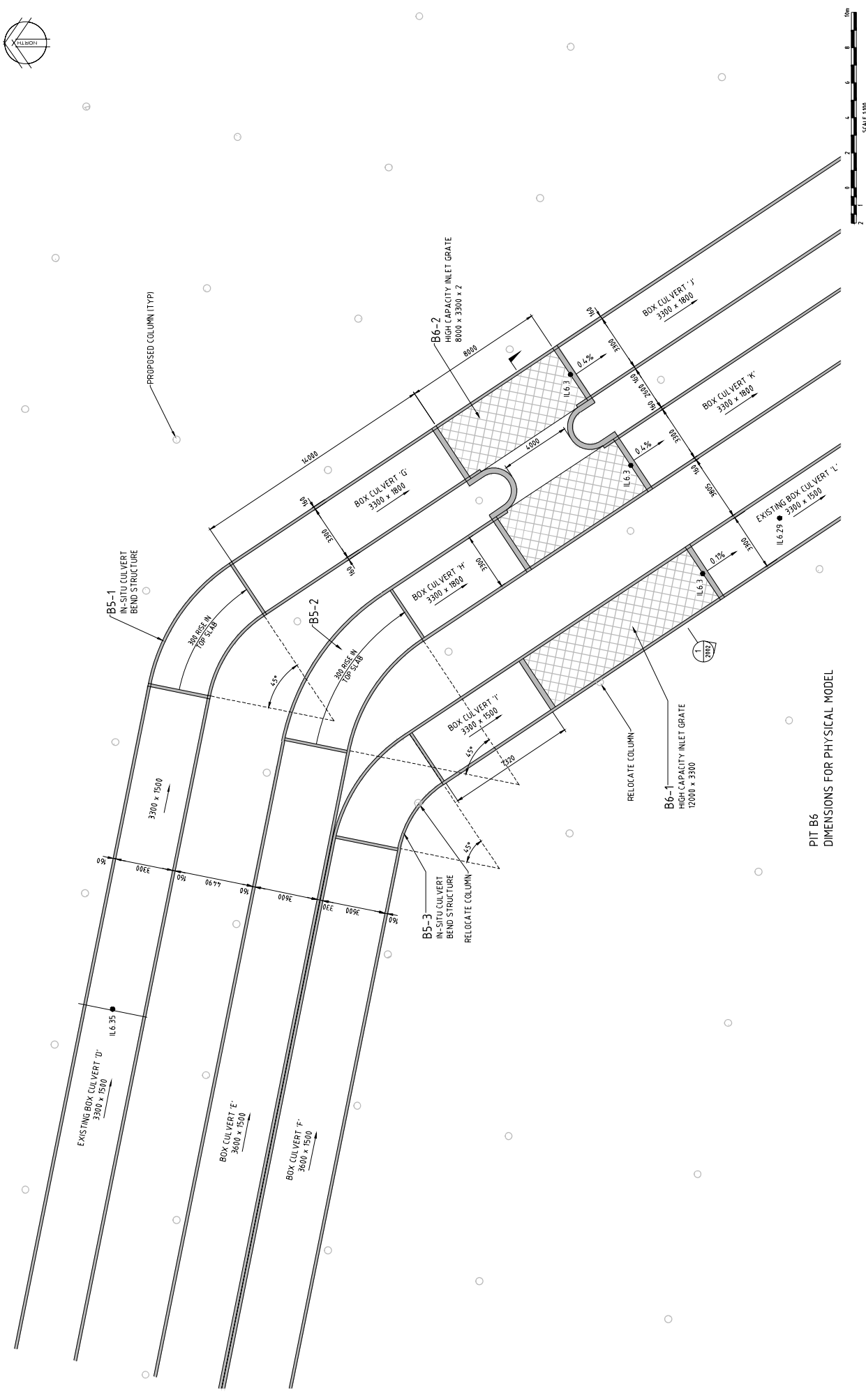


**WARRINGAH MALL  
TRUNK DRAINAGE AUGMENTATION WORKS**

PIT B4 DIMENSIONS FOR PHYSICAL MODEL  
PREPARED: SCOTT BRISBIN (CARDNO) 19/8/13  
DRAWING No: 89914004-SK106, REV 1



SECTION   
NTS



PIT B6  
DIMENSIONS FOR PHYSICAL MODEL

Scale	NOT FOR CONSTRUCTION	
Date	13/08/2013	Scale
Drawn	AHD	Scale
Checked	SK2001	Scale
Project No.	89914,004 - SK2001	Scale
Sheet No.	2	Scale

Client	WESTFIELD
Project	WARRINGAH MALL TRUNK DRAINAGE
Design	DNA
Checked	DNA
Drawn	DNA
Scale	1:100
Date	13/08/2013
Author	
Checked	
Date	
Scale	
Date	

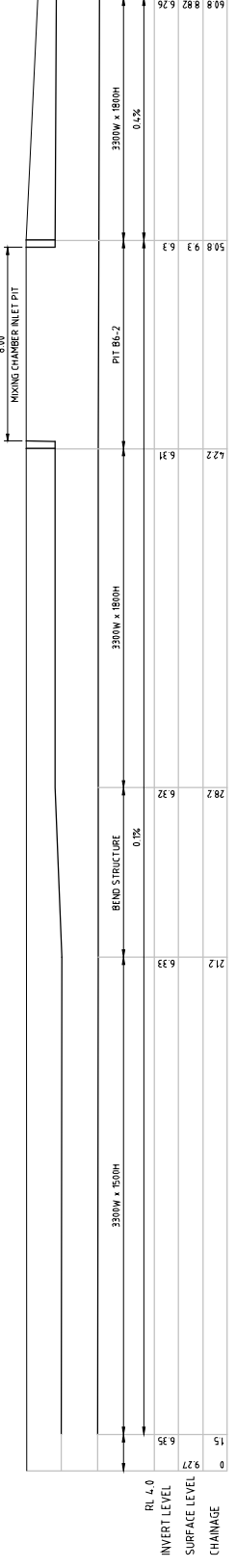
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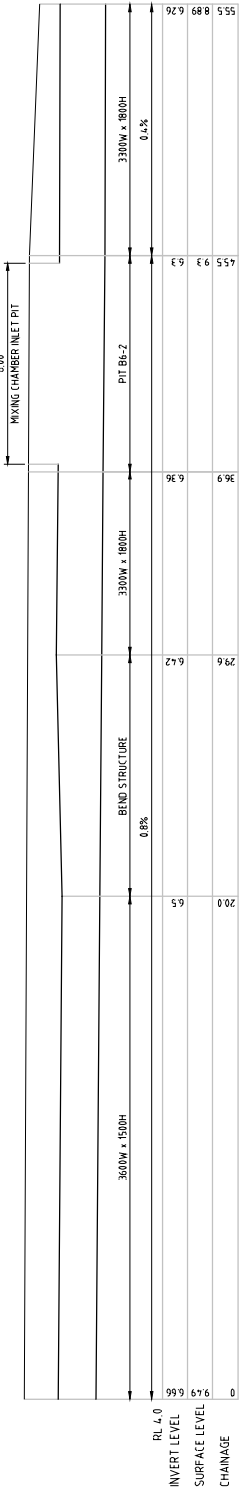
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Rev	Date	Description
1	13/08/2013	PRELIMINARY
2	21/08/2013	PRELIMINARY



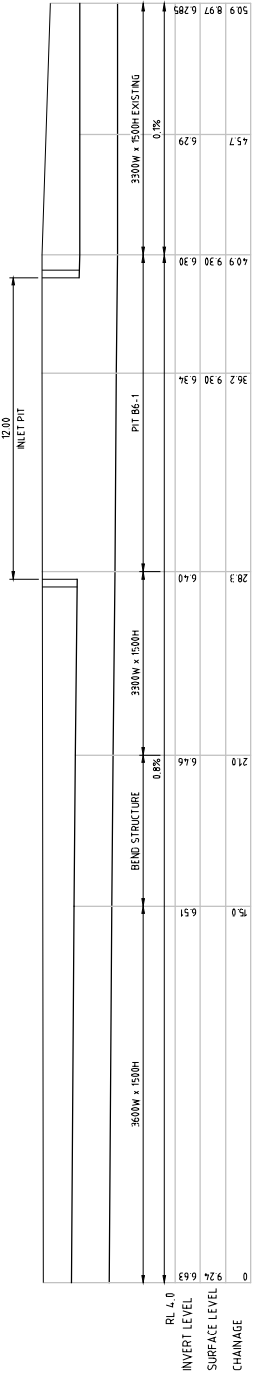
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SCALE 1:100



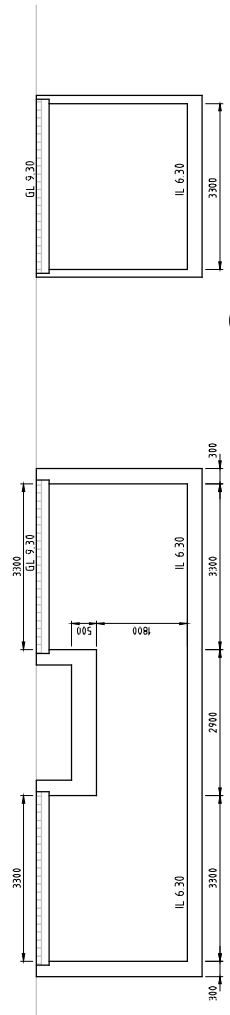
BOX CULVERT LONGSECTION 'E', 'H' & 'K'

SCALE 1:100

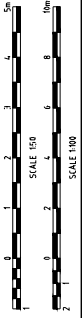


BOX CULVERT LONGSECTION 'F', 'I' & 'L'

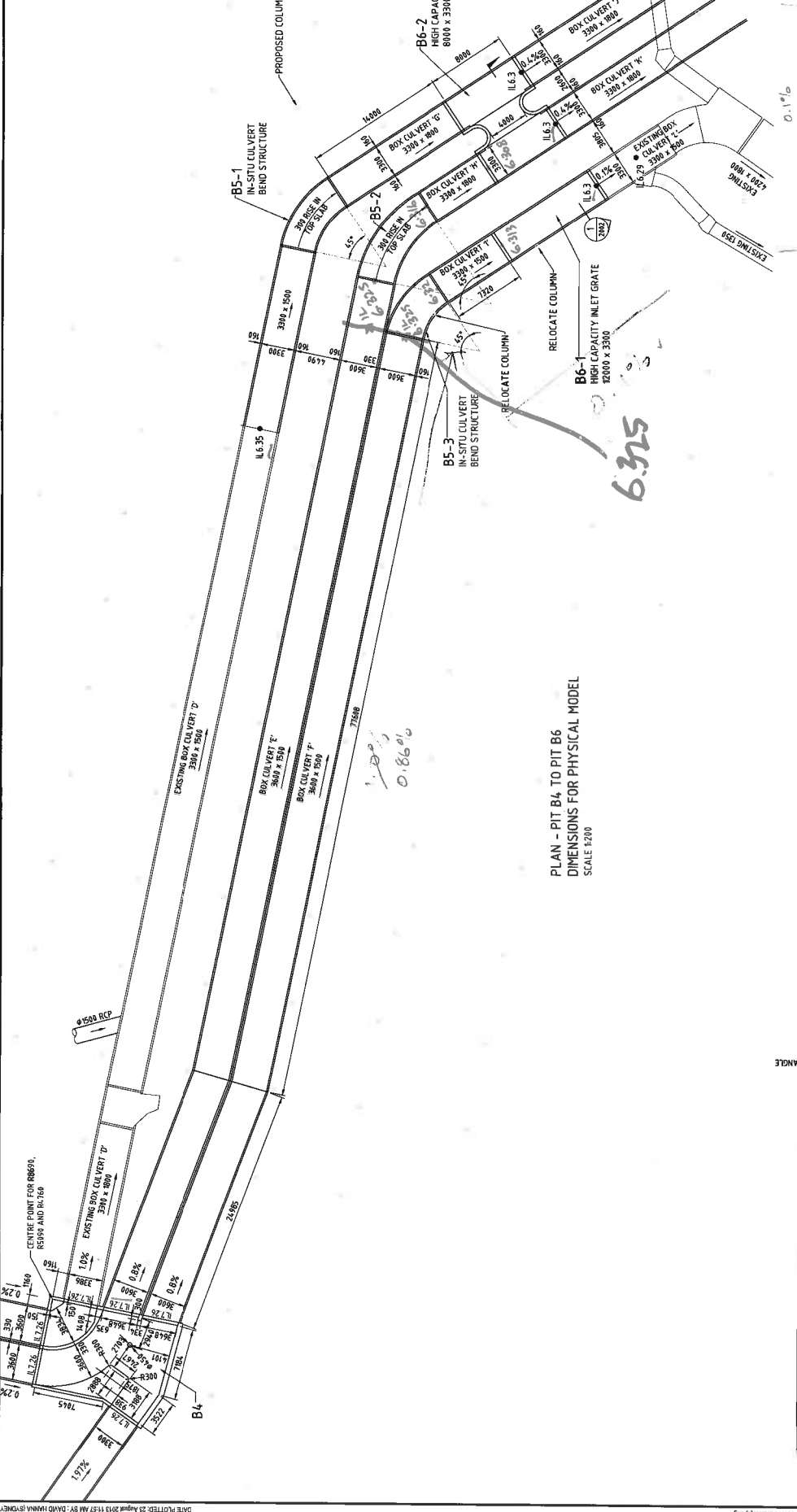
SCALE 1:100



SECTION 1  
SCALE 1:50



		<b>WESTFIELD</b> WARRINGAH MALL TRUNK DRAINAGE	
Date: 13/08/2013 Drawn: DNA Checked: DNA Title: NOT FOR CONSTRUCTION	Date: 13/08/2013 Drawn: DNA Checked: DNA Title: NOT FOR CONSTRUCTION	Date: 13/08/2013 Drawn: DNA Checked: DNA Title: NOT FOR CONSTRUCTION	Date: 13/08/2013 Drawn: DNA Checked: DNA Title: NOT FOR CONSTRUCTION
Project No: 89914004-SK2002 Revision: 2		Typical Sections	
Scale: 1:100 Date: 13/08/2013 Drawn: DNA Checked: DNA Title: NOT FOR CONSTRUCTION		Scale: 1:100 Date: 13/08/2013 Drawn: DNA Checked: DNA Title: NOT FOR CONSTRUCTION	



PLAN - PIT B4 TO PIT B6  
DIMENSIONS FOR PHYSICAL MODEL  
SCALE 1:200

LONGITUDINAL SECTION - PIT B4 TO PIT B6 SCALE 1:200																																																			
<table border="1"> <tr> <td>BOX CULVERT 'E'</td> <td>3000 x 1500</td> <td>1.0%</td> <td>0.86%</td> <td>0.86%</td> </tr> <tr> <td>BOX CULVERT 'F'</td> <td>3000 x 1500</td> <td>1.0%</td> <td>0.86%</td> <td>0.86%</td> </tr> <tr> <td>BOX CULVERT 'G'</td> <td>3000 x 1500</td> <td>1.0%</td> <td>0.86%</td> <td>0.86%</td> </tr> <tr> <td>BOX CULVERT 'H'</td> <td>3000 x 1500</td> <td>1.0%</td> <td>0.86%</td> <td>0.86%</td> </tr> <tr> <td>BOX CULVERT 'I'</td> <td>3000 x 1500</td> <td>1.0%</td> <td>0.86%</td> <td>0.86%</td> </tr> <tr> <td>BOX CULVERT 'J'</td> <td>3000 x 1500</td> <td>1.0%</td> <td>0.86%</td> <td>0.86%</td> </tr> <tr> <td>BOX CULVERT 'K'</td> <td>3000 x 1500</td> <td>1.0%</td> <td>0.86%</td> <td>0.86%</td> </tr> <tr> <td>BOX CULVERT 'L'</td> <td>3000 x 1500</td> <td>1.0%</td> <td>0.86%</td> <td>0.86%</td> </tr> </table>	BOX CULVERT 'E'	3000 x 1500	1.0%	0.86%	0.86%	BOX CULVERT 'F'	3000 x 1500	1.0%	0.86%	0.86%	BOX CULVERT 'G'	3000 x 1500	1.0%	0.86%	0.86%	BOX CULVERT 'H'	3000 x 1500	1.0%	0.86%	0.86%	BOX CULVERT 'I'	3000 x 1500	1.0%	0.86%	0.86%	BOX CULVERT 'J'	3000 x 1500	1.0%	0.86%	0.86%	BOX CULVERT 'K'	3000 x 1500	1.0%	0.86%	0.86%	BOX CULVERT 'L'	3000 x 1500	1.0%	0.86%	0.86%	<table border="1"> <tr> <td>PIT B4</td> <td>3000 x 1500</td> <td>1.0%</td> <td>0.86%</td> <td>0.86%</td> </tr> <tr> <td>PIT B6</td> <td>3000 x 1500</td> <td>1.0%</td> <td>0.86%</td> <td>0.86%</td> </tr> </table>	PIT B4	3000 x 1500	1.0%	0.86%	0.86%	PIT B6	3000 x 1500	1.0%	0.86%	0.86%
BOX CULVERT 'E'	3000 x 1500	1.0%	0.86%	0.86%																																															
BOX CULVERT 'F'	3000 x 1500	1.0%	0.86%	0.86%																																															
BOX CULVERT 'G'	3000 x 1500	1.0%	0.86%	0.86%																																															
BOX CULVERT 'H'	3000 x 1500	1.0%	0.86%	0.86%																																															
BOX CULVERT 'I'	3000 x 1500	1.0%	0.86%	0.86%																																															
BOX CULVERT 'J'	3000 x 1500	1.0%	0.86%	0.86%																																															
BOX CULVERT 'K'	3000 x 1500	1.0%	0.86%	0.86%																																															
BOX CULVERT 'L'	3000 x 1500	1.0%	0.86%	0.86%																																															
PIT B4	3000 x 1500	1.0%	0.86%	0.86%																																															
PIT B6	3000 x 1500	1.0%	0.86%	0.86%																																															
<p>INVERT LEVEL</p> <p>SURFACE LEVEL</p> <p>CHAMBER</p>																																																			
<p>ANGLE</p> <p>SCALE 1:200</p>																																																			

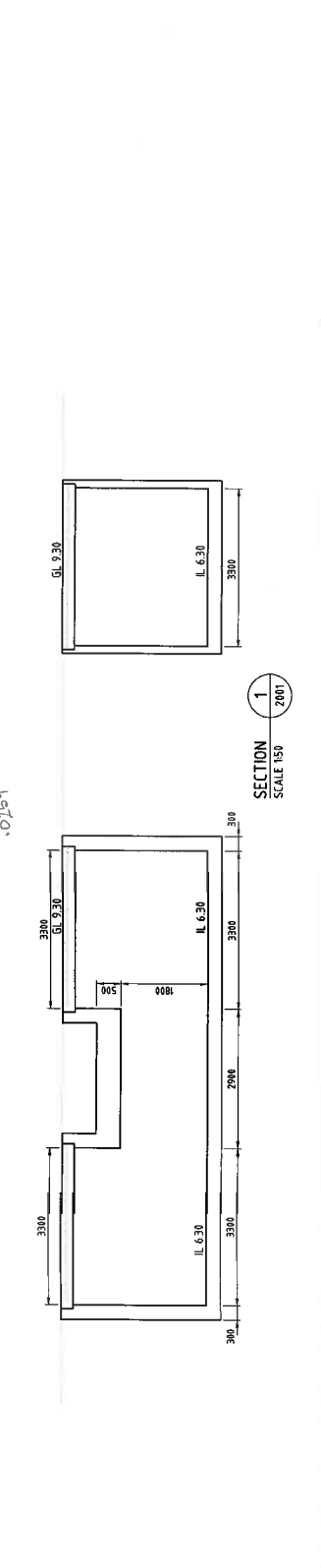
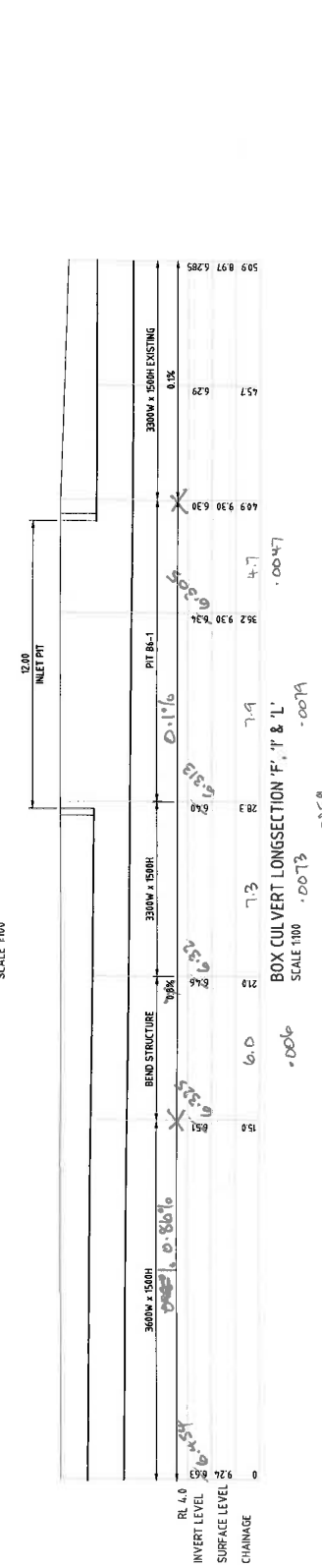
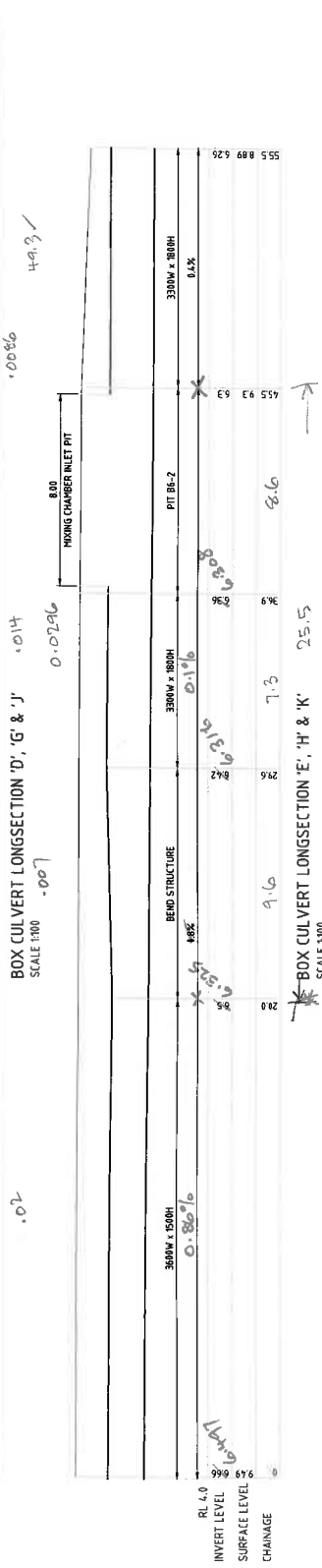
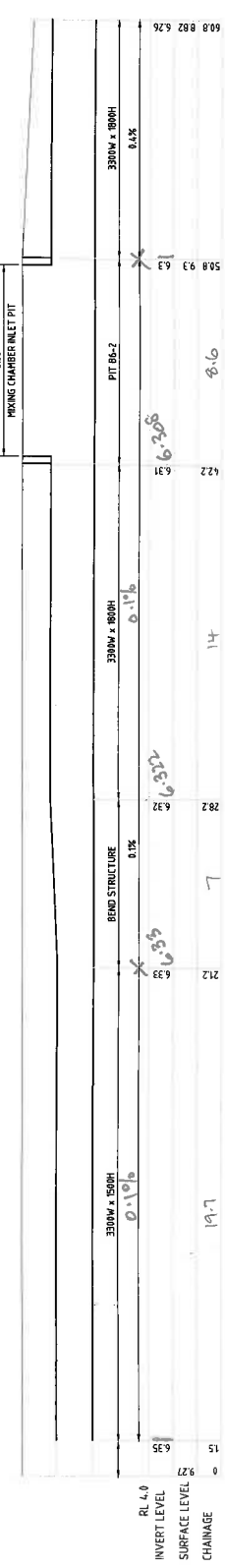
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Client: WESTFIELD  
WARRINGHAM MALL  
THURK DRAINAGE

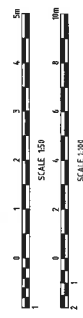
19/08/2013  
19/08/2013  
19/08/2013

NOT FOR CONSTRUCTION  
DATE: 19/08/2013  
DRAWN: AHD  
CHECKED: AHD  
SCALE: 1:200  
PROJECT NO: 899174-004-SK2003

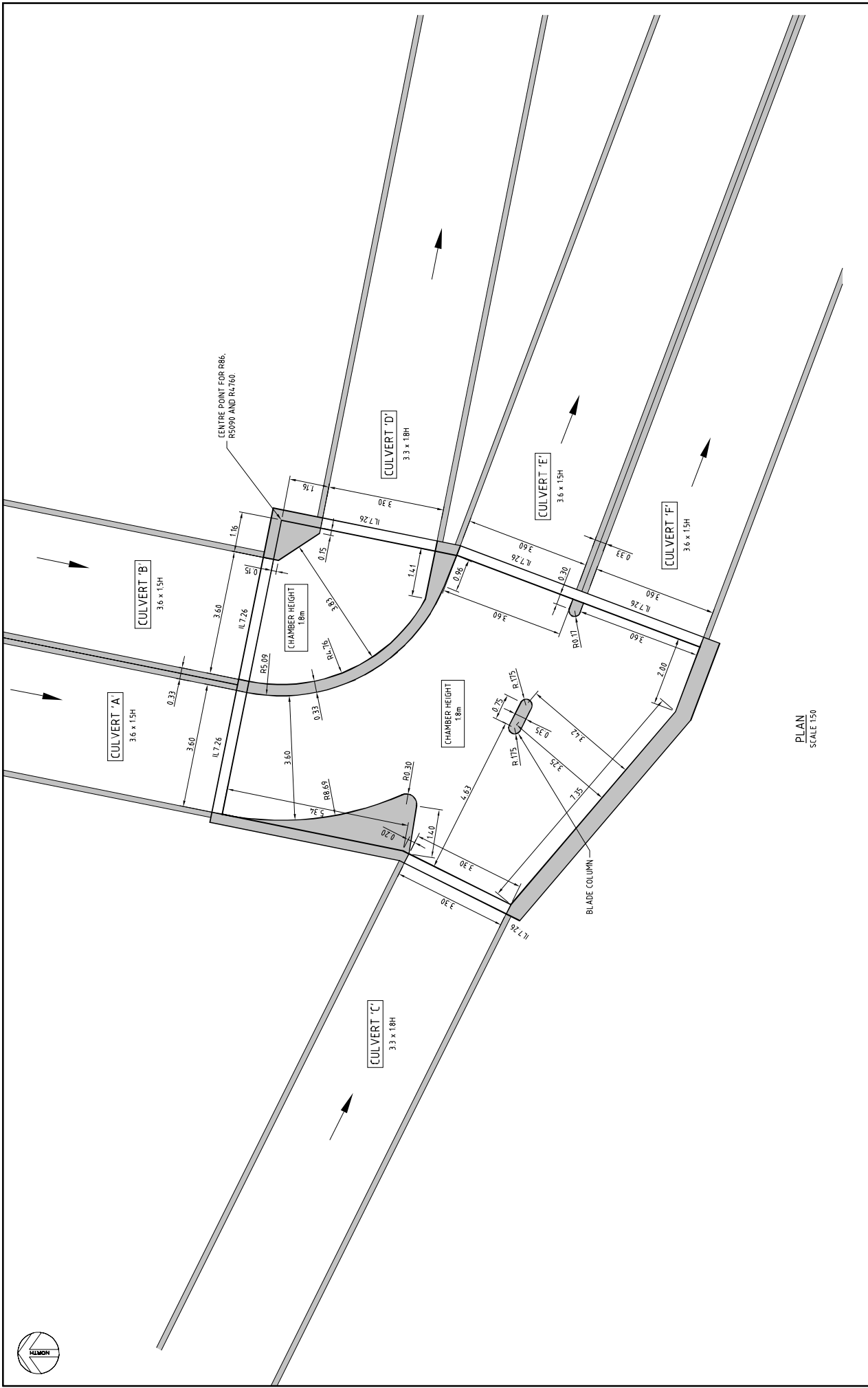
PIT B4 TO PIT B6  
PLAN & LONGITUDINAL SECTION  
ON: ORIGINAL



SECTION 1  
SCALE 1/8" = 1'-0"



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<p>Client: WESTFIELD          Date: 13/08/2013</p>	<p>Design: WESTFIELD          Date: 13/08/2013</p>	<p>Checked: WESTFIELD          Date: 13/08/2013</p>	<p>Drawn: WESTFIELD          Date: 13/08/2013</p>
<p>Project: 89514004-SK2002</p>	<p>Sheet: 2</p>	<p>Scale: AS SHOWN</p>	<p>Revision: 2</p>
<p>NOT FOR CONSTRUCTION</p>		<p>89514004-SK2002</p>	
<p>DATE PLOTTED: 21 August 2013 11:05 AM BY: DAVID HANNA (SYNCR)</p>		<p>SCALE 1/8" = 1'-0"</p>	

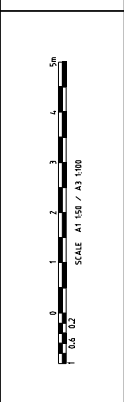


PLAN  
SCALE 1:50

Item	LOB	Date	Drawn	Checked	Scale	Sheet
1	SCB	28/10/13	WARRINGAH MALL	WARRINGAH MALL	1:50	A1
2	SCB	28/10/13	STAGE1 TRUNK DRAINAGE AUGMENTATION WORKS	STAGE1 TRUNK DRAINAGE AUGMENTATION WORKS	1:50	A1
3	SCB	28/10/13	PIT B4 TO B6 - DIMENSIONS FOR PHYSICAL MODEL	PIT B4 TO B6 - DIMENSIONS FOR PHYSICAL MODEL	1:50	A1
4	SCB	28/10/13	SHEET 1 OF 2	SHEET 1 OF 2	1:50	A1
5	SCB	28/10/13	89914004-SK100	89914004-SK100	1:50	A1

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NO	DATE	DESCRIPTION	BY	CHECKED
1				
2				
3				
4				
5				

Attachment A

DATE PLOTTED: 6 September 2013 1:51 PM BY: DAVID HANNA (SYDNEY)

XREF: \\X:\GP\A1\SHIT\_X\SYD\A1\Loop\_X\Boundary\_X\Survey\_X\Surf\Est\_X\Design\_X\Plan\_X\Drawings\Bldg\Drawings\89914004-2022\1.dwg  
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Rev	Date	Description	Drawn	Appr.
1	7/7	PRELIMINARY	DH	DNA

**Westfield**  
 Westfield Design & Construction Pty Limited  
 100 Market Street, Sydney NSW 2011  
 Phone (02) 9358 7000 Fax (02) 9028 8500  
 GPO Box 4004 Sydney NSW 2001  
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Drawn	DH	Date	30/08/2013
Checked	DNA	Date	30/08/2013
Designed	DNA	Date	30/08/2013
Verified		Date	
Approved		Date	

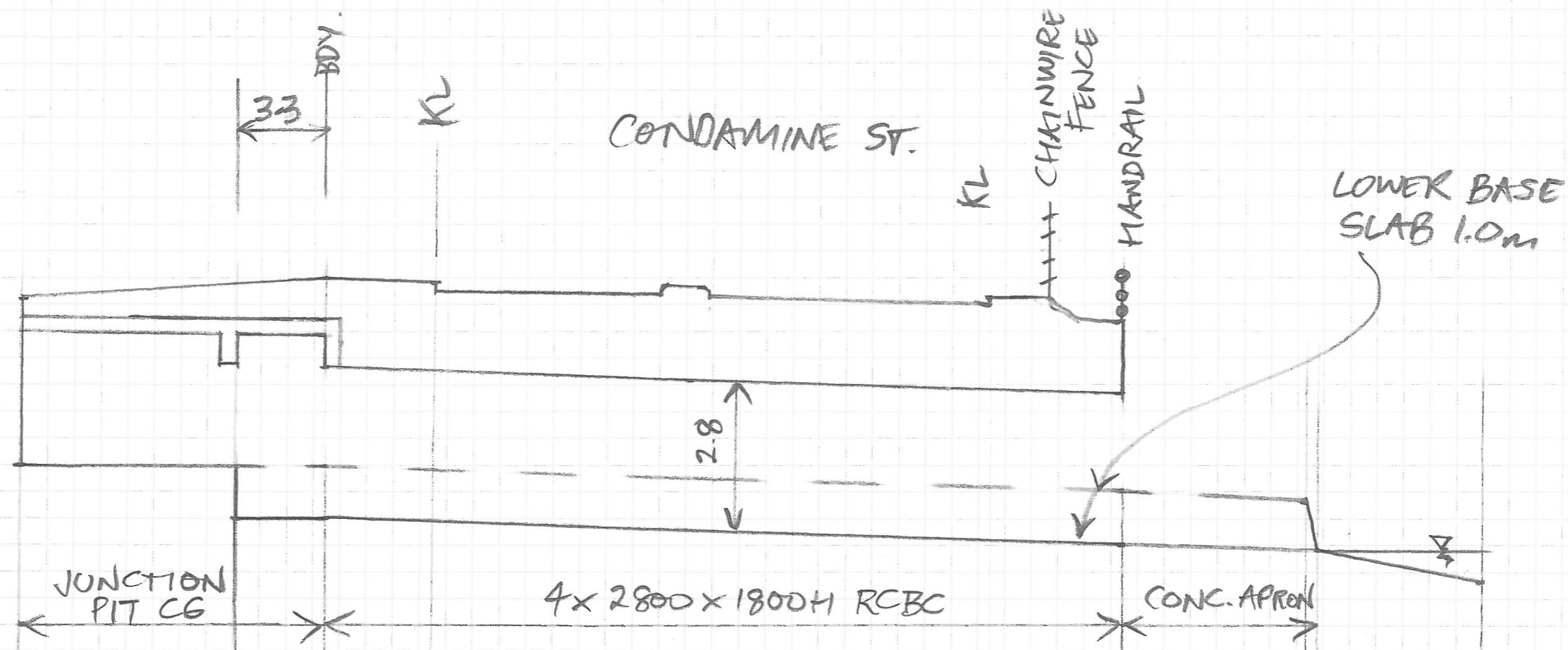
Client: WESTFIELD DESIGN AND CONSTRUCTION			
WARRINGAH MALL TRUNK DRAINAGE			
Status: NOT FOR CONSTRUCTION			
Date	Datum	Scale	Size
30/08/2013	AHD	1:200	A1
Drawing Number: 89914004-2022			Revision: 1
GENERAL ARRANGEMENT - SHEET 3 of 3			

0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150mm ON A1 ORIGINAL

WARRINGAH MALL, WESTFIELD

DATE  
SEPT. 13

DOCUMENT NO.



PROPOSED INVERT LEVEL	DATUM 0.0	5.0	5.0			4.5	4.44	
EXISTING INVERT LEVEL	6.0	6.0	6.0			5.51	5.33	4.44
EXIST. LEVEL	9.10		9.49	9.41		9.17	8.70	3.80
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								48.6
								55.0

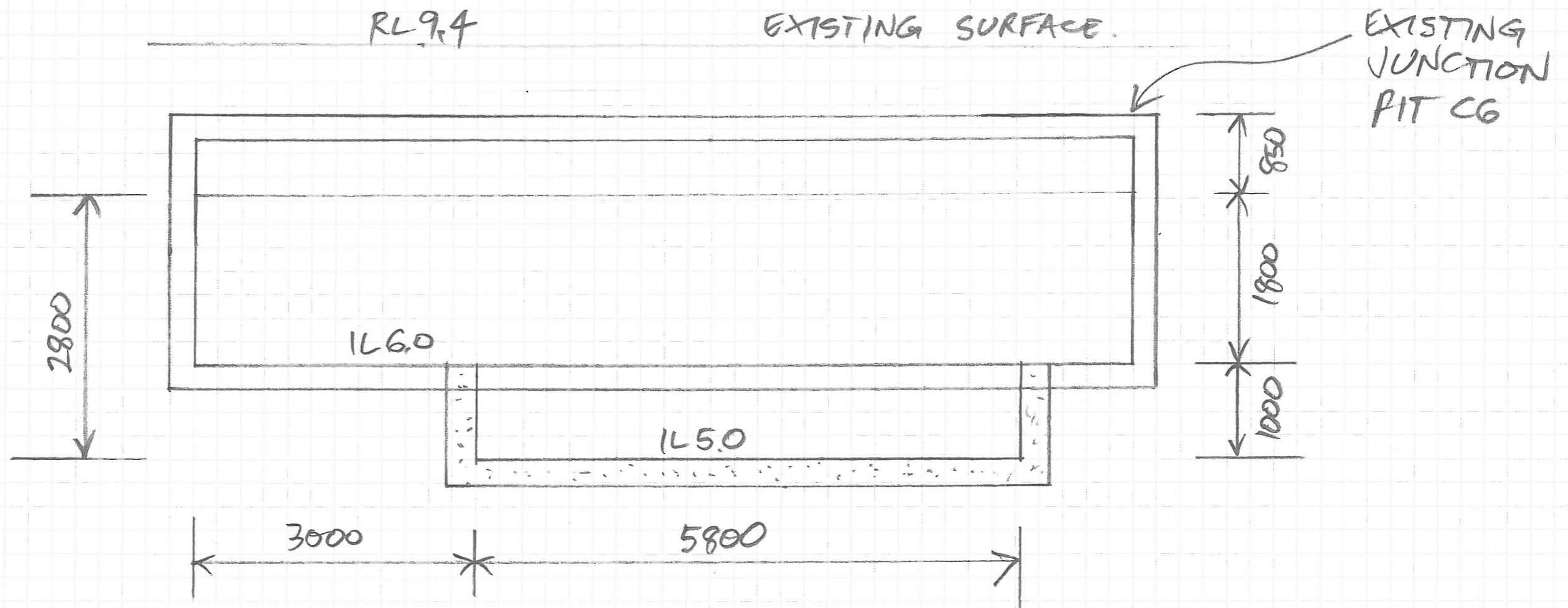
LONGITUDINAL SECTION  
BOX CULVERTS CONDAMINE ST.

HORIZ. 1:200  
VERT 1:100

SEPT. 13.  
WARRINGAH  
MALL  
89914004-SK  
2100



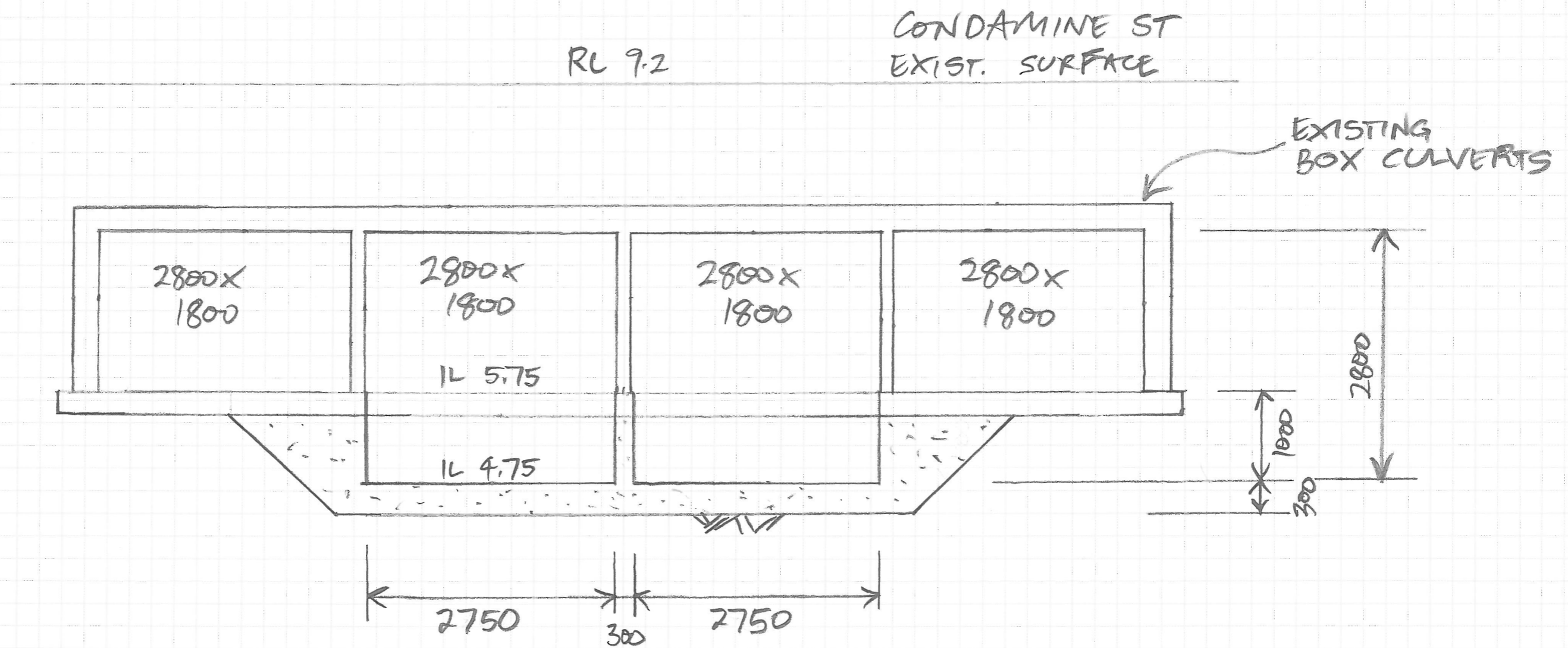
DATE	DOCUMENT NO.
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SECTION ①  
1:50

SEPT 13.  
WARRINGAH  
MALL  
999 14004-  
SK 2401

DATE	DOCUMENT NO.
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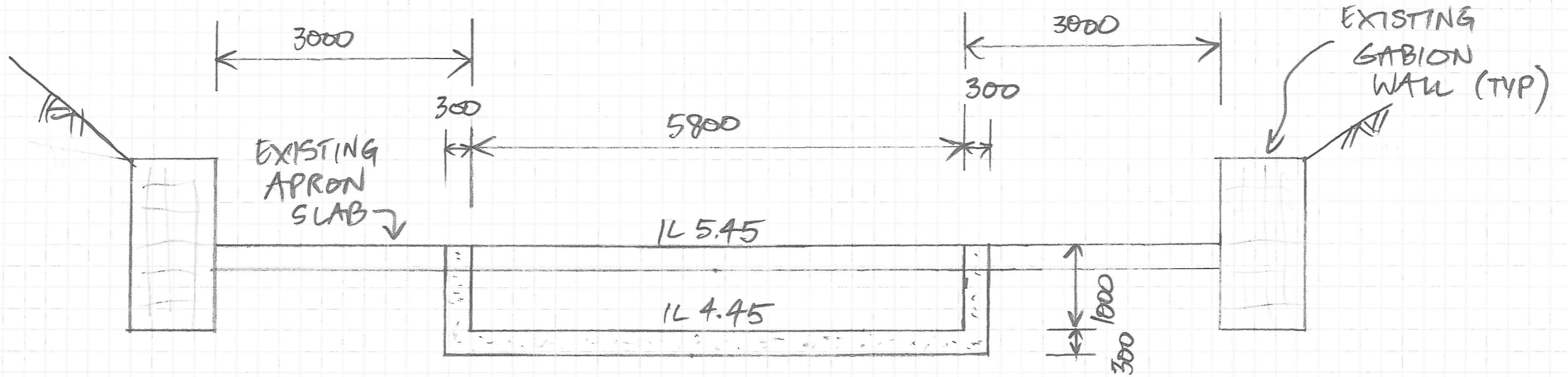


SECTION (2)  
1:50

SEPT 13  
WARRINGAH  
MAU  
89914004-SK  
2102

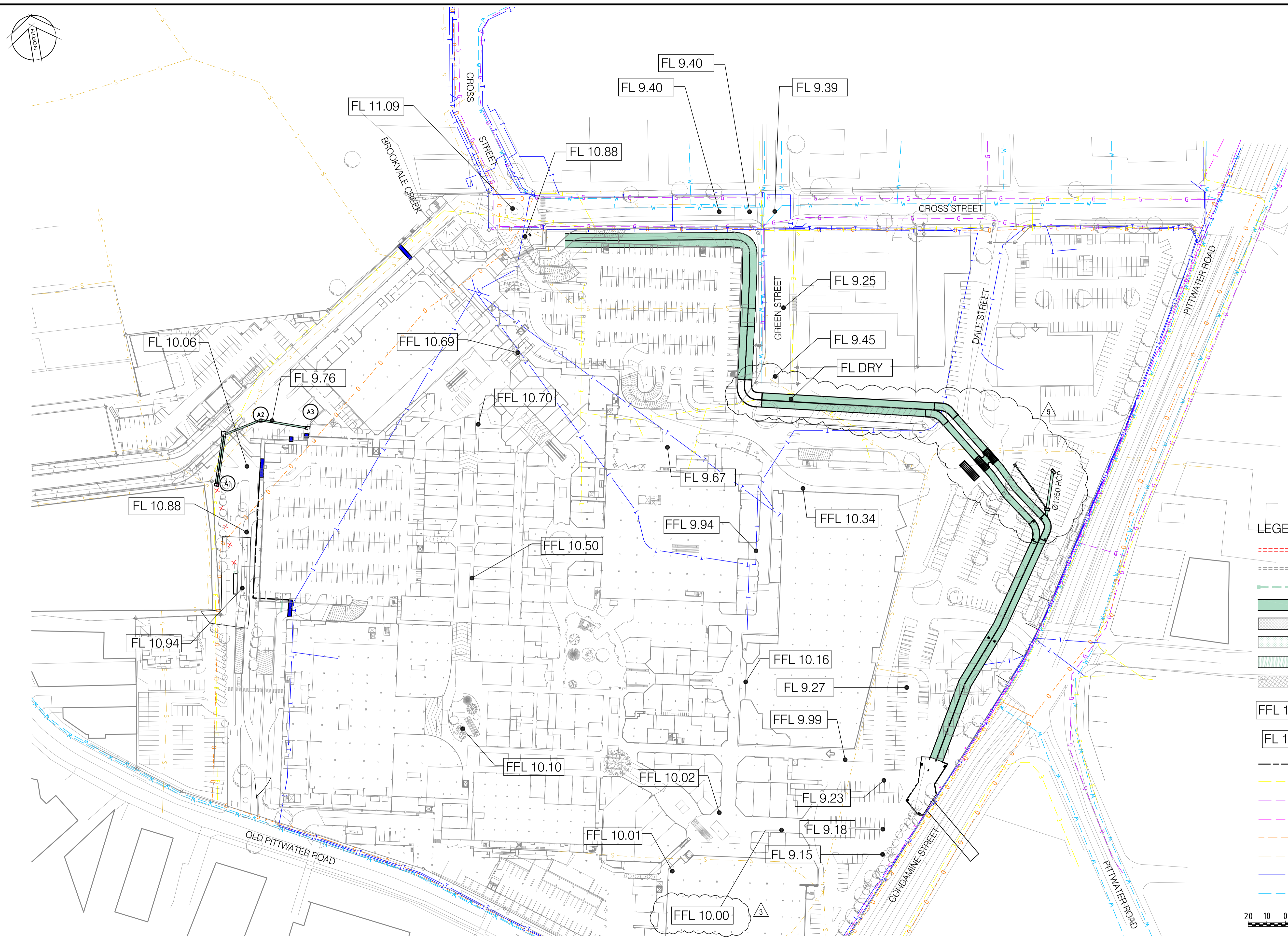
DATE

DOCUMENT NO.



SECTION 3  
1:50

SEPT 13.  
WARRINGAH  
MALL  
89914004-SK  
2103



NOTE:  
LOCATION AND LEVEL OF ALL  
SERVICES TO BE CONFIRMED ON SITE

**LEGEND**

- EXISTING STORMWATER TO REMAIN
- EXISTING STORMWATER TO BE REMOVED
- PROPOSED STORMWATER (RCP)
- PROPOSED STORMWATER (RCBC)
- PROPOSED HIGH CAPACITY INLET
- APPROXIMATE AREAS OF REGRADING
- APPROXIMATE AREA OF CHANNEL REGRADING
- PROPOSED STAGE 1 BUILDING WORKS
- FFL 10.50 FINISHED FLOOR LEVEL (m AHD)
- FL 10.17 100 YEAR ARI FLOOD LEVEL (m AHD)
- BOUNDARY
- ELECTRICITY
- GAS
- OPTICAL FIBRE
- OPTUS
- SEWER
- TELSTRA
- WATER

0 10 20 40 60 80 100  
SCALE OF METRES (1:1000 BEFORE REDUCTION)

XREFs: X:Survey; X:Drainage; X:Architect; X:Carpark; X:GRP-A1-SHT; X:SYD-A1-Logo  
 CAD FILE: N:\Projects\9999\FY13\W4548\1332\_WARRINGAH MALL FLOODING AND DRAINAGE\01+Package\_1\_Data\Drawn\DW4548-420(5).dwg

Rev	Date	Description	Drawn	Appr.
5	5/11/2014	FLOOD BARRIERS UPDATED	LLC	DNA
4	15/10/2014	FLOOD LEVELS AMENDED	JT	DNA
3	13/08/2014	FLOOD LEVELS REVISED	GV	DNA
2	30/07/2014	CULVERT AMENDED THROUGH BING LEE	DH	DNA
1	20/11/2013	PRELIMINARY	DH	DNA



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Drawn	GV	Date	13/08/2014
Checked	DNA	Date	13/08/2014
Designed	DNA	Date	18/11/2013
Verified		Date	
Approved		Date	

Client: **WESTFIELD DESIGN & CONSTRUCTION**  
**WARRINGAH MALL**  
**CONCEPT DRAINAGE AUGMENTATION WORKS**  
 Status: **PRELIMINARY**  
 Date: 12/08/2014 Datum: AHD Scale: 1:1000 Size: A1  
 Drawing Number: **W4548-420** Revision: **5**  
 DETAIL ARRANGEMENT PLAN  
 GENERAL ARRANGEMENT PLAN

Table A.5 Updated Comparison of Updated Stormwater DA with Existing Conditions (xpswmv10)

Existing Conditions

Updated Stormwater DA

New Stormwater DA 1742 (Stage2)

New Stormwater DA 1742 (Stage2)

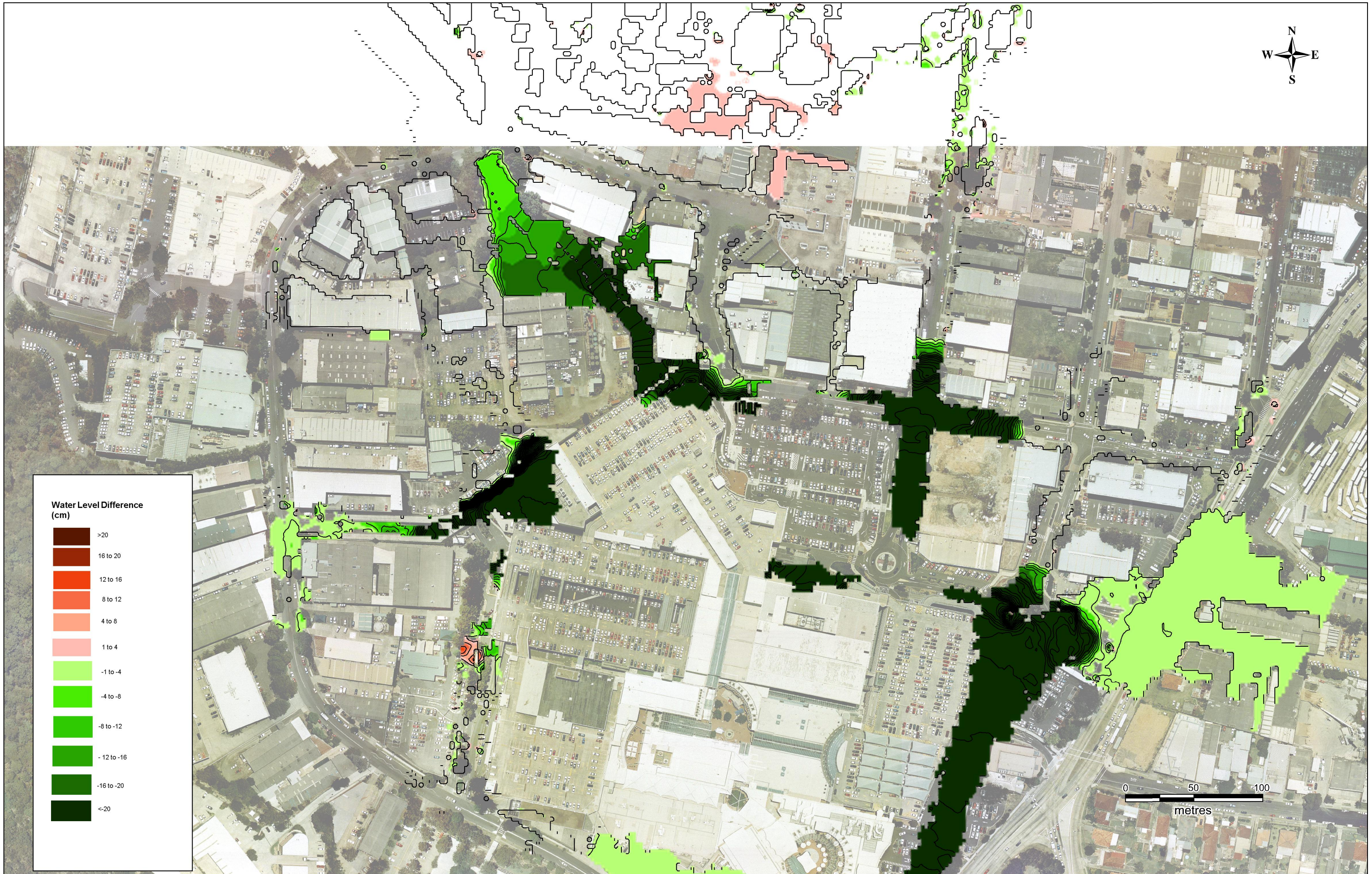
Table with 26 columns: Survey Point, Floor Level (mAH/D), Street Address, Flood Level (mAH/D), Depth over Ground (cm), Depth over Floor (cm), Diff (cm), Max FL (m AH/D), and multiple columns for Flood Level, Depth over Ground, Depth over Floor, and Diff for various ARI and Blockage scenarios.

Existing Conditions

Updated Stormwater DA

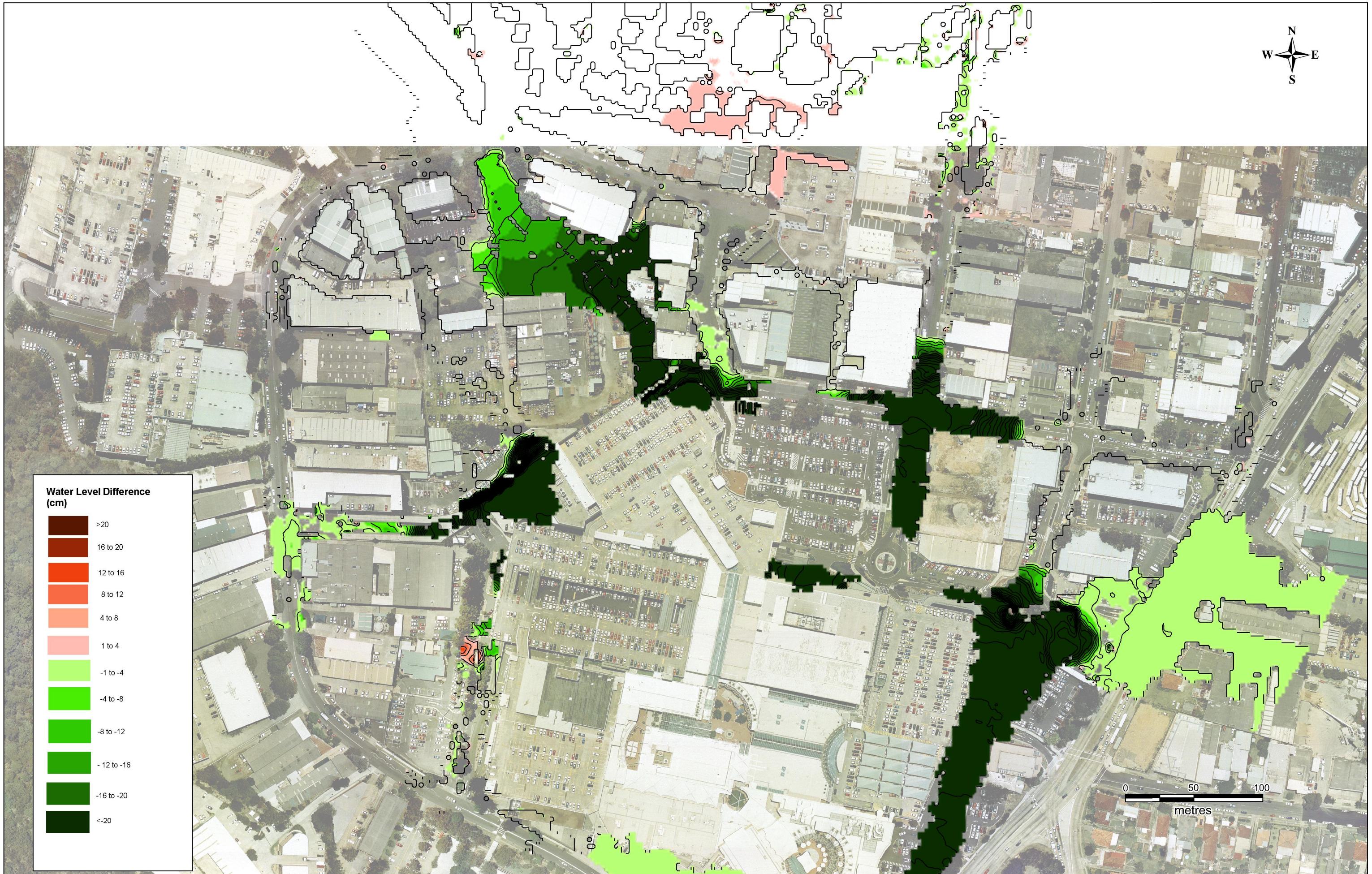
Updated Stormwater DA with Brookvale Creek Design

Table with columns: Survey Point, Floor Level (mAHD), Street Address, Flood Level (mAHD), Depth over Ground (cm), Depth over Floor (cm), Diff (cm), Max FL (mAHD), and various ARI (100yr 1.5h ARI) and DA (0% Blockage) data for Existing Conditions, Updated Stormwater DA, and Updated Stormwater DA with Brookvale Creek Design.











NA49913132 Construction Scenario A for Chamber C6

Existing Conditions

10 yr ARI Existing Conditions

10 yr ARI C6A

5 yr ARI Existing Conditions

5 yr ARI C6A

Run 2E

21 August 2013

3 December 2013

21 August 2013

3 December 2013

Survey Point	Ground (mAHD)	Floor Level (mAHD)	Street Address	100yr 1.5h ARI 50% Blockage			10yr 1.5h ARI Existing 50% Blockage			10yr 1.5h ARI C6A 50% Blockage			5 yr 1.5h ARI Existing 50% Blockage			5 yr 1.5h ARI C6A 50% Blockage					
				Flood Level (mAHD)	Depth over Ground (cm)	Depth over Floor (m)	Flood Level (mAHD)	Depth over Ground (cm)	Diff (cm)	Flood Level (mAHD)	Depth over Ground (cm)	Diff (cm)	Flood Level (mAHD)	Depth over Ground (cm)	Diff (cm)	Flood Level (mAHD)	Depth over Ground (cm)	Diff (cm)	Flood Level (mAHD)	Depth over Ground (cm)	Diff (cm)
				(a)	(b)	(c)	(d)	(e)	(f)	(g)	(h)	(i)	(j)	(k)	(l)	(m)	(n)	(o)	(p)	(q)	(r)
P1	19.37	19.43	15 Clearview Place	20.46	109	1.03	19.14	4	-132	19.14	4	-132	18.60	4	-186	18.60	4	-186			
P2	19.20	19.40	15 Clearview Place	20.35	115	0.95	19.24	4	-111	19.24	4	-111	19.24	4	-111	19.24	4	-111			
P3	19.19	20.23	14 Clearview Place	19.88	69	0.00	19.25	6	-63	19.25	6	-63	19.25	6	-63	19.25	6	-63			
P4	19.01	19.23	Unit 11/16-18 Clearview Place	19.10	9	0.00	18.88	9	-22	18.88	9	-22	18.88	9	-22	18.88	9	-22			
P5	19.08	19.25	Unit 10B/16-18 Clearview Place	19.10	2	0.00	18.96	7	-14	18.96	7	-14	18.96	7	-14	18.96	7	-14			
P6	19.20	19.30	Unit 10/16-18 Clearview Place	19.27	7	0.00	19.27	7	0	19.27	7	0	19.27	7	0	19.27	7	0			
P7	19.27	19.30	Unit 9/16-18 Clearview Place	19.38	11	0.08	19.38	11	0	19.38	11	0	19.38	11	0	19.38	11	0			
P8	19.24	19.30	Unit 8/16-18 Clearview Place	19.35	11	0.05	19.35	11	0	19.35	11	0	19.35	11	0	19.35	11	0			
P9	19.23	19.26	Unit 7A/16-18 Clearview Place	19.33	10	0.07	19.33	10	0	19.33	10	0	19.33	10	0	19.33	10	0			
P10	19.21	19.26	Unit 7/16-18 Clearview Place	19.31	10	0.05	19.31	10	0	19.31	10	0	19.31	10	0	19.31	10	0			
P11	19.26	19.26	Unit 6/16-18 Clearview Place	19.30	4	0.04	19.29	3	-1	19.29	3	-1	19.29	3	-1	19.29	3	-1			
P12	19.18	19.28	Unit 5/16-18 Clearview Place	19.30	12	0.02	19.29	11	-1	19.29	11	-1	19.29	11	-1	19.29	11	-1			
P13	19.19	19.29	Unit 4/16-18 Clearview Place	19.29	10	0.00	19.28	9	-1	19.28	9	-1	19.28	9	-1	19.28	9	-1			
P14	18.86	19.26	Unit 3/16-18 Clearview Place	19.29	43	0.03	19.28	42	-1	19.28	42	-1	19.28	42	-1	19.28	42	-1			
P15	18.59	19.27	Unit 2/16-18 Clearview Place	19.27	68	0.00	19.27	68	0	19.27	68	0	19.27	68	0	19.27	68	0			
P16	19.13	19.23	Unit 12/16-18 Clearview Place	19.31	18	0.08	19.30	17	-1	19.30	17	-1	19.30	17	-1	19.30	17	-1			
P17	19.24	19.27	Unit 13/16-18 Clearview Place	19.30	6	0.03	19.29	5	-1	19.29	5	-1	19.29	5	-1	19.29	5	-1			
P18	19.00	19.25	Unit 13/16-18 Clearview Place	19.08	8	0.00	18.94	0	-14	18.94	0	-14	18.94	0	-14	18.94	0	-14			
P19	19.19	19.26	Unit 12/16-18 Clearview Place	19.16	2	0.00	19.18	2	0	19.18	2	0	19.18	2	0	19.18	2	0			
P20	18.84	19.26	Unit 1 / 16-18 Clearview Place	19.02	18	0.00	18.79	0	-23	18.79	0	-23	18.79	0	-23	18.79	0	-23			
P21	16.41	16.51	Unit 1 / 16-18 Clearview Place	18.92	251	2.41	18.48	207	-44	18.48	207	-44	18.48	207	-44	18.48	207	-44			
P22	17.19	17.35	19 Clearview Place	18.25	106	0.90	17.53	34	-72	17.53	34	-72	17.43	24	-82	17.43	24	-82			
P23	16.88	18.21	19 Clearview Place	18.25	137	0.04	17.55	67	-70	17.55	67	-70	17.45	57	-80	17.45	57	-80			
P24	17.01	17.04	Unit 1/20 Clearview Place	18.25	124	1.21	17.50	49	-75	17.50	49	-75	17.41	40	-84	17.41	40	-84			
P25	16.60	16.69	21 Clearview Place	18.21	161	1.52	17.49	89	-72	17.49	89	-72	17.39	79	-82	17.39	79	-82			
P26	17.05	17.06	Unit 2/20 Clearview Place	17.98	93	0.92	17.37	32	-61	17.37	32	-61	17.29	24	-69	17.29	24	-69			
P27	18.14	18.21	19 Clearview Place	18.73	59	0.52	18.57	43	-16	18.57	43	-16	18.52	38	-21	18.52	38	-21			
P28	16.52	16.62	Unit 5/22 Clearview Place	17.60	108	0.98	17.05	53	-55	17.05	53	-55	16.94	42	-66	16.94	42	-66			
P29	16.54	16.58	Unit 4/22 Clearview Place	17.58	104	1.00	17.04	50	-54	17.04	50	-54	16.93	39	-65	16.93	39	-65			
P30	16.50	16.55	Unit 3/22 Clearview Place	17.58	108	1.03	17.04	54	-54	17.04	54	-54	16.93	43	-65	16.93	43	-65			
P31	16.61	16.63	21 Clearview Place	17.14	53	0.51	16.58	0	-56	16.58	0	-56	16.58	0	-56	16.58	0	-56			
P33	16.30	16.30	21 Clearview Place	17.14	84	0.84	16.27	0	-87	16.27	0	-87	16.27	0	-87	16.27	0	-87			
P34	17.70	19.04	21 Clearview Place	18.06	36	0.00	17.85	15	-21	17.85	15	-21	17.84	14	-22	17.84	14	-22			
P35	16.20	16.67	close to 20 Clearview Place	17.13	93	0.46	16.01	0	-112	16.01	0	-112	16.01	0	-112	16.01	0	-112			
P36	15.90	16.67	close to 20 Clearview Place	17.13	123	0.60	15.97	7	-116	15.97	7	-116	15.94	4	-119	15.94	4	-119			
P37	15.73	15.83	close to 20 Clearview Place	17.13	140	1.30	15.96	23	-117	15.96	23	-117	15.80	7	-133	15.80	7	-133			
P38	16.20	16.25	Unit 2 / 22 Clearview Place	17.40	120	1.15	16.93	73	-47	16.93	73	-47	16.83	63	-57	16.83	63	-57			
P39	16.48	16.57	Unit 1 / 22 Clearview Place	17.17	69	0.60	16.73	25	-44	16.73	25	-44	16.61	13	-56	16.61	13	-56			
P40	16.80	16.81	Clearview Place	17.20	40	0.39	16.86	6	-34	16.86	6	-34	16.82	2	-38	16.82	2	-38			
P41	16.75	16.90	Unit 1/84-85A Old Pittwater Road	16.83	8	0.00	16.77	2	-6	16.77	2	-6	16.77	2	-6	16.77	2	-6			
P42	16.96	17.17	82B Old Pittwater Road	16.98	2	0.00	16.98	2	0	16.98	2	0	16.95	0	-3	16.95	0	-3			
P43	16.60	16.65	Clearview Place	16.84	24	0.19	16.50	0	-34	16.50	0	-34	16.44	0	-40	16.44	0	-40			
P44	16.42	16.52	22A Clearview Place	17.02	60	0.50	16.63	21	-39	16.63	21	-39	16.53	11	-49	16.53	11	-49			
P45	16.73	17.01	22 Clearview Place	16.88	15	0.00	16.59	0	-29	16.59	0	-29	16.59	0	-29	16.59	0	-29			
P48	15.82	15.98	22B Clearview Place	16.84	102	0.86	16.46	64	-38	16.46	64	-38	16.38	56	-46	16.38	56	-46			
P50	16.03	16.21	98 Clearview Place	16.84	81	0.63	16.47	44	-37	16.47	44	-37	16.38	35	-46	16.38	35	-46			
P51	15.83	19.94	Old Pittwater Road	15.84	1	0.00	15.84	1	0	15.84	1	0	15.84	1	0	15.84	1	0			
P52	15.76	16.5	Old Pittwater Road	15.80	4	0.00	15.80	4	0	15.80	4	0	15.80	4	0	15.80	4	0			
P53	15.53	16.45	Old Pittwater Road	15.53	0	0.00	15.53	0	0	15.53	0	0	15.53	0	0	15.53	0	0			
P54	14.75	14.97	Old Pittwater Road	15.52	77	0.55	15.10	35	-42	15.10	35	-42	15.01	26	-51	15.01	26	-51			
P55	14.86	14.93	Old Pittwater Road	15.25	39	0.32	15.01	15	-24	15.01	15	-24	14.95	9	-30	14.95	9	-30			
P56	14.79	14.94	Old Pittwater Road	15.14	35	0.20	14.84	5	-30	14.84	5	-30	14.81	2	-33	14.81	2	-33			
P57	14.78	14.93	Old Pittwater Road	15.14	36	0.21	14.85	7	-29	14.85	7	-29	14.81	3	-33	14.81	3	-33			
P58	14.85	14.93	Old Pittwater Road	15.14	29	0.21	14.85	0	-29	14.85	0	-29	14.80	0	-34	14.80	0	-34			
P59	14.85	14.94	Old Pittwater Road	15.14	29	0.20	14.86	1	-28	14.86	1	-28	14.80	0	-34	14.80	0	-34			
P60	14.78	14.94	Old Pittwater Road	15.14	36	0.20	14.84	6	-30	14.84	6	-30	14.80	2	-34	14.80	2	-34			
P61	14.80	14.96	Old Pittwater Road	15.14	34	0.18	14.86	6	-28	14.86	6	-28	14.83	3	-31	14.84	4	-30			
P62	14.82	14.94	Old Pittwater Road	15.14	32	0.20	14.86	4	-28	14.86	4	-28	14.82	0	-32	14.82	0	-32			
P63	14.82	14.95	Old Pittwater Road	15.14	32	0.19	14.86	4	-28	14.86	4	-28	14.82	0	-32	14.82	0	-32			
P64	14.73	14.89	Suit2/98 Old Pittwater Road	15.14	41	0.25	14.85	12	-29	14.85	12	-29	14.64	0	-50	14.64	0	-50			
P65	14.72	14.89	Suit2/98 Old Pittwater Road	15.14	42	0.25	14.85	13	-29	14.85	13	-29	14.71	0	-43	14.71	0	-43			
P66	14.71	14.91	Suit1/98 Old Pittwater Road	15.14	43	0.23	14.85	14	-29	14.85	14	-29	14.70	0	-44	14.70	0	-44			
P67	14.77	14.91	Suit1/98 Old Pittwater Road	15.14	37	0.23	14.85	8	-29	14.85	8	-29	14.73	0	-41	14.73	0	-41			
P69	15.08	16.45	Unit 2/100 Old Pittwater Road	15.08	0	0.00	15.12	4	4	15.12	4	4	15.12	4	4	15.12	4	4			
P71	14.98	16.41	Unit 2/100 Old Pittwater Road	15.08	10	0.00	15.03	5	-5	15.03	5	-5	15.03	5	-5	15.03	5	-5			
P72	15.05	17.48	Unit 1/100 Old Pittwater Road	15.05	0	0.00	15.01	0	-4	15.01	0	-4	15.01	0	-4	15.01	0	-4			
P73	14.53	14.74	98 Old Pittwater Road	15.14	61	0.40	14.85	32	-29	14.85	32	-29	14.50	0	-64	14.50	0	-64			
P74	14.63	14.72	98 Old Pittwater Road	15.14	51	0.42	14.85	22	-29	14.85	22	-29	14.61	0	-53	14.61	0	-53			
P75	14.53	14.71	98 Old Pitt																		

Survey Point	Ground (mAHD)	Floor Level (mAHD)	Street Address	100yr 1.5h ARI 50% Blockage			10yr 1.5h ARI Existing 50% Blockage			10yr 1.5h ARI C6A 50% Blockage			5 yr 1.5h ARI Existing 50% Blockage			5 yr 1.5h ARI C6A 50% Blockage				
				Flood Level (mAHD)	Depth over Ground (cm)	Depth over Floor (m)	Flood Level (mAHD)	Depth over Ground (cm)	Diff (cm)	Flood Level (mAHD)	Depth over Ground (cm)	Diff (cm)	Flood Level (mAHD)	Depth over Ground (cm)	Diff (cm)	Flood Level (mAHD)	Depth over Ground (cm)	Diff (cm)	Flood Level (mAHD)	Depth over Ground (cm)
P155	11.87		Harrison Property	(a)			(g)	(g)-(a)	(h)	(h)-(g)	(h)-(a)	(i)	(i)-(a)	(j)	(j)-(i)	(j)-(a)				
P156	12.12			12.20	33		11.61	-59	11.64	3	-56	11.44	-76	11.47	3	-73				
P157	12.48			12.26	14		11.79	-47	11.80	1	-46	11.74	-52	11.75	1	-51				
P158	12.06			12.51	3		11.95	-56	11.97	2	-54	11.85	-66	11.86	1	-65				
P159	12.23			12.66	60		12.20	14	12.20	0	-46	12.08	2	12.08	2	0	-58			
P160	11.81			12.71	48		12.21	-50	12.21	0	-50	12.09	-62	12.09	0	-62				
P161	12.85			12.80	99		12.36	55	12.37	56	1	-43	12.31	50	12.31	50	0	-49		
P162	13.41			12.96	11		12.95	10	12.95	10	0	-1	12.95	10	12.95	10	0	-1		
P163	12.47		13.65	24		13.50	9	13.50	9	0	-15	13.50	9	13.50	9	0	-15			
			12.78	31		12.29	-49	12.29	0	-49	12.23	-55	12.23	0	-55					
P164	9.87		Driveway to Woolworth Loading Dock	10.96	109		9.98	11	9.98	12	1	-97	9.92	5	-104	9.92	5	0	-104	
P165	10.19			11.01	82		10.26	7	10.26	9	-73	10.20	1	10.20	1	-81	10.20	1	0	-81
P166	10.54			11.11	57		10.63	9	10.63	10	1	-47	10.58	4	-53	10.58	4	0	-53	
P167	10.66		Roundabout in Cross Street	11.34	68		10.83	17	10.84	18	1	-50	10.80	14	-54	10.80	14	0	-54	
P168	10.93			11.19	26		11.04	11	11.04	11	0	-15	11.02	9	-17	11.02	9	0	-17	
P169	10.39		Palm Tree Carpark	10.54	15		10.46	7	10.46	7	0	-8	10.45	6	-9	10.45	6	0	-9	
P170	9.88			10.15	27		9.99	11	9.99	11	0	-16	9.96	8	-19	9.97	9	1	-18	
P171	9.60			10.15	55		9.98	38	9.99	39	1	-16	9.96	36	-19	9.97	37	1	-18	
P172	9.52			10.15	63		9.98	46	9.99	47	1	-16	9.96	44	-19	9.97	45	1	-18	
P173	9.76			10.15	39		9.98	22	9.99	23	1	-16	9.96	20	-19	9.97	21	1	-18	
P174	10.59			10.74	15		10.65	6	10.65	6	0	-9	10.61	2	-13	10.61	2	0	-13	
P175	9.16			10.15	99		9.99	83	10.00	84	1	-15	9.96	80	-19	9.98	82	2	-17	
P176	9.09			10.15	106		9.99	90	9.99	90	0	-16	9.96	87	-19	9.97	88	1	-18	
P177	9.01		10.15	114		9.98	97	9.99	98	1	-16	9.96	95	-19	9.97	96	1	-18		
P178	8.95		Green Street	10.15	120		9.98	103	9.99	104	1	-16	9.96	101	-19	9.97	102	1	-18	
P179	8.85			10.15	130		9.98	113	9.99	114	1	-16	9.96	111	-19	9.97	112	1	-18	
P180	8.83			10.15	132		9.98	115	9.99	116	1	-16	9.96	113	-19	9.97	114	1	-18	
P181	9.35			10.15	80		9.98	63	9.99	64	1	-16	9.96	61	-19	9.97	62	1	-18	
P182	9.97			10.08	11		9.93	0	9.93	0	-15	9.93	-15	9.90	-3	-18				
P183	9.46			9.91	45		9.71	25	9.72	26	1	-19	9.67	21	-24	9.69	23	2	-22	
P184	9.25			9.83	58		9.64	39	9.66	41	2	-17	9.58	33	-25	9.59	34	1	-24	
P185	9.19			9.83	64		9.64	45	9.65	46	1	-18	9.57	38	-26	9.58	39	1	-25	
P186	8.95			9.83	88		9.63	68	9.65	70	2	-18	9.57	62	-26	9.58	63	1	-25	
P187	8.61			9.81	120		9.62	101	9.63	102	1	-18	9.56	95	-25	9.57	96	1	-24	
P188	8.39			9.77	138		9.60	121	9.61	122	1	-16	9.54	115	-23	9.55	116	1	-22	
P189	8.35			9.74	139		9.58	123	9.59	124	1	-15	9.52	117	-22	9.53	118	1	-21	
P190	8.99			9.63	64		9.49	50	9.50	51	1	-13	9.45	46	-18	9.46	47	1	-17	
P191	9.04		9.48	44		9.37	33	9.38	34	1	-10	9.33	29	-15	9.34	30	1	-14		
P192	10.08		Starfish Carpark Entrance	10.96	88		10.17	9	10.17	9	0	-79	10.17	9	-79	10.17	9	0	-79	
P193	10.99			11.00	1		11.00	1	11.00	1	0	0	11.00	1	0	11.00	1	0	0	
P194	11.06			11.07	1		11.07	1	11.07	1	0	0	11.07	1	0	11.07	1	0	0	
P192_1	10.05		Starfish Carpark	10.96	91		10.06	1	10.06	1	0	-90	10.06	1	-90	10.06	1	0	-90	
P193_1	10.81			10.96	15		10.80	-16	10.80	11	0	-16	10.80	-16	10.80	8	0	-10	-16	
P194_1	10.90			11.08	18		11.01	11	11.01	11	0	-7	10.98	8	-10	10.98	8	0	-10	
P217	9.50			10.96	146		9.80	30	9.83	33	3	-113	9.53	3	-143	9.53	3	0	-143	
P195	10.95			10.98	3		10.98	3	10.98	3	0	0	10.98	3	0	10.98	3	0	0	
P196	10.64			10.80	16		10.64	-16	10.64	0	-16	10.64	-16	10.64	-16	10.64	0	0	-16	
P197	10.22			10.80	-		10.22	-58	10.22	-	-58	10.22	-58	10.22	-58	10.22	-58	0	-58	
P198	10.51			10.80	-		10.51	-29	10.51	-	-29	10.51	-29	10.51	-29	10.51	-29	0	-29	
P199	10.53			10.80	27		10.60	7	10.60	7	0	-20	10.57	4	-23	10.57	4	0	-23	
P200	10.76			10.86	10		10.82	6	10.82	6	0	-4	10.82	6	-4	10.82	6	0	-4	
P201	10.77			10.83	6		10.78	1	10.78	1	0	-5	10.78	1	-5	10.78	1	0	-5	
P202	10.45		10.80	35		10.53	8	10.53	8	0	-27	10.51	6	-29	10.51	6	0	-29		
P203	10.43		10.80	-		10.39	-41	10.39	-	-41	10.39	-41	10.39	-41	10.39	-41	0	-41		
P204	5.07		Warringah Golf Course	5.78	71		5.66	59	5.66	59	0	-12	5.63	56	-15	5.63	56	0	-15	
P205	4.98			5.19	21		5.09	11	5.09	11	0	-10	5.07	9	-12	5.07	9	0	-12	
P206	3.42			4.18	76		4.07	65	4.07	65	0	-11	4.04	62	-14	4.04	62	0	-14	
P207	2.81			3.70	89		3.62	81	3.62	81	0	-8	3.60	79	-10	3.60	79	0	-10	
P208	1.13			3.38	225		3.31	218	3.30	217	-1	-8	3.29	216	-9	3.28	215	-1	-10	
P209	2.66			3.37	71		3.30	64	3.30	64	0	-7	3.28	62	-9	3.27	61	-1	-10	
P210	2.52			3.34	82		3.27	75	3.27	75	0	-7	3.25	73	-9	3.25	73	0	-9	
P211	1.55			2.90	135		2.85	130	2.85	130	0	-5	2.84	129	-6	2.84	129	0	-6	
P212	0.45			2.74	229		2.73	228	2.73	228	0	-1	2.73	228	-1	2.72	227	-1	-2	
P213	1.77			2.71	94		2.71	94	2.71	94	0	0	2.71	94	0	2.71	94	0	0	
P214	1.80			2.71	91		2.71	91	2.71	91	0	0	2.71	91	0	2.71	91	0	0	
P215	2.05			2.74	69		2.73	68	2.73	68	0	-1	2.72	67	-2	2.72	67	0	-2	
P216	1.63			2.71	108		2.71	108	2.71	108	0	0	2.71	108	0	2.71	108	0	0	
P300	10.68	11.12		4 Cross St	10.78	10		10.68	-10	10.68	0	-10	10.68	-10	10.68	0	-10	10.68	0	-10
P301	9.22	10.12	2 Cross St	10.15	93	3	9.99	77	10.00	78	1	-15	9.96	74	-19	9.98	76	2	-17	
P302	9.29	10.08	(Cross St)	10.15	86	7	9.99	70	10.00	71	1	-15	9.96	67	-19	9.98	69	2	-17	
P303	10.27	10.45	Cross St	10.37	10		10.36	9	10.36	9	0	-1	10.34	7	-3	10.34	7	0	-3	
P304	10.86	10.87	2 Dale St	10.95	9	8	10.90	4	10.90	4	0	-5	10.89	3	-6	10.89	3	0	-6	
P305	9.24	10.08	26 Green St	10.15	91	7	9.99	75	10.00	76	1	-15	9.97	73	-18	9.98	74	1	-17	
P306	9.65	10.65	24 Green St	10.15	50		9.99	34	10.00	35	1	-15	9.97	32	-18	9.98	33	1	-17	
P307	10.13	11.55	20 Green St	10.27	14		10.23	10	10.23	10	0	-4	10.23	10	-4	10.23	10	0	-4	
221	9.2		DJ Loading Dock	10.24	104		9.33	13	9.41	21	8	-83	9.25	5	-99	9.30	10	5	-94	
222	9.1		DJ Loading Dock	10.21	111		9.33	23	9.41	31	8	-80	9.25	15	-96	9.30	20	5	-91	
401			Bus Depot Sucatchment east of Pittwater Rd	11.07			10.96	-11	10.96	0	-11	10.92	-15	10.92	0	-15	10.92	0	-15	
402			Bus Depot Sucatchment east of Pittwater Rd	11.06			10.96	-10	10.96	0	-10	10.92	-14	10.92	0	-14	10.92	0	-14	
403			Bus Depot Sucatchment east of Pittwater Rd	11.06			10.96	-10	10.96	0	-10	10.92	-14	10.92	0	-14	10.92	0	-14	
404			Bus Depot Sucatchment east of Pittwater Rd	11.06			10.96	-10	10.96	0	-10	10.92	-14	10.92						

Model done 21Aug 2013

Model done 3Dec 2013

Model done 21Aug 2013

Model done 3Dec 2013

NA49913132 Construction Scenario B for Chamber C6

Existing Conditions

10 yr ARI Existing Conditions

10 yr ARI C6B

5 yr ARI Existing Conditions

5 yr ARI C6B

Run 2E

21 August 2013

3 December 2013

21 August 2013

3 December 2013

Table with columns: Survey Point, Ground (mAHD), Floor Level (mAHD), Street Address, Flood Level (mAHD), Depth over Ground (cm), Depth over Floor (m), Diff (cm), and multiple columns for 100yr 1.5h ARI 50% Blockage, 10yr 1.5h ARI Existing 50% Blockage, 10yr 1.5h ARI C6B 50% Blockage, 5 yr 1.5h ARI Existing 50% Blockage, and 5 yr 1.5h ARI C6B 50% Blockage.

Survey Point	Ground (mAHD)	Floor Level (mAHD)	Street Address	100yr 1.5h ARI 50% Blockage			10yr 1.5h ARI Existing 50% Blockage			10yr 1.5h ARI C6B 50% Blockage			5 yr 1.5h ARI Existing 50% Blockage			5 yr 1.5h ARI C6B 50% Blockage			
				Flood Level (mAHD)	Depth over Ground (cm)	Depth over Floor (m)	Flood Level (mAHD)	Depth over Ground (cm)	Diff (cm)	Flood Level (mAHD)	Depth over Ground (cm)	Diff (cm)	Diff (cm)	Flood Level (mAHD)	Depth over Ground (cm)	Diff (cm)	Flood Level (mAHD)	Depth over Ground (cm)	Diff (cm)
P155	11.87		Harrison Property	(a)			(g)	(g)-(a)	(h)	(h)-(g)	(hh)-(a)	(i)	(i)-(a)	(j)	(j)-(i)	(jj)-(a)			
P156	12.12			12.20	33		11.61	-59	11.75	14	-45	11.44	-76	11.58	14	-62			
P157	12.48			12.26	14		11.79	-47	11.83	4	-43	11.74	-52	11.78	4	-48			
P158	12.06			12.51	3		11.95	-56	12.04	9	-47	11.85	-66	11.90	5	-61			
P159	12.23			12.66	60		12.20	14	12.21	1	-45	12.08	2	12.08	2	0	-58		
P160	11.81			12.71	48		12.21	-50	12.23	2	-48	12.09	2	12.09	2	0	-62		
P161	12.85			12.80	99		12.36	55	12.38	57	-42	12.31	50	12.31	50	0	-49		
P162	13.41			12.96	11		12.95	10	12.95	10	0	12.95	10	12.95	10	0	-1		
P163	12.47			13.65	24		13.50	9	13.50	9	0	13.50	9	13.50	9	0	-15		
P164	9.87		Driveway to Woolworth Loading Dock	12.78	31		12.29	-49	12.30	1	-48	12.23	9	12.23	9	0	-55		
P165	10.19			10.96	109		9.98	11	10.09	22	11	-87	9.92	5	10.00	8	3	-101	
P166	10.54			11.01	82		10.26	7	10.33	14	7	-68	10.20	1	10.25	6	5	-76	
P167	10.66		Roundabout in Cross Street	11.11	57		10.63	9	10.63	9	-48	10.58	4	10.60	6	2	-51		
P168	10.93			11.34	68		10.83	17	10.83	27	10	-41	10.80	14	10.81	15	1	-53	
P169	10.39		Palm Tree Carpark	11.19	26		11.04	11	11.04	11	-15	11.02	9	11.02	9	0	-17		
P170	9.88			10.54	15		10.46	7	10.46	7	0	-8	10.45	6	10.45	6	0	-9	
P171	9.60			10.15	27		9.99	11	10.02	14	3	-13	9.96	8	10.00	12	4	-15	
P172	9.52			10.15	55		9.98	38	10.01	41	3	-14	9.96	36	10.00	40	4	-15	
P173	9.76			10.15	63		9.98	46	10.01	49	3	-14	9.96	44	10.00	48	4	-15	
P174	10.59			10.15	39		9.98	22	10.01	25	3	-14	9.96	20	10.00	24	4	-15	
P175	9.16			10.74	15		10.65	6	10.65	6	0	-9	10.61	2	10.61	2	0	-13	
P176	9.09			10.15	99		9.99	83	10.01	85	2	-14	9.96	80	10.00	84	4	-15	
P177	9.01			10.15	106		9.99	90	10.01	92	2	-14	9.96	87	10.00	91	4	-15	
P178	8.95			10.15	114		9.98	97	10.01	100	3	-14	9.96	95	10.00	99	4	-15	
P178	8.95		Green Street	10.15	120		9.98	103	10.01	106	3	-14	9.96	101	10.00	105	4	-15	
P179	8.85			10.15	130		9.98	113	10.01	116	3	-14	9.96	111	10.00	115	4	-15	
P180	8.83			10.15	132		9.98	115	10.01	118	3	-14	9.96	113	10.00	117	4	-15	
P181	9.35			10.15	80		9.98	63	10.01	66	3	-14	9.96	61	10.00	65	4	-15	
P182	9.97			10.08	11		9.93	0	9.93	0	-15	9.93	0	9.93	0	0	-15		
P183	9.46			9.91	45		9.71	25	9.74	28	3	-17	9.67	21	9.72	26	5	-19	
P184	9.25			9.83	58		9.64	39	9.68	43	4	-15	9.58	33	9.62	37	4	-21	
P185	9.19			9.83	64		9.64	45	9.68	49	4	-15	9.57	38	9.62	43	5	-21	
P186	8.95			9.83	88		9.63	68	9.67	72	4	-16	9.57	62	9.61	66	4	-22	
P187	8.61			9.81	120		9.62	101	9.66	105	4	-15	9.56	95	9.60	99	4	-21	
P188	8.39			9.77	138		9.60	121	9.63	124	3	-14	9.54	115	9.58	119	4	-19	
P189	8.35			9.74	139		9.58	123	9.61	126	3	-13	9.52	117	9.56	121	4	-18	
P190	8.99			9.63	64		9.49	50	9.52	53	3	-11	9.45	46	9.48	49	3	-15	
P191	9.04			9.48	44		9.37	33	9.39	35	2	-9	9.33	29	9.36	32	3	-12	
P192	10.08			Starfish Carpark Entrance	10.96	88		10.17	9	10.17	9	0	-79	10.17	9	10.17	9	0	-79
P193	10.99		11.00		1		11.00	1	11.00	1	0	0	11.00	1	11.00	1	0	0	
P194	11.06		11.07		1		11.07	1	11.07	1	0	0	11.07	1	11.07	1	0	0	
P192_1	10.05		Starfish Carpark	10.96	91		10.06	1	10.08	3	2	-88	10.06	1	10.06	1	0	-80	
P193_1	10.81			10.96	15		10.80	0	10.80	0	-16	10.80	0	10.80	0	0	-16		
P194_1	10.90			11.08	18		11.01	11	11.01	11	0	-7	10.98	8	10.98	8	0	-10	
P217	9.50			10.96	146		9.80	30	9.83	33	3	-113	9.53	3	9.65	15	12	-131	
P195	10.95			10.98	3		10.98	3	10.98	3	0	0	10.98	3	10.98	3	0	0	
P196	10.64			10.80	16		10.64	0	10.64	0	-16	10.64	0	10.64	0	0	-16		
P197	10.22			10.80	-		10.22	-58	10.22	-58	-58	-58	10.22	-58	10.22	-58	-58		
P198	10.51			10.80	-		10.51	-29	10.51	-29	-29	-29	10.51	-29	10.51	-29	-29		
P199	10.53			10.80	27		10.60	7	10.60	7	0	-20	10.57	4	10.58	5	1	-22	
P200	10.76			10.86	10		10.82	6	10.82	6	0	-4	10.82	6	10.82	6	0	-4	
P201	10.77			10.83	6		10.78	1	10.78	1	0	-5	10.78	1	10.78	1	0	-5	
P202	10.45		10.80	35		10.53	8	10.52	7	-1	-28	10.51	6	10.50	5	-1	-30		
P203	10.43		10.80	-		10.39	-41	10.39	-41	-41	-41	10.39	-41	10.39	-41	-41			
P204	5.07		Warringah Golf Course	5.78	71		5.66	59	5.64	57	-2	-14	5.63	56	5.62	55	-1	-16	
P205	4.98			5.19	21		5.09	11	5.08	10	-1	-11	5.07	9	5.06	8	-1	-13	
P206	3.42			4.18	76		4.07	65	4.05	63	-2	-13	4.04	62	4.02	60	-2	-16	
P207	2.81			3.70	89		3.62	81	3.61	80	-1	-9	3.60	79	3.58	77	-2	-12	
P208	1.13			3.38	225		3.31	218	3.29	216	-2	-9	3.29	216	3.27	214	-2	-11	
P209	2.66			3.37	71		3.30	64	3.28	62	-2	-9	3.28	62	3.26	60	-2	-11	
P210	2.52			3.34	82		3.27	75	3.25	73	-2	-9	3.25	73	3.24	72	-1	-10	
P211	1.55			2.90	135		2.85	130	2.84	129	-1	-6	2.84	129	2.83	128	-1	-7	
P212	0.45			2.74	229		2.73	228	2.73	228	0	-1	2.73	228	2.72	227	-1	-2	
P213	1.77			2.71	94		2.71	94	2.71	94	0	0	2.71	94	2.71	94	0	0	
P214	1.80			2.71	91		2.71	91	2.71	91	0	0	2.71	91	2.71	91	0	0	
P215	2.05			2.74	69		2.73	68	2.72	67	-1	-2	2.72	67	2.72	67	0	-2	
P216	1.63			2.71	108		2.71	108	2.71	108	0	0	2.71	108	2.71	108	0	0	
P300	10.68	11.12		4 Cross St	10.78	10		10.68	-10	10.68	0	-10	10.68	-10	10.68	0	-10		
P301	9.22	10.12		2 Cross St	10.15	93	3	9.99	77	10.01	79	2	-14	9.96	74	10.00	78	4	-15
P302	9.29	10.08		(Cross St)	10.15	86	7	9.99	70	10.01	72	2	-14	9.96	67	10.00	71	4	-15
P303	10.27	10.45	Cross St	10.37	10		10.36	9	10.36	9	0	-1	10.34	7	10.34	7	0	-3	
P304	10.86	10.87	2 Dale St	10.95	9	8	10.90	4	10.90	4	0	-5	10.89	3	10.89	3	0	-6	
P305	9.24	10.08	26 Green St	10.15	91	7	9.99	75	10.01	77	2	-14	9.97	73	10.00	76	3	-15	
P306	9.65	10.65	24 Green St	10.15	50		9.99	34	10.01	36	2	-14	9.97	32	10.00	35	3	-15	
P307	10.13	11.55	20 Green St	10.27	14		10.23	10	10.23	10	0	-4	10.23	10	10				