# WHALE BEACH NEIGHBOURHOOD CENTRE 231 WHALE BEACH ROAD, WHALE BEACH, NSW 2107 **DEVELOPMENT APPLICATION** STORMWATER CONCEPT PLAN



R	REV	ISIONS	AMENDMENTS		REV	ISIONS	/ AMENDMENTS	
	Rev	Date	Description	Verified	Rev	Date	Description	Verified
7	P1	18.12.19	ISSUED FOR DA	J.S.				
	P2	13.03.20	ISSUED FOR DA	J.S.				
						NORTH	All dimensions to be verifie commencement of on-site prefabrication. Figured dim preference to scaled dimen copyright and remains the Consulting Engineers. Rep part of these drawings with constitutes an infringement	work and/ or off-site ension to be taken in isions. This drawing is property of JHA roduction in whole or out written consent
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NORTHERN BEACHES COUNCIL

CLIENT L. CASSAR

**RICHARD COLE** ARCHITECTURE

ARCHITECT



PROJECT WHALE BEACH **NEIGHBOURHOOD DEVELOPMENT APPLICATION** 231 WHALE BEACH ROAD, WHALE BEACH, NSW

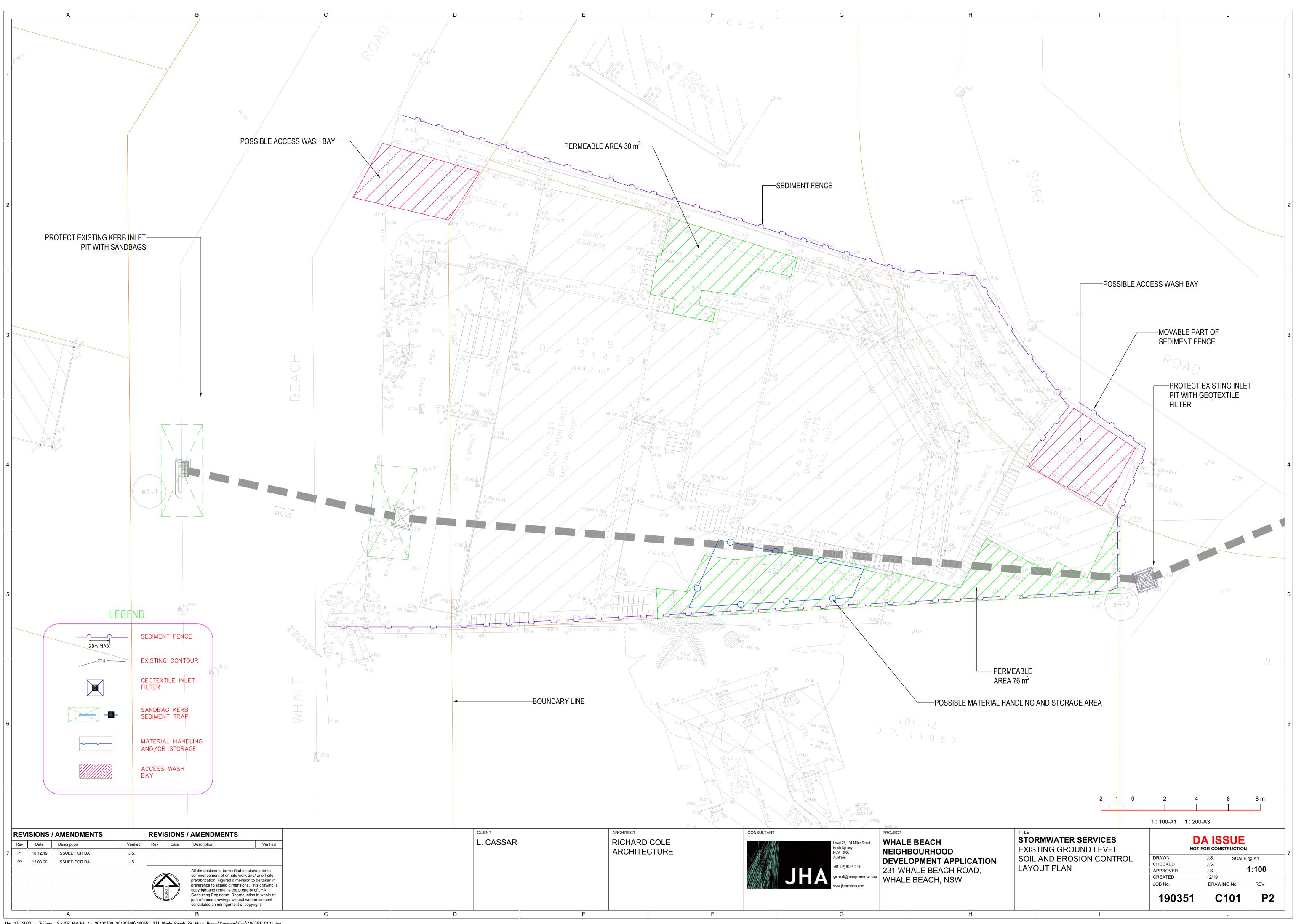
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TITLE
STORMWATER SERVICES
COVER SHEET

	A ISS			
DRAWN	J.S.	SCALE @	A1	1 '
CHECKED	J.S.			
APPROVED	J.S.		ITS	
CREATED	12/19			
JOB No.	DRAWING	G No.	REV	
190351	CO	00	P2	

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Mar 13, 2020 - 3:55pm S:\JOB No\Job No 20190300-20190399\190351 231 Whale Beach Rd Whale Beach\Drawings\Civil\190351\_C101.dwg

SEDIMENT	&	EROSION	CONTROL	NOTES

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- THE CONTRACTOR SHALL IMPLEMENT ALL SOIL EROSION AND SEDIMENT CONTROL MEASURES PRIOR TO THE COMMENCEMENT OF ANY WORKS BEING CARRIED OUT. ALL SOIL AND EROSION MEASURES SHALL BE MAINTAINED AND KEPT IN PLACE FOR THE FULL DURATION OF THE WORKS AND SHALL ONLY BE REMOVED AT FINAL STABILISATION OF THE WORKS. WHERE IT IS NECESSARY TO UNDERTAKE STRIPPING IN ORDER TO CONSTRUCT A SEDIMENT CONTROL DEVICE ONLY SUFFICIENT GROUND SHALL BE STRIPPED TO ALLOW CONSTRUCTION.
- 2. ALL SOIL EROSION AND SEDIMENT CONTROL MEASURES SHALL BE CONSTRUCTED AND MAINTAINED AS INDICATED ON THESE DRAWINGS. LOCATION AND EXTENT OF SOIL AND WATER MANAGEMENT DEVICES IS DIAGRAMMATIC ONLY AND THE ACTUAL REQUIREMENTS SHALL BE CONFIRMED ON SITE PRIOR TO COMMENCEMENT.
- 3. CONFORMITY WITH THIS PLAN SHALL IN NO WAY REDUCE THE RESPONSIBILITY OF THE CONTRACTOR TO PROTECT AGAINST WATER DAMAGE DURING THE COURSE OF THE CONTRACT. IT SHALL BE THE CONTRACTORS RESPONSIBILITY TO ENSURE THAT ANY NECESSARY CONTROL IS IN PLACE EVEN THOUGH SUCH CONTROL MAY NOT BE SHOWN ON THE PLAN.
- 4. THE CONTRACTOR SHALL INFORM ALL SUBCONTRACTORS AND ALL EMPLOYEES OF THEIR RESPONSIBILITIES IN MINIMISING THE POTENTIAL FOR SOIL EROSION AND POLLUTION TO DOWNSTREAM AREAS
- 5. APART FROM SEDIMENT BASINS, THE CONTRACTOR SHALL REGULARLY MAINTAIN SEDIMENT AND EROSION CONTROL STRUCTURES AND DESILT SUCH STRUCTURES PRIOR TO THE REDUCTION IN CAPACITY OF 30% DUE TO ACCUMULATED SEDIMENT. THE SEDIMENT SHALL BE DISPOSED OF ON SITE IN A MANNER APPROVED BY THE ENGINEER.
- THE CONTRACTOR SHALL TEMPORARILY REHABILITATE WITHIN TEN (10) DAYS ANY DISTURBED AREAS PROVIDING A MINIMUM 60% COVER. FINAL REHABILITATION IS TO BE PROVIDED WITHIN A FURTHER 60 DAYS WITH A MINIMUM 70% COVER.
- 8. THE CONTRACTOR SHALL PROVIDE WATERING OF THE VEGETATED BATTERS FOR MAINTENANCE PERIOD. PLANT, MACHINERY AND VEHICLES SHALL NOT BE DRIVEN OVER GRASSED AREAS UNLESS ON AN APPROVED HAULAGE ROUTE.
- 9. ALL DRAINAGE WORKS SHALL BE CONSTRUCTED AND STABILISED AS QUICKLY AS POSSIBLE TO MINIMISE RISK OF EROSION.
- 10. SITE ACCESS SHALL BE RESTRICTED TO THE NOMINATED POINTS. THE CONTRACTOR SHALL PROVIDE STABILISED SITE ACCESS.
- 11. DUST AND SITE DISTURBANCE MUST BE KEPT TO A MINIMUM. DURING WINDY WEATHER, LARGE, UNPROTECTED AREAS MUST BE KEPT MOIST (NOT WET) BY SPRINKLING WITH WATER TO REDUCE WIND EROSION. ERECT BARRIER FENCING TO MINIMISE LAND DISTURBANCE BY PREVENTING VEHICULAR AND PEDESTRIAN ACCESS TO AREAS BEING REHABILATATED AND LANDS THAT DO NOT NEED TO BE DISTURBED BY THIS PROJECT.
- 12. STOCKPILE TOPSOILS, SUBSOILS AND OTHER MATERIALS SEPARATELY.
- 13. TOPSOIL SHALL BE STORED IN LOW MOUNDS NO MORE THAN 2 METRES HIGH AND RE-USED WITHIN TWO MONTHS TO MAINTAIN ACTIVE POPULATIONS OF BENEFICIAL SOIL MICROBES AND SEED.
- 14. PLACE ALL STOCKPILES AT LEAST FIVE METRES FROM AREAS OF LIKELY CONCENTRATED OR HIGH VELOCITY FLOWS, ESPECIALLY EARTH BANKS AND ROADS. IF NECESSARY, EARTH BANKS OR DRAINS WILL BE CONSTRUCTED TO DIVERT LOCALISED RUN-ON.
- 15. TURN TOPSOIL STOCKPILES OVER TO AERATE THEM AT MONTHLY INTERVALS. ENSURE VEGETATION IS NOT INCORPORATED INTO THE SOIL.
- 16. AVOID REVERSING THE SOIL PROFILE MATERIALS DURING FILL OPERATIONS -REPLACE DISTURBED SOILS IN THEIR ORIGINAL ORDER.

17. ON COMPLETION OF MAJOR EARTHWORKS AND BEFORE ADDING TOPSOIL DISTURBED LANDS WITH A LOOSE SURFACE. ALTERNATELY, DISTURBED A PREVIOUSLY COMPACTED BY CONSTRUCTION WORKS WILL BE RIPPED TO 200-MM ALONG THE CONTOUR BEFORE APPLYING TOPSOIL

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- 18. PROVIDING MATERIALS ARE AVAILABLE, SPREAD TOPSOIL TO A MINIMUM I 75mm IN REVEGETATION AREAS ON SLOPES OF 4(H):1(V) OR LESS AND TO OF 40 TO 60mm IN REVEGETATION AREAS STEEPER THAN 4:1.
- 19. LEAVE TOPSOIL IN A SCARIFIED OR ROUGH CONDITION ONCE REPLACED THELP MOISTURE INFILTRATION AND REDUCE SOIL EROSION.
- 20. ENSURE SOIL IS THOROUGHLY SOAKED TO A DEPTH OF 75mm (RAIN OR IRRIGATION) IMMEDIATELY BEFORE PLANTING.
- 21. HANDLE TOPSOIL ONLY WHEN IT IS MOIST (NOT WET OR DRY) TO AVOID DECLINE OF SOIL STRUCTURE
- 22. SEDIMENT BASINS SHALL BE MAINTAINED FOR THE ENTIRE DURATION OF PROJECT OR UNTIL SUCH TIME AS ALL DISTURBED AREAS ARE HYDROMULCHED.
- 23. WHERE FLOCCULATION OF BASINS IS REQUIRED UNLESS OTHERWISE SP THE RECOMMENDED INITIAL DOSING IS 30KG OF GYPSUM PER 100 CUBIC METRES OF BASIN VOLUME. THE CONTRACTOR MAY VARY THIS RATE SUE TO TESTING OF PREVIOUS WATER SAMPLES AND THE ACHIEVEMENTS OF REQUIRED WATER QUALITY STANDARDS.
- 24. ANY DAMS TO BE DESILTED SHALL BE FLOCCULATED TO SETTLE ANY SUSPENDED SOLIDS CLEAR WATER SHALL THEN BE PUMPED OUT IN A MANNER THAT WILL NOT CAUSE DOWNSTREAM EROSION. THE DAM WALL SHALL THEN BE BREACHED AND ANY SILT REMOVED AND PLACED IN A SUITABLY CONSTRUCTED DRYING BASIN. WHEN DRY, THE SILT SHALL BE REMOVED FROM SITE OR MIXED WITH TOP SOIL FOR FUTURE SPREADING
- 25. THE CONTRACTOR SHALL MAINTAIN A LOG BOOK DETAILING: - RECORDS OF ALL RAINFALL
- CONDITION OF SOIL AND WATER MANAGEMENT STRUCTURES
   ANY APPLICATION OF FLOCCULATING AGENTS TO SEDIMENT BASIN
   VOLUMES OF ALL WATER DISCHARGED FROM SEDIMENT BASINS
   ANY ADDITIONAL REMEDIAL WORKS REQUIRED.
- 26. THE LOG BOOK SHALL BE MAINTAINED ON A WEEKLY BASIS AND BE MADE AVAILABLE TO ANY AUTHORISED PERSON UPON REQUEST. THE ORIGINAL BOOK SHALL BE ISSUED TO THE PROJECT MANAGER AT THE COMPLETION WORKS
- 27. ALL ROAD EMBANKMENTS TO BE STABILISED AS PER LANDSCAPE ARCHIT DETAILS.
- 28. A SELF AUDITING PROGRAM SHOULD BE ESTABLISHED BASED ON A CHEC SHEET DEVELOPED FOR THE SITE. A SITE INSPECTION USING THE CHECK SHEET SHOULD BE MADE BY THE SITE MANAGER AT LEAST WEEKLY, IMMEDIATELY BEFORE SITE CLOSURE AND IMMEDIATELY FOLLOWING RAIL EVENTS THAT CAUSE RUNOFF.
- 29. UNDERTAKE THE SELF AUDIT BY:
- WALKING AROUND THE SITE SYSTEMATICALLY (E.G. CLOCKWISE) - RECORDING THE CONDITION OF EVERY BMP EMPLOYED
- RECORDING MAINTENANCE REQUIREMENTS (IF ANY) FOR EACH BMP
- RECORDING THE VOLUMES OF SEDIMENT REMOVED FROM THE SEDIMEN
   RETENTION SYSTEMS WHERE APPLICABLE
- RECORDING THE SITE WHERE SEDIMENT IS DISPOSED
- FORWARDING A SIGNED DUPLICATE OF THE COMPLETED CHECK SHEET PROJECT MANAGER/DEVELOPER/SITE OPERATOR FOR THEIR INFORMAT

WIRE OR STEEL MESH-LOCALISED CHANNEL DEPRESSION TO INTERCEPT FLOWS-MA BE DISTURBED AREA DIRECTION OF FLOW MAX 0.6m FENCE HEIGHT 0.2m EMBEDMENT OF **GROUND SURF** FILTER FABRIC **TEMPORARY SEDIMENT FENCE** NOT TO SCALE BUILDER TO COORDINATE APPROPRIATE CONSTRUCTION SEQUENCE WITH CONSIDERA FOR MATERIAL STORAGE AND ANTICIPATED SEDIMENT MOVEMENT DURING CONSTRUCTION

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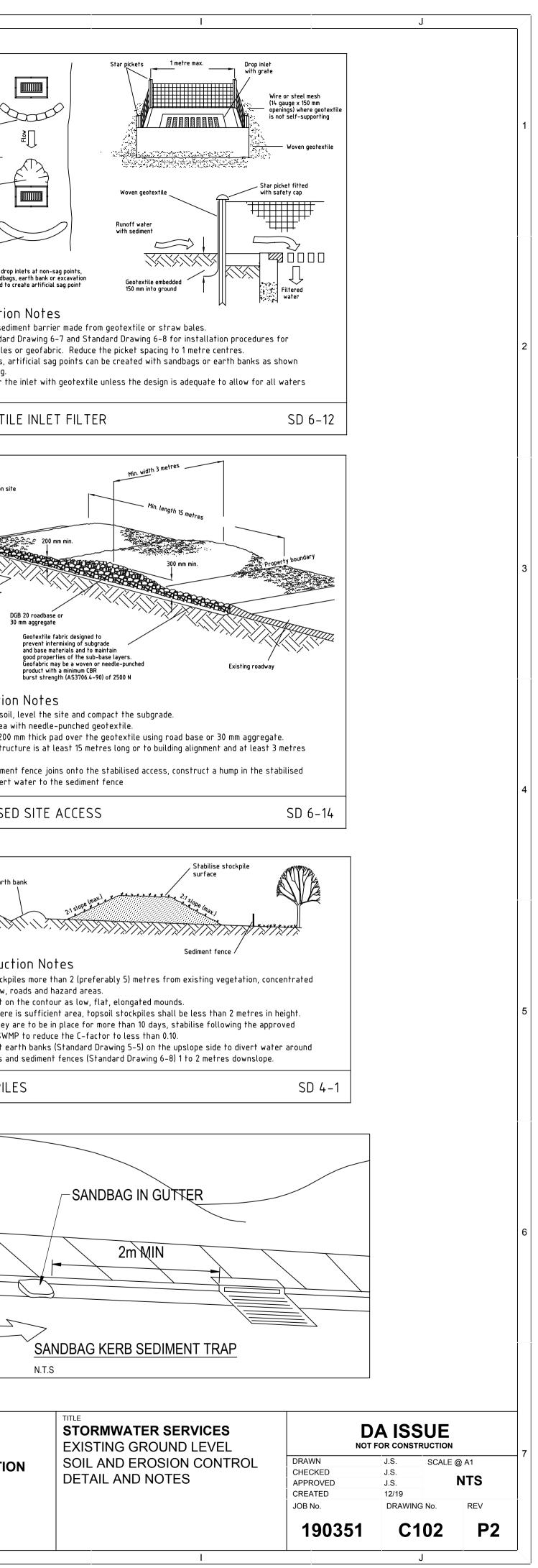
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M DEPTH OF TO A DEPTH	- ALL DISCH - CONTROL	ARGE POINTS, TO ASSESS WHETHER THE ERO MEASURES ARE EFFECTIVE IN PREVENTING IM IG WATERS			Excavation
ED TO	MANAGER A	ECTION USING THE CHECK SHEET WILL BE MAI T LEAST WEEKLY, IMMEDIATELY BEFORE SITE Y FOLLOWING RAINFALL EVENTS GREATER TH	CLOSURE, AND		
R					Earth bank For drop sandbags used to c
					Constructio
OF THE SPECIFIED SUBJECT OF THE					<ol> <li>Fabricate a sedir</li> <li>Follow Standard the straw bales</li> <li>In waterways, ar in the drawing.</li> <li>Do not cover the to bypass it.</li> </ol>
					GEOTEXTIL
LL BE NG.					Construction site
DE IAL LOG ION OF HITECTS					Runoff directed to sediment trap/fence D 3
ECK CK RAINFALL					Construction 1. Strip the topsoil, 2. Cover the area w 3. Construct a 200 4. Ensure the struc wide.
IENT					5. Where a sedimen access to divert
ET TO THE ATION					STABILISEI
					Earth I
MAX 3m BETWE					Construct
BETWEEN POST		GEOTEXTILE FILTER			1. Place stockpi water flow, r 2. Construct on 3. Where there 4. Where they a ESCP or SWM 5. Construct ea stockpiles an
	VHERE REQUIRED: SF	PLICE FILTER FABRIC AT POST	F WITH MIN. 150 OVERLAP		
FACE	UND	DISTURBED AREA			
RATION					RUNOFF
ION					
CASSAR		ARCHITECT RICHARD COLE ARCHITECTURE	CONSULTANT	Level 23, 101 Miller Street, North Sydney NSW 2060 Australia +61 (02) 9437 1000 general@jhaengineers.com.au www.jhaservices.com	PROJECT WHALE BEACH NEIGHBOURHOOD DEVELOPMENT APPLICATIO 231 WHALE BEACH ROAD, WHALE BEACH, NSW

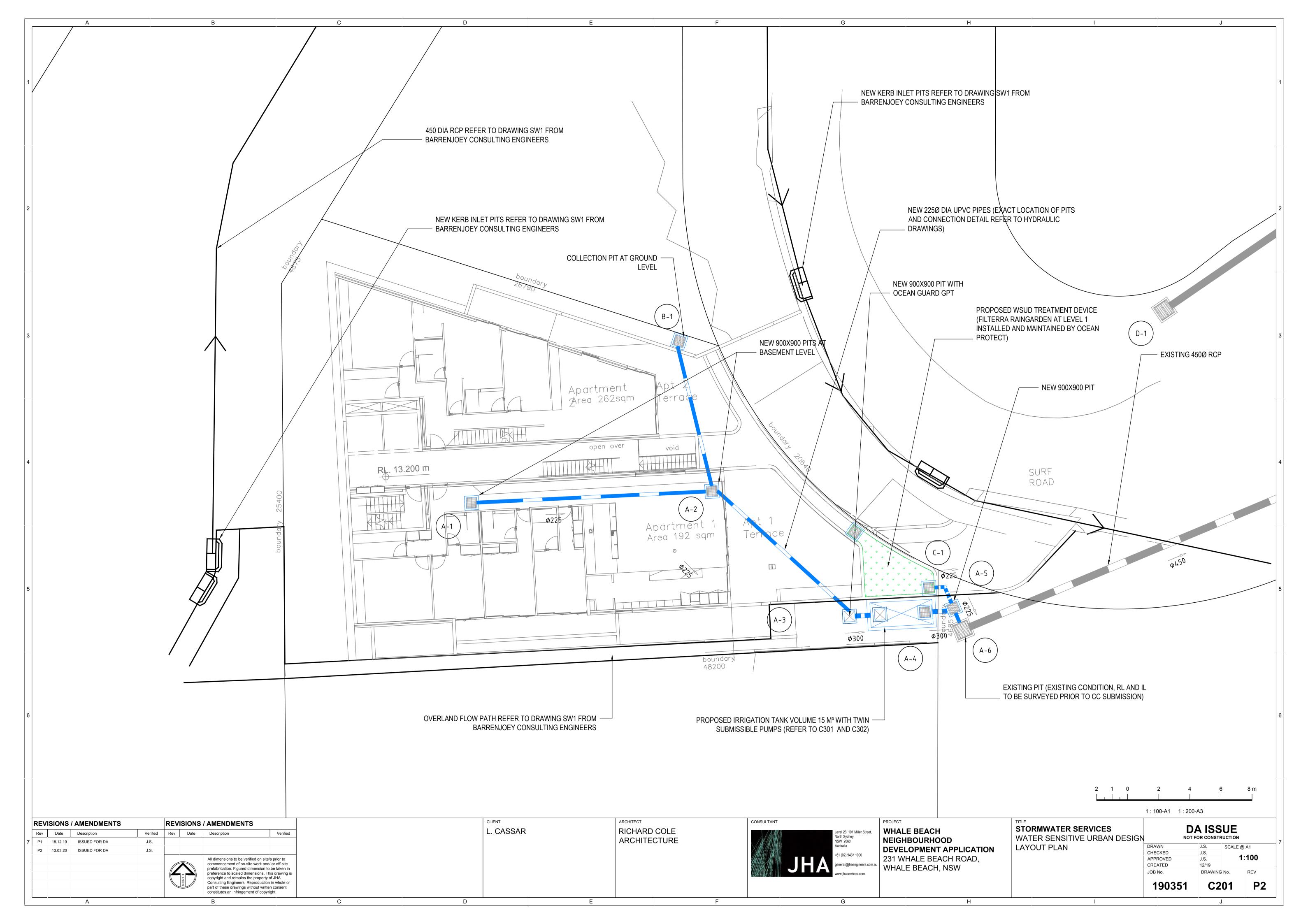
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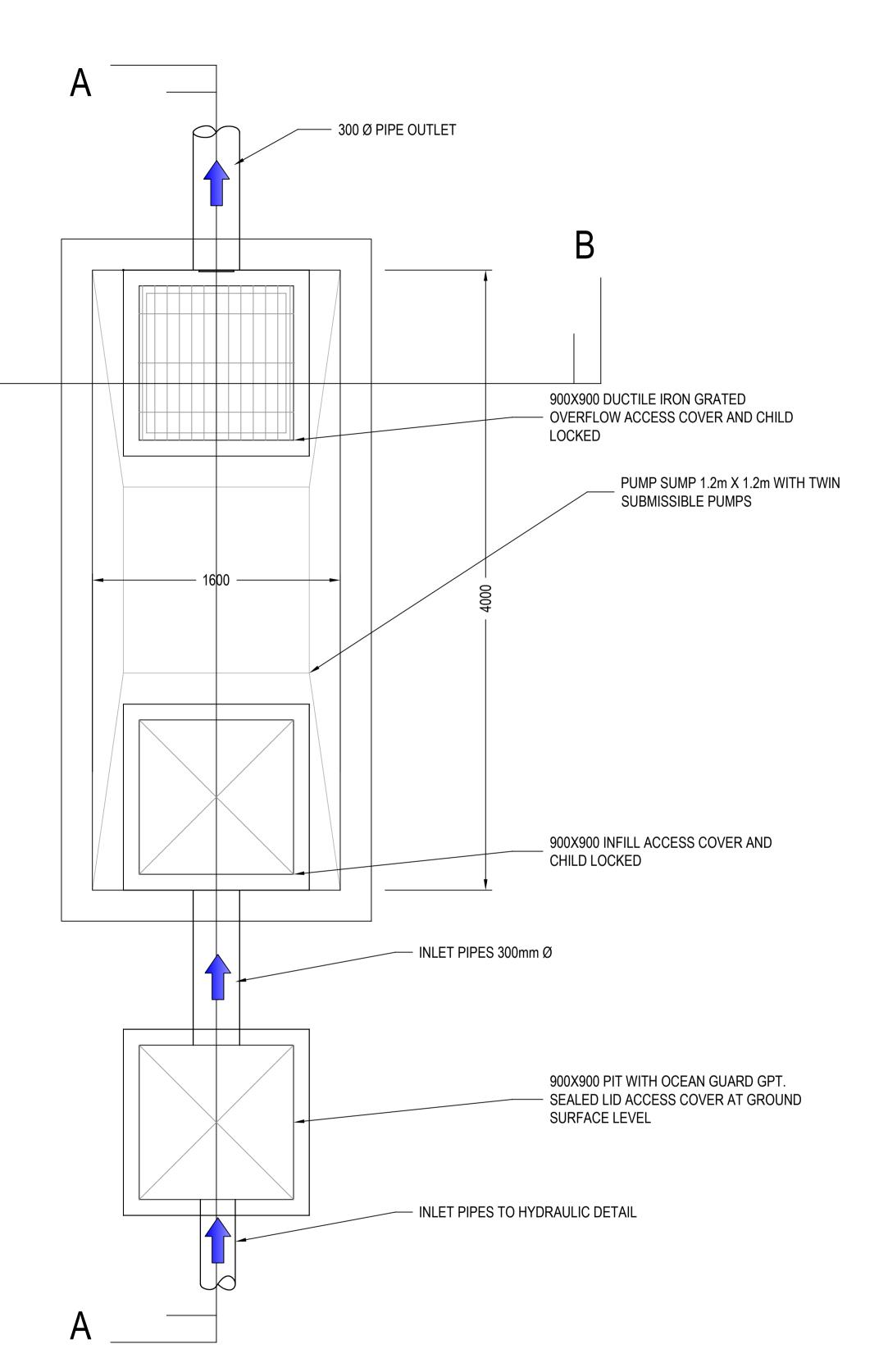
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	P2 13.03.20 ISSUED FOR DA	J.S.	All dimensions to be verified on site/s prior to commencement of on-site work and/ or off-site prefabrication. Figured dimensions This drawing is			
		NORTH	prefabrication. Figured dimension to be taken in prefabrication. Figured dimensions. This drawing is copyright and remains the property of JHA Consulting Engineers. Reproduction in whole or part of these drawings without written consent constitutes an infringement of copyright.			

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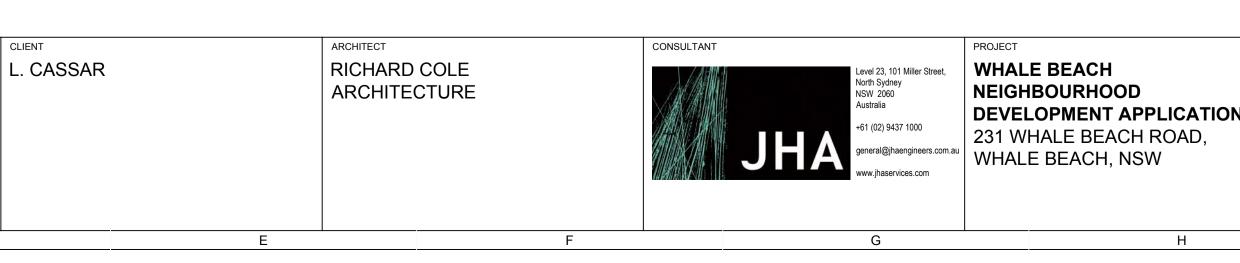
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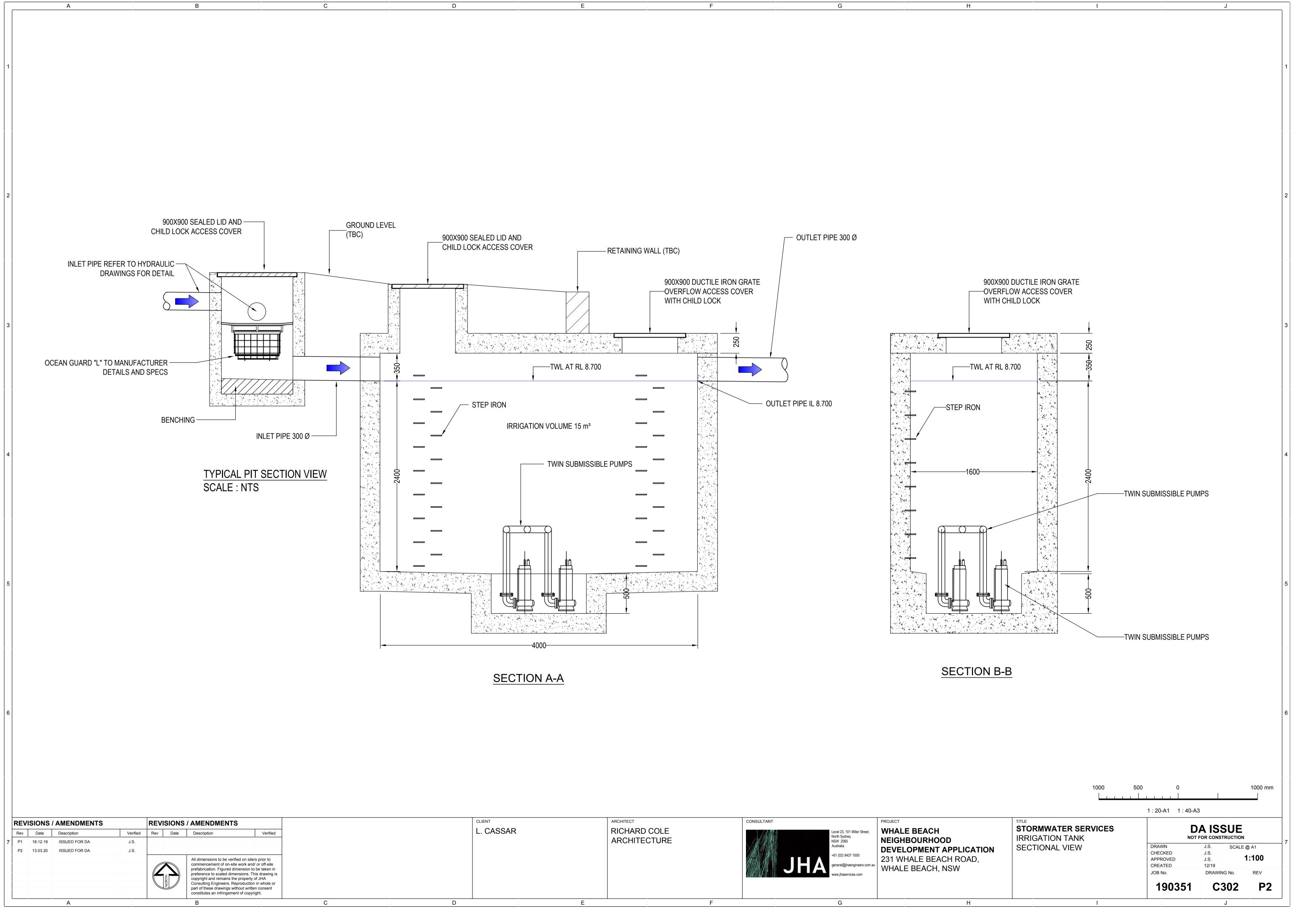
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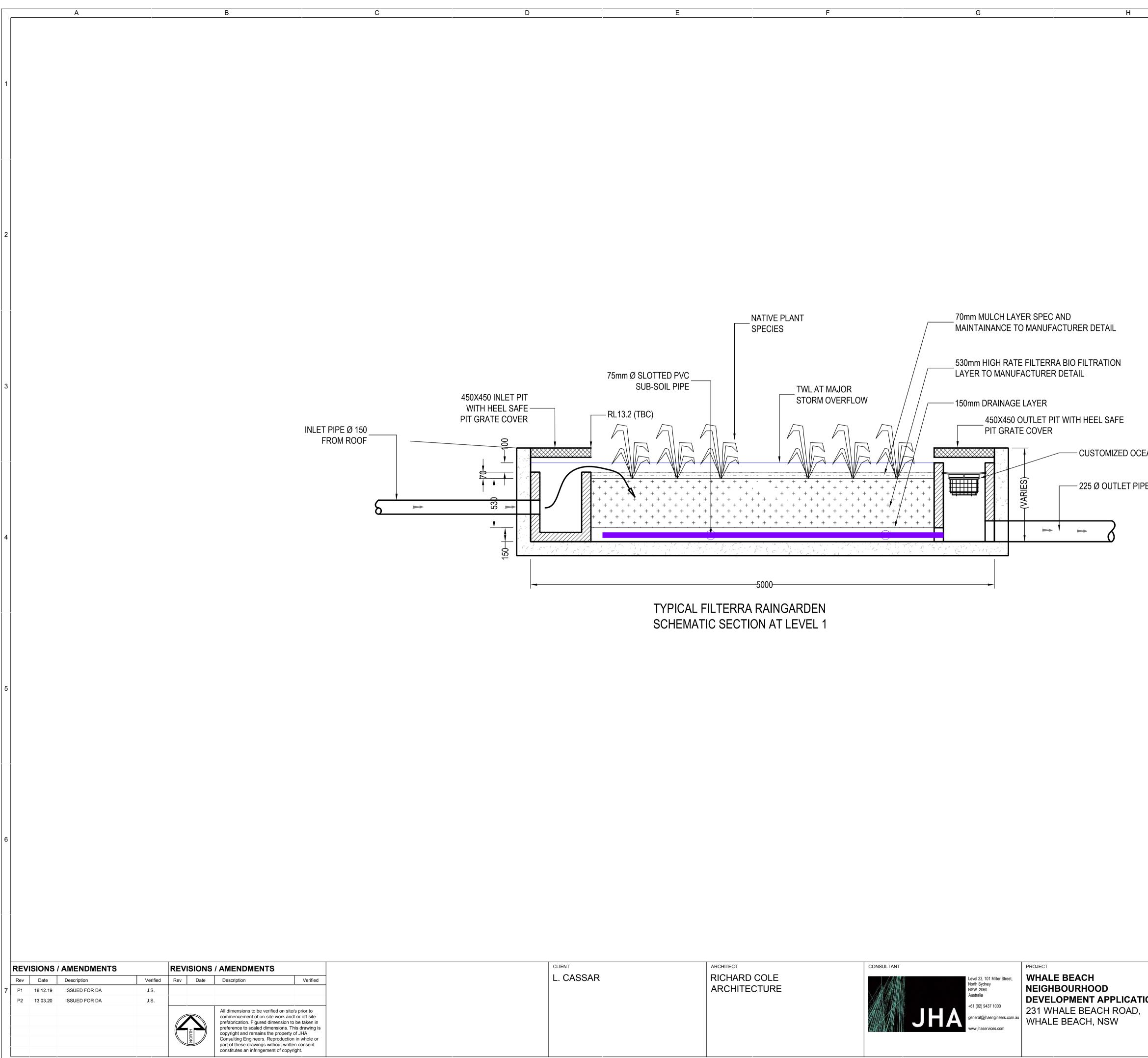
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CASSAR		CHARD COLE CHITECTURE			Level 23, 101 Miller Street, North Sydney NSW 2060 Australia +61 (02) 9437 1000 general@jhaengineers.com.au	<b>DEVELO</b> 231 WHA	BEACH DURHOOD PMENT APPLICATION LE BEACH ROAD, BEACH, NSW
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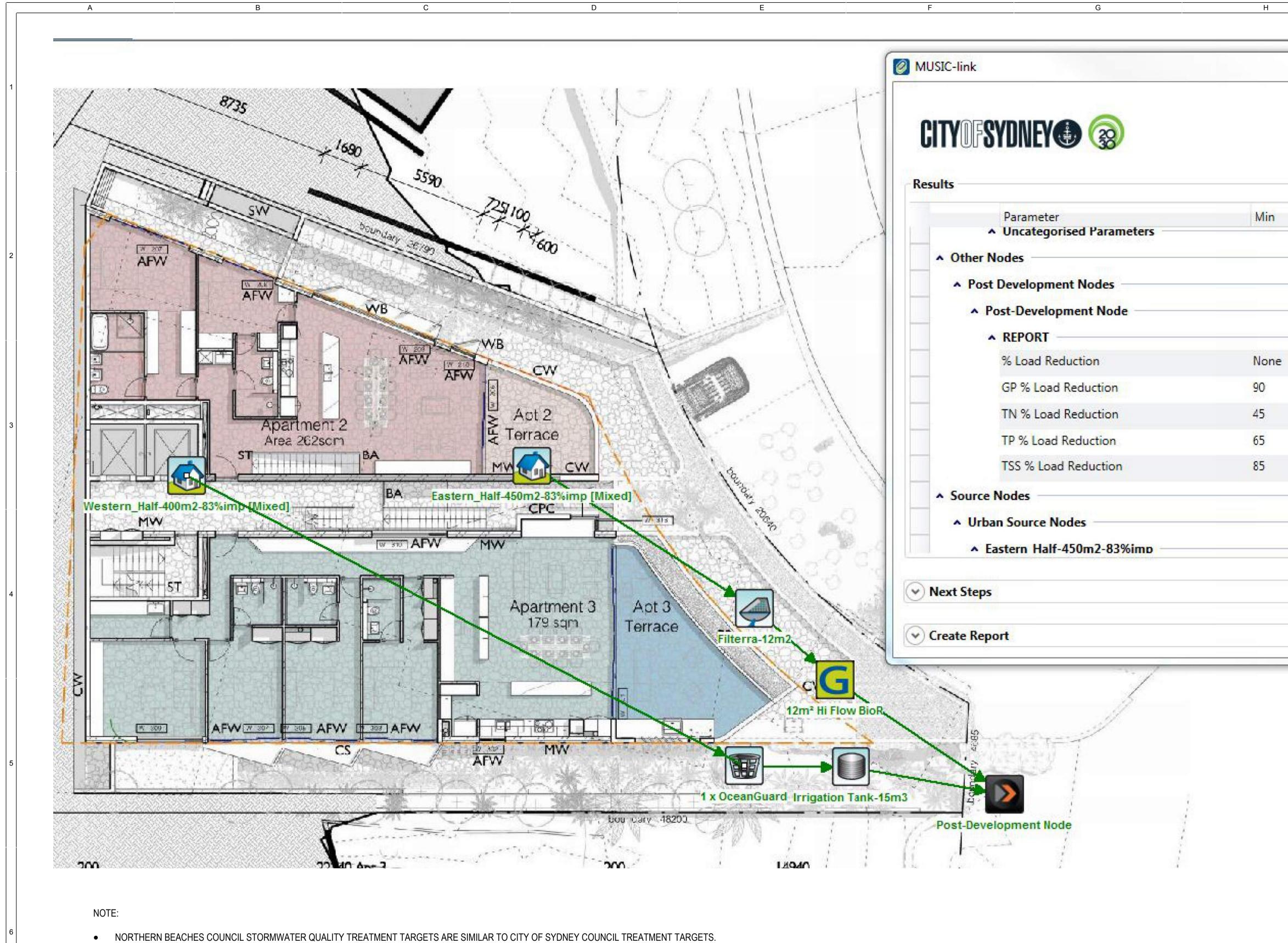
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75mm Ø SLOTTED PVC SUB-SOIL PIPE	NATIVE PLANT SPECIES TWL AT MAJOR STORM OVERFLO	W 150X450	TO MANUFACTURER DETAIL TE FILTERRA BIO FILTRATION FACTURER DETAIL E LAYER OUTLET PIT WITH HEEL SAFE TE COVER CUSTOMIZED OCEAN GUA	RD TO MANUFACTURER DETAIL	3
			SINCE SERVICE AND A STREET PIPE FOR B		4
					6
T CASSAR	ARCHITECT RICHARD COLE ARCHITECTURE F	CONSULTANT Korth Sydney North Sydney Nort	PROJECT WHALE BEACH NEIGHBOURHOOD DEVELOPMENT APPLICATION 231 WHALE BEACH ROAD, WHALE BEACH, NSW	TITLE <b>STORMWATER SERVICES</b> TYPICAL FILTERRA GARDEN CROSS SECTION	

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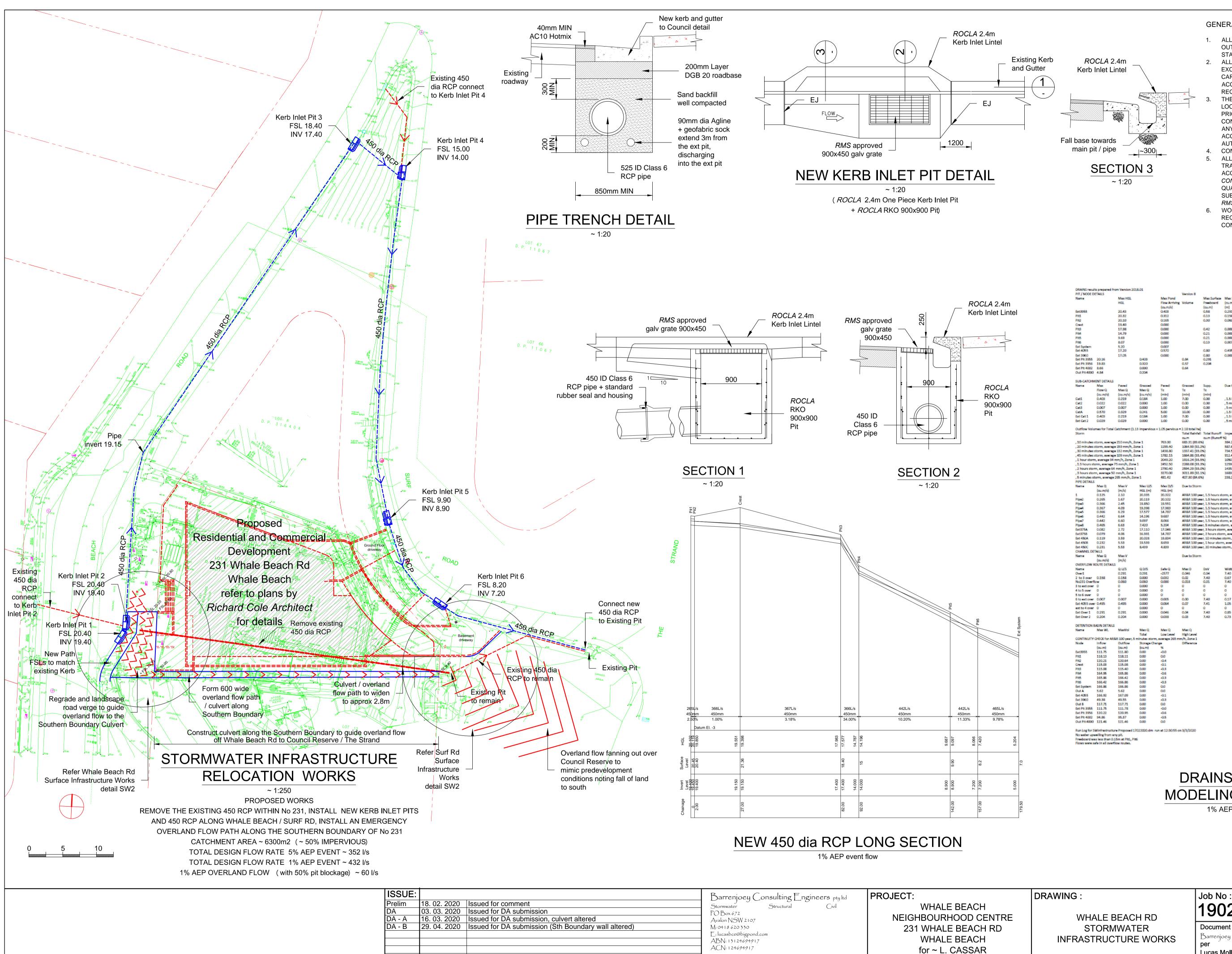
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- IN THIS MUSIC ANALYSIS, WE ARE USING THE LATEST MUSIC LINK DOWNLOADED FROM CITY OF SYDNEY COUNCIL PORTAL.
- THIS MUSIC MODEL WILL BE PROVIDED DURING CC SUBMISSION STAGE.

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	Results	Parameter	Min	Max	Actual	Result	*
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11	A Po	st Development Nodes					-
initial (		Post-Development Node					-
1)		▲ REPORT					-
. //		% Load Reduction	None	None	25.9	0	
1/		GP % Load Reduction	90	None	100	0	
A ST		TN % Load Reduction	45	None	50.5	0	
1/		TP % Load Reduction	65	None	66.6	0	
and N		TSS % Load Reduction	85	None	93.5	0	
8 3 T 0	A Source	e Nodes					-
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	4	Eastern Half-450m2-83%imp	9				
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Light	ioR k-15m3 Post-Dev	elopment Node		TITI F		0 1 : 20-A1 1 : 40-A3	
ARCHITECT RICHARD COL		NSULTANT	PROJECT WHALE BEACH BEIGHBOURHOOD			1 : 20-A1 1 : 40-A3 <b>DAIS</b> NOT FOR CON	STRUCTION
ACHITEC		NSULTANT	WHALE BEACH	STORMWATER		1 : 20-A1 1 : 40-A3	



r comment r DA submission r DA submission, culvert altered r DA submission (Sth Boundary wall altered)	Barrenjoey Consulting Engineers pty ltd Stormwater Structural Civil PO Box 672 Avalon NSW 2107 M: 0418 620 330 E: lucasbce@bigpond.com ABN: 13124694917 ACN: 124694917	PROJECT: WHALE BEACH NEIGHBOURHOOD CENTRE 231 WHALE BEACH RD WHALE BEACH for ~ L. CASSAR	

# **GENERAL NOTES -**

1. ALL WORKS ARE TO BE CARRIED OUT IN ACCORDANCE WITH RMS STANDARDS AND REQUIREMENTS.

- 2. ALL WORKS INCLUDING EXCAVATION AND SHORING TO BE CARRIED OUT IN A SAFE MANNER IN ACCORDANCE WITH ALL OH&S REQUIREMENTS.
- THE BUILDER / CONTRACTOR SHALL LOCATE ALL EXISTING SERVICES PRIOR TO THE COMMENCEMENT OF CONSTRUCTION AND COORDINATE ANY REQUIRED WORKS IN ACCORDANCE WITH THE RELEVANT AUTHORITIES CONSENT.
- CONCRETE STRENGTH 32MPa. ALL PEDESTRIAN AND ROAD TRAFFIC SHALL BE CONTROLLED IN ACCORDANCE WITH A TRAFFIC CONTROL PLAN PREPARED BY A QUALIFIED TRAFFIC CONSULTANT SUBMITTED AND APPROVED BY THE RMS.
- 6. WORKS TO BE INSPECTED AS REQUIRED BY THE RMS DURING CONSTRUCTION.

PIT / NODE DI	ts prepared fro ETAILS	m Version 2018	8.01		Version 8						
Name		Max HGL HGL		Max Pond Flow Arriving	Volume	Max Surface Freeboard	Max Pond (oum/s)	Min	Overflow	Constraint	
				(au.m/s)		(aum)	(m)				
Ext3955 Pit1		20.43		0.408		0.58	0.291			Inlet Cepecity Inlet Cepecity	
Pitt2		20.10		0.165		0.30	0.060			Inlet Capacity	
Creat Pit3		19.40		0.000		0.42	0.000			None	
P164		14.79		0.000		0.21	0.000			None	
Pites		9.69		0.000		0.21	0.000			None Inlet Capacity	
Ext System Ext 4035		5.20 17.20		0.007		0.80	0.495				
Ext 3960		17.05		0.000		0.80	0.000			Inlet Capacity None	
Ext Pit 3955 Ext Pit 3956	20.16		0.408		0.84	0.291			Inlet Capacity Inlet Capacity		
Ext Pit 4002	8.66		0.000		0.64	Color Martin			None		
Out PIt 4000	4.84		0.204								
SUB-CATCHM											
Name	Max Flow Q	Paved Max Q	Grassed Max Q	Paved	Grassed To	Supp. Te	Due to Storm				
	(cu.m/s)	(cu.m/s)	(cu.m/s)	(min)	(min)	(min)					
Cat1 Cat2	0.405	0.219	0.184	1.00	7.00	0.00			5 mm/h, Zone 3 65 mm/h, Zone		
Cat3 CatA	0.007	0.007	0.000	1.00	0.00	0.00			65 mm/h, Zone 5 mm/h, Zone 3		
Ext Cat 1	0.405	0.219	0.184	1.00	7.00	0.00	, 1.5 hours sta	orm, everage 75	5 mm/h, Zone 1		
Ext Cat 2	0.029	0.029	0.000	1.00	0.00	0.00	, 5 minutes st	orm, average 2	65 mm/h, Zone	•1	
	mes for Total C	atchment (1.15	Impervious +	1.05 pervious •		Tested Barris	lange of the second		Reading Pro-		
Storm					ou.m	Total Runoff ourm (Runoff)	Impervious R %)	cuim (Runoff)	Pervious Runo %)	cu.m (Runoff %)	
	torm, evenage torm, evenage			763.00	683.51 (89.69		384.20 (97.19 587.60 (98.19		299.11 (81.4% 477.33 (85.8%		
, 30 minutes a	torm, everage	152 mm/h, Zor	ne 1	1438.80	1337.41 (93.0	(%)	734.50 (98.59	ý.	602.91 (87.0%	á l	
	torm, average 1, average 94 m		n# 1	1782.15 2049.20	1664.86 (93.4 1916.24 (93.5		912.46 (98.89 1050.89 (98.9		752.40 (87.7% 865.34 (87.7%		
,1.5 hours sto	arm, average 7	5 mm/h, Zone 3	1	2452.50	2288.08 (93.3	56)	1259.96 (99.1	56)	1028.12 (87.0	nc)	
	m, everage 54 m, everage 50 m			2790.40 3270.00	2594.29 (93.0 3011.89 (92.1		1435.10 (99.2 1683.71 (99.3		1139.20 (86.2) 1328.18 (84.3)		
, 5 minutes at	orm, average 2		.1	481.42	407.30 (84.69		238.24 (95.59		169.06 (72.9%		
PIPE DETAILS Name	Max Q	MexV	Max U/S	Max D/S	Due to Storm						
	(cu.m/s)	(m/s)	HGL (m)	HGL (m)							
1 Pipe2	0.125	2.10	20.335 20.119	20.322 20.102				5 mm/h, Zone : 5 mm/h, Zone :			
Pipe3 Pipe4	0.366	2.45	19.850	19.551	AR&R 100 yes	r, 1.5 hours sta	orm, average 7	5 mm/h, Zone 1 5 mm/h, Zone 1	1.		
Pipes	0.366	6.29	17.577	14.787	AR&R 100 yes	r, 1.5 hours sta	orm, average 7	5 mm/h, Zone 1	1.		
Pipe5 Pipe7	0.442	6.64	14.196 9.097	9.687				5 mm/h, Zone 1 5 mm/h, Zone 1			
Pipeli	0.465	6.63	7.420	5.204	AR&R 100 year	r, 5 minutes st	orm, evenage 3	55 mm/h, Zon			
Ext375A Ext3758	0.082	2.72	17.110	17.046			m, average 50 m, average 64				
Ext 450A	0.119	3.59	20.028	19.834	AR&R 100 yes	r, 10 minutes a	torm, average	200 mm/h, Zor	ne 1		
Ext 4508 Ext 4500	0.232	5.53	19.539 8.439	8.659			n, average 94 m storm, average	159 mm/h, Zone 1	ne 1		
CHANNEL DET Name	Max Q	MaxV			Due to Storm						
	(cu.m/s)	(m/s)			ove to storm						
OVERFLOW R	OUTE DETAILS	qu/s	Q D/S	Safe Q	Max D	DxV	Width	Max V	Due to Storm		
Over1		0.291	0.291	-2577	0.046	0.04	7.40	0.85	, 1.5 hours		
2 to 3 over No231 Overfi	0.158	0.158	0.000	0.032	0.02	7.40	0.67	, 1.5 hours sta 0.45	, 1.5 hours sto	arm,	
3 to ext over 4 to 5 over	0	0	0.000	0	0	0	0				
5 to 6 over	0	0	0.000	0	0	0	0				
5 to extrover Ext 4055 over		0.007	0.000	0.005	0.00	7.40	0.17	, 5 minutes str , 1.5 hours sto			
ext to 4 over	0	0	0.000	0	0	0	0				
Ext Over 1 Ext Over 2	0.291	0.291	0.000	0.046	0.04	7.40	0.85		, 1.5 hours sto , 1.5 hours sto		
DETENTION B	ACTA DETAILS										
Name	Max WL	MaxVol	Max Q	Max Q	Max Q						
CONTINUETY	CHECK for ARM	100 year 5 m	Total linutes storm, (	Low Level werage 265 m	High Level m/h, Zone 1						
Node	Inflow	Outflow	Storage Chan	g=	Difference						
Ext3955	(au.m) 111.75	(cu.m) 111.80	(ou.m) 0.00	-0.0							
Pitt1	118.13	118.11	0.00	-0.4							
Pit2 Creat	115.08	120.64	0.00	-0.1							
PIES PI54	115.08	115.40	0.00	-0.5							
PRS-	165.86	166.42	0.00	-0.3							
Pits Ext System	166.42	166.86	0.00	-0.3							
Out A Ext 4055	5.62	5.62	0.00	-0.1							
Ext 3960	49.38	49.55	0.00	-0.3							
Out 8 Ext Pit 3955	117.71 111.75	117.71	0.00	-0.0							
Ext Pit 3956	120.22	120.95	0.00	-0.6							
Ext Pit 4002 Out Pit 4000	94.86 121.46	95.37 121.46	0.00	-05							
					1.11 1.11						
	Winfrestructure relling from any		azuzu.dm rur	ne 12:30:55 of	n ay ay 2020						
	a less than 0.15 de in all overfic										
							_			_	
					nR.	ΔΙΝΙ	SS	SV2	TEN	М	
				-		<u> </u>				V I	
					יחר					тο	
	MODELING RESULTS										
						40/ 4					
						1% A	CH eve	ent flow	/		

190210 SW1 DA-B **Document Certification** Barrenjoey Consulting Engineers pty ltd LUCAS MOILOY MIEA CPEng NER Director



Drawing No

