

REPORT TO FOREST CENTRAL BUSINESS PARK PTY LTD

ON

HYDROGEOLOGICAL INVESTIGATION AND ANALYSIS

FOR PROPOSED MEDICAL CENTRE

AT Lot 7, DP1020015, FOREST CENTRAL BUSINESS PARK, 49 FRENCHS FOREST ROAD EAST, FRENCHS FOREST, NSW

Date: 3 August 2020 Ref: 32505BMrpt2

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ATTACHMENTS

Monitoring Well logs: M201 to M203

- Figure 1: Site Location Plan
- Figure 2: Investigation Location Plan
- Figure 3: Groundwater Level and Daily Rainfall -v-Time Plot M201
- Figure 4: Groundwater Level and Daily Rainfall -v-Time Plot M202
- Figure 5: Groundwater Level and Daily Rainfall -v-Time Plot M203
- Figure 6: Graphical Borehole Section A-A
- Figure 7: Section A-A Hydrogeological Model
- Figure 8: Section A-A Seepage Analysis Results- Total Head Contours
- Figure 9: Section A-A Seepage Analysis Results- Water Flow Contours

Appendix A: Relevant Borehole Logs (BH1 to BH9, BH101 to BH102) Graphical Borehole Section B-B

Appendix B: WaterNSW Exemptions- Construction Dewatering- Fact Sheet



1 INTRODUCTION

This report presents the results of a hydrogeological investigation and analysis for the proposed medical centre known as 'Project Maui Oncology' at Lot 7, DP1020015, which is part of Forest Central Business Park, 49 Frenchs Forest Road East (also known as 502B Warringah Road), Frenchs Forest, NSW. A site location plan is presented as Figure 1. The investigation was commissioned by Ms Joanna Karamihas of Ascot Project Management, on behalf of Forest Central Business Park Pty Ltd. The commission dated 30 June 2020 was on the basis of Scope Items 2.2 and 2.3 of our fee proposal (Ref: P52067BM dated 23 June 2020).

We previously completed a geotechnical investigation of the site as detailed in our report dated 17 February 2020 (Ref: 32505BMrpt Rev4). Reference should be made to our previous geotechnical investigation report for the subsurface profile and our recommendations for design and construction of the proposed development. The purpose of this additional investigation was to install 3 groundwater monitoring wells in order to record seepage inflows to estimate groundwater permeability and record groundwater levels. The results of the investigation were then used to carry out 2D seepage analysis of a section through the proposed basement to assess the potential seepage volumes into the basement excavation during construction and in the long term. The analysis was carried out using the 2D finite element computer program Seep/W 2019 (from Geo-Slope International Ltd).

The groundwater quality has also been assessed by JK Environments as detailed in their separate report, Ref E32505BTrpt3 dated August 2020, which also provides advice on any treatment required prior to potential discharge of groundwater from the site.

2 PROPOSED DEVELOPMENT

From the previously supplied DA drawings (Project No. 856, Drawing No. DA-099 ^{Rev 1}, 100 to 103 ^{Rev 6}, and 300 & 301 ^{Rev 4}, all dated 4 December 2019) prepared by Team 2 Architects we understand the development includes excavation for 4 basement levels, the lowest being at RL 146.95m requiring excavation to depths ranging from about 12m to 14m below the existing surface levels. The proposed basement outline is indicated on Figure 2.

3 PREVIOUS GEOTECHNICAL INVESTIGATION

We previously carried out a detailed geotechnical investigation and report on this site for the proposed development (Ref: 32505BMrpt Rev4, dated 17 February 2020). The geotechnical investigation included the drilling of 11 boreholes (BH1 to BH9 and BH101 and BH102) to maximum depths of 17.3m. Reference should be made to our previous report for detailed descriptions of the subsurface conditions encountered. However, copies of the boreholes logs are provided in Appendix A, together with Section B. Section A is provided as Figure 6, which has had the monitoring wells and measured groundwater levels added to the section provided within our previous geotechnical investigation report.



In summary, the subsurface profile comprises silty clay fill overlaying residual silty clay grading into weathered siltstone at depths of about 1.4m to 3.2m below surface levels. The weathered siltstone was underlain by sandstone bedrock below depths of 4.75m to 5.12m.

A groundwater monitoring well was installed in BH7 as part of our previous geotechnical investigation and groundwater was measured at a depth of 5.5m on the completion of core drilling (where water is introduced in the ground as part of the drilling process). The well was then pumped dry to allow the groundwater to recharge, with the recharge rate measured using a data logger to assess the permeability of the weathered sandstone. A site visit was again made 26 days after pumping it dry and groundwater was measured in the well at a depth of 7.05m (\approx RL152.3m). A final site visit was made on 3 days after pumping and groundwater was measured at a depth of 7.8m (\approx RL151.5m).

Based on the recharge rate into the well in BH7, the permeability of the weathered sandstone bedrock was calculated to be about 6×10^{-8} m/s, which is in the order expected for sandstone bedrock with relatively few defects. The actual water level measured may have been artificially high due to the water used in the drilling of the borehole.

Since the above geotechnical investigation was completed the well in BH7 has been destroyed by others.

4 ADDITIONAL GROUNDWATER INVESTIGATION AND MONITORING

In accordance with WaterNSW guidelines, and to assist with the groundwater seepage analysis the following was undertaken:

- Installation of 3 new groundwater monitoring wells (M201, M202 and M203) in auger drilled boreholes at the locations shown on the attached Figure 2. No water was used in the drilling process of these holes and all wells were dry on installation. The well construction details are presented on the attached Monitoring Well logs. The subsurface profile within the wells was not logged in detail due to the amount of geotechnical information already available. Therefore, the descriptions given on the monitoring well logs are approximate only,
- Continuous groundwater level monitoring using electronic data loggers in each of the wells and a site specific barometer, over an approximate 2 week period between 3 July and 16 July 2020, and
- Using the recorded infiltration from initially dry conditions to steady water levels to assess the permeability of the sandstone. The results of the groundwater monitoring have been plotted against rainfall and are presented in the attached Figures 3 to 5.

The results of the above monitoring have been used to develop a groundwater flow direction and approximate contour plan, as shown in Figure 2. The contours are very approximate as they are based on only three monitoring well locations inside the site boundaries. The water levels are also shown in the attached Section A, Figure 6.

Based on the results, the direction of groundwater flow appears to be down towards the south, from RL152.0m at M201 to RL150.0m at M203, as shown on Figure 2.





Using established correlations, we have estimated the bulk permeability of the weathered sandstone within each of the three monitoring wells. The estimated coefficients of permeability are summarised in the table below.

Well	Material	Coefficient Permeability, k, Range (m/sec)
M201	Sandstone Bedrock	1 x 10 ⁻⁷ to 5 x 10 ⁻⁸
M202	Sandstone Bedrock	9 x 10 ⁻⁸ to 4 x 10 ⁻⁸
M203	Sandstone Bedrock	3 x 10 ⁻⁸ to 1 x 10 ⁻⁸

The total range for each of the three wells was 1×10^{-7} to 1×10^{-8} m/sec.

5 HYDROGEOLOGICAL MODEL

5.1 Subsurface Profile

Based on the results of the geotechnical investigation and the details of the proposed development, a geotechnical model was developed as shown in the attached Figure 7. The section was predominantly based on Section A.

5.2 Hydraulic Model and Boundary Conditions

The saturated coefficient of permeability values adopted in the geotechnical model for the rock unit within which groundwater was identified were assessed based on the calculated coefficients given in Section 4 above. The results indicate a coefficient of permeability ranging from 1×10^{-7} to 1×10^{-8} m/sec and the analysis has been completed by varying the coefficient of permeability within the estimated range. The results of the analysis are not sensitive to the permeability of the fill, soil, and siltstone as the groundwater table is entirely within the sandstone bedrock profile and as such the permeability of these layers has not been varied.

Based on experience, seepage through the rock mass would dominantly occur within the defects. The most common continuous defect in the Hawksbury Sandstone and Ashfield Shale are near horizontal bedding partings. Therefore, it is appropriate to apply a lower vertical permeability value than that of the horizontal permeability. Therefore, for the purposes of the analysis we have varied the ratio of vertical to horizontal permeability (k_y/k_x) from 0.1 to 0.5.

The groundwater table adopted for the analysis has been taken as the measured groundwater level within the wells of RL152m at the northern end of the site to RL150m at the southern end. As the model boundaries extend beyond the site boundaries, the adopted groundwater level was extrapolated to the model boundaries. In order to model a potential rise in the groundwater level, analysis has also been completed for a groundwater level 1m higher than the measured levels, i.e. RL153m at the northern end to RL151m at the southern end.





5.3 Shoring System

We expect the shoring system around the perimeter of the proposed basement will likely be a soldier pile shoring system with reinforced shotcrete panels. Therefore, seepage will flow between the piles. In the permanent case we have assumed that drainage placed at the rear of the shotcrete panels will collect seepage and divert the seepage to an underfloor drainage system. As a result we have modelled the excavation to allow seepage to flow freely from the excavation faces and base.

If the basement was to be tanked, the temporary soldier pile shoring system would allow the same temporary drainage so the same seepage model and results which is expressed as a rate (ML/year) would apply to a temporary dewatering case. The total estimated volume of seepage during the construction period may be determined by multiplying the estimated seepage by the length of the construction period.

6 **RESULTS OF ANALYSIS**

As discussed above, several analysis cases have been carried out by varying the coefficient of permeability, the ratio of vertical permeability to horizontal permeability and the groundwater level. An example of the plots showing the Total Head Contours and the Water Flow Contours from the analysis for Case 1.1 are presented as Figures 8 and 9, respectively. For each case we have assessed the expected inflow for the section per metre width and then calculated the total inflow by multiplying the inflow through the base by the approximate average width of excavation of about 35m and the inflow through the sides by the permitter of the basement of about 70m.

	Case	Groundwater Level (RLmAHD)	Horizontal Permeability k _x (m/sec)	Ky/Kx	Rate of Inflow, Q (ML/yr)
1.1	Measured groundwater levels, highest calculated permeability, lowest vertical permeability	152 to 150	1 x 10 ⁻⁷	0.1	0.4
1.2	Measured groundwater levels, highest calculated permeability, highest vertical permeability	152 to 150	1 x 10 ⁻⁷	0.5	0.9
1.3	Measured groundwater levels, lowest calculated permeability, lowest vertical permeability	152 to 150	1 x 10 ⁻⁸	0.1	0.04
1.4	Measured groundwater levels, lowest calculated permeability, highest vertical permeability	152 to 150	1 x 10 ⁻⁸	0.5	0.1
2.1	Elevated groundwater levels, highest calculated permeability, lowest vertical permeability	153 to 151	1 x 10 ⁻⁷	0.1	0.4
2.2	Elevated groundwater levels, highest calculated permeability, highest vertical permeability	153 to 151	1 x 10 ⁻⁷	0.5	1.0



	Case	Groundwater Level (RLmAHD)	Horizontal Permeability k _x (m/sec)	K _y /K _x	Rate of Inflow, Q (ML/yr)
2.3	Elevated groundwater levels, lowest calculated permeability, lowest vertical permeability	153 to 151	1 x 10 ⁻⁸	0.1	0.04
2.4	Elevated groundwater levels, lowest calculated permeability, highest vertical permeability	153 to 151	1 x 10 ⁻⁸	0.5	0.1

The above results show little difference in the total seepage volume with variations in the groundwater level, or the ratio of vertical permeability to horizontal permeability. The main parameter that affects the measured seepage is the permeability adopted for the weathered sandstone.

7 COMMENTS

The results of the seepage analysis show that for the measured range of permeability of the weathered sandstone that seepage into the basement for the measured groundwater is expected to be in the order of 0.1ML/year to 0.9ML/year. When the groundwater levels were raised by 1m the estimated seepage into the basement was of about the same order of 0.1ML/year to 1ML/year.

We note that since the basement will be excavated into the sandstone bedrock the flow will occur through defects, such as joints and bedding parting, within the rock and will vary throughout the excavation due to the jointing present in different areas of the excavation. The estimates given above assume homogenous materials and in practice lower and higher inflows may be experienced. We recommend that the inflow into the excavation by monitored during construction.

All the analysed inflows are well below 3ML/year, which in this regard would comply with the WaterNSW exemption from a Construction Dewatering Licence, as detailed in the WaterNSW Fact Sheet provided in Appendix B.

8 GENERAL COMMENTS

The analysis detailed in this report is only related to seepage analysis and not stability analysis or design of the shoring system or other geotechnical issues relating to the proposed development. Reference should be made to the geotechnical investigation report for comments on other geotechnical issues.

Occasionally, the subsurface conditions between the completed boreholes may be found to be different (or may be interpreted to be different) from those expected. Variation can also occur with groundwater conditions, especially after climatic changes. If such differences appear to exist, we recommend that you immediately contact this office.

This report provides advice on geotechnical aspects for the proposed civil and structural design. As part of the documentation stage of this project, Contract Documents and Specifications may be prepared based on





our report. However, there may be design features we are not aware of or have not commented on for a variety of reasons. The designers should satisfy themselves that all the necessary advice has been obtained. If required, we could be commissioned to review the geotechnical aspects of contract documents to confirm the intent of our recommendations has been correctly implemented.

This report has been prepared for the particular project described and no responsibility is accepted for the use of any part of this report in any other context or for any other purpose. If there is any change in the proposed development described in this report then all recommendations should be reviewed. Copyright in this report is the property of JK Geotechnics. We have used a degree of care, skill and diligence normally exercised by consulting engineers in similar circumstances and locality. No other warranty expressed or implied is made or intended. Subject to payment of all fees due for the investigation, the client alone shall have a licence to use this report. The report shall not be reproduced except in full.



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Client: Project: Locatior	PROF	OSE	D M	IEDICA	L CEN	ESS PARK PTY LTD ITRE HS FOREST ROAD EAST, FI	RENCH	SFOR	EST, I	NSW	
Date: 3/7						thod: SPIRAL AUGER	Da	L. Sur atum:		~160.3	m
	pe: JK305					gged/Checked By: W.S./M.P.			er Pa)		
Groundwater Record U50 DB DB DB DB DB DB DB DB DB DB DB DB DB	Field Tests	RL (m AHD)	Depth (m)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition/ Weathering	Strength/ Rel Density	Hand Penetrometer Readings (kPa)	Well Details	Remarks and Well Details
DRY ON COMPLETION		- 160 - - -	- - - - -			FILL: /Soil. Refer to nearest borehole log (BH8) for description					 Gatic Cover Concrete
		159 - -	2-								
		158 - - -	3-								 Cuttings
		157	4-			SANDSTONE:					_
		156 - - - - 155 -	5-								Bentonite
			6-								_
COPYRIGHT		-	- - -								—— Start of Class 18 Machine Slotted Scree





Client: Project:	FOREST PROPOS				ESS PARK PTY LTD ITRE				
Location:	LOT 7 DF	1020	015, FF	RENC	HS FOREST ROAD EAST, FI	RENCH	SFOR	REST, NSW	
Job No.: 32	2505BM			Me	thod: SPIRAL AUGER	R.	L. Sur	rface: ~160.	3 m
Date: 3/7/20	C						atum:	AHD	
Plant Type:	JK305			Lo	gged/Checked By: W.S./M.P				
Groundwater Record DB DB DS DS	Field Tests RL (m AHD)	Depth (m)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition/ Weathering	Strength/ Rel Density	Hand Penetrometer Readings (kPa) Well Details	Remarks and Well Details
	153	-		-	SANDSTONE: (continued)				
	152	- 8-							
	151	- - - - - - - - -							
	150	- - - - -							
	149	- 11 - 							
	148	- - - - -							
	147	- 13-			END OF BOREHOLE AT 13.50 m				
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Client:FOREST CENTRAL BUSINESS PARK PTY LTDProject:PROPOSED MEDICAL CENTRELocation:LOT 7 DP1020015, FRENCHS FOREST ROAD EAST, FRENCHS FOREST, NSW											
Job No.: 32			,		thod: SPIRAL AUGER				~159.2	m	
Date: 3/7/20							atum:	AHD			
Plant Type:	JK305	1		Lo	gged/Checked By: W.S./M.P.						
Groundwater Record DB DB DS S31dWYS C3 DB DB	Field Tests RL (m AHD)	Depth (m)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition/ Weathering	Strength/ Rel Density	Hand Penetrometer Readings (kPa)	Well Details	Remarks and Well Details	
COMPLETION	159 - - - -				FILL: /Soil. Refer to nearest borehole log for description					_← Gatic Cover	
	158 - - - - -	2-								- 	
	157 - - - 156 -	3-								Cuttings	
	- - - - - - - - - - - - - - -	4			SANDSTONE:			-		- - - - - - - - - - - - - - - - - - -	
	- 154 - - -	5-								-	
COPYRIGHT	- 153 - - - -	6-								Start of Class 18 Machine Slotted Scre Action Slotted Scree	





Client: Project: Location:	Project: PROPOSED MEDICAL CENTRE											
Job No.: 32	2505BM			Me	thod: SPIRAL AUGER	R.	L. Sur	face:	~159.2	m		
Date: 3/7/20				_			atum:	AHD				
Plant Type:	JK305	1	1	Loę	gged/Checked By: W.S./M.P.			<u> </u>				
Groundwater Record DB DB DB DB BC DB	Field Tests RL (m AHD)	Depth (m)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition/ Weathering	Strength/ Rel Density	Hand Penetrometer Readings (kPa)	Well Details	Remarks and Well Details		
	152			-	SANDSTONE: (continued)					-		
	152 151 150 149 148 147									- 2mm Sand		
										- - -		
			-		END OF BOREHOLE AT 12.70 m					End Cap - -		
	146	- 13 - - ·	-							-		

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Client: Project:	PROPOSE	ED N	IEDICAI	L CEN						
Location:		1020	015, FF		HS FOREST ROAD EAST, FI					
Job No.: 3 Date: 3/7/2				Me	thod: SPIRAL AUGER		L. Sur atum:	face: ~	158.2	m
Plant Type:				Loo	gged/Checked By: W.S./M.P.		atum.	AND		
								oa)		
Groundwater Record ES U50 DB DB DB	Field Tests RL (m AHD)	Depth (m)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition/ Weathering	Strength/ Rel Density	Hand Penetrometer Readings (kPa)	Well Details	Remarks and Well Details
DRY ON COMPLETION	158 -				FILL: /Soil. Refer to nearest borehole log (BH1) for description					 Gatic Cover Concrete
	157 -	- 1-								- 50mm DIA. PVC Standpipe ← Cuttings
	156 -	2-								_
	155 -	- 3-								-
	154 -	4								 Bentonite
	153 -	- 5-			SANDSTONE:	<u> </u>				-
COPYRIGHT	152 -	6								— Start of Class 18 Machine Slotted Scre





С	lier	t:		FOREST CENTRAL BUSINESS PARK PTY LTD PROPOSED MEDICAL CENTRE										
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Groundwater Record	SAN		S	Field Tests	RL (m AHD)	Depth (m)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition/ Weathering	Strength/ Rel Density	Hand Penetrometer Readings (kPa)	Well Details	Remarks and Well Details
					151 -			-	SANDSTONE: (continued)					
						- 8								
					- 149 -	9								
					- 148 - -	10								
					- 147 - -	· 11								
					- +146-	12-								— - ——— End Cap
COF					145	13-	-		END OF BOREHOLE AT 12.20 m					· · · · · · · · · · · · · · · · · · ·



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INVESTIGATION LOCATION PLAN

Location: FRENCHS FOREST BUSINESS PARK FRENCHS FOREST, NSW

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Groundwater Level and Daily Rainfall -v- Time Plot



Groundwater Level and Daily Rainfall -v- Time Plot



Groundwater Level and Daily Rainfall -v- Time Plot





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APPENDIX A





С	lier	nt:	ERILY	(AN								
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				510				ESS PARK, FRENCHS FORE			6	450.70
		NO.: : 1/8/	32505S				Method: SPIRAL AUGER R.L. Surface: 1 Datum: AHD				158.72 m	
			e: JK250)			Lo	gged/Checked By: W.S./P.S.		atunn	/ 10	
Groundwater Record	SAI		Field Tests	RL (m AHD)	Depth (m)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition/ Weathering	Strength/ Rel Density	Hand Penetrometer Readings (kPa)	Remarks
DRY ON COMPLETION			N = 7 3,4,3	- - - 158 -	-			FILL: Silty gravelly clay, medium plasticity, dark grey and red brown, fine to medium grained igneous gravel.	w>PL			GRASS COVER APPEARS MODERATELY COMPACTED
2				-	1- -		СН	Silty CLAY: high plasticity, orange brown mottled red brown.	w>PL	St		RESIDUAL
			N = 18 11,9,9	 157 - - -	2-			as above, but light grey.	w~PL	VSt - Hd	350 450 480	-
				156							-	-
				- - - 155 - -	4	-		END OF BOREHOLE AT 3.00 m				
an forman a second second second second				- 154 - -	5							-
				- 153	6	-						- - - - - - - - - -
		GHT		152 -	-	-						-





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L	ocat	ion:	FORE	ST C	EN	TRAL E	BUSIN	ESS PARK, FRENCHS FORE	ST, NS	W				
J	ob N	lo.:	32505S				Me	thod: SPIRAL AUGER	R	.L. Sur	face: [^]	159.3 m		
	ate:			_			_		D	Datum: AHD				
P	lant	Тур	e: JK250)		1	Lo	gged/Checked By: W.S./P.S.		1				
Groundwater Record	SAMI D20	PLES BO	Field Tests	RL (m AHD)	Depth (m)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition/ Weathering	Strength/ Rel Density	Hand Penetrometer Readings (kPa)	Remarks		
DRY ON COMPLETION				- 159 -				FILL: Silty clay, medium plasticity, dark brown, trace of fine to medium grained igneous gravel and ash.	w>PL			GRAVEL COVER		
0			N = 9 7,4,5				СН	SILTY CLAY: high plasticity, red brown mottled orange brown, trace of fine to medium grained ironstone gravel.	w>PL	VSt - Hd	450 550 540	RESIDUAL		
				158 -	-			SILTY CLAY: high plasticity, yellow	w~PL			- - - -		
			N = 12 5,6,6		2-			as above, but light grey.	W~PL		520 550 600	- - - - - -		
				157 —	-			as above, but light grey and red brown.				-		
				-								-		
2				156		-		END OF BOREHOLE AT 3.00 m				-		
				- - 155 — -	4	-						- - - - - - -		
				- - 154 -	5	-						- 		
				- 153 — -	6							- - - - - - - -		
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		atic			510	EN	I RAL E		ESS PARK, FRENCHS FORE				
				2505S				Me	thod: SPIRAL AUGER				160.01 m
			/8/1								atum:	AHD	
P	lar	nt I	ype:	JK250)		1 1	LO	gged/Checked By: W.S./P.S.	1			
Groundwater Record	SA SI	MPL DB		Field Tests	RL (m AHD)	Depth (m)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition/ Weathering	Strength/ Rel Density	Hand Penetrometer Readings (kPa)	Remarks
DRY ON COMPLETION					-				FILL: Silty clay, medium plasticity, dark grey and brown, with fine to coarse grained igneous gravel.	w>PL		-	GRAVEL COVER
0				N = 7 4,4,3	-	1 -		СН	Silty CLAY: high plasticity, light grey and red brown, trace of fine to medium grained ironstone gravel.	w>PL	St	300 350 510	- RESIDUAL - - -
					159	1			Silty CLAY: high plasticity, pale grey.	w~PL	F - St		-
				N = 3 1,2,1	- - 158	2-						100 150 160	-
						2						-	- - - -
						3	-					-	-
					-		-		END OF BOREHOLE AT 3.00 m				-
					- 156 -	4 -	-					-	- - - -
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	Job) No	5.: 3	2505S	EST C	EN	FRAL E		ESS PARK, FRENCHS FORE thod: SPIRAL AUGER	R	.L. Sur		160.0 m		
			1/8/1 Г уре	9 : JK250)			Lo	gged/Checked By: W.S./P.S.	D	atum:				
Groundwater	Record ES 0			Field Tests	RL (m AHD)	Depth (m)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition/ Weathering	Strength/ Rel Density	Hand Penetrometer Readings (kPa)	Remarks		
DRY ON					-	-			FILL: Silty clay, dark brown, medium plasticity, trace of fine to medium grained igneous gravel.	w>PL			-		
				N = 8 4,4,4	- - - 159	- - - 1-		СН	Silty CLAY: high plasticity, light grey mottled orange brown and red brown.	w>PL	VSt - Hd	450 500 550	- REFUSAL - - - - - - -		
				N = 14		-			as above, but mottled light grey and orange brown.	w~PL	Hd	500	-		
				5,6,8	- 158 - - -	2						560 >600			
	-				-157-	-3-			END OF BOREHOLE AT 3.00 m				-		
					- - - 156 - - -		-								
					- 155 — -	5	-						- - - - - - -		
						- - 6 - -									





	lier roje	nt: ect:	ERILY PROF		D M	EDICA		ITRE					
L	oca	tion	FORE	ST C	CEN	TRAL E	BUSIN	ESS PARK, FRENCHS FORE	EST, NS	W			
J	ob	No.:	32505S				Me	thod: SPIRAL AUGER	R.	L. Sur	face:	159.9 m	
		: 1/8							Datum: AHD				
P	lan	t Тур	e: JK250)	1		Lo	gged/Checked By: W.S./P.S.					
Groundwater Record	SAI		Field Tests	RL (m AHD)	Depth (m)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition/ Weathering	Strength/ Rel Density	Hand Penetrometer Readings (kPa)	Remarks	
DRY ON COMPLETION					-			FILL: Silty clay, medium plasticity, dark brown, trace of fine to medium grained igneous gravel and ash.				-	
			N = 16 19,10,6	- 159 -	1-		СН	SILTY CLAY: high plasticity, light grey mottled orange brown.	w~PL	Hd	>600 >600 >600	RESIDUAL	
				-	-			as above, but light grey and orange brown.		VSt		-	
			N = 11 5,5,6	158 -	2-						480 520 530	-	
				-	-			as above, but light grey.				-	
	\parallel			157 -	3-			END OF BOREHOLE AT 3.00 m				-	
				- - - 156		-						-	
				- - - 155 –		-						-	
				155 -	5								
		GHT		- - 153 –	-	-						- - - - -	





	lier	nt: ect:					IEDICA		ITDE						
	-	atio							ESS PARK, FRENCHS FORE	EST, NS	W				
Jo	b	No.	: 32	2505S				Me	thod: SPIRAL AUGER	R.	L. Sur	face: ´	159.3 m		
		e: 1/								Datum: AHD					
Ρ	lan	t Ty	vpe:	JK250)		, ,	Log	gged/Checked By: W.S./P.S.	, , , , , , , , , , , , , , , , , , , ,					
Groundwater Record	SAN	MPLE	S	Field Tests	RL (m AHD)	Depth (m)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition/ Weathering	Strength/ Rel Density	Hand Penetrometer Readings (kPa)	Remarks		
DRY ON COMPLETION					- 159 —				FILL: Silty clay, medium plasticity, dark brown, with fine to medium grained igneous gravel.	w>PL		-	-		
				N = 11 11,6,5		1-		СН	Silty CLAY: high plasticity, light grey mottled orange brown and red brown, trace of root fibres.	w~PL	VSt	400 350	RESIDUAL		
				N = 10	158						Hd	>600 >600	-		
				4,4,6		2-						>600	-		
					-							-	-		
					- 156 -	-3-	-		END OF BOREHOLE AT 3.00 m						
					-	4 -	-					-	-		
					155 -		-						-		
					-	5-	-						- - - -		
					154		-						-		
					- - 153 —	6-							- - - -		
					-								-		





	lier	-	ERILY											
	-	ect:	PROP						OT NO					
		ation:		SIC	EN	I RAL E		ESS PARK, FRENCHS FORE				150.04		
		NO.: 3 : 1/8/1	32505S				IVIE [®]	thod: SPIRAL AUGER	R.L. Surface: 159.31 m Datum: AHD					
			e: JK250				Logged/Checked By: W.S./P.S.							
			Field Tests	RL (m AHD)	Depth (m)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition/ Weathering	Strength/ Rel Density	Hand Penetrometer Readings (kPa)	Remarks		
שפ			<u> </u>	<u>∽</u> 159 –			20	FILL: Silty clay, medium plasticity, dark brown, trace of fine to medium grained igneous and ironstone gravel, and ash.	≥o≤ w>PL	ώ κ	ΤŒΥ			
			N = 4 9,2,2	-	- - 1—		СН	Silty CLAY: high plasticity, light grey mottled orange brown and red brown.	w>PL	VSt	300 350	RESIDUAL		
				- 158 -	-							-		
				- - 157 - -	2 - - -			as above, but light grey.				- - - - - - - - - - - -		
			N = 32 11,16,16	- 156 — -	3		-	Extremely Weathered siltstone: silty CLAY, high plasticity, light grey.	XW	Hd	300 550 >600	- HAWKESBURY - SANDSTONE - VERY LOW 'TC' BIT - RESISTANCE		
0				- - 155 — -	4 — - -							- 		
				- 154	5			REFER TO CORED BOREHOLE LOG				 MODERATE RESISTANC GROUNDWATER MONITORING WELL INSTALLED TO 12.0m. CLASS 18 MACHINE SLOTTED 50mm DIA. PVI STANDPIPE 12.0m TO 9.0m. CASING 9.0m TO 0m. 2mm SAND FILTER 		
				- - 153 — -	- 6 - -							PACK 12.0m TO 8.0m. BENTONITE SEAL 8.0m TO 0.1m. BACKFILLED WITH SAND TO THE SURFACE. COMPLETED WITH A CONCRETED GATIC COVER.		



CORED BOREHOLE LOG



		nt:		ERILY							
		ject: ation			OSED MEDICAL CENTRE ST CENTRAL BUSINESS PAF	RK, F	REN	CHS FORE	EST, NS	W	
	lob	No.:	32	505S	Core Size:	NML	С		R	.L. Surface: 159.31 m	
[Date	e: 1/8	/19		Inclination:	VER	RTICA	L	Da	atum: AHD	
F	Plar	nt Typ	e:	JK250	Bearing: N	/A			Lo	ogged/Checked By: W.S./P.S.	
					CORE DESCRIPTION			POINT LOAD STRENGTH		DEFECT DETAILS	
Water	Barrel Lift	RL (m AHD)	Depth (m)	Graphic Log	Rock Type, grain characteristics, colour, texture and fabric, features, inclusions and minor components	Weathering	Strength	INDEX I _s (50)	SPACING (mm)	DESCRIPTION Type, orientation, defect shape and roughness, defect coatings and seams, openness and thickness Specific General	Formation
		- 155		- - - - - - - - -	START CORING AT 4.90m						
AFTERIA	26 DAYS 1	- 154 — - -	5-		SANDSTONE: fine to medium grained, orange brown, red brown and light grey, bedded at 0-35°.	HW	VL-L	0.10 0.20 		— (5.06m) Be, 11°, P, R, Cn (5.09m) Be, 4°, P, R, Clay Ct (5.23m) Be, 4°, P, R, Clay Ct (5.30m) Be, 6°, P, R, Fe Sn (5.39m) Be, 11°, P, R, Fe Sn (5.46m) XWS, 20°, 50 mm.t (5.72m) Be, 19°, P, R, Cn	
		- 153 — - - -	7-		SANDSTONE: fine to medium grained, light grey and orange brown, bedded at 0-20°.	_		+0.030 +0.090 		(6.22-6.32m) CS, 6°, 100 mm.t (6.76m) CS, 6°, 10 mm.t	
%0	RETURN	152 - - 151 - - - -	8-		as above, but light grey.			€0.10 0.10 0.20 0.20 0.050 1.1 0.050 1.1 0.10 1.1 0.10 1.1 0.10 1.1 0.10 1.1 0.10 1.1 1.1		— (7.25m) Be, 4°, P, R, Clay Ct — (8.57m) Be, 12°, P, R, Clay Vn — (8.63m) Be, 12°, Ir, R, Clay Vn — (8.70m) Be, 14°, P, R, Cn	Hawkesbury Sandstone
		- 150 — - - -	9 - 10 -		SANDSTONE: fine to medium grained,	-		•0.070 •0.050 •0.20		(9.57-9.65m) XWS, 14° (9.95-10.00m) XWS, 6°	
		149- - - RIGHT			light grey.			•0.20		- - - - - - - - - - - - - - - - - - -	



CORED BOREHOLE LOG



(Clie	ent:		ERILY	AN						
	-	ject:			DSED MEDICAL CENTRE	אר בי				14	
_		ation			ST CENTRAL BUSINESS PAP			JHS FURE			
		• No.: e: 1/8		5058	Core Size: Inclination:			A I		. L. Surface: 159.31 m atum: AHD	
				JK250	Bearing: N		1107			ogged/Checked By: W.S./P.S.	
					CORE DESCRIPTION			POINT LOAD		DEFECT DETAILS	
Water	Loss\Level Barrel Lift	RL (m AHD)	Depth (m)	Graphic Log	Rock Type, grain characteristics, colour, texture and fabric, features, inclusions and minor components	Weathering	Strength	STRENGTH INDEX I _s (50)	SPACING (mm)	DESCRIPTION Type, orientation, defect shape and roughness, defect coatings and seams, openness and thickness Specific General	Formation
0%	RETURN	- 148			SANDSTONE: fine to medium grained, light grey. (<i>continued</i>)	DW	VL	•0.080 		-	
		- 147 – -	-12-		END OF BOREHOLE AT 12.00 m					-	
		- - 146 - -	13-							- 	
		- 145 -	14 -						-	-	
		- 144 - -	15-								
		- 143 - - -	16 -	- - - - - - - - - - - - -							
						FRACT	JRES	 		- - - - - - - - - - DERED TO BE DRILLING AND HANDLING BR	REAKS







C	lient:	ERILY	AN												
	roject:	PROP		DМ	EDIC	CA	L CEN	ITRE							
	ocation:							ESS PARK, FRENCHS FORE	EST, NS	W					
J	ob No.: 32	2505S					Me	thod: SPIRAL AUGER	R	R.L. Surface: 160.25 m					
D	ate: 2/8/19)			Datum: AH										
Р	lant Type:	JK250					Log	gged/Checked By: W.S./P.S.							
Groundwater Record	SAMPLES	Field Tests	RL (m AHD)	Depth (m)	Graphic Log		Unified Classification	DESCRIPTION	Moisture Condition/ Weathering	Strength/ Rel Density	Hand Penetrometer Readings (kPa)	Remarks			
DRY ON COMPLETION OF AUGERING			- 160	_				FILL: Silty clayey gravel, fine to medium grained sub-angular igneous gravel, trace of fine to medium grained sand.	м			GRAVEL COVER			
CON		N = 4 3,2,2	-	- - 1-			CL	Silty CLAY: high plasticity, red brown and orange brown, trace of fine to medium grained ironstone gravel and root fibres.	w>PL	St - VSt	150 350 450	RESIDUAL			
			159	-				Silty CLAY: high plasticity, light grey.	w~PL	VSt - Hd		-			
		N = 19 6,8,11	- - 158 -	2-			-	Extremely Weathered siltstone: silty	XW	Hd	420 550 >600	- - - - - - - - - - - - - - - - - - -			
			- - 157 -					CLAY, high plasticity, light grey.				- SANDSTONE - - - - - - - - - - - - - - - - - - -			
			- 156 — -	4								- - - - - - - - - - -			
			-	5—				SILTSTONE: dark grey.	HW	L		LOW RESISTANCE			
			155	6				REFER TO CORED BOREHOLE LOG							
			154 — - -	-								-			
JKGeotechnics









BOREHOLE LOG



Client:ERILYANProject:PROPOSED MEDICLocation:FOREST CENTRAL									ITRE ESS PARK, FRENCHS FORE	ST, NS	W		
Jo	ob No).: 32	2505S					Me	thod: SPIRAL AUGER	R	L. Sur	face:	159.98 m
D	ate: 2	2/8/19	9							Da	atum:	AHD	
Ρ	lant T	ype:	JK250					Log	gged/Checked By: W.S./P.S.				
Groundwater Record	SAMPLES DO SO SO SO SO SO SO SO SO SO SO SO SO SO		Field Tests	RL (m AHD)	Depth (m)	Graphic Log		Unified Classification	DESCRIPTION	Moisture Condition/ Weathering	Strength/ Rel Density	Hand Penetrometer Readings (kPa)	Remarks
ETION				-	_	\bigotimes	Ø	011	FILL: Silty clay, medium plasticity, dark $_{\rm J}$ grey, with fine to medium grained $_{\rm L}$	w>PL			-
COMPLETION OF AUGERING			N = 11 6,6,5	- - 159 —	- - 1—			CH	\igneous gravel, trace of sand. Silty CLAY: high plasticity, light grey, dark grey and orange brown, trace of fine to medium grained ironstone gravel, ash and root fibres.	w>PL		350 400 500	- RESIDUAL
				-	-				Silty CLAY: high plasticity, light grey.				-
			N > 7 6,7/ 10mm REFUSAL	-	=			-	Extremely Weathered siltstone: silty CLAY, high plasticity, light grey.	XW	Hd	>600 >600 >600	HAWKESBURY SANDSTONE
				158 — - - 157 — - -	2				SILTSTONE: light grey.	HW	VL - L		VERY LOW 'TC' BIT RESISTANCE
				- - 156 - -	- 4 - -								-
				- 155 — -	5				REFER TO CORED BOREHOLE LOG				GROUNDWATER MONITORING WELL INSTALLED TO 12.1m. CLASS 18 MACHINE SLOTTED 50mm DIA. PVC STANDPIPE 12.1m TO 9.1m. CASING 9.1m TO
				- - 154 - - -	- 6 - -								 Om. 2mm SAND FILTER PACK 12.1m TO 8.0m. BENTONITE SEAL 8.0m TO 0.1m. BACKFILLED WITH SAND TO THE SURFACE. COMPLETED WITH A CONCRETED GATIC COVER.
	PYRIGI			_									-





•	Clie	nt:												
		ject: ation			DSED MEDICAL CENTRE	סוג בו			000	- NI	S/W			
	Location: FOREST CENTRAL BUSINESS PAR Job No.: 32505S Core Size: 1													
		e: 2/8		0000	Inclination:		-	AL.		R.L. Surface: 159.98 m Datum: AHD				
				JK250						Logged/Checked By: W.S./P.S.				
-	Т				CORE DESCRIPTION			POINT LOAI			DEFECT DETAILS			
Water	Loss/Level Barrel Lift	RL (m AHD)	Depth (m)	Graphic Log	Rock Type, grain characteristics, colour, texture and fabric, features, inclusions and minor components	Weathering	Strength	STRENGTH INDEX I₅(50)	(r	CING nm)	Type, orientation, defect shape and roughness, defect coatings and seams, openness and thickness	Formation		
			5. 6. 		SANDSTONE: fine to medium grained, light grey and orange brown, bedded at 0-25°. as above, but light grey and red brown. SANDSTONE: fine to medium grained, light grey, bedded at 0-20°.	HW	M	+0.040 +0.040 +0.040 +0.030 +0.040 +0.10 +0.10 +0.10 +0.090 +0.040 +0.090 +0.040 +0.10 +0.				Hawkesbury Sandstone		
						FRACTI				 8 8 	E SIDERED TO BE DRILLING AND HANDLING BRI			





	Cli	ier	nt:		ERILY	AN						
	•		OSED MEDICAL CENTRE									
	Location: FOREST CENTRAL BUSINESS PARK, FRENCHS FOREST, NSW											
Job No.: 32505S Core Size: NMLC R.L. Surface: 159								L. Surface: 159.98 m				
			: 2/8			Inclination:		TICA	NL.		atum: AHD	
	Pla	ant	t Typ	e:	JK250	Bearing: N	/A		,		ogged/Checked By: W.S./P.S.	_
			(C		D	CORE DESCRIPTION	D		POINT LOAD STRENGTH INDEX	SPACING	DEFECT DETAILS DESCRIPTION	
Water	Loss/Leve	Barrel Lift	RL (m AHD)	Depth (m)	Graphic Log	Rock Type, grain characteristics, colour, texture and fabric, features, inclusions and minor components	Weathering	Strength	INDEX الم الم 1° (20) EH 1° (20)	(mm)	Type, orientation, defect shape and roughness, defect coatings and seams, openness and thickness Specific General	Formation
			-			SANDSTONE: fine to medium grained, light grey.	HW	VL - L	•0.060			tone
~	z		-		-	as above, but light grey and red brown.						Sands
100	RETURN		-									bury :
			- 148	40	-				0.10			Hawkesbury Sandstone
_	+			12-	_	END OF BOREHOLE AT 12.10 m						<u> </u>
7-00-01 07			-		-							
0.10.8 M			-		-							
10-21 Fil:			- 147	13-	-						_	
72.4 2013			-	15	-							
			-									
01-020			-									
			- 146	14 -	-							
si Lad and			-	14-	_						-	
1.01 Datg			-							250 00 00 00 00 00 00 00 00 00 00 00 00 0		
0.10.01			-									
2.118 14:2			145	15-	-						_	
R0/00 <<			-	15	-							
rawingrie			-									
6FJ <<0			-									
PORES			- 144 —	16-	-						-	
LIKENOLIS				10	-							
S0025 7			-		-							
- MAO I EL			-		-							
REPOLE			143	17-	-						· ·	
			-	.,	-							
n vr Gon			-		-							
LID.GLD			-		-							
470.8 VL					-					- 660 - - 260 - - 260 - - 260 -		
CC	ΝP		GHT				FRACTI	IRES			ERED TO BE DRILLING AND HANDLING BR	EVKG







Client:			ERILY	AN									
1	Pro	ject:		PROP	OSED MEDICAL CENTRE								
Location: FOREST CENTRAL BUSINE						ESS PARK, FRENCHES FOREST, NSW							
.	Job No.: 32505S2				Core Size:	R.	.L. Surface: ~159.0 m						
	Dat	e: 22/	1/2	0	Inclination:	VER	TICA	L	Da	atum: AHD			
	Pla	nt Typ	oe:	JK305	Bearing: N	/A			Logged/Checked By: W.S./M.P.				
				D	CORE DESCRIPTION	_		POINT LOAD STRENGTH	SPACING	DEFECT DETAILS DESCRIPTION	_		
Water		RL (m AHD)	Depth (m)	Graphic Log	Rock Type, grain characteristics, colour, texture and fabric, features, inclusions and minor components	Weathering	gth	INDEX I _s (50)	(mm)	Type, orientation, defect shape and roughness, defect coatings and	Formation		
Wate	Loss/Leve Rarrel I ift	RL (n	Dept	Grapl		Weat	Strength	ст ст ст ст ст ст ст ст ст ст ст ст ст с	600 60 2 20 0	seams, openness and thickness Specific General	Form		
		_		-									
		-	-	-						-			
		-	-							-			
		-		-						-			
3		148-	11-	-						-			
2		-	-	-						-			
		-	-							-			
-	+				START CORING AT 11.80m SANDSTONE: fine to medium grained,	MW	VL - L	0.20		WATER LEVEL AFTER CORING APPROX. 3.2m			
		147 -	12-	-	light grey, bedded at 0-10°.					-			
]				•0.20		-	tone		
2		-	-	-						- (12.61m) J, 14°, Ir, R, Cn	Hawkesbury Sandstone		
		-	-					•0.060		-	pury S		
3		146 -	13-	-				•0.090		-	wkes		
afino In		-								-	На		
001001		1 -	-	_					 	-			
100%	TURN	-	-		Silty CLAY: medium plasticity, light grey.	RS	Hd	0.080		- ─ ─── (13.80m) HPR: >550kPa -			
	R	145 -	14 -	-	SANDSTONE: fine to medium grained,	MW	VL - L			-	bne		
Coll Bull		-			light grey, bedded bedded at 0-10°.			0.30		- (14.30m) Be, 8°, P, R, Clay Vn (14.40m) J 10° h P, Ca	andsto		
								0.10		– —— (14.40m) J, 12°, Ir, R, Cn – –	ury S		
2.4		-	-	-						- -	vkesb		
		144 -	15-		Silty CLAY: medium plasticity, light grey.	RS	Hd			-	Hav		
		-						•0.050		(15.20m) HPR: ≻600kPa -	dstone		
		-			SANDSTONE: fine to medium grained, light grey, bedded at 0-10°.	MW	VL - M	0.030		-	y San		
		-	-	_						- 	esbur		
		143-	16-	_	END OF BOREHOLE AT 15.90 m						Hawkesbury Sandstone Hawkesbury Sandstone		
		-		-						-			
5				-						-			
		-		-						-			
	PYI	 RIGHT		1		 FRACTI	JRES N			- DERED TO BE DRILLING AND HANDLING BR			







Client: E		ERILYAN									
F	roj	ect:		PROP	OSED MEDICAL CENTRE						
Location: FOREST CENTRA			:	FORE	ST CENTRAL BUSINESS PAF	CENTRAL BUSINESS PARK, FRENCHES FOREST, NSW					
J	ob	No.:	32	505S2	Core Size:	NML	R	.L. Surface: ~160.0 m			
C	ate	e: 22/	1/2	0	Inclination: VERTICAL				D	atum: AHD	
F	lan	t Typ	e:	JK305	Bearing: N	/A			Le	ogged/Checked By: W.S./M.P.	
					CORE DESCRIPTION			POINT LOAD		DEFECT DETAILS	_
Nater oss\I evel	Barrel Lift	RL (m AHD)	Depth (m)	Graphic Log	Rock Type, grain characteristics, colour, texture and fabric, features, inclusions and minor components	Weathering	Strength	INDEX I _s (50)	(mm)	DESCRIPTION Type, orientation, defect shape and roughness, defect coatings and seams, openness and thickness Specific General	Formation
		-148-			START CORING AT 12.00m					WATER LEVEL AFTER CORING APPROX. 4.7m	
		- - - 147 - -	13·		NO CORE 1.55m						
100%		- - 146 - -	14-		SANDSTONE: fine to medium grained, light grey and brown, bedded between 0-10°.	MW	VL L	•0.060 •0.070 •1 •0.070 	600 - 200 - 200 - - -		
		-			Extremely weathered sandstone: silty sandy CLAY, medium plasticity, light	xw	Hd			– – —— (14.80m) HPR: 380kPa –	e
b		- 145	15		grey, with occasional low strength bands.			•0.20			Hawkesbury Sandston
		- - 144 —	16 ·					•0.020 		- (15.71m) HPR: 530kPa 	Hawkest
		-			SANDSTONE: fine to medium grained, light grey, bedded between 0-10°.	MW	L - M				
		- 143 - -	17·		Extremely weathered sandstone: silty sandy CLAY, medium plasticity, light	XW	(Hd)			- (17.05m) HPR: ????kPa 	
		- -		- - - - -	grey. SANDSTONE: fine to medium grained, light grey, bedded between 0-10°. END OF BOREHOLE AT 17.30 m				- 600	- - - - -	
<u> </u>				1	1						

COPYRIGHT

FRACTURES NOT MARKED ARE CONSIDERED TO BE DRILLING AND HANDLING BREAKS





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	<u>+</u>	
	 +	
	<u> </u>	
SUMMARY	DRAWN D.M.	DATE 18/02/2020
JUIVIIVIAK I	CHECKED	DATE
IESS PARK,	M.P.	18/02/2020
, NSW	H 1:100 V 1:10	00 A3
CENTRE	^{PROJECT №} 32505BMrev4	A



APPENDIX B

Fact sheet

Exemptions Construction dewatering





This fact sheet provides information to local authorities and applicants seeking development consent that may involve dewatering activities. It outlines exemptions from the need to obtain certain approvals/licences under the *Water Management Act 2000* (WMA). This fact sheet should be read in conjunction with the <u>Dewatering information for councils and applicants fact sheet</u>.

If intending to rely on one of the exemptions below, it is necessary to understand requirements that may involve recording water extraction and the time limits that apply to the exemption. It is important to obtain any further advice before commencing the development application process.

Note that as part of dewatering works you may need other approvals included under the Local Government Act 1993 or Roads Act 1993 to dispose of the water into council drains, use any part of the public footpath or carriageway or undertake any work within the public road.

There are two exemptions that apply from 6 December 2019.

Exemption 1: For extraction of less than 3 ML of water per year

When water extraction is taken as part of approved development (or exempt development), up to 3 ML of water may be taken in any one year (commencing on 1 July each year) without the need for:

- a water access licence (WAL)
- a water use approval.

A water supply work approval will be required for any works that are to be constructed or used to drain or pump the water.

To rely on this exemption, certain requirements must be met to record the water extraction and ensure that less than 3 ML of water is taken. These requirements are set out in clause 21(6) of the Water Management (General) Regulation 2018 and include requirements to:

- record the water take within 24 hours in the approved form and manner (see the Completion report fact sheet)
- keep the water take records for a period of five years
- provide the water take records to the Minister (or WaterNSW) by no later than 28 July for the year ending 1 July during which the water was taken.

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Exemptions Construction dewatering

Exemption 2: For construction activities that take water from the Botany Sands Groundwater Source only

When water extraction is taken as part of construction activities for a building, road or other infrastructure from the Botany Sands Groundwater Source, it may not require:

- a water access licence (WAL) or
- a water use approval

if a water supply work approval (e.g. for a pump) has already been obtained.

The water supply work approval is required to specify the maximum amount of water that can be taken during a year.

This exemption is only applicable until the earlier of:

- a controlled allocation of the water in the water source is opened by the Department of Industry, Planning and Environment (DPIE)
- 1 July 2021.

This exemption cannot be relied upon if construction activity is likely to continue beyond 1 July 2021 (unless the law changes).

In order to take water after 1 July 2021, the required water allocation must be purchased in the water market from that water source (or a transferable water source allocation).

To rely on the exemption, the conditions of the water supply work approval must be met and may require metering of the water take.

A water supply work approval will still need to be obtained for any works that are to be constructed or used to drain or pump the water.

Currently, the exemption only applies to the Botany Sands Groundwater Source, but other sources may be added and it is best to check with WaterNSW for the latest information.

More information

If you have any questions, please contact one of our friendly Customer Service team on 1300 662 077 or email <u>Customer.Helpdesk@waternsw.com.au</u>

Disclaimer:

This fact sheet is provided for general information purposes only and may not cover the precise circumstances of your development. It is only relevant to the particular matters identified in this fact sheet. There may be other processes and relevant fact sheets that are also relevant to your development. Links to all fact sheets related to construction dewatering may be found at <u>waternsw.com.au/dewatering</u>. This fact sheet is not legal advice and should not be relied upon as such. Interested persons should obtain their own advice. This fact sheet does not represent the views of any council or the Department of Planning, Industry and Environment or Natural Resources Access Regulator. This fact sheet represents an interim position and may be updated at any time. Please check the WaterNSW website for the current version. WaterNSW is not liable for consequences of actions taken in reliance of information provided or omitted from this document.

