GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER
FORM NO. 1 – To be submitted with Development Application

Development Application for _____
Name of Applicant

Address of site 103 Narrabeen Park Parade, Mona Vale

The following checklist covers the minimum requirements to be addressed in a Geotechnical Risk ***Declaration made by geotechnical engineer or engineering geologist or coastal engineer (where applicable) as part of a geotechnical report***

I, Ben White *on behalf of* White Geotechnical Group Pty Ltd
(Insert Name) (Trading or Company Name)

on this the 29/7/20 certify that I am a geotechnical engineer or engineering geologist or coastal engineer as defined by the Geotechnical Risk Management Policy for Pittwater - 2009 and I am authorised by the above organisation/company to issue this document and to certify that the organisation/company has a current professional indemnity policy of at least $10million.

I:
Please mark appropriate box

☒ have prepared the detailed Geotechnical Report referenced below in accordance with the Australia Geomechanics Society's Landslide Risk Management Guidelines (AGS 2007) and the Geotechnical Risk Management Policy for Pittwater - 2009

☒ am willing to technically verify that the detailed Geotechnical Report referenced below has been prepared in accordance with the Australian Geomechanics Society's Landslide Risk Management Guidelines (AGS 2007) and the Geotechnical Risk Management Policy for Pittwater - 2009

☐ have examined the site and the proposed development in detail and have carried out a risk assessment in accordance with Section 6.0 of the Geotechnical Risk Management Policy for Pittwater - 2009. I confirm that the results of the risk assessment for the proposed development are in compliance with the Geotechnical Risk Management Policy for Pittwater - 2009 and further detailed geotechnical reporting is not required for the subject site.

☐ have examined the site and the proposed development/alteration in detail and I am of the opinion that the Development Application only involves Minor Development/Alteration that does not require a Geotechnical Report or Risk Assessment and hence my Report is in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009 requirements.

☐ have examined the site and the proposed development/alteration is separate from and is not affected by a Geotechnical Hazard and does not require a Geotechnical Report or Risk Assessment and hence my Report is in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009 requirements.

☐ have provided the coastal process and coastal forces analysis for inclusion in the Geotechnical Report

Geotechnical Report Details:

Report Title: Geotechnical Report **103 Narrabeen Park Parade, Mona Vale**

Report Date: 29/7/20

Author: BEN WHITE

Author's Company/Organisation: **WHITE GEOTECHNICAL GROUP PTY LTD**

Documentation which relate to or are relied upon in report preparation:

Australian Geomechanics Society Landslide Risk Management March 2007.

White Geotechnical Group company archives.

I am aware that the above Geotechnical Report, prepared for the abovementioned site is to be submitted in support of a Development Application for this site and will be relied on by Pittwater Council as the basis for ensuring that the Geotechnical Risk Management aspects of the proposed development have been adequately addressed to achieve an "Acceptable Risk Management" level for the life of the structure, taken as at least 100 years unless otherwise stated and justified in the Report and that reasonable and practical measures have been identified to remove foreseeable risk.

Signature _____

Name Ben White

Chartered Professional Status MScGEOLAusIMM CP GEOL

Membership No. 222757

Company White Geotechnical Group Pty Ltd

GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER
FORM NO. 1(a) - Checklist of Requirements for Geotechnical Risk Management Report for Development Application

Development Application for _____
Name of Applicant

Address of site 103 Narrabeen Park Parade, Mona Vale

The following checklist covers the minimum requirements to be addressed in a Geotechnical Risk Management Geotechnical Report. This checklist is to accompany the Geotechnical Report and its certification (Form No. 1).

Geotechnical Report Details:

Report Title: Geotechnical Report **103 Narrabeen Park Parade, Mona Vale**

Report Date: 29/7/20

Author: BEN WHITE

Author's Company/Organisation: WHITE GEOTECHNICAL GROUP PTY LTD

Please mark appropriate box

- ☒ Comprehensive site mapping conducted **28/11/19** (date)
- ☒ Mapping details presented on contoured site plan with geomorphic mapping to a minimum scale of 1:200 (as appropriate)
- ☒ Subsurface investigation required
  - ☐ No Justification _____
  - ☒ Yes Date conducted **28/11/19**
- ☒ Geotechnical model developed and reported as an inferred subsurface type-section
- ☒ Geotechnical hazards identified
  - ☒ Above the site
  - ☒ On the site
  - ☒ Below the site
  - ☐ Beside the site
- ☒ Geotechnical hazards described and reported
- ☒ Risk assessment conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
  - ☒ Consequence analysis
  - ☒ Frequency analysis
- ☒ Risk calculation
- ☒ Risk assessment for property conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
- ☒ Risk assessment for loss of life conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
- ☒ Assessed risks have been compared to "Acceptable Risk Management" criteria as defined in the Geotechnical Risk Management Policy for Pittwater - 2009
- ☒ Opinion has been provided that the design can achieve the "Acceptable Risk Management" criteria provided that the specified conditions are achieved.
- ☒ Design Life Adopted:
  - ☒ 100 years
  - ☐ Other _____ specify
- ☒ Geotechnical Conditions to be applied to all four phases as described in the Geotechnical Risk Management Policy for Pittwater - 2009 have been specified
- ☒ Additional action to remove risk where reasonable and practical have been identified and included in the report.
- ☐ Risk assessment within Bushfire Asset Protection Zone.

I am aware that Pittwater Council will rely on the Geotechnical Report, to which this checklist applies, as the basis for ensuring that the geotechnical risk management aspects of the proposal have been adequately addressed to achieve an "Acceptable Risk Management" level for the life of the structure, taken as at least 100 years unless otherwise stated, and justified in the Report and that reasonable and practical measures have been identified to remove foreseeable risk.

Signature _____

Name Ben White

Chartered Professional Status MScGEOLAusIMM CP GEOL

Membership No. 222757

Company White Geotechnical Group Pty Ltd

GEOTECHNICAL INVESTIGATION:

New Garage, Storeroom and Lift at **103 Narrabeen Park Parade, Mona Vale**

1. Proposed Development

1.1 Demolish the existing garage.

1.2 Construct a new suspended driveway.

1.3 Construct a new garage with a storeroom below by excavating to a maximum depth of 2.3m. Landscape a lawn/garden area uphill of the proposed garage by filling to a maximum depth of 1.2m.

1.4 Construct a new lift connecting the proposed garage with the pavement on the uphill side of the house.

1.5 Details of the proposed development are shown on 8 drawings prepared by James de Soyres + Associates Architects, project number 1905a, drawings numbered DA-01, DA-06, DA-10 to DA-12, DA-20, DA-21 and DA-30, Revision A, dated 24/7/20.

2. Site Description

2.1 The site was inspected on the 28th of November, 2019.

2.2 This residential property is on the low side of the road and has a SE aspect. The block is located on the moderately graded middle reaches of a hillslope. The natural surface falls across the property at an average angle of ~15°. The slope above the property decreases in grade and the slope below the property increases in grade.

2.3 From the road frontage, steps and a pedestrian ramp lead to a concrete and brick paved driveway, which runs down the slope to a garage just below the uphill boundary of the property (Photos 1, 2 & 3). The fill batter for the road merges into the

natural slope. In a small section adjacent to the stairs it is lined with bricks (Photo 4). The timber and weatherboard clad garage is supported on a concrete slab which shows some cracking but no movement related to slope instability (Photo 5). Between the driveway and house are garden beds and a lawn (Photos 6 & 7). An excavation has been made in the slope to provide a level platform for the house. The cut below the garden is supported by a stable stack rock retaining wall ~1.0m high (Photo 8). The cut below the lawn is supported by a stable sandstone retaining wall ~1.0m high (Photo 9).

The two storey timber and weatherboard clad house is supported on brick walls, brick piers and concrete piers (Photos 10 to 14). The supporting walls and piers show no significant signs of movement. A stable timber retaining wall supports a cut batter located underneath the downhill side of the house (Photo 13). A suspended timber deck supported on timber posts extends off the downhill side of the house. (Photo 15). A fill that provides a level platform for a stone pavement is supported by a stable timber retaining wall (Photos 12, 16 & 17). A moderately sloping lawn descends downhill to the SE boundary of the property (Photo 18). Below the boundary the surface is covered in coastal shrub and continues to Warriewood Beach (Photos 19 & 20).

3. Geology

The Sydney 1:100 000 Geological sheet indicates the site is underlain by the Newport Formation of the Narrabeen Group. This is described as interbedded laminite, shale, and quartz to lithic quartz sandstone.

4. Subsurface Investigation

One auger hole was put down to identify the soil materials. Four Dynamic Cone Penetrometer (DCP) tests were put down to determine the relative density of the overlying soil and the depth to weathered rock. The locations of the tests are shown on the site plan. It should be

noted that a level of caution should be applied when interpreting DCP test results. The test will not pass through hard buried objects so in some instances it can be difficult to determine whether refusal has occurred on an obstruction in the profile or on the natural rock surface. This is not expected to be an issue for the testing on this site. But due to the possibility that the actual ground conditions vary from our interpretation there should be allowances in the excavation and foundation budget to account for this. We refer to the appended "Important Information about Your Report" to further clarify. The results are as follows:

AUGER HOLE 1 (~RL34.5) **–** AH1 (photo 21)

Depth (m)	**Material Encountered**
0.0 to 0.2	**SANDY SOIL**, grey, fine to medium grained.
0.2 to 0.5	**CLAYEY SOIL**, grey brown, dry.
0.5 to 0.8	**SANDY CLAY**, orange brown, firm to stiff, moist.

End of hole @ 0.8m in firm to stiff sandy clay. No watertable encountered.

DCP TEST RESULTS – Dynamic Cone Penetrometer				
Equipment: 9kg hammer, 510mm drop, conical tip.			Standard: AS1289.6.3.2 - 1997	
Depth(m) Blows/0.3m	**DCP 1** (~RL36.5)	**DCP 2** (~RL34.5)	**DCP 3** (~RL33.9)	**DCP 4** (~RL32.5)
0.0 to 0.3	17	9	13	11
0.3 to 0.6	20	13	24	10
0.6 to 0.9	35	14	17	8
0.9 to 1.2	40	38	30	14
1.2 to 1.5	#	#	40	25
1.5 to 1.8			#	30
1.8 to 2.1				29
2.1 to 2.4				40
2.4 to 2.7				#
	End of Test @ 1.2m	End of Test @ 1.2m	End of Test @ 1.4m	End of Test @ 2.3m

#refusal/end of test. F=DCP fell after being struck showing little resistance through all or part of the interval.

DCP Notes:

DCP1 – End of test @ 1.2m, DCP still very slowly going down, orange shale on dry tip.
DCP2 – End of test @ 1.2m, DCP still very slowly going down, sandy soil on dry tip.
DCP3 – End of test @ 1.4m, DCP still very slowly going down, white brown clay on dry tip.
DCP4 – End of test @ 2.3m, DCP still very slowly going down, orange and maroon shale on dry tip.

5. Geological Interpretation

The slope materials are colluvial at the near surface and residual at depth. They consist of a sandy topsoil over clayey soils and sandy clays. In the test locations, the clays merge into the weathered zone of the underlying rocks at a depth from between ~1.2m to ~2.3m below the current surface. The weathered zone of the underlying rock is interpreted as Extremely Low Strength Shale. It is to be noted that this material is a soft rock and can appear as a mottled stiff clay when it is cut up by excavation equipment. See Type Section attached for a diagrammatical representation of the expected ground materials.

6. Groundwater

Normal ground water seepage is expected to move over the buried surface of the clay and rock and through the cracks in the rock.

Due to the slope and elevation of the block, the water table in the location is expected to be many metres below the base of the proposed excavation.

7. Surface Water

No evidence of significant surface flows were observed on the property during the inspection. Normal sheet wash from the slope above will be intercepted by the street drainage system for Narrabeen Park Parade above.

8. Geotechnical Hazards and Risk Analysis

No geotechnical hazards were observed beside the property. The moderate to steeply graded slope that falls across the property and continues above and below is a potential hazard

(**Hazard One**). The excavation for the proposed garage and store room is a potential hazard until retaining walls are place (**Hazard Two**).

Geotechnical Hazards and Risk Analysis - Risk Analysis Summary

HAZARDS	Hazard One	Hazard Two
TYPE	The moderate to steep slope that falls across the property and continues above and below failing and impacting on the property.	The proposed excavation collapsing onto the worksite during the excavation process.
LIKELIHOOD	'Unlikely' (10^{-4})	'Possible' (10^{-3})
CONSEQUENCES TO PROPERTY	'Medium' (12%)	'Medium' (15%)
RISK TO PROPERTY	'Low' (2×10^{-5})	'Moderate' (2×10^{-4})
RISK TO LIFE	8.3×10^{-7}/annum	8.3×10^{-6}/annum
COMMENTS	'ACCEPTABLE' level of risk to life & property.	This level of risk to life and property is 'UNACCEPTABLE'. To move the risk to 'ACCEPTABLE' levels, the recommendations in **Section 13** are to be followed.

(See Aust. Geomech. Jnl. Mar 2007 Vol. 42 No 1, for full explanation of terms)

9. Suitability of the Proposed Development for the Site

The proposed development is suitable for the site. No geotechnical hazards will be created by the completion of the proposed development provided it is carried out in accordance with the requirements of this report and good engineering and building practice.

10. Stormwater

Warriewood Beach Reserve is immediately below the property so stormwater can be piped to the downhill boundary through a diffuser or spreader.

11. Excavations

An excavation to a maximum depth of ~2.3m will be required to construct the proposed garage and storeroom. The excavation is expected to be through soil and fill over clay, with Extremely Low Strength Shale expected at a depth of ~1.2m. Excavations through soil, fill, clay and Extremely Low Strength Shale can be carried out with an excavator and bucket.

12. Vibrations

It is expected the proposed excavation will be carried out with an excavator and bucket and the vibrations produced will be below the threshold limit for building or infrastructure damage.

13. Excavation Support Requirements

The excavation for the proposed garage and storeroom will reach a maximum depth of ~2.3m. The excavation is set back sufficiently from the property boundaries and adjoining structures to negate excavation induced instability.

The soil portion of the proposed excavation is to be battered temporarily at 1.0 Vertical to 1.7 Horizontal (30°) until the retaining walls are in place. Cut batters through clay and Extremely Low Strength Shale will stand at near-vertical angles for short periods of time until the retaining walls are installed, provided the cut batters are kept from becoming saturated.

During the excavation process, the geotechnical consultant is to inspect the cut face in 1.5m intervals as it is lowered to ensure ground materials are as expected and that additional support is not required.

Upslope runoff is to be diverted from the cut faces by sandbag mounds or other diversion works. All unsupported cut batters are to be covered to prevent access of water in wet weather and loss of moisture in dry weather. The materials and labour to construct the retaining walls are to be organised so on completion of the excavations they can be constructed as soon as possible. The excavations are to be carried out during a dry period. No

excavations are to commence if heavy or prolonged rainfall is forecast. If the retaining walls are not constructed within a few days of the excavation being completed temporary shoring will be required.

All excavation spoil is to be removed from site or is to be supported by engineered retaining walls.

14. Fill

Fill will be placed on the slope on the uphill side of the proposed garage for landscaping. Excavation spoil may be used as fill but it is to be stored well back and outside the zone of influence of the excavation faces. No fills are to be laid until the retaining walls are in place.

The fill will reach a maximum depth of ~1.2m. The surface is to be prepared before any fills are laid by removing any organic matter and topsoil. Fills for landscaping purposes are to be laid in a loose thickness not exceeding 0.3m before being moderately compacted. Tracking the machine over the loose fill in 1 to 2 passes should be sufficient. Immediately behind the retaining walls (say to 1.5m), the fills are to be compacted with light weight equipment such as a hand-held plate compactor so as not to damage the retaining walls. Where light weight equipment is used, fills are to be laid in a loose thickness not exceeding 0.2m before being compacted. No structures are to be supported on landscaped fill.

15. Retaining Walls

For cantilever or singly propped retaining structures it is suggested the design be based on a triangular distribution of lateral pressures using the parameters shown in Table 1.

TABLE 1 ON NEXT PAGE

Table 1 – Likely Earth Pressures for Retaining Structures

Unit	Earth Pressure Coefficients		
	Unit weight (kN/m^3)	'Active' K_a	'At Rest' K_0
Sandy Soil, Clayey Soil	20	0.40	0.55
Residual Clays	20	0.35	0.45
Extremely Low Strength Shale	22	0.25	0.35

For rock classes refer to Pells et al "Design Loadings for Foundations on Shale and Sandstone in the Sydney Region". Australian Geomechanics Journal 1978.

It is to be noted that the earth pressures in Table 1 assume a level surface above the structure, do not account for any surcharge loads and assume retaining structures are fully drained.

Rock strength and relevant earth pressure coefficients are to be confirmed on site by the geotechnical consultant.

All retaining structures are to have sufficient back-wall drainage and be backfilled immediately behind the structure with free draining material (such as gravel). This material is to be wrapped in a non-woven Geotextile fabric (i.e. Bidim A34 or similar), to prevent the drainage from becoming clogged with silt and clay. If no back-wall drainage is installed in retaining structures the full hydrostatic pressures are to be accounted for in the retaining structure design.

16. Foundations

The proposed suspended driveway and lift are to be supported on piers embedded into the underlying Extremely Low Strength Shale. This ground material is expected at depths of between ~1.2m to ~1.4m below the natural surface. The garage and storeroom is expected to be seated in Extremely Low Strength Shale. On the downhill side where the rock drops away with the slope, shallow spread footings will be required to maintain a uniform bearing

pressure across the structure. A maximum allowable bearing pressure of 600kPa can be assumed for footings embedded in Extremely Low Strength Shale. It should be noted that this material is a soft rock and a rock auger will cut through it so the builders should not be looking for refusal to end the footings.

As the bearing capacity of shale reduces when it is wet we recommend the footings be dug, inspected and poured in quick succession (ideally the same day if possible). If the footings get wet, they will have to be drained and the soft layer of shale on the footing surface will have to be removed before concrete is poured.

If a rapid turnaround from footing excavation to the concrete pour is not possible a sealing layer of concrete may be added to the footing surface after it has been cleaned.

NOTE: If the contractor is unsure of the footing material required, it is more cost-effective to get the geotechnical consultant on site at the start of the footing excavation to advise on footing depth and material. This mostly prevents unnecessary over-excavation in clay like shaly rock but can be valuable in all types of geology.

17. Inspections

The client and builder are to familiarise themselves with the following required inspections as well as council geotechnical policy. We cannot provide geotechnical certification for the owners or the Occupation Certificate if the following inspections have not been carried out during the construction process.

- During the excavation process, the geotechnical consultant is to inspect the cut face in 1.5m intervals as it is lowered to ensure ground materials are as expected and that additional support is not required.

- All footings are to be inspected and approved by the geotechnical consultant while the excavation equipment is still onsite and before steel reinforcing is placed or concrete is poured.

White Geotechnical Group Pty Ltd.

Ben White M.Sc. Geol.,
AusIMM., CP GEOL.
No. 222757
Engineering Geologist

Photo 1

Photo 2

Photo 3

Photo 4

Photo 5

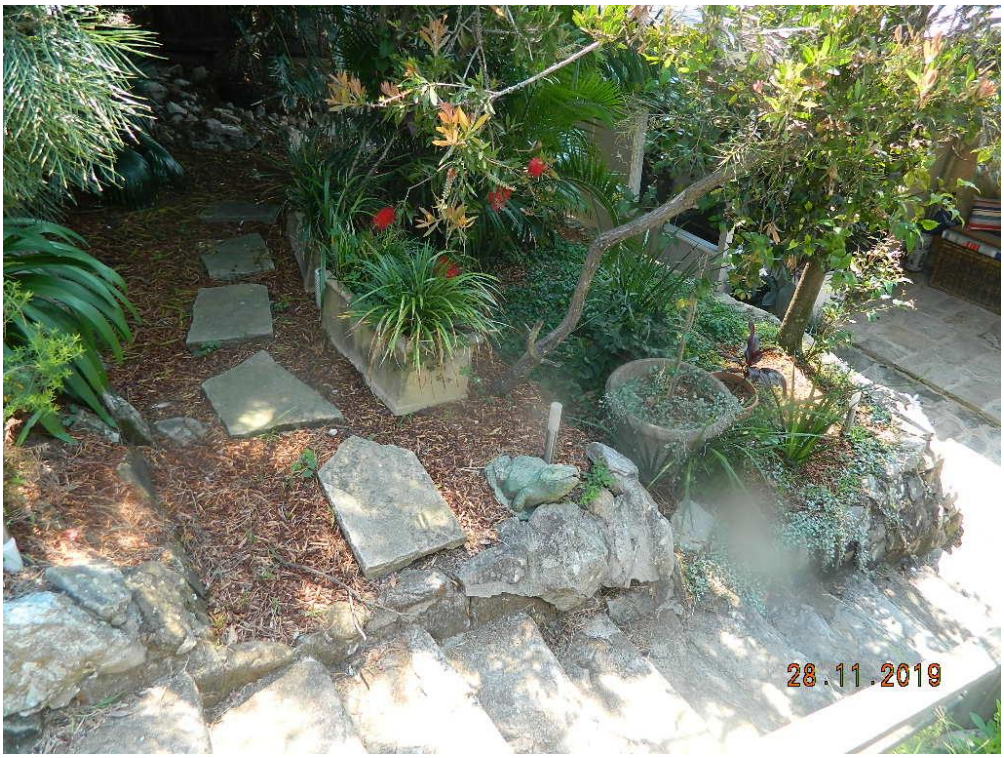

Photo 6

Photo 7

Photo 8

Photo 9

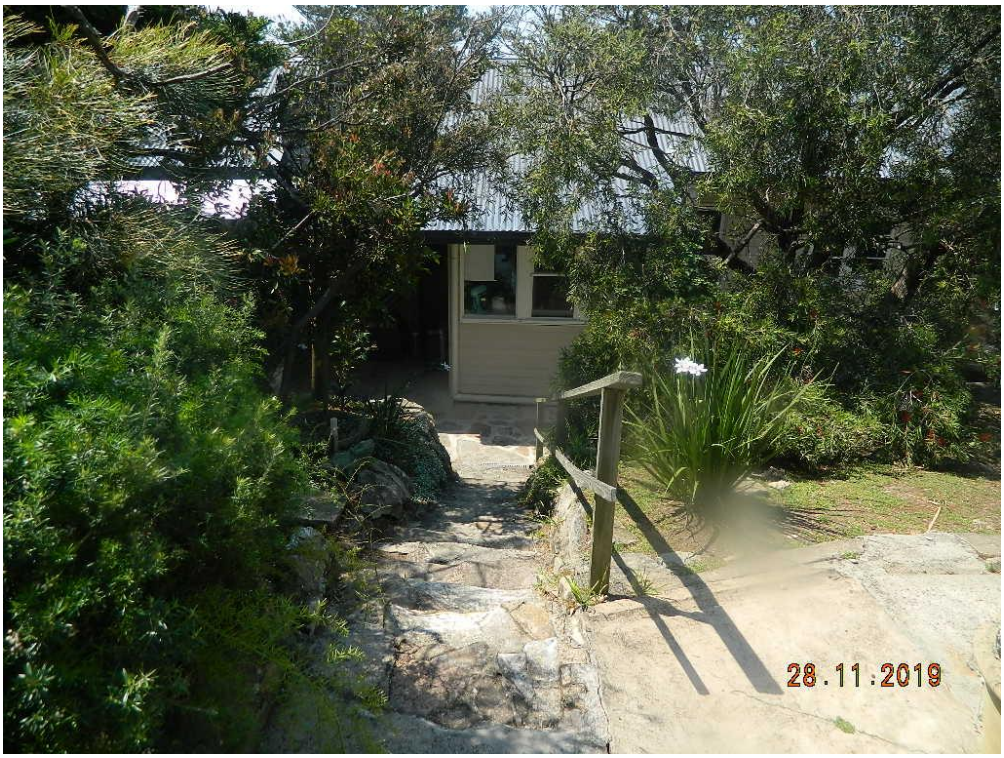

Photo 10

Photo 11

Photo 12

Photo 13

Photo 14

Photo 15

Photo 16

Photo 17

Photo 18

Photo 19

Photo 20

Photo 21: AH1 – Downhole is from top to bottom

Important Information about Your Report

It should be noted that Geotechnical Reports are documents that build a picture of the subsurface conditions from the observation of surface features and testing carried out at specific points on the site. The spacing and location of the test points can be limited by the location of existing structures on the site or by budget and time constraints of the client. Additionally, the test themselves, although chosen for their suitability for the particular project, have their own limiting factors. The testing gives accurate information at the location of the test, within the confines of the test's capability. A geological interpretation or model is developed by joining these test points using all available data and drawing on previous experience of the geotechnical consultant. Even the most experienced practitioners cannot determine every possible feature or change that may lie below the earth. All of the subsurface features can only be known when they are revealed by excavation. As such, a Geotechnical report can be considered an interpretive document. It is based on factual data but also on opinion and judgement that comes with a level of uncertainty. This information is provided to help explain the nature and limitations of your report.

With this in mind, the following points are to be noted:

- If upon the commencement of the works the subsurface ground or ground water conditions prove different from those described in this report, it is advisable to contact White Geotechnical Group immediately, as problems relating to the ground works phase of construction are far easier and less costly to overcome if they are addressed early.

- If this report is used by other professionals during the design or construction process, any questions should be directed to White Geotechnical Group as only we understand the full methodology behind the report's conclusions.

- The report addresses issues relating to your specific design and site. If the proposed project design changes, aspects of the report may no longer apply. Contact White Geotechnical if this occurs.

- This report should not be applied to any other project other than that outlined in section 1.0.

- This report is to be read in full and should not have sections removed or included in other documents as this can result in misinterpretation of the data by others.

- It is common for the design and construction process to be adapted as it progresses (sometimes to suit the previous experience of the contractors involved). If alternative design and construction processes are required to those described in this report, contact White Geotechnical Group. We are familiar with a variety of techniques to reduce risk and can advise if your proposed methods are suitable for the site conditions.

SITE PLAN – showing test locations

NOTES 1. ONLY VISIBLE UTILITY SERVICES HAVE BEEN LOCATED. RELEVANT AUTHORITIES SHOULD BE NOTIFIED PRIOR TO EXCAVATION ON OR NEAR THE SITE.
2. NO BOUNDARY DEFINITION HAS BEEN CREATED. IF ANY CONSTRUCTION IS PROPOSED IN PROXIMITY OF BOUNDARIES, IT IS RECOMMENDED THAT A FURTHER SURVEY BE REQUESTED FOR MARKING OF RELEVANT BOUNDARIES.
3. COVENANTS, RESTRICTIONS ON LAND USE (IF ANY) NOT INVESTIGATED.
4. ROOF LINES SHOWN ARE APPROXIMATE. SIGNIFICANT TREES, GREATER THAN 0·4 DIAM. SHOWN
5. APPROX. SEWER LINE MUST BE VERIFIED BY SYDNEY WATER.

NOTE: CONTOURS ARE FOR INDICATIVE PURPOSES ONLY. FOR MORE CONCLUSIVE EVIDENCE OF LEVELS USE THE SPOT HEIGHTS GIVEN.

103–105 NARRABEEN PARK PARADE MONA VALE

DETAILED SURVEYS
(A.B.N. 36 233 529 164)
CONSULTING SURVEYORS
87 ELANORA ROAD, ELANORA HEIGHTS, 2101
PHONE: 9913–9525
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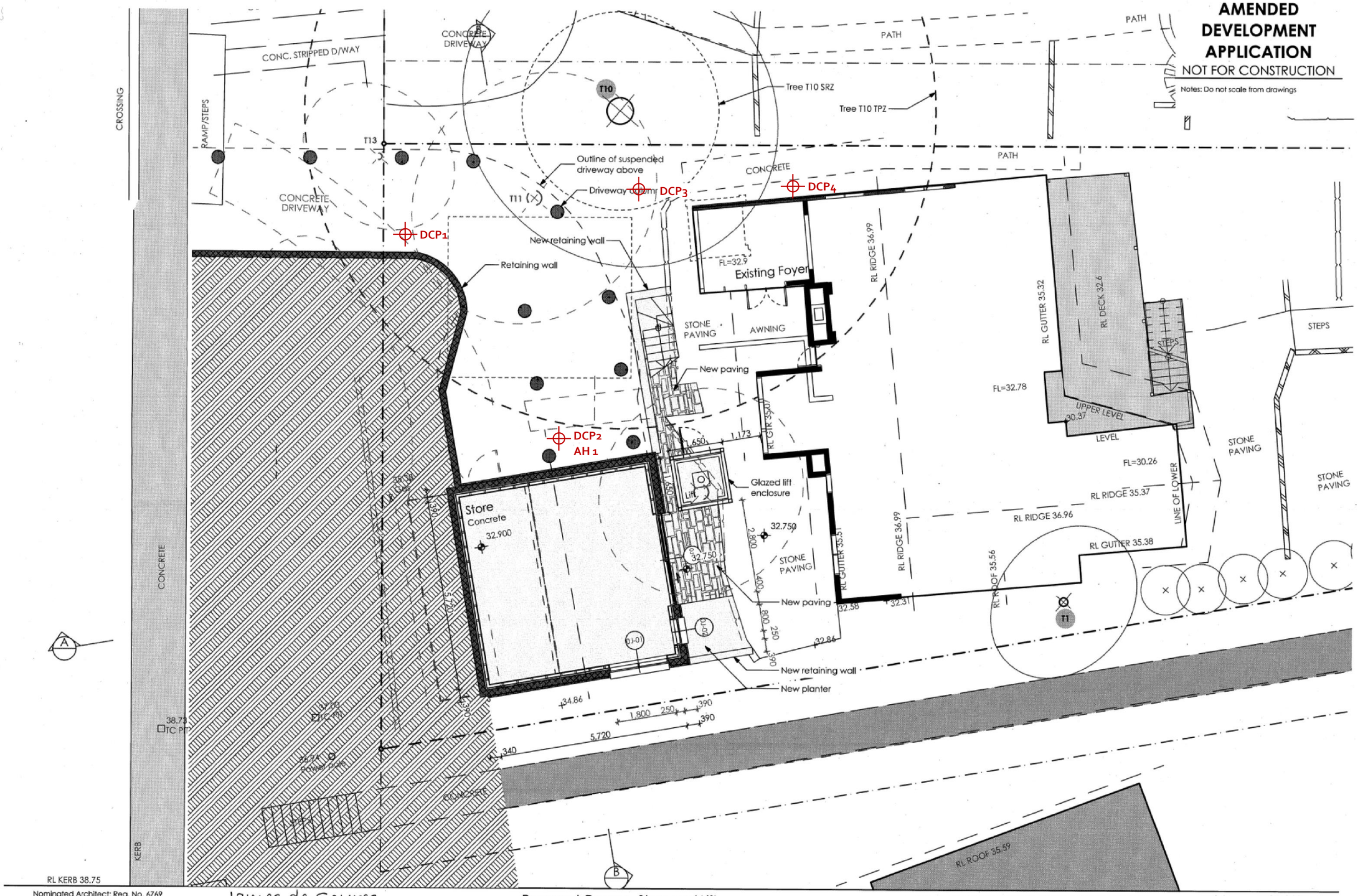
J. MCCLURE
SURVEYOR REGISTERED UNDER THE SURVEYING AND SPATIAL INFORMATION ACT, 2002

R & S NICHOLS

LOTS 2 & 3 DP 16692
LEVELS TO AHD

A1 REDUCTION RATIO: 1:100			DRAWING No.
SURVEY	J.McC		1
DRAWN	A1 J.McC		
REFERENCE	025/18	26 MAR 2019	

SITE PLAN – showing test locations



Nominated Architect: Reg. No. 6769
m PO Box 657, Newport Beach, NSW 2106
t (02) 9979 1823
e contact@jdesa.com.au
w www.jdesa.com.au

James de Soyres + Associates
a r c h i t e c t s

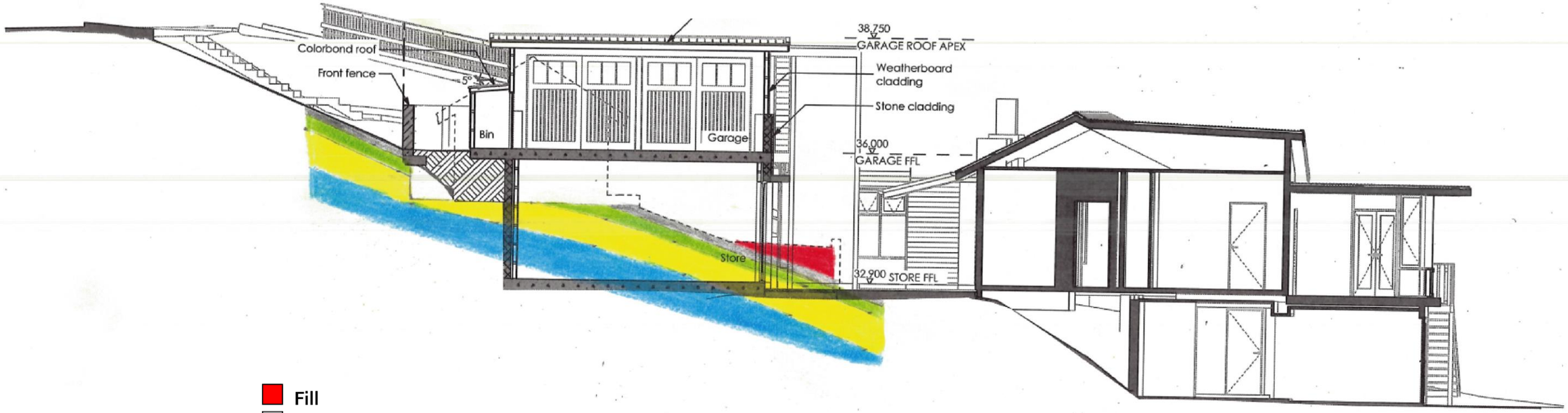
Proposed Garage, Store and Lift
at 103 Narrabeen Park Parade Mona Vale NSW 2103
for R & S Nichols

AMENDED DEVELOPMENT APPLICATION

Store and Entry Floor Plan

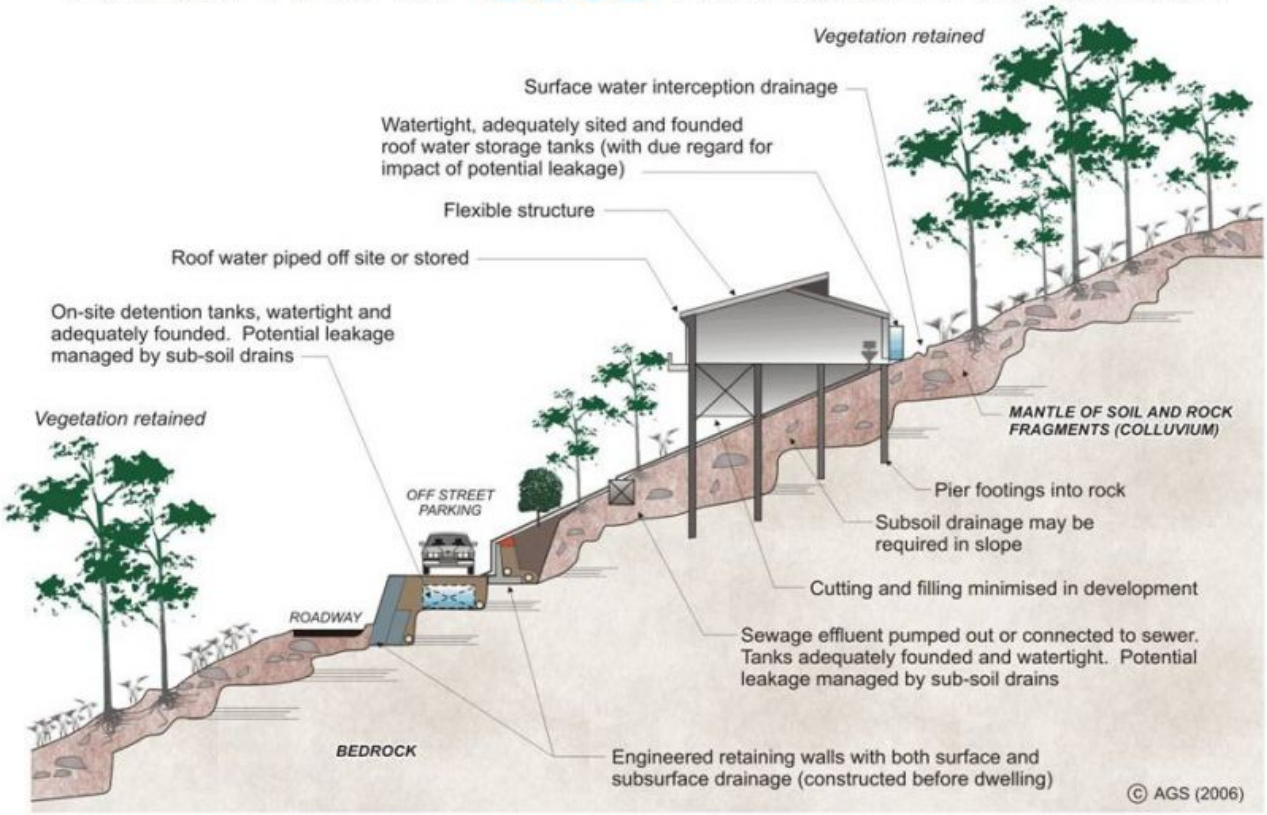
Scale: 1:100
Date of printing: 24/07/2020
Project No: 1905a
Drawing No: **DA-10**
Rev: A

TYPE SECTION – Diagrammatical Interpretation of expected Ground Materials



- Fill
- Sandy Soil
- Clayey Soil
- Sandy Clay
- Narrabeen Group Rocks – Extremely Low Strength Shale - after being cut up by excavation equipment can resemble a stiff to hard clay.

EXAMPLES OF **GOOD** HILLSIDE PRACTICE



EXAMPLES OF **POOR** HILLSIDE PRACTICE