GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER
FORM NO. 1 – To be submitted with Development Application

Development Application for _____
Name of Applicant

Address of site 51 Alleyne Avenue, North Narrabeen

The following checklist covers the minimum requirements to be addressed in a Geotechnical Risk ***Declaration made by geotechnical engineer or engineering geologist or coastal engineer (where applicable) as part of a geotechnical report***

I, Ben White *on behalf of* White Geotechnical Group Pty Ltd
(Insert Name) (Trading or Company Name)

on this the 24/7/25 certify that I am a geotechnical engineer or engineering geologist or coastal engineer as defined by the Geotechnical Risk Management Policy for Pittwater - 2009 and I am authorised by the above organisation/company to issue this document and to certify that the organisation/company has a current professional indemnity policy of at least $10million.

I:
Please mark appropriate box

☒ have prepared the detailed Geotechnical Report referenced below in accordance with the Australia Geomechanics Society's Landslide Risk Management Guidelines (AGS 2007) and the Geotechnical Risk Management Policy for Pittwater - 2009

☒ am willing to technically verify that the detailed Geotechnical Report referenced below has been prepared in accordance with the Australian Geomechanics Society's Landslide Risk Management Guidelines (AGS 2007) and the Geotechnical Risk Management Policy for Pittwater - 2009

☐ have examined the site and the proposed development in detail and have carried out a risk assessment in accordance with Section 6.0 of the Geotechnical Risk Management Policy for Pittwater - 2009. I confirm that the results of the risk assessment for the proposed development are in compliance with the Geotechnical Risk Management Policy for Pittwater - 2009 and further detailed geotechnical reporting is not required for the subject site.

☐ have examined the site and the proposed development/alteration in detail and I am of the opinion that the Development Application only involves Minor Development/Alteration that does not require a Geotechnical Report or Risk Assessment and hence my Report is in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009 requirements.

☐ have examined the site and the proposed development/alteration is separate from and is not affected by a Geotechnical Hazard and does not require a Geotechnical Report or Risk Assessment and hence my Report is in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009 requirements.

☐ have provided the coastal process and coastal forces analysis for inclusion in the Geotechnical Report

Geotechnical Report Details:

Report Title: Geotechnical Report **51 Alleyne Avenue, North Narrabeen**
Report Date: 24/7/25

Author: BEN WHITE

Author's Company/Organisation: **WHITE GEOTECHNICAL GROUP PTY LTD**

Documentation which relate to or are relied upon in report preparation:

Australian Geomechanics Society Landslide Risk Management March 2007.

White Geotechnical Group company archives.

I am aware that the above Geotechnical Report, prepared for the abovementioned site is to be submitted in support of a Development Application for this site and will be relied on by Pittwater Council as the basis for ensuring that the Geotechnical Risk Management aspects of the proposed development have been adequately addressed to achieve an "Acceptable Risk Management" level for the life of the structure, taken as at least 100 years unless otherwise stated and justified in the Report and that reasonable and practical measures have been identified to remove foreseeable risk.

Signature _____

Name Ben White

Chartered Professional Status MScGEOLAusIMM CP GEOL

Membership No. 222757

Company **White Geotechnical Group Pty Ltd**

GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER
FORM NO. 1(a) - Checklist of Requirements for Geotechnical Risk Management Report for Development Application

Development Application for _____
Name of Applicant

Address of site 51 Alleyne Avenue, North Narrabeen

The following checklist covers the minimum requirements to be addressed in a Geotechnical Risk Management Geotechnical Report. This checklist is to accompany the Geotechnical Report and its certification (Form No. 1).

Geotechnical Report Details:

Report Title: Geotechnical Report **51 Alleyne Avenue, North Narrabeen**

Report Date: 24/7/25

Author: BEN WHITE

Author's Company/Organisation: WHITE GEOTECHNICAL GROUP PTY LTD

Please mark appropriate box

☒ Comprehensive site mapping conducted **23/7/25**
(date)

☒ Mapping details presented on contoured site plan with geomorphic mapping to a minimum scale of 1:200 (as appropriate)

☒ Subsurface investigation required
- ☐ No Justification _____
- ☒ Yes Date conducted **23/7/25**

☒ Geotechnical model developed and reported as an inferred subsurface type-section

☒ Geotechnical hazards identified
- ☒ Above the site
- ☒ On the site
- ☒ Below the site
- ☐ Beside the site

☒ Geotechnical hazards described and reported

☒ Risk assessment conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009
- ☒ Consequence analysis
- ☒ Frequency analysis

☒ Risk calculation

☒ Risk assessment for property conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009

☒ Risk assessment for loss of life conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009

☒ Assessed risks have been compared to "Acceptable Risk Management" criteria as defined in the Geotechnical Risk Management Policy for Pittwater - 2009

☒ Opinion has been provided that the design can achieve the "Acceptable Risk Management" criteria provided that the specified conditions are achieved.

☒ Design Life Adopted:
- ☒ 100 years
- ☐ Other _____
specify

☒ Geotechnical Conditions to be applied to all four phases as described in the Geotechnical Risk Management Policy for Pittwater - 2009 have been specified

☒ Additional action to remove risk where reasonable and practical have been identified and included in the report.

☐ Risk assessment within Bushfire Asset Protection Zone.

I am aware that Pittwater Council will rely on the Geotechnical Report, to which this checklist applies, as the basis for ensuring that the geotechnical risk management aspects of the proposal have been adequately addressed to achieve an "Acceptable Risk Management" level for the life of the structure, taken as at least 100 years unless otherwise stated, and justified in the Report and that reasonable and practical measures have been identified to remove foreseeable risk.

Signature _____

Name Ben White

Chartered Professional Status MScGEOLAusIMM CP GEOL

Membership No. 222757

Company **White Geotechnical Group Pty Ld**

GEOTECHNICAL INVESTIGATION:

New Granny Flat at **51 Alleyne Avenue, North Narrabeen**

1. Proposed Development

1.1 Construct a new granny flat at the uphill side of the property by excavating to a maximum depth of ~1.5m.

1.2 Details of the proposed development are shown on 7 drawings prepared by RK Designs, project number 25-11, drawings numbered A0 to A6, Issue D, dated 29/4/25.

2. Site Description

2.1 The site was inspected on the 23rd July, 2025 and previously on the 4th May, 2015.

2.2 This residential property is on the low side of the road and has an E aspect. It is located on the steeply graded upper to middle reaches of a hillslope. The natural slope falls across the property at an average angle of ~25°. The slope below the property continues at similar steep angles for some 40m before easing to near level angles. The grade above rises similarly but quickly eases as the crest of the hill is approached.

2.3 Low to Medium Strength Sandstone bedrock outcrops at the uphill side of the road (Photo 1). A suspended timber parking platform is located at the road frontage. The timber posts that support the parking platform stand vertical (Photo 2). Beneath the parking platform, a large boulder is partially embedded in the slope (Photo 2). It is considered stable provided the drainage regime on the slope is not altered (i.e. so water flows are not directed to the uphill side of the boulder). Cuts and fills provide level platforms for lawn, garden and paved areas between the parking platform and

the house. The cuts and fills are lined by stable stacked rocks up to ~1.4m high (Photos 3 to 5). In this area, there is also a scattering of detached sandstone joint blocks that appear to be stable (Photo 6).

The two storey split level timber framed house is supported on brick piers and timber posts (Photos 7 to 9). The supporting piers and posts stand vertical (Photos 8 & 9). The downhill side of the property has been terraced with two timber retaining walls up to ~1.3m high. The lower wall is currently bulging and some of the central supporting posts are tilting downslope significantly (Photo 10). However, due to its low height and location, it is not considered a significant threat to life or property. The owner has informed us this area will be re-landscaped and the retaining wall will be replaced as part of a separate development.

3. Geology

The Sydney 1:100 000 Geological Sheet indicates the site is underlain by the Newport Formation of the Narrabeen Group. This is described as interbedded laminite, shale, and quartz to lithic quartz sandstone.

4. Subsurface Investigation

One hand Auger Hole (AH) was put down to identify the soil materials. Five Dynamic Cone Penetrometer (DCP) tests were put down to determine the relative density of the overlying soil and the depth to weathered rock. The locations of the tests are shown on the site plan attached. It should be noted that a level of caution should be applied when interpreting DCP test results. The test will not pass through hard buried objects so in some instances it can be difficult to determine whether refusal has occurred on an obstruction in the profile or on the natural rock surface. This is expected to have occurred for DCP1 and may have occurred for DCP2. Due to the possibility that the actual ground conditions vary from our interpretation there should be allowances in the excavation and foundation budget to account for this. We

refer to the appended "Important Information about Your Report" to further clarify. The results are as follows:

AUGER HOLE 1 (~RL42.0) **–** AH1 (Photo 11)

Depth (m)	**Material Encountered**
0.0 to 0.8	**FILL**, sandy soil and sand, with some rock fragments, dark brown, brown, orange, grey, moist, fine to coarse grained.
0.8 to 1.4	**COLLUVIUM**, sandy soil and sand, with some rock fragments, dark brown, grey, orange, moist to damp, fine to coarse grained.
1.4 to 1.6	**SANDY CLAY**, light brown/orange and light grey/white, firm to very stiff, moist.

End of hole @ 1.6m in firm to very stiff clay. No water table encountered.

DCP TEST RESULTS – Dynamic Cone Penetrometer

Equipment: 9kg hammer, 510mm drop, conical tip. Standard: AS1289.6.3.2 -1997

Depth(m) Blows/0.3m	**DCP 1** (~RL42.0)	**DCP 2** (~RL38.7)	**DCP 3** (~RL35.1)	**DCP 4** (~RL38.0)	**DCP 5** (~RL41.9)
0.0 to 0.3	1F	1F	2	1	2
0.3 to 0.6	9	4	26	4	2
0.6 to 0.9	11	19	17	4	23
0.9 to 1.2	28	#	22	24	20
1.2 to 1.5	#		31	30	12
1.5 to 1.8			#	36	14
1.8 to 2.1				#	45
2.1 to 2.4					28
2.4 to 2.7					#
	Refusal @ 1.2m	Refusal @ 0.9m	Refusal on Rock @ 1.4m	Refusal on Rock @ 1.7m	Refusal on Rock @ 2.4m

#refusal/end of test. F=DCP fell after being struck showing little resistance through all or part of the interval.

DCP Notes:

DCP1 – Refusal @ 1.2m, DCP bouncing, white impact dust on dry tip.
DCP2 – Refusal @ 0.9m, DCP bouncing, white impact dust on dry tip.
DCP3 – Refusal on Rock @ 1.4m, DCP bouncing off rock surface, white impact dust on dry tip.
DCP4 – Refusal on Rock @ 1.7m, DCP thudding on rock surface, dark brown soil on dry tip, orange clay on collar above tip.
DCP5 – Refusal on Rock @ 2.4m, DCP bouncing off rock surface, orange brown clay and dark brown soil on damp tip.

5. Geological Observations/Interpretation

The slope materials are colluvial at the near surface and residual at depth. In the test locations, the ground materials consist of fill and colluvium over Firm to Very Stiff Clays. Fill to a maximum depth of ~1.3m provides level platforms for landscaping and paved areas across the property. In the test locations, the clays merge into the weathered zone of the underlying rock at depths of between ~0.9m to ~1.8m below the current surface, being deeper where the fill and colluvium is deeper. DCP1 is expected to have encountered refusal on a sandstone boulder within the colluvium or another obstruction in the profile. This may have also occurred for DCP2. The weathered zone of the underlying rock is interpreted as Extremely Low to Low Strength Rock. It is to be noted that this material is a soft rock and can appear as a mottled stiff clay when it is cut up by excavation equipment. See Type Section attached for a diagrammatical representation of the expected ground materials.

6. Groundwater

Ground water seepage is expected to move over the denser and less permeable clay and weathered rock layers in the sub-surface profile. Due to the slope and elevation of the block, the water table is expected to be many metres below the base of the proposed works.

7. Surface Water

No evidence of surface flows were observed on the property during the inspection. It is expected that normal sheet wash will move onto the site from above the property during heavy down pours.

8. Geotechnical Hazards and Risk Analysis

No geotechnical hazards were observed beside the property. The steeply graded slope that falls across the property and continues above and below is a potential hazard (**Hazard One**). The proposed excavation is a potential hazard until retaining structures are in place (**Hazard Two**).

Geotechnical Hazards and Risk Analysis - Risk Analysis Summary

HAZARDS	Hazard One	Hazard Two
TYPE	The steep slope that falls across the property and continues above and below failing and impacting on the house or the proposed works.	The proposed excavation for the granny flat collapsing onto the worksite and impacting the neighbouring properties or the road reserve during the excavation process.
LIKELIHOOD	'Unlikely' (10^{-4})	'Possible' (10^{-3})
CONSEQUENCES TO PROPERTY	'Medium' (15%)	'Medium' (13%)
RISK TO PROPERTY	'Low' (2×10^{-5})	'Moderate' (2×10^{-4})
RISK TO LIFE	8.3×10^{-7}/annum	3.7×10^{-5}/annum
COMMENTS	This level of risk is 'ACCEPTABLE', provided the recommendations in **Section 17** are carried out.	This level of risk to life and property is 'UNACCEPTABLE'. To move the risk to 'ACCEPTABLE' levels, the recommendations in **Section 13** are to be followed.

(See Aust. Geomech. Jnl. Mar 2007 Vol. 42 No 1, for full explanation of terms)

9. Suitability of the Proposed Development for the Site

The proposed development is suitable for the site. No geotechnical hazards will be created by the completion of the proposed development provided it is carried out in accordance with the requirements of this report and good engineering and building practice.

10. Stormwater

The fall is away from the street. The stormwater engineer is to refer to council stormwater policy for suitable options.

11. Excavations

An excavation to a maximum depth of ~1.5m is required to construct the proposed granny flat. The excavation is expected to be through fill, colluvium and clay. Excavations through fill, colluvium and clay are expected to be carried out with an excavator and toothed bucket.

12. Vibrations

It is expected the proposed excavation will be carried out with an excavator and toothed bucket and the vibrations produced will be below the threshold limit for building or infrastructure damage using a domestic sized excavator up to 16 tonne.

13. Excavation Support Requirements

An excavation to a maximum depth of ~1.5m is required to construct the proposed granny flat. Allowing 0.5m for backwall drainage, the excavation is set back ~0.8m from the S common boundary and ~1.8m from the road reserve. The S common boundary and road reserve will be within the zone of influence of the excavation. In this instance, the zone of influence is the area above a theoretical 30° line (from horizontal) through fill/soil/colluvium and a 45° line through clay from the base of the excavation towards the surrounding structures and boundaries.

Due to the steep grade of the slope, the depth of the fill/colluvium and the proximity to the common boundaries, the excavation will need to be temporarily supported during the excavation process in a staged manner, so cut batters are not left unsupported. The support will need to be designed or approved by the structural engineer. See the site plan attached for the minimum extent of the required shoring shown in blue.

Upslope runoff is to be diverted from the cut faces by sandbag mounds or other diversion works. The materials and labour to construct the retaining walls are to be organised so on completion of the excavation they can be constructed as soon as possible. The excavation is to be carried out during a dry period. No excavations are to commence if heavy or prolonged rainfall is forecast.

All excavation spoil is to be removed from site following the current Environmental Protection Agency (EPA) waste classification guidelines.

14. Retaining Structures

For cantilever or singly propped retaining structures it is suggested the design be based on a triangular distribution of lateral pressures using the parameters shown in Table 1.

Table 1 – Likely Earth Pressures for Retaining Structures

Unit	Earth Pressure Coefficients		
	Unit weight (kN/m^3)	'Active' K_a	'At Rest' K_0
Fill and Colluvium	20	0.40	0.55
Residual Clays	20	0.35	0.45

For rock classes refer to Pells et al "Design Loadings for Foundations on Shale and Sandstone in the Sydney Region".

It is to be noted that the earth pressures in Table 1 assume a level surface above the wall, do not account for any surcharge loads and assume retaining walls are fully drained. Ground Materials and relevant earth pressure coefficients are to be confirmed on site by the geotechnical consultant.

All retaining structures are to have sufficient back-wall drainage and be backfilled immediately behind the structure with free draining material (such as gravel). This material is to be wrapped in a non-woven Geotextile fabric (i.e. Bidim A34 or similar), to prevent the

drainage from becoming clogged with silt and clay. If no back-wall drainage is installed in retaining structures the full hydrostatic pressures are to be accounted for in the retaining structure design.

15. Site Classification

The site classification in accordance with AS2870-2011 is Class P due to the steep grade of the slope and the depth of the fill and colluvium. The natural clays underlying the fill/colluvium are interpreted to be moderately reactive.

16. Foundations

The proposed granny flat is to be supported on piers taken to and embedded no less than 0.6m into Extremely Low Strength rock or better from the downhill edge of the footing. This ground material is expected at depths of between ~0.9m to ~2.1m below the current surface, being deeper where the fill and colluvium is deeper. A maximum allowable bearing pressure of 600kPa can be assumed for footings embedded in Extremely Low Strength Rock or better. It should be noted that this material is a soft rock and a rock auger will cut through it so the builders should not be looking for refusal to end the footings.

As the bearing capacity of weathered rock reduces when it is wet we recommend the footings be dug, inspected and poured in quick succession (ideally the same day if possible). If the footings get wet, they will have to be drained and the soft layer of weathered rock on the footing surface will have to be removed before concrete is poured.

If a rapid turnaround from footing excavation to the concrete pour is not possible a sealing layer of concrete may be added to the footing surface after it has been cleaned and inspected.

NOTE: If the contractor is unsure of the footing material required it is more cost effective to get the geotechnical professional on site at the start of the footing excavation to advise on footing depth and material. This mostly prevents unnecessary over excavation in clay like shaly rock but can be valuable in all types of geology.

17. Ongoing Maintenance

Where slopes are steep and exceed 25°, such as on this site, it is prudent for the owners to occasionally inspect the slope (say annually or after heavy and prolonged rainfall events, whichever occurs first). Should any of the following be observed: movement or cracking in retaining walls, cracking in any structures, cracking or movement in the slope surface, tilting or movement in established trees, leaking pipes, or newly observed flowing water, or changes in the erosional process or drainage regime, then a geotechnical consultant should be engaged to assess the slope. We can carry out these inspections upon request. The risk assessment in **Section 8** is subject to this ongoing maintenance being carried out.

18. Geotechnical Review

The structural plans are to be checked and certified by the geotechnical engineer as being in accordance with the geotechnical recommendations. On completion, a Form 2B will be issued. This form is required for the Construction Certificate to proceed.

19. Inspection

The client and builder are to familiarise themselves with the following required inspection as well as council geotechnical policy. We cannot provide geotechnical certification for the Occupation Certificate if the following inspection has not been carried out during the construction process.

- All footings are to be inspected and approved by the geotechnical consultant while the excavation equipment and contractors are still onsite and before steel reinforcing is placed or concrete is poured.

White Geotechnical Group Pty Ltd.

Dion Sheldon
BEng(Civil)(Hons) MIEAust NER,
Geotechnical Engineer.

Reviewed By:

Nathan Gardner B.Sc. (Geol. & Geophys. & Env. Stud.)
AIG., RPGeo Geotechnical & Engineering.
No. 10307
Engineering Geologist & Environmental Scientist.

Photo 1

Photo 2

Photo 3

Photo 4

Photo 5

Photo 6

Photo 7

Photo 8

Photo 9

Photo 10

Photo 11: AH1 – Downhole is from top to bottom.

Important Information about Your Report

It should be noted that Geotechnical Reports are documents that build a picture of the subsurface conditions from the observation of surface features and testing carried out at specific points on the site. The spacing and location of the test points can be limited by the location of existing structures on the site or by budget and time constraints of the client. Additionally, the test themselves, although chosen for their suitability for the particular project, have their own limiting factors. The testing gives accurate information at the location of the test, within the confines of the test's capability. A geological interpretation or model is developed by joining these test points using all available data and drawing on previous experience of the geotechnical consultant. Even the most experienced practitioners cannot determine every possible feature or change that may lie below the earth. All of the subsurface features can only be known when they are revealed by excavation. As such, a Geotechnical report can be considered an interpretive document. It is based on factual data but also on opinion and judgement that comes with a level of uncertainty. This information is provided to help explain the nature and limitations of your report.

With this in mind, the following points are to be noted:

- If upon the commencement of the works the subsurface ground or ground water conditions prove different from those described in this report, it is advisable to contact White Geotechnical Group immediately, as problems relating to the ground works phase of construction are far easier and less costly to overcome if they are addressed early.

- If this report is used by other professionals during the design or construction process, any questions should be directed to White Geotechnical Group as only we understand the full methodology behind the report's conclusions.

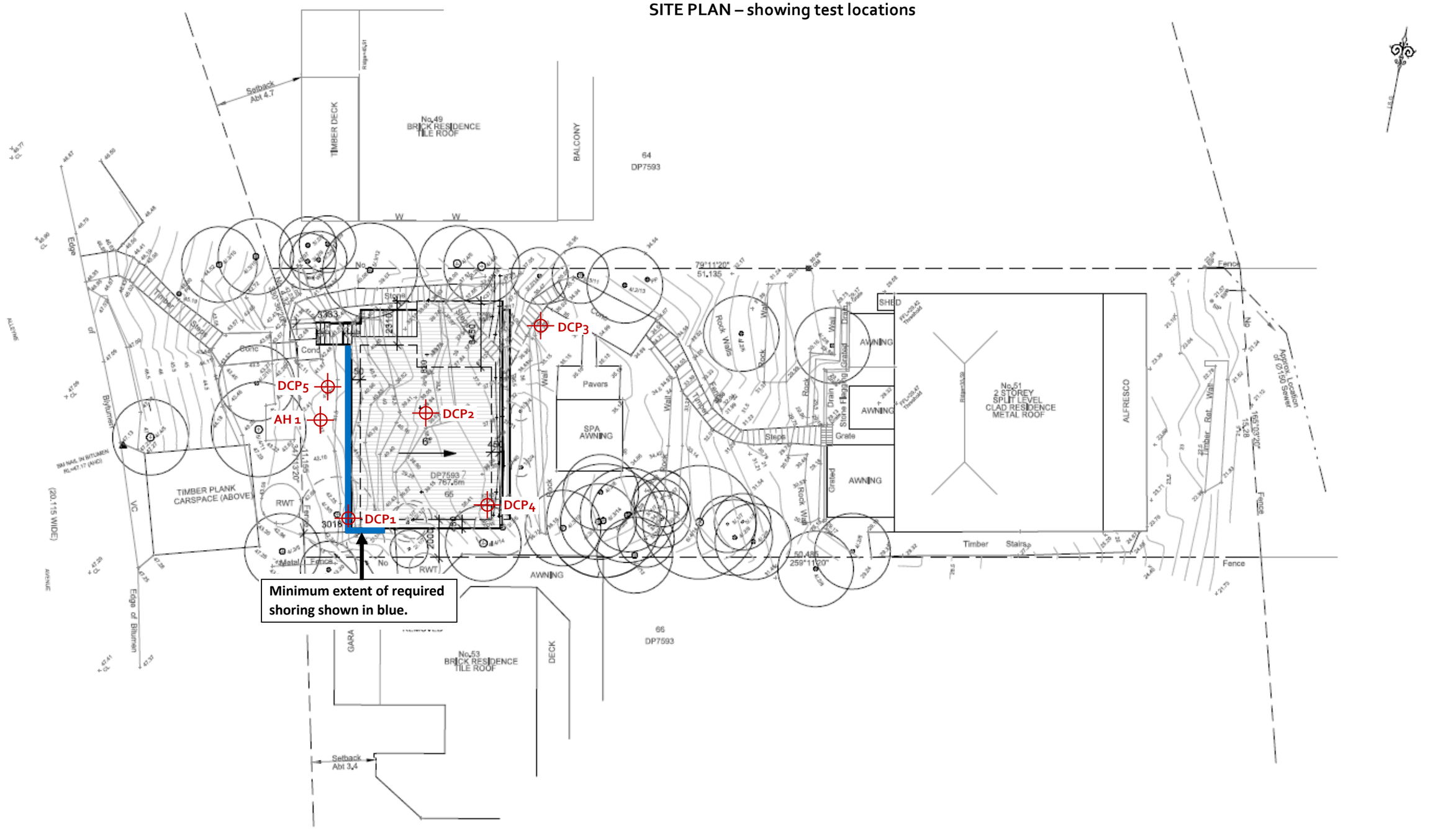
- The report addresses issues relating to your specific design and site. If the proposed project design changes, aspects of the report may no longer apply. Contact White Geotechnical if this occurs.

- This report should not be applied to any other project other than that outlined in section 1.0.

- This report is to be read in full and should not have sections removed or included in other documents as this can result in misinterpretation of the data by others.

- It is common for the design and construction process to be adapted as it progresses (sometimes to suit the previous experience of the contractors involved). If alternative design and construction processes are required to those described in this report, contact White Geotechnical Group. We are familiar with a variety of techniques to reduce risk and can advise if your proposed methods are suitable for the site conditions.

SITE PLAN – showing test locations



PRELIMINARY
DO NOT USE FOR CONSTRUCTION

1 SITE AND ROOF PLAN

1 : 200

DIAL BEFORE YOU DIG www.1100.com.au

GENERAL NOTES
. All dimensions are to be confirmed on site by the builder/subcontractor, any incongruencies must be reported to the Designer before commencement of any work.
. No Survey has been made on the boundaries, all bearings, distances and areas have been taken from the contour survey plan, A Survey must be carried out to confirm the exact boundary locations.
. No construction work shall commence until a site survey confirming the site boundaries has been completed, The contractor is to ensure that the boundary setbacks are confirmed and used, the boundary setbacks take precedence over all other dimensions. The Survey work must be performed by a registered Surveyor.
. In the event of encountering any discrepancies on these drawings, specification or subsequent instructions issued, the Builder/Subcontractor shall contact the designer before proceeding further with any work.
. All construction, control joints and expansion joints in the wall, floors, other locations shall be in strict accordance with the Structural Engineering details. No joints or breaks other than specified, are allowed without written permission from the Engineer.
. Measurements for the fabrication of secondary components such as, windows, doors, internal frames, structural steel components and the like, are not to be taken from these documents, Measurements must be taken on site to suit the work as constructed.
. All structural components shall be in strict accordance to details and specifications as prepared by a structural engineer.
. All existing structures need to be examined for structural adequacy, and it is the Contractor's responsibility to ensure that a certificate of structural adequacy is available prior to the start of any work.

drawn	date	issue	description
LS	03/03/25	A	ISSUE FOR DA
LS	07/04/25	B	ISSUE FOR DA
LS	24/04/25	C	ISSUE FOR DA
LS	29/04/25	D	ISSUE FOR DA

project
PROPOSED CONSTRUCTION OF A GRANNY FLAT AT 51 ALLEYNE AVENUE NORTH NARRABEEN NSW 2101 LOT 65/-/DP7593

bdaa ACCREDITED BUILDING DESIGNER

6/7 Parkes Street, Parramatta NSW 2150
www.rkdesigns.com.au
admin@rkdesigns.com.au
02 9633 4797
abn. 68 659 200 389
spaces designed for life

client
STEVE AND WENDY MACALPINE
Bungalow Homes

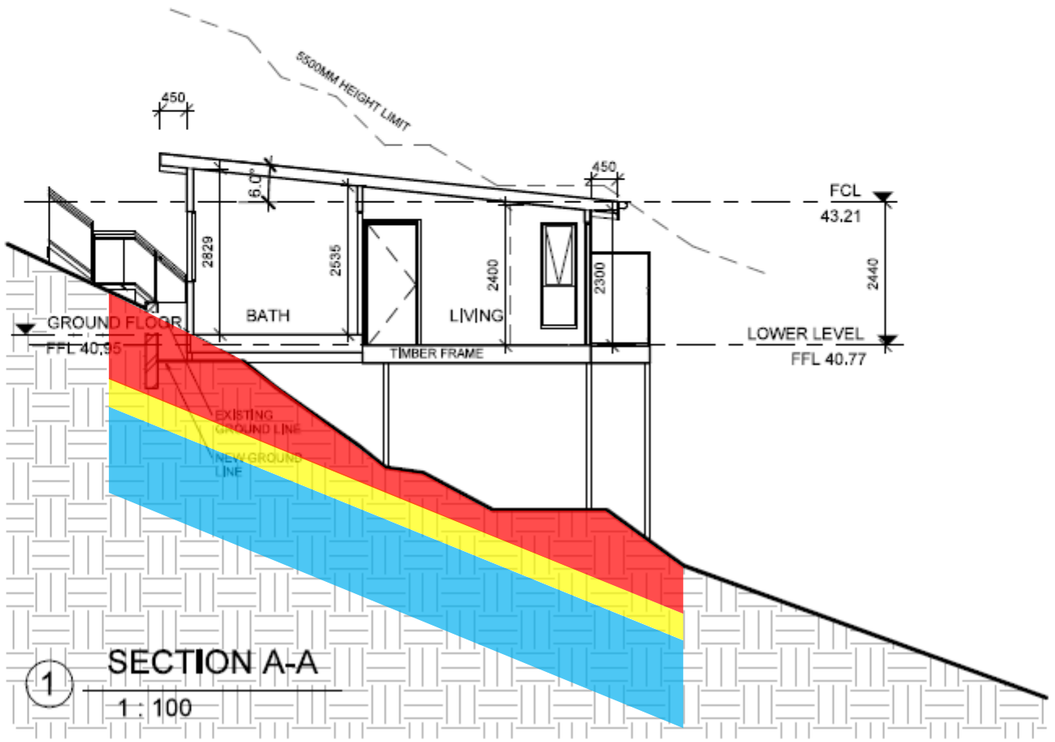
true north

drawing
SITE AND ROOF PLAN

project no	date	sheet no.	scale @ A3	issue	checked
25-11	03/03/25	A 2	1 : 200	D	?

RK DESIGNS

TYPE SECTION – Diagrammatical Interpretation of expected Ground Materials



- Fill and Colluvium
- Clay – Firm to Very Stiff
- Narrabeen Group Rocks – Extremely Low to Low Strength Rock - after being cut up by excavation equipment can resemble a stiff to hard clay.

GENERAL NOTES
. All dimensions are to be confirmed on site by the builder/subcontractor, any incongruencies must be reported to the Designer before commencement of any work.
. No Survey has been made on the boundaries, all bearings, distances and areas have been taken from the contour survey plan, A Survey must be carried out to confirm the exact boundary locations.
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. All existing structures need to be examined for structural adequacy, and it is the Contractor's responsibility to ensure that a certificate of structural adequacy is available prior to the start of any work.

drawn	date	issue	description
LS	03/03/25	A	ISSUE FOR DA
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LS	24/04/25	C	ISSUE FOR DA
LS	29/04/25	D	ISSUE FOR DA

project
PROPOSED CONSTRUCTION OF A GRANNY FLAT AT 51 ALLEYNE AVENUE NORTH NARRABEEN NSW 2101 LOT 65/-/DP7593

client
STEVE AND WENDY MACALPINE
Bungalow Homes

true north

drawing
SECTION

project no	date	sheet no,	scale @ A3	issue	checked
25-11	03/03/25	A 5	1 : 100	D	?

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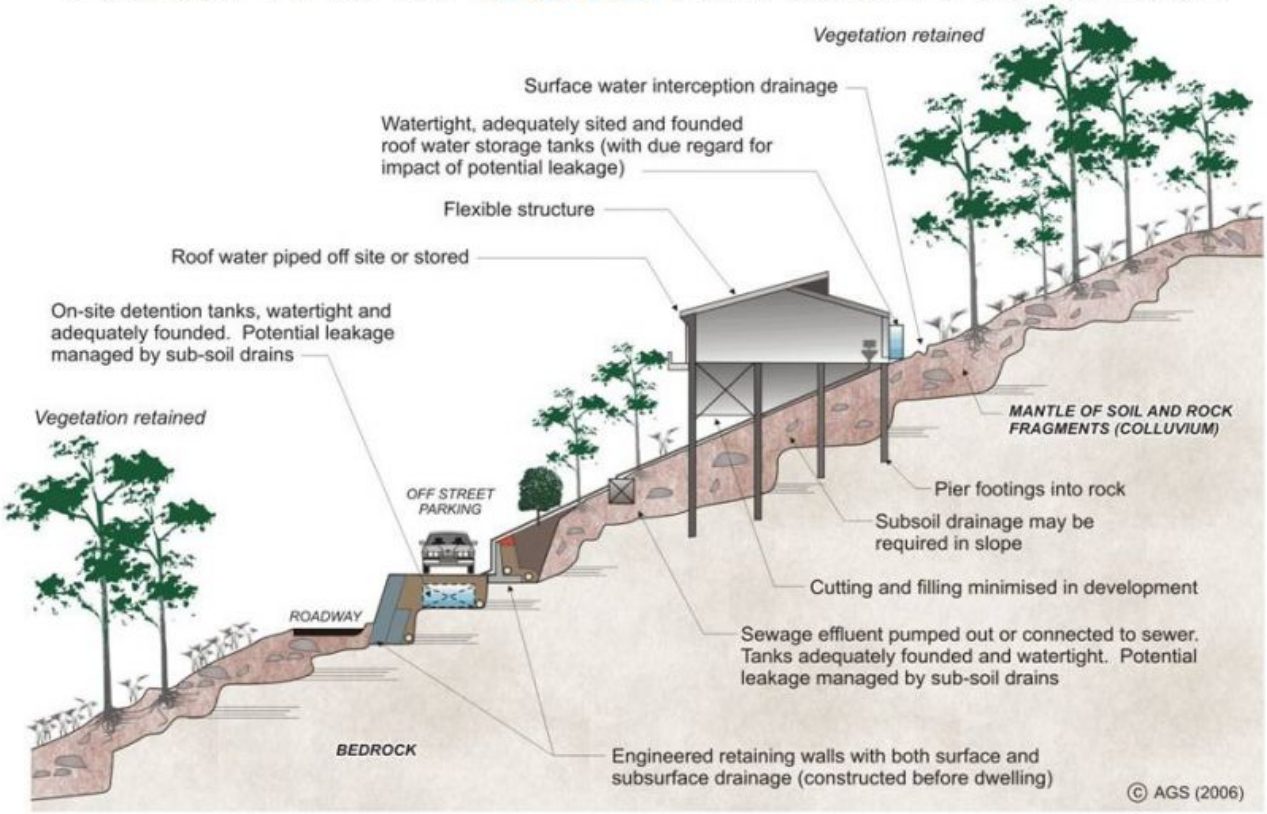
admin@rkdesigns.com.au
02 9633 4797
abn. 68 659 200 389
spaces designed for life
RK DESIGNS

EXAMPLES OF **GOOD** HILLSIDE PRACTICE



EXAMPLES OF **POOR** HILLSIDE PRACTICE