

01 July, 2014.

Pittwater Council
P.O. Box 882
Mona Vale NSW 2103

Dear Sir /Madam

Re:

Lodgement of CC2014-153 for DA No. N0248/11

Site address:

No. 28 Woorarra Avenue, North Narrabeen.

RECEIVED
4 . JUL 2014
PITTWATER COUNCIL

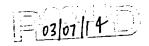
Please find attached all required documentation relied upon to issue Construction Certificate and Notice of Commencement for the above development:

- Part 4A Lodgement Fee \$36.00 payable to Council.
- Copy of Home Owner's Warranty Insurance.
- Sydney Water approval
- Full set of council approved 'stamped' plans.
- 1 full set of Construction Certificate plans and specifications.
- 1 Structural Engineer's Plans & Certificate of Structural Adequacy
- Geotechnical Assessment Report
- Certificate showing the Engineering plans cover the stipulations set out in C2
- Stormwater Management Plans.
- Receipt for payment of Long Service Levy.
- Detail of Roofing material- type and colour.
- Schedule of finishes
- Form 2A & Form 2B of the Geotechnical Risk Management Policy for Pittwater
- 1 Basix Certificate

Yours faithfully

Craig Formosa

136 Rec: 5020 41/11.





# CONSTRUCTION CERTIFICATE #2014-153 - Stage 1

Approved 01/07/14

Issued in accordance with the provisions of the Environmental & Assessment Act 1979 under Sections 109C(1)(b) and 109F

Date Application Received	26-06-14						
Council	Pittwater Council						
Development Consent No.	N0248/11	Da	ate Approved		19-09-11		
Certifying Authority	Craig Formosa	Ad	ccredited Certi	fier	Craig Form	nosa -	BPB0124
Accreditation Body	Building Professionals Board	В	CA in Force		BCA2014		
APPLICANT DETAILS							
Name	Rob Phipps		Ph	No.	0417 556 6	521	
Address	28 Woorarra Avenue, North Narrabeen	NSW	2101				
OWNER DETAILS							
Name	Rob Phipps						
Address	28 Woorarra Avenue, North Narrabeen	NSW	2101				
DEVELOPMENT DETAILS	and the second s	·			AMARICAN MILITERATURE (III) AND		
Subject Land	28 Woorarra Avenue, North Narrabeen	NSW	2101 Lot	No.	25	DP	23429
Description of Development	Stage 1 - Alterations & additions to the external stair)	existing	dwelling (no	t incl	uding Kitcher	n exte	ension &
Class of Building	1a, 10a	V	alue of Work		\$240,000.0	00	
BUILDER DETAILS							
Name	Shane Doherty						
Address	15 Grasmere Crescent, Wheeler Height	s NSW	/ 2097				
Contact Number	0423 580 442	Li	cence No.	•	247869C		
APPROVED PLANS & DOCU	MENTS						
Plans Prepared By	Sally Gardner Design & Draft						
Drawing Numbers	A1 – A7		Da	ted	09-06-11		
Engineer Details Prepared By	Waddington Consulting Pty Ltd - structure	a!	Sally Gardr	ner D	esign & Draft	- hyd	raulic
Drawing Numbers	S0.00, S0.01, S1.00, S1.01, S2.00, S3.00 - structural	raulic	Da	ted	18-06-14	- 1	09-06-11, 02/07/14
Basix Certificate No.	A115956		Da	ted	05-07-11		

- I, Craig Formosa, as the certifying authority am satisfied that;
  - (a) The requirements of the regulations referred to in s81A (5) have been complied with. That is, work completed in accordance with the documentation accompanying the application for this certificate (with such modifications verified by the certifying authority as may be shown on that documentation) will comply with the requirements of the Regulation as referred to in section 81A (5) of the Act, and b)Long Service Levy has been paid where required under s34 of the Building & Construction Industry Long Service Payments Act 1986.

Signed:

Date: 01/07/14



# **Waddington Consulting Pty Ltd**

ACN 130 522 851 Structural and Civil Engineering Suite 506, Level 5 22 Central Ave, Manly NSW P.O. Box 1044 Manly NSW 1655

> P (02) 9976 0070 F (02) 9976 0095

2 July 2014

Mr Craig Formosa Form Building Certifiers by email:<craig@formbc.com

Dear Craig,

Subject:

Proposed Alterations and Additions - 28 Woorarra Ave, North Narrabeen

Stormwater Management Plan

The *Stormwater Concept Plan* (drawing S7 by Sally Gardner Design and Draft) has been prepared to satisfy the requirements of Pittwater Council's Development Control Plan. Further details will be provided during construction to ensure full compliance with the DCP, required standards and the BCA.

Please do not hesitate to contact me if you have any queries regarding this design.

Yours sincerely,

Kate Waddington

Director

Waddington Consulting Pty Ltd

K Waddington

THIS PLAN / DOCUMENT FORMS

CERTIFIERS CC / CDC



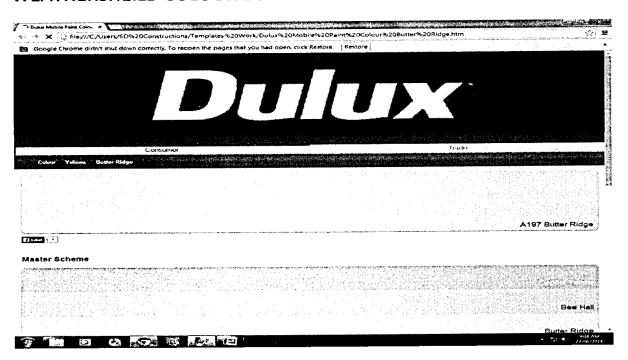
JOB; 28 WOORARRA AVE, NORTH NARRABEEN

THIS PLAN/DOCUMENT FORMS
PART OF FORM BUILDING
CERTIFIERS CC/CDC

# **SCHEDULE OF FINISHES FOR EXTERNAL**

- 1)BOTTOM FLOOR TO HAVE SANDSTONE STACK STONE BLOCKS
- 2)TOP FLOOR WEATHER BOARDS, PAINTED IN DULUX

# WEATHERSHEILD COLOUR BUTTER RIDGE



Shane Doherty Phone: 0423580442

Email: shane.doherty@live.com.au ABN: 36 144 642 437 Licence No:247869c

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Given names (if person)	203 Ex	AC T	MES						 
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Local Council Area	PITTL	SATER							
1 DA/CC/CDC No.	NO248	3/11							
Estimated value of work (see note on back)	s	2400	00.	Levy payabl	e \$		84	Ο.	
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Contact person (Signature)				Date	e D	M	Y [		

Any false or misleading information provided on this form may result in prosecution under Section 58A.

I hereby declare that the information provided on this form is true and correct to the best of my knowledge

Name PSRM PCIPIC -- Signature Date D 16 M 06 Y 201

Exemption Approval Certificate No.

COST OF CORRS STATE

\$840.00 REC: 362244. 25/6/2014.

Long Service Corporation, Locked Bag 3000, Central Coast MC NSW 2252
Tel: 13 14 41 Fax: (02) 9287 5685 Email: levy@longservice.nsw.gov.au www.longservice.nsw.gov.au

ASN 93 645 090 808

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# Tax invoise Official Receipt

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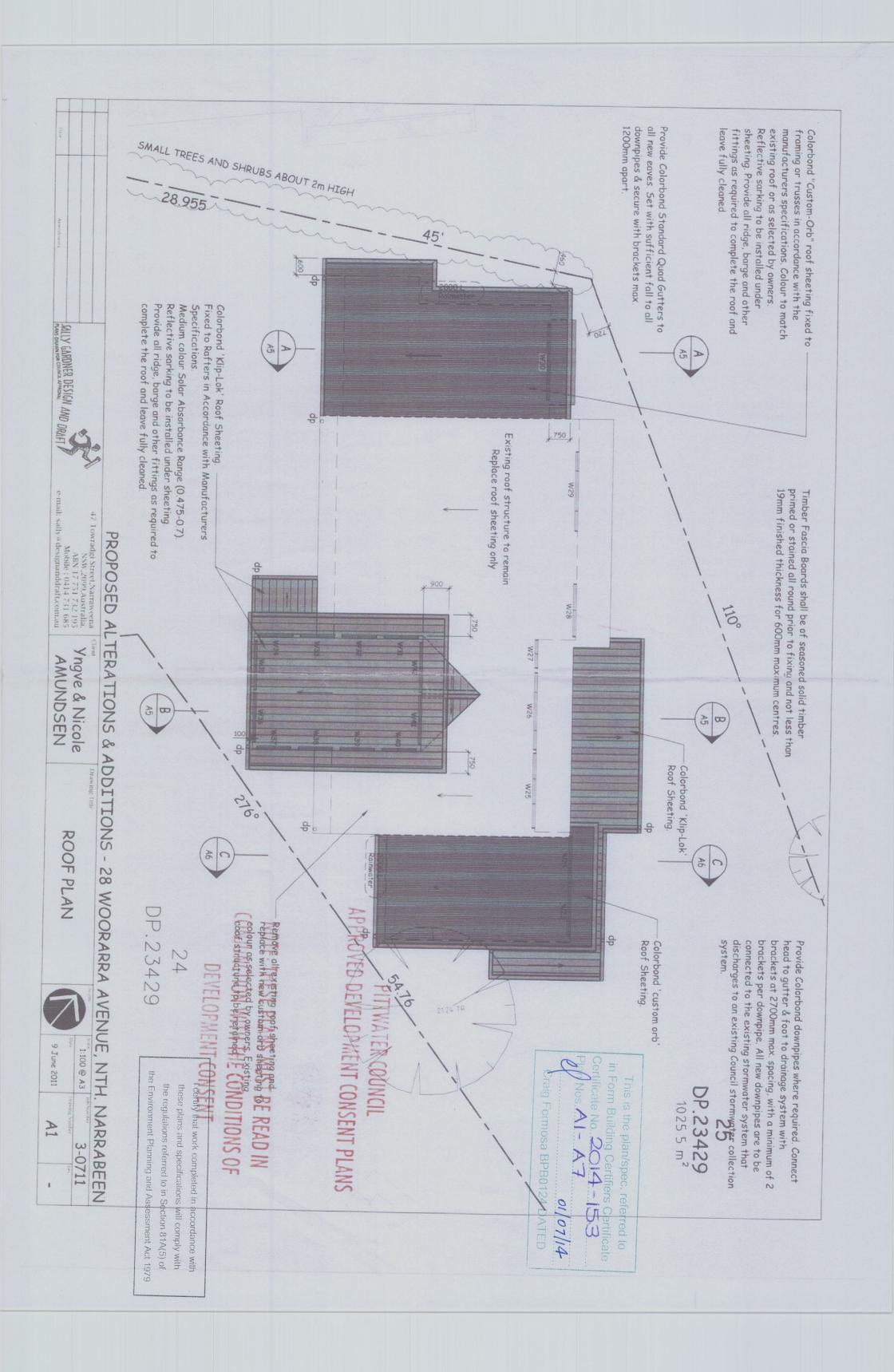
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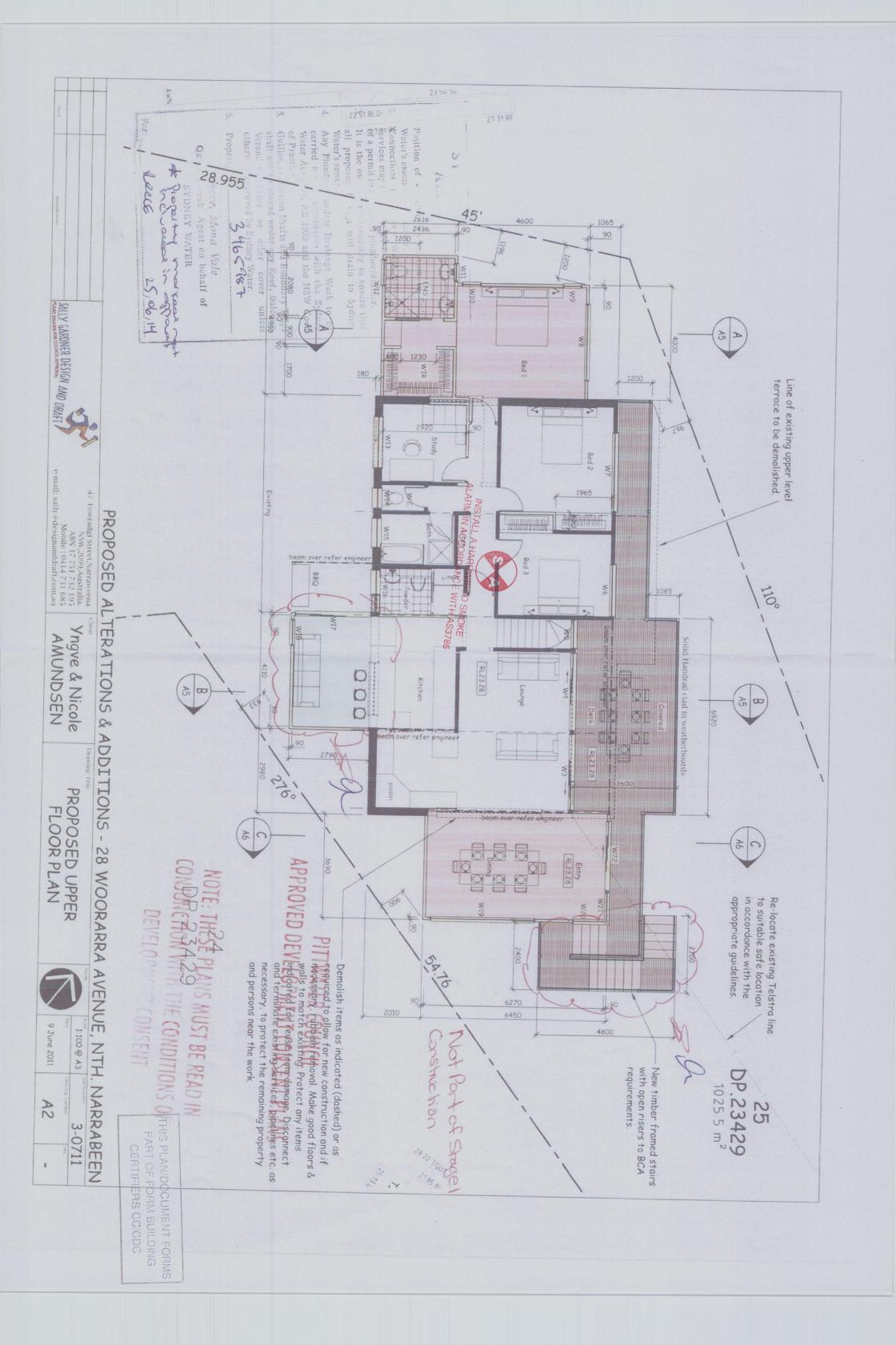
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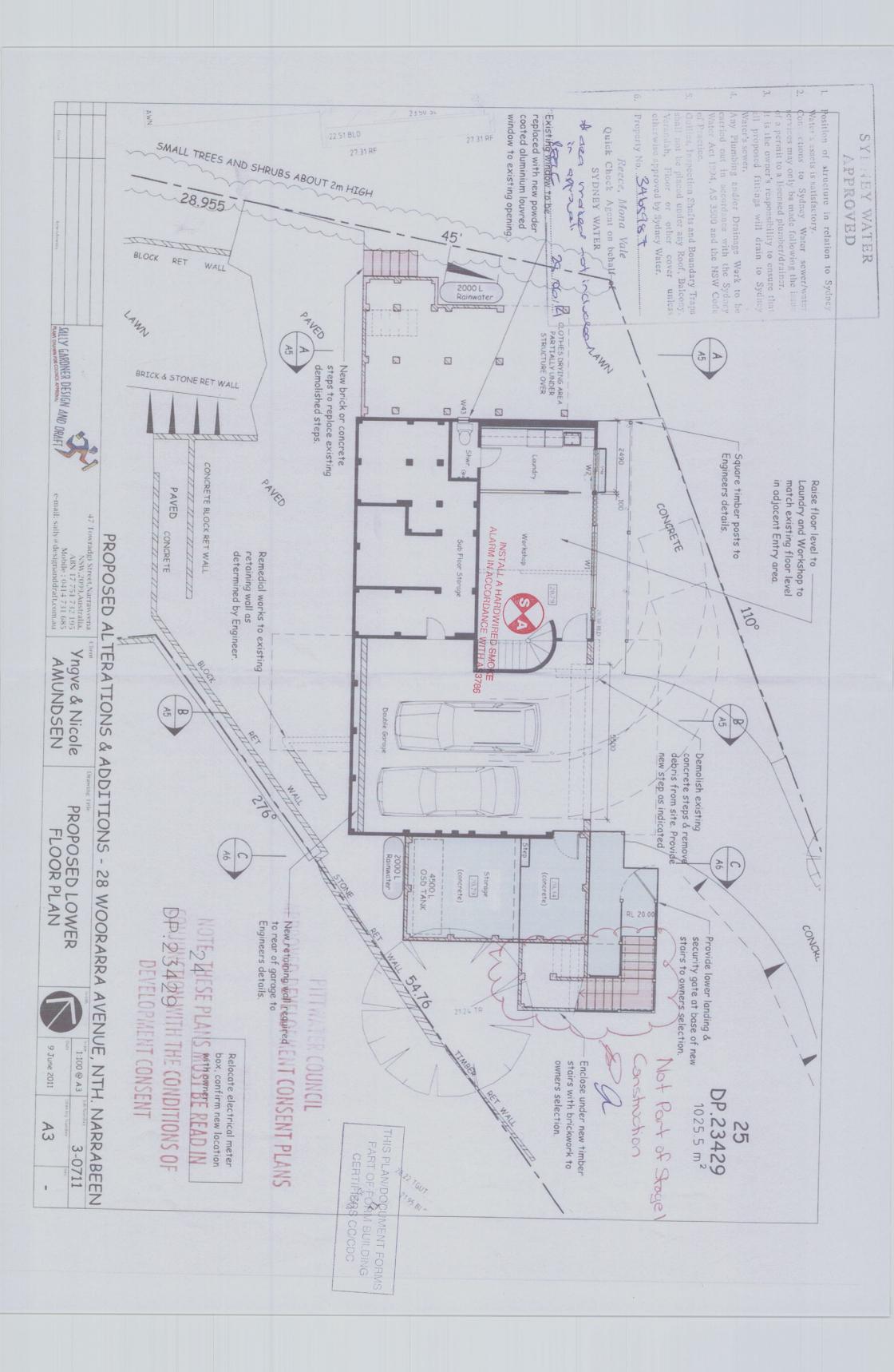
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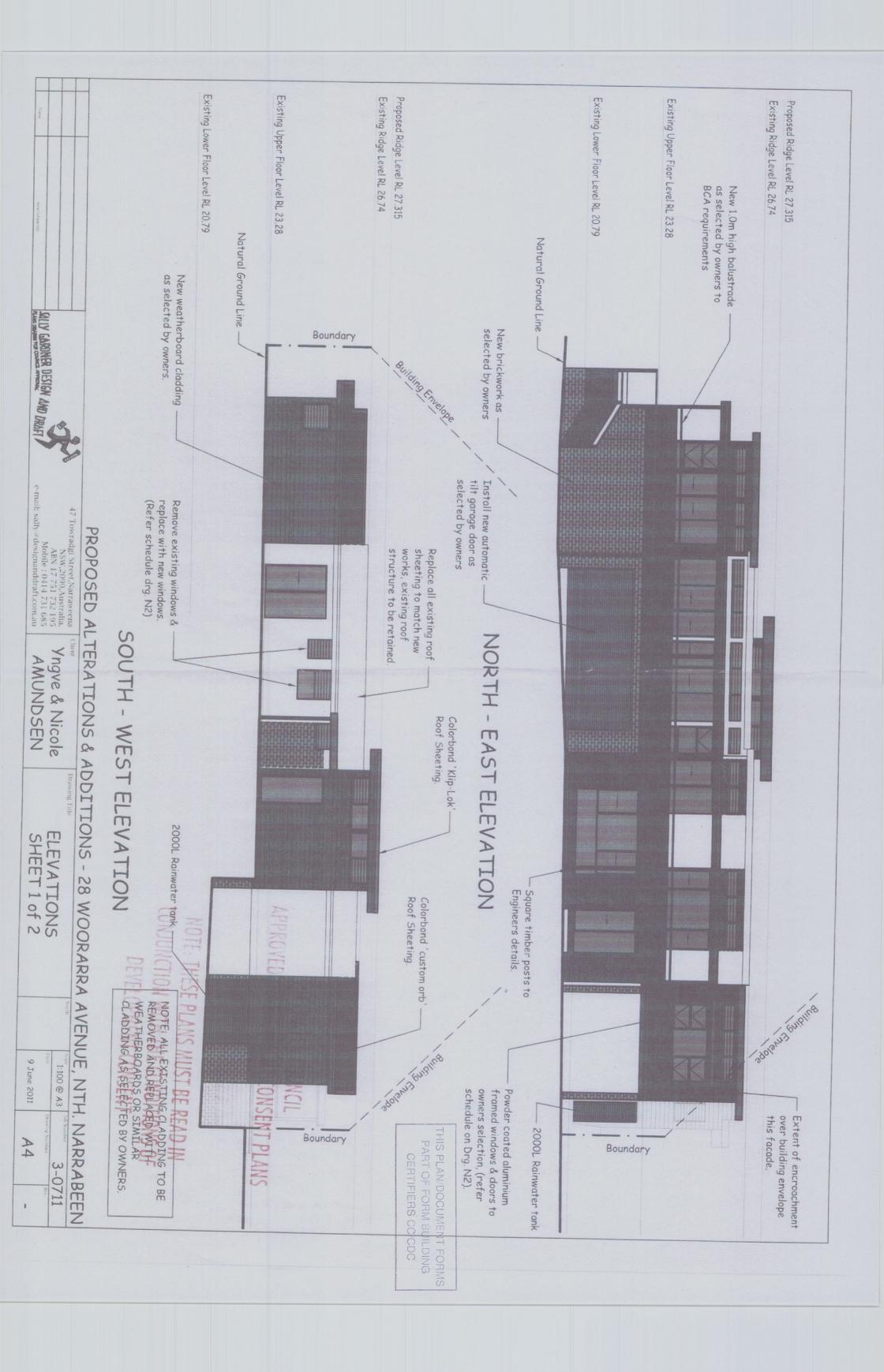
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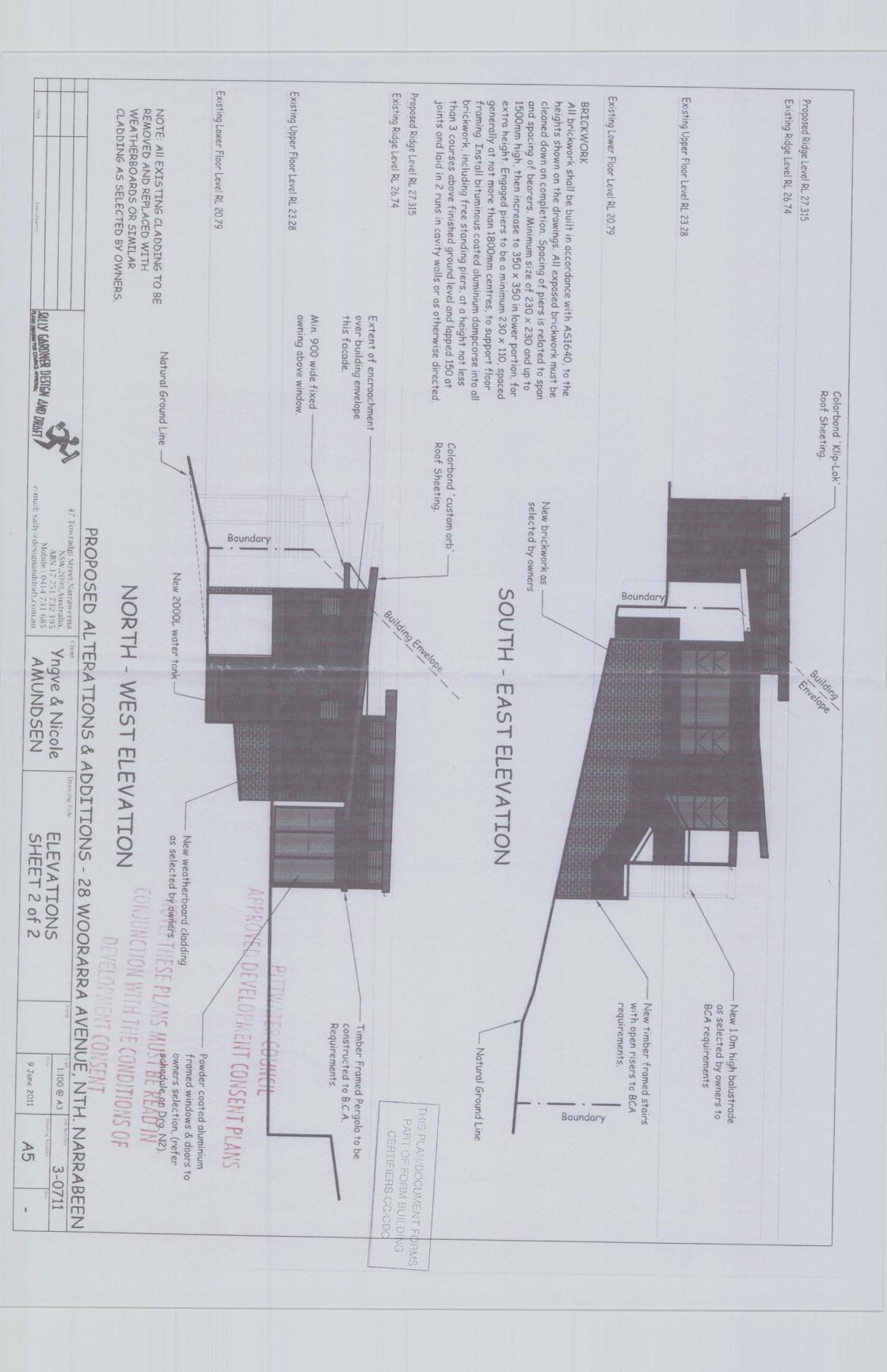
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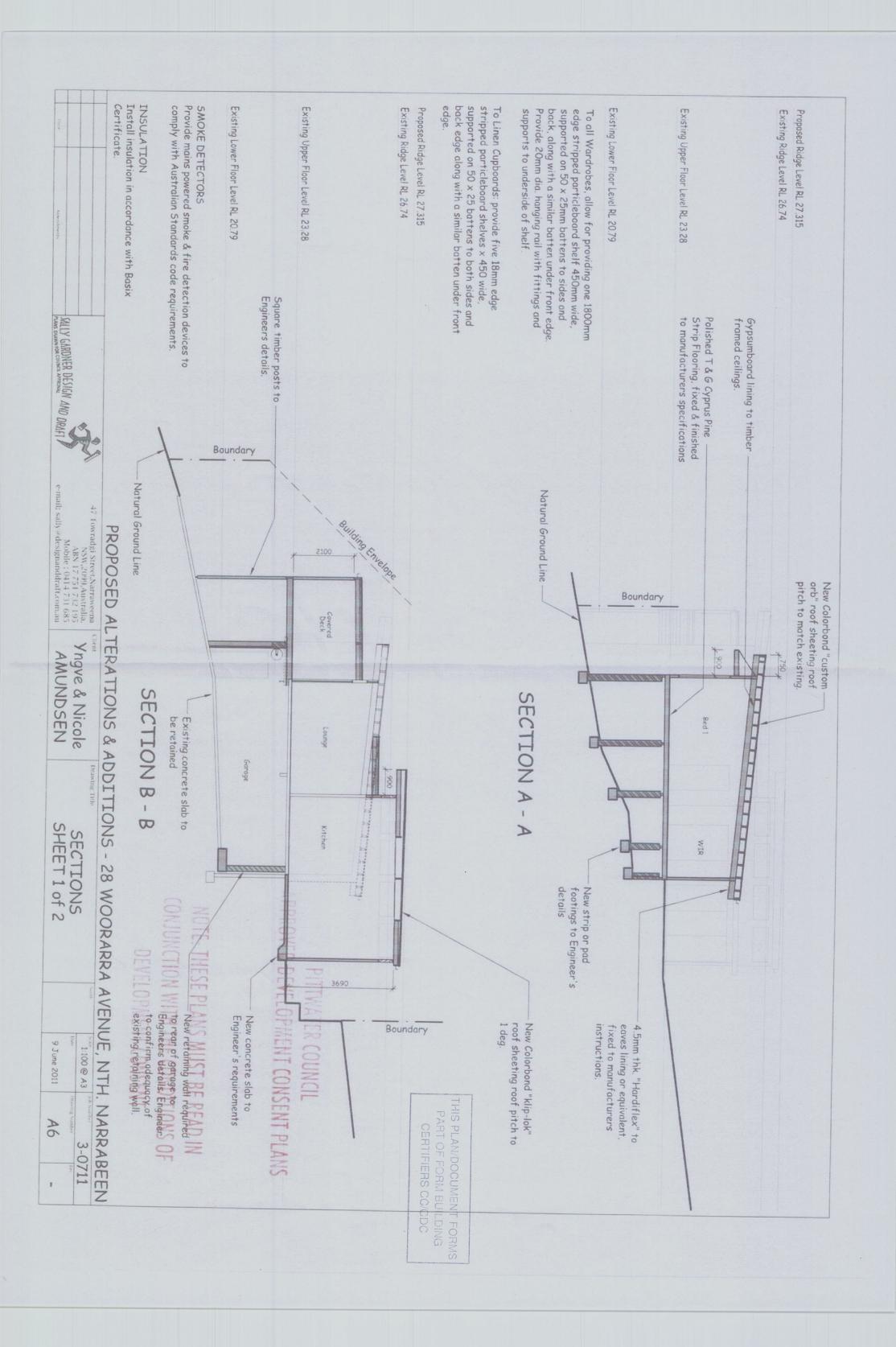












Existing Lower Floor Level RL 20.79 Allow for separate taps in location indicated for the washing machine and keep separate to those for the Existing Upper Floor Level RL 23.28 shower area shall be flashed to a height of at least Taps: Must have a flow rate no greater than 9 litres per minute or a minimum 3 star water rating. Toilets: Must have a flow rate no greater than 4 appropriate flexible epoxy coating to the whole of the shower floor area. All wall junctions in the Shower recesses shall be waterproofed with an no water penetration to adjoining construction. Wet areas: shall be adequately flashed to ensure Install aerators on bathroom hand basins & kitchen Showerheads: Must have a flow rate no greater Existing Ridge Level RL 26.74 than 9 litres per minute or a 3 star rating litres per average flush or a minimum 3 star water subject to attack by subteranean termites, then provide lining to all general areas and 'Hardies' Villaboard or similar to all protection to the new work by 1 or more of the means allowed specifications. wet' areas. Fixed & Finished in accordance with manufacturers All new Internal timber framed walls to be provided with gypsumboard If a member which provides structural support to the work is TIMBER WALLS for under AS 3660.1 TERMITE CONTROL SALLY GARDNER DESIGN AND DRAFT eaves lining or equivalent, fixed to manufacturers 4.5mm thk. "Hardiflex" to 450mm high splashbacks over tubs and vanities 2100mm high to all bathrooms and ensuites 1500mm high tiled skirting generally Walls: Provide to all W.C., Bathroom, Laundry indicated, with falls where required Floors: Provide tiles as selected to all W.C. and other wet areas indicated as follows :-Bathroom, Laundry and other wet areas Boundary 47 Towradgi Street,Narraweena NSW,2099,Australia, ABN 17-751-732-195 Mobile : 0414-731-685 e-mail: sally@designanddraft.com.au details under new addition concrete slab to Engineers Provide new reinforced PROPOSED ALTERATIONS & ADDITIONS - 28 WOORARRA AVENUE, NTH. 900 Yngve & Nicole fully lagged to all new & existing fittings as Hotwater: Provide copper water service which is terminate with pressure cocks all to Authorities Water Service: Extend from existing service required. requirements. fittings & hose cocks as indicated on plan & with copper pipe to allow new & relocated relocated fittings indicated to drainage system through wastes & traps as required by the Sanitary Plumbing: Provide & connect all new & AMUNDSEN 20.340 SECTION C -Wine Cellar Storage pitch to match existing orb" roof sheeting roof New Colorbond "custom SHEET 2 of 2 SECTIONS chosen by painted surfaces, allow for repainting existing surfaces to provide uniform appearances. Only ZERO-VOC or LOW-VOC paints and primers are to be used. applied over them. External joinery intended to be painted, shall be primed on all faces at the place of assembly. Colours to be approved brand and compatible to the finishing coats to be å of approved manufacture. All priming materials shall be of an All paints or other coatings shall be of the best quality materials PAINTING Boundary Owners. Where new or altered works adjoin existing on all faces at the place of assembly. Colours to be Engineers details. New retaining wall required to rear of storage room to suitable and available used in preference to plantation timbers if or second hand timbers are to be sourced and to rainforest or old growth timbers. Recycled Sustainable timbers will be used in preference 9 June 2011 1:100 @ A3 NARRABEEN A7 3-0711



calliden

NSWBIBHWI/177212-PermitAuthority

11/06/2014

Shane Doherty Constructions Pty Ltd trading as Shane Doherty Constructions Pty Ltd
15 Grasmere Crescent
WHEELER HEIGHTS NSW 2097

Calliden Insurance Ltd
ABN 47 004 125 268 AFS Licence 234438
Level 9, 11-33 Exhibition Street
MELBOURNE VIC 3000
Phone: (03) 9637 1300 FAX: 1300 662 215

# Certificate of Insurance RESIDENTIAL BUILDING WORK BY CONTRACTORS

A contract of insurance complying with sections 92 and 96A of the <u>Home Building Act 1989</u> has been issued by **Calliden Insurance Limited** (ABN 47 004 125 268) (AFSL 234438) as agent for and on behalf of the NSW Self Insurance Corporation (SICorp) (ABN 97 369 689 650) who is responsible for management of the Home Warranty Insurance Fund.

In respect of:

Structural Alterations/Additions

At.

28 Woorarra Avenue

NORTH NARRABEEN NSW 2101

Carried out by:

Shane Doherty Constructions Pty Ltd trading as Shane Doherty Constructions Pty Ltd

Licence Number:

247869C

ABN:

36 144 642 437

For:

Robert James Phipps

In the amount of:

\$240,000.00

Subject to the Act and the <u>Home Building Regulation 2004</u> and the conditions of the insurance contract, cover will be provided to:

- a beneficiary described in the contract and successors in title to the beneficiary,

OR

- the immediate successor in title to the contractor or developer who did the work and subsequent successors in title.

**Authorisation:** Signed by Calliden Insurance Ltd (ABN 47 004 125 268) (AFSL 234438) as agent for and on behalf of the NSW Self Insurance Corporation (SICorp) (ABN 97 369 689 650)

Issued on the 11th day of June, 2014.

NOTICE: To download a copy of your insurance policy wording visit http://www.policywording.com.au.

Page 1 of 1

PART OF FORM BUILDING

- The Owner will directly pay the fees associated with the following: local council, insurance fee to Building Services Corporation, Long building approval from council, footpath and kerb deposits with the Approved means by the 'The Relevant Local Authority'or 'Council'
- authority deposits which are forfeited due to damage or other cause All other fees are to be paid by the builder. The amount of any local will be deducted from the payments due to the builder. Service Leave levy fee and approval fee by water and sewerage authority.
- All tenderers are to visit the site to satisfy themselves as to the The Builder is to provide at his/her own expence adequate Public Risk Act. Works insurance to be as stated in the contract conditions Insurance and arrange indemnification under the Workers Compensation
- All work and materials to comply with the current Australian Standards at the time of commencement where applicable. arising owing to neglect of this clause.

entailed in the works as Variations will not be allowed due to work

nature and extent of the Works, facilities available and difficulties

- These drawings shall be read in conjunction with all structural and on site before commencement of any work. Dimensions shall not be obtained by scaling the drawings. Use only figured dimensions. All dimensions are Set out dimensions shown on this drawing shall be verified by the builder other consultants drawings and specifications and with any such written instructions as may be issued during the course of the contract
- with the conditions of the Building Approval.

  The Builder is to comply with all Ordinances, Local Authority regulations The Builder is to ensure all construction, levels and other items comply
- jurisdiction over the works. and the requirements of all Services Supply Authorities having
- system that discharges to an existing Council Stormwater system.

  All Power and Stormwater outlet locations shall be determined on site by All new downpipes are to be connected to the existing Stormwater

the Owner

- the Owner and the Builder to the Owner's approval, except for any structural details or design which is to be supplied by the Engineer. Any detailing in addition to what is supplied shall be resolved between All timber sizes and concrete details to be confirmed by the builder
- and is to be done as part of the contract. construction and/or finish is to be considered as shown and specified shown on the plan which is obviously necessary as part of proper Any work indicated on the plans but not specified, and any item not

prior to commencement of any work

- The Builder shall provide sediment and siltration control measures as Variations will not be permitted without the written consent of the Owner
- on the job at all times. Hours of construction shall be restricted to A legible copy of the plans bearing approval stamps must be maintained the times as required by the Building Approval. required by council and maintain them throughout the duration of the works.
- and lending institutions to their requirements. The Builder is to arrange for all inspections required by the authorities
- The Builder is to obtain approval for interruptions to existing services and existing services and equipment to be to by appropriatey skilled tradesmen minimise the duration and number of interruptions. Any interruptions with
- The Builder shall restore, reinstate or replace any damage caused Provide protection to existing trees to remain as required by to existing structures or landscaping by construction work or workmen.

# BASIX INSULATION SCHEDULE

Construction	Additional insulation required (R-value) Other specifications	Other specifications
concrete slab on ground floor.	nil	
suspended floor with open subfloor: framed (RO.7).	R0.8 (down) (or R1.50 including construction)	
external wall: framed (weatherboard, fibro, metal clad) (R0.40) R1.30 (or R1.70 including construction	R1.30 (or R1.70 including construction)	
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# DRAWING SCHEDULE

Title Sheet

N1. Notes and Schedules

N2. Window & Door Schedule

E2. Existing Lower Floor Plan E1. Existing Upper Floor Plan

A1. Roof Plan

A3. Proposed Lower Floor Plan A2. Proposed Upper Floor Plan

A4. Elevations Sheet 1 of 2

A5. Elevations Sheet 2 of 2

A6. Sections

S1. Site Analysis

53. Site Management Control Plan S2. Site Plan and Calculations

S4. Shadow Diagram at 9am

S5. Shadow Diagram at 12 noon
S6. Shadow Diagram at 3pm

S7. Stormwater Concept Plan

must be fitted with flourescent, compact A minimum of 40% of new or altered light fixtures LIGHTING : Basix requirements flourescent or light-emitting diode (LED) lamps.

THIS PLAN/DOCUMENT FORMS PART OF FORM BUILDING

# PROPOSED ALTERATIONS & ADDITIONS - 28 WOORARRA AVENUE, NTH. NARRABEEN

47 Towradgi Street,Narraweena NSW,2099,Australia. NSW,72099,Australia. ABN 17 751 732 195 Mobile : 0414 731 685 e-mail: sally@designanddraft.com.au

1/8/11

Roof colour altered

DALLY GARDNER DESIGN AND DRAFT

Yngve & Nicole AMUNDSEN

NOTES & SCHEDULES

9 June 2011 3-0711

Z

# GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER FORM NO. 2 – PART A – To be submitted with detailed design for Construction Certificate

	Development Application for MR ROB PHIPPS  Name of Applicant
	Traine or Appropri
	Address of site 28 WOORARRA AVENUE, NORTH NARRABEEN
RT A:	Declaration made by Structural or Civil Engineer in relation to the incorporation of the Geotechnical issues into the lesign
SIM	ION WADDINGTON On behalf of WADDINGTON CONSULTING PTY LTD
	(insert name) (trading or company name)
this the	e 19 June 2014
	(date)
the ab	at Lam a Structural or Civil Engineer as defined by the Geotechnical Risk Management Policy for Pittwater - 2009. Lam authorise love organisation/company to issue this document and to certify that the organisation/company has a current professional indemnity at least \$2million. It also certify that I have prepared the below listed structural documents in accordance with the indations given in the Geotechnical Report for the above development and that
ase m	nark appropriate box
<b>'</b>	the structural design meets the recommendations as set out in the Geotechnical Report or any revision thereto, the structural design has considered the requirements set out in the Geotechnical Report for Excavation and Landfill both for the excavation/construction phase and the final installation in accordance with Clause 3.2 (b)(iv) of the Geotechnical Ris Management Policy.
otech	nical Report Details:
	Report Title: RISK ANALYSIS & MANAGEMENT FOR PROPOSED DEVELOPMENT AT 28 WOORARRA AVE.
	NORTH NARRABEEN
	Report Date 2 AUGUST 2011
	Author MR BEN WHITE
	Author's Company/Organisation: JACK HODGSON CONSULTANTS PTY LTD
	Structural Documents list:
	10885-S0.00, 10885-S0.01, 10885-S1.00, 10885-S1.01, 10885-S2.00, 10885-S3.00,
	o aware that Pittwater Council relies on the processes covered by the Geotechnical Risk Management Policy, including the

THIS PLAN/DOCUMENT FORMS
PART OF FORM BUILDING
CERTIFIERS CC/CDC

# GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER FORM NO. 2 – PART B - To be submitted with detailed design for construction certificate

PART B Declaration made by Geotechnical Engineer or Engineering Geologist and/or Coastal Engineer (where applicable) in relation to the incorporation of the Geotechnical issues into the project design

I,	Peter Thompson	on behalf of	Jack Hodgson Consultants Pty Ltd
-	(insert name)		(trading or company name)
on th	his the 20 <sup>TH</sup> JUNE, 2014		
On a	(date	)	
Polic	cy for Pittwater – 2009 and I inization/company has a current p	am authorised by the aborrofessional indemnity policy	at and/or Coastal Engineer as defined by the Geotechnical Risk Management we organization/company to issue this document and to certify that the of at least \$2million. I also certify that I have reviewed the design plans and ate Stage and that I am satisfied that:
Plea	se mark appropriate box		
	the structural design has consid	ered the requirements set ou	in the Geotechnical Report or any revision thereto at in the Geotechnical Report for Excavation and Landfill both in in accordance with Clause 3.2 (b)(iv) of the Geotechnical Risk
	Geotechnical Report De	tails :	
	Report Title: RISK ANA AVENUE, NORTH NAF		NT FOR PROPOSED DEVELOPMENT AT 28 WOORARRA
	Report Date: 2 <sup>ND</sup> AUGUS	ST, 2011	
	Author: BEN WHITE		
	ARCHITECTURAL PLAI JUNE 2011	PREPARED BY WADDINGT	GARDNER DESIGN & DRAFT DWG NO: A1 TO A6 DATED ON CONSULTING DWG NOS: S0.00, S0.01, S1.00, S1.01,
certi	fication as the basis for ensuring	that the geotechnical risk ma	ered by the Geotechnical Risk Management Policy, including this nagement aspects of the proposed development have been adequately the life of the structure taken as at least 100 years unless otherwise stated
P	eter Thompson		Pt Dlambal (signature)
	(name)		(signature)
the b	pasis for ensuring that the geotect eve an "Acceptable Risk Manage	hnical risk management aspe ment" level for the life of the	ered by the Geotechnical Risk Management Policy, including this certification as ects of the proposed development have been adequately addressed to structure taken as at least 100 years unless otherwise stated and justified in identified to remove foreseeable risk
		Signature Pata	Thamps of
		Name Peter Thom	pson
		Chartered Professional St	atus MIE Aust CPEng
		Membership No. 146	800
		Company Jac	k Hodgson Consultants Pty Ltd

Policy of Operations and Procedures

Council Policy - No 178

THIS PLAN/DOCUMENT FORMS
PART OF FORM BUILDING
CERTIFIERS CC/CDC
Page 22



Jack Hodgson Consultants Pty Limited

CONSULTING CIVIL, GEOTECHNICAL AND STRUCTURAL ENGINEERS

ABN: 94 053 405 011

# RISK ANALYSIS & MANAGEMENT FOR PROPOSED DEVELOPMENT

# AT 28 WOORARRA AVENUE, NORTH NARRABEEN



DIRECTOR: J.D. HODGSON, M. Eng. Sc., F.I. E. Aust., Nper3 Struc. Civil 149788 67 Darley Street. Mona Vale NSW 2103

PO Box 389 Mona Vale NSW 1660

THIS PLAN/DOCUMENT FORMS PART OF FORM BUILDING

RECEIVED MONA VALE

0 8 AUG 2011

CUSTOMER SERVICE

# GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER FORM NO. 1 – To be submitted with Development Application

Address of site  Declaration made by geotechnical engineer or engineering geologist or coastal engineer (where applicable) as part of a geotechnic report  Ben White on behalf of Jack Hodgson Consultants Pty Ltd  [merh name)  I certy that I an a geotechnical engineer or engineering geologist or coastal engineer (where applicable) as part of a geotechnical engineer or engineering geologist or coastal engineer is defined by the Geotechnical Risk Management Policy for Pittwater - 2009 and I am authorised to the above organization/company to salue his document and to certify that the organisation/company has a current professional indeening policy of all least \$2 million have  Please mark appropriate box  Prepared the declared Geotechnical Report referenced below in accordance with the Australia Geomechnical Society 1 and such development and programment Geogramment Geomechnical Society 1 and such development and programment Geomechnical Risk Management Geotechnical Risk Management Policy for Pittwater - 2009 I and the detailed geotechnical Risk Management Policy for Pittwater - 2009 I and further detailed geotechnical Risk Management Policy for Pittwater - 2009 I confirm the results of the risk assessment for the proposed development are no morphicance with the Geotechnical Risk Management Policy for Pittwater - 2009 I confirm the results of the risk assessment for the proposed development Arie attains that do not require to first subject site.  Have examined the site and the proposed development and an off the geometrical Risk Management Arie attains that do not require to first subject site.  Have examined the site and the proposed development and the proposed development Arie attai		Development Application for Name of Applicant
Ben White Insert name)  Jack Hodgson Consultants Pty Ltd (Trading or Company Name)  certify that I am a geotechnical engineer or engineering geologist or coastal engineers as defined by the Geotechnical Risk Management Policy for Pittwater 2008 and I am autonosed by the above organisation/company to issue his document and to certify that the organisation/company has a current professional indemnity policy of at least \$2million. There is document and to certify that the organisation/company has a current professional indemnity policy of at least \$2million. There is a current professional indemnity policy of at least \$2million. There is a current professional indemnity policy of at least \$2million. There is a current professional indemnity policy of at least \$2million. There is a current professional indemnity policy of professional profe		
Interest name)  (Interest (2" August 2011)  (Interest (2" August 2)  (Interest	Deci	aration made by geotechnical engineer or engineering geologist or coastal engineer (where applicable) as part of a geotechnic report
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# GEOTECHNICAL RISK MANAGEMENT POLICY FOR PITTWATER FORM NO. 1(a) - Checklist of Requirements for Geotechnical Risk Management Report for Development Application

	Development Application for
	Name of Applicant Address of site 28 Woorarra Avenue, North Narrabeen
The fi	ollowing checklist covers the minimum requirements to be addressed in a Geotechnical Risk Management Geotechnical this checklist is to accompany the Geotechnical Report and its certification (Form No. 1)
(	Geotechnical Report Details:
	Report Title RISK ANALYSIS & MANAGEMENT FOR PROPOSED DEVELOPMENT AT 28 WOORARRA AVENUE. NORTH NARRABEEN
	Report Date: 2 <sup>nd</sup> August, 2011
	Author BEN WHITE
	Author's Company/Organisation, JACK HODGSON CONSULTANTS PTY LTD
Pleas	e mark appropriate box
123	Comprehensive site mapping conducted 29/01/2017
₩.	Mapping details presented on contoured site plan with geomorphic mapping to a minimum scale of 1.200 (as appropriate)
	Subsurface investigation required
	☐ No Justification  ☐ Yes Date conducted 29/07/2911
	Geotechnical model developed and reported as an interred subsurface type-section
[X]	Geotechnical hazards identified  Above the site
	☑ On the site
	Below the site
	Construct to avoid described and recorded
×	Risk assessment conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009  Consequence analysis  Frequency analysis
	Risk datculation Risk assessment for property conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009 Risk assessment for loss of life conducted in accordance with the Geotechnical Risk Management Policy for Pittwater - 2009 Assessed risks have been compared to "Acceptable Risk Management" criteria as defined in the Geotechnical Risk Management Policy for Pittwater - 2009
Ø	Opinion has been provided that the design can achieve the "Acceptable Risk Management" criteria provided that the specified conditions are achieved.
	Design Life Adopted   ☐ 100 years
	Dother
	specify  specify  Genterhood Risk Management Policy for
×	Geotechnical Conditions to be applied to all four phases as described in the Geotechnical Risk Management Policy for Pittwater – 2009 have been specified
	Additional action to remove risk where reasonable and practical have been identified and included in the report.  Risk Assessment within Bushfire Asset Protection Zone
the g	aware that Pittwater Council will rely on the Geotechnical Report, to which this checklist applies, as the basis for ensuring that leotechnical risk management aspects of the proposal have been adequately addressed to achieve an "Acceptable Risk igement" level for the life of the structure, taken as at least 100 years unless otherwise stated, and justified in the Report and easonable and practical measures have been identified to remove foreseeable risk.
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	Signature Belliele
	Name Ben White
	Chartered Professional Status MScGEOLAusIMM CP GEOL
	Membership No. 222757
	Company Jack Hodgson Consultants Pty Ltd



# Jack Hodgson Consultants Pty Limited

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# RISK ANALYSIS & MANAGEMENT FOR PROPOSED DEVELOPMENT AT 28 WOORARRA AVENUE, NORTH NARRABEEN

# 1. INTRODUCTION.

- 1.1 This assessment has been prepared to accompany an application for development approval. The requirements of the Geotechnical Risk Management Policy for Pittwater, 2009 have been met.
- 1.2 The definitions used in this Report are those used in the Geotechnical Risk Management Policy for Pittwater, 2009.
- 1.3 The methods used in this Assessment are based on those described in Landslide Risk Management March 2007, published by the Australian Geomechanics Society and as modified by the Geotechnical Risk Management Policy for Pittwater, 2009.
- 1.4 The experience of Jack Hodgson spans some 50 years in many areas of Australia and in the Pittwater area, particularly in the last 30 years as Principal of Jack Hodgson Consultants Pty Limited.

### PROPOSED DEVELOPMENT.

- 2.1 Extend the footprint of the house to the south east and north west.
- 2.2 Construct a deck
- 2.3 Construct a pergola.
- 2.3 Details of the proposed development are shown on 6 drawings prepared by Sally Gardener Design and Draft numbered A1 A6 and dated June 2011.



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# 3. DESCRIPTION OF SITE & SURROUNDING AREA.

- 3.1 The site was inspected on the 29th of July 2011.
- 3.2 The property has a north easterly aspect with an average slope of 20 degrees across the site. Above the site the slope eases to 12 degrees and continues for  $\sim$ 85 metres to the ridge of the hill. Below the property the slope continues for  $\sim$ 100 metres with an average gradient of 8 degrees.
- 3.3 The frontage consists of a concrete driveway and lawn that continue from the road to the house (Photos 1 & 2). Above the lawn a stable timber cantilever wall and stack rock wall run along the property boundary (Photos 3 & 4). On the south eastern side of the house an access path rises to the uphill side (Photo 5). On the north western side of the house there is a lawn area and a small stable brick retaining wall (Photo 6). Uphill of the house there is a large level paved area that is cut into the slope (Photo 7). Adjacent to the paved area there is a concrete block retaining wall. The retaining wall has a couple of cracks though this is attributed to differential settlement and not slope instability. Overall the wall is in good condition, it stands vertical and appears to be engineered (Photos 8 & 9). To the west of the paved area there is a concrete gravity retaining wall (Photo 10). The wall has a large vertical crack but there were no signs of deflection in the wall and it appeared stable at the time of inspection (Photo 11). The upper portion of the site is covered in a lawn area with a metal shed at the uphill boundary (Photo 12 & 13).
- 3.4 The two storey brick house is in good condition for its age (Photo 14). It is supported on brick piers and steel posts. Generally the brick walls and piers are in good condition and the steel posts stand vertical (Photos 15). Along the south eastern wall of the house there is a large horizontal crack near the bottom of the brick wall (Photo 16). The crack is attributed to differential settlement rather than slope instability.

# GEOLOGY OF THE SITE.

4.1 The site is underlain by interbedded sandstones, siltstones and shales of the Upper Narrabeen Group. The Narrabeen Group Rocks are Late Permian to Middle Triassic in age with the early rocks not outcropping in the area under discussion. The materials from which the rocks were formed consist of gravels, coarse to fine sands, silts and clays. They were deposited in a riverine type environment with larger floods causing fans of finer materials. The direction of deposition changed during the period of formation. The lower beds are very variable with the variations decreasing as the junction with the Hawkesbury Sandstones is approached. This is marked by the highest of persistent shale beds over thicker sandstone beds which are similar in composition to the Hawkesbury Sandstones.

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# GEOLOGY OF THE SITE (CONTINUED).

The slope materials are colluvial at the surface and residual at depth. They consist of sandy loam topsoil over sandy clays and clays with rock fragments and some floaters through out the profile. The sandy clays and clays merge into the weathered zone of the under lying rocks at depths expected to be in the range 0.5 to 2.0 metres.

# SUBSURFACE INVESTIGATION.

Three Dynamic Cone Penetrometer (DCP) tests were conducted in the locations shown on the site plan. The results of these tests are as follows.

DEPTH (m)	NUMBER OF B	LOWS - conducted	with Pointed Tip
	DCP 1	DCP 2	DCP 3
0.0 to 0.3	2	8	2
0.3 to 0.6	5	7	28
0.6 to 0.9	5	10	28
0.9 to 1.2		13	
1.2 to 1.5		21	
1.5 to 1.8		25	
1.8 to 2.1		47	
	Refusal @ 0.6m	End of test @ 2.1m	Refusal @ 0.9m

# DCP NOTES:

DCP 1 – Refusal @ 0.6m, maroon and orange impact dust on tip.

DCP 2 - End of test @ 2.1m, grey soil on moist tip.

DCP3 - Refusal @ 0.9m, brown topsoil on tip.

One Auger Hole test was conducted in the location shown on the site plan. The log of the auger hole is as follows:

# HAND AUGER 1

0.0 to 0.8m Brown, organic top soil. 0.8 to 1.1m Stiff yellow clay.

End of test @ 1.1m in stiff yellow clay



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# 6. DRAINAGE OF THE SITE.

# 6.1 ON THE SITE.

The site is well drained with no natural water courses

# 6.2 SURROUNDING AREA.

Normal sheet wash will enter the site from above during heavy down pours.

# 7. GEOTECHNICAL HAZARDS.

# 7.1 ABOVE THE SITE.

The slope above the site is considered as part of Hazard One in 7.2.

# 7.2 ON THE SITE.

The slope of the land surface that rises across the site is a potential hazard. (HAZARD ONE).

# 7.3 BELOW THE SITE.

The slope below the site eases to 8 degrees and the property below is developed and appears to be in good order as observed from the subject site. The slope below the site is not considered a geotechnical hazard.

# 7.4 BESIDE THE SITE.

The properties beside the site are at similar elevations and have similar geomorphology. The house's and grounds were in good condition as observed from the subject property. No geotechnical hazards likely to adversely affect the subject property were observed beside the site.

# 8. RISK ASSESSMENT.

# 8.1 ABOVE THE SITE.

The slope above is considered in Hazard One as part of 8.2

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# RISK ASSESSMENT (CONTINUED).

### 8.2 ON THE SITE.

# 8.2.1 HAZARD ONE Qualitative Risk Assessment on Property

The slope of the land surface rises across the property at angles of some 20 degrees. The house has been cut into the slope and the area uphill of the house has been landscaped with concrete block and concrete retaining walls. The retaining walls had cracks but were standing vertical and appeared stable. No evidence of tension cracking, slumping or any other signs of movement were observed on the site. The adjoining properties are developed and in good condition as observed from the subject property. The likelihood of the slope failing and impacting on the house is assessed as 'Rare' (10°). The consequences to property of such a failure are assessed as 'Medium' (20%). The risk to property is 'Low' (2 x 10°).

# 8.2.2 HAZARD ONE Quantitative Risk Assessment on Life

For loss of life risk can be calculated as follows:

 $\mathbf{R}_{(Loi)} = \mathbf{P}_{(H)} \times \mathbf{P}_{(SH)} \times \mathbf{P}_{(TS)} \times \mathbf{V}_{(DT)}$  (See Appendix for full explanation of terms)

### 8.2.2.1 Annual Probability

No evidence of movement was observed on the site

P(II) = 0.00001/annum

# 8.2.2.2 Probability of Spatial Impact

The house is located on a slope approximately half way up the slope.  $P_{(SH)} = 0.5$ 

# 8.2.2.3 Possibility of the Location Being Occupied During Failure

The average household is taken to be occupied by 4 people. It is estimated that I person is in the house for 20 hours a day, 7 days a week. It is estimated 3 people are in the house 12 hours a day, 5 days a week.

For the person most at risk

$$\frac{20}{24}x\frac{7}{7} = 0.83$$

 $P_{(1S)} = 0.83$ 



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# 8. RISK ASSESSMENT (CONTINUED).

# 8.2.2.4 Probability of Loss of Life on Impact of Failure

Based on the volume of land sliding and its likely velocity when it hits the house, it is estimated that the vulnerability of a person to being killed in the house when a landslide hits is 0.3

 $V_{(DT)} = 0.3$ 

## 8.2.2.5 Risk Estimation

 $\mathbf{R}_{(1.00)} = 0.00001 \times 0.5 \times 0.83 \times 0.3$ = 0.00000125

 $R_{(Lob)} = 1.25 \times 10^{-6}$ /annum NOTE: This level of risk is 'ACCEPTABLE'

# 8.3 BELOW THE SITE.

As no geotechnical hazards likely to adversely impact upon the subject site were observed below the site, no risk analysis is required.

# 8.4 BESIDE THE SITE.

As no geotechnical hazards likely to adversely impact upon the subject site were observed beside the site, no risk analysis is required.

# 9. SUITABILITY OF DEVELOPMENT FOR SITE.

# 9.1 GENERAL COMMENTS.

The proposed development is suitable for the site

# 9.2 GEOTECHNICAL COMMENTS.

No geotechnical hazards will be created by the completion of the proposed development if it is done in accordance with the requirements of this Report and good engineering and building practice.

# 9.3 CONCLUSIONS.

The site and the proposed development can achieve the Acceptable Risk Management criteria outlined in the Pittwater Geotechnical Risk Policy provided the recommendations given in **Section 10** are undertaken.

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### 10. RISK MANAGEMENT.

# 10.1. TYPE OF STRUCTURE.

The types of structure are suitable.

# 10.2. EXCAVATIONS.

A small excavation of 0.5m will be required to extend the house to the south east. The excavation is expected to be through -0.3 metres of topsoil and -0.2 metres of clay. Provided the soil materials are kept dry the excavation can stand unsupported for short periods of time until permanent support can be put in place.

# 10.3. FILLS.

If material from the excavation is to be used for landscaping on site it is to be battered at an angle of 1:2 or supported by an engineered retaining wall. The fill is to be placed in layers not more than 250 mm thick and compacted to not less than 95% of Standard Optimum Dry Density at plus or minus 2% of Standard Optimum Moisture Content.

# 10.4. FOUNDATION MATERIALS AND FOOTINGS.

It is recommended that the footings for the proposed development be taken to underlying rock using piers as necessary. The design ultimate bearing pressures are 1.2 MPa for spread footings or shallow piers.

# 10.5. STORM WATER DRAINAGE.

Storm water run off from the proposed addition is to be connected to the existing

# 10.6. SUBSURFACE DRAINAGE.

10.6.1 Retaining walls are required to have adequate back wall drainage.

10.6.2 A new retaining wall is shown in the garage. The space between the existing brick wall and the new wall is to be back filled with non cohesive free draining material to provide a drainage layer immediately behind the wall. The space is to be filled as close to the floor above as possible. The free draining material is to be separated from the ground materials by geotextile fabric.



MM 27702. 2<sup>nd</sup> August, 2011. Page 8.

### RISK MANAGEMENT (CONTINUED). 10.

# 10.7. INSPECTIONS.

10.7.1 It is recommended that the foundation materials of all footing excavations be inspected and approved before concrete is placed.

# GEOTECHNICAL CONDITIONS FOR ISSUE OF CONSTRUCTION CERTIFICATE.

It is recommended that the following geotechnical conditions be applied to the Development Approval:

The work is to be carried out in accordance with the Risk Management Report MM 27702 dated 2<sup>nd</sup> August, 2011.

The Geotechnical Engineer is to inspect and approve the foundation materials of all footing excavations before concrete is placed.

### 12. GEOTECHNICAL CONDITIONS FOR ISSUE OF OCCUPATION CERTIFICATE.

The Geotechnical Engineer is to certify the following geotechnical aspects of the development:

The work has been carried out in accordance with the Risk Management Report MM 27702 dated 2<sup>nd</sup> August, 2011.

The Geotechnical Engineer has inspected and approved the foundation materials of all footing excavations before concrete was placed.



MM 27702. 2<sup>nd</sup> August, 2011. Page 9.

# RISK ANALYSIS SUMMARY.

HAZARDS	Hazard One
ТҮРЕ	The slope on or above the site failing and impacting on the subject property.
LIKELIHOOD	'Rare' (10 <sup>-5</sup> )
CONSEQUENCES TO PROPERTY	'Medium' (20%)
RISK TO PROPERTY	'Low'(2 x 10')
RISK TO LIFE	1.25 x 10 <sup>-6</sup> /annum
COMMENTS	'Acceptable' level of risk.

JACK HODGSON CONSULTANTS PTY. LIMITED.

Ben White M.Sc. Geol., AusIMM., CP GEOL.

Fellate-

No. 222757

Engineering Geologist.

MM 27702. 2<sup>nd</sup> August, 2011. Page 10.



\* ,3

Photo 1



Photo 2

MM 27702. 2<sup>nd</sup> August, 2011. Page 11.



Photo 3



Photo 4

MM 27702. 2<sup>nd</sup> August, 2011. Page 12.



Photo 5



Photo 6

MM 27702. 2<sup>nd</sup> August, 2011. Page 13.



Photo 7



Photo 8

MM 27702, 2<sup>nd</sup> August, 2011, Page 14.



Photo 9

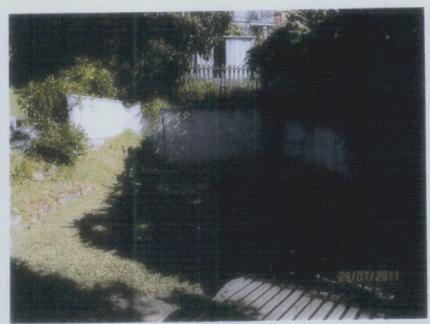


Photo 10

MM 27702. 2<sup>nd</sup> August, 2011. Page 15.



Photo 11



Photo 12

MM 27702. 2<sup>nd</sup> August, 2011. Page 16.



Photo 13



Photo 14

MM 27702. 2<sup>nd</sup> August, 2011. Page 17.



Photo 15



Photo 16

- Natural Ground Line New timber framed stairs Roundary New 1.0m high balustrade as selected by awners to BCA requirements with open risers to BCA requirements SOUTH - EAST ELEVATION Supplied Supplied Lippunda selected by owners New brickwork as -

Sandy Clay
Narrabeen Rock Group

\* Sandy Loam

Fill

Type Section 28 Woorarra Ave, Nth Narrabeen MM 27702

Scale 1:100



#### RISK ESTIMATION

7.1 QUANTITATIVE RISK ESTIMATION

Quantitative risk estimation involves integration of the frequency analysis and the consequences. For property, the risk can be calculated from:

 $\mathbf{R}_{(Prop)} = \mathbf{P}_{(H)} \times \mathbf{P}_{(S:H)} \times \mathbf{P}_{(T:S)} \times \mathbf{V}_{(Prop:S)} \times \mathbf{E} \tag{1}$ 

Where

 $R_{(Prop)}$  is the risk (annual loss of property value)  $P_{OP}$  is the annual probability of the landslide.

P<sub>(S:H)</sub> is the probability of spatial impact by the landslide on the property, taking into account the travel distance and travel direction.

 $P_{(T;S)}$  is the temporal spatial probability. For houses and other buildings  $P_{(T;S)} = 1.0$ . For Vehicles and other

moving elements at risk1.0<  $P_{(T:S)}>0$ .  $V_{(Prop:S)}$  is the vulnerability of the property to the spatial impact (proportion of property value lost).

E is the element at risk (e.g. the value or net present value of the property).

For loss of life, the individual risk can be calculated from:

 $\mathbf{R}_{(L \circ L)} = \mathbf{P}_{(H)} \times \mathbf{P}_{(S:H)} \times \mathbf{P}_{(T:S)} \times \mathbf{V}_{(D:T)} \tag{2}$ 

Where

 $R_{(LoL)}$  is the risk (annual probability of loss of life (death) of an individual).

P(th) is the annual probability of the landslide

P(S:H) is the probability of spatial impact of the landslide impacting a building (location) taking into account the travel distance and travel direction given the event.

P<sub>(T:S)</sub> is the temporal spatial probability (e.g. of the building or location being occupied by the individual) given the spatial impact and allowing for the possibility of evacuation given there is warning of the landslide occurrence.

V(D(I) is the vulnerability of the individual (probability of loss of life of the individual given the impact).

A full risk analysis involves consideration of all landslide hazards for the site (e.g. large, deep seated landsliding, smaller slides, boulder falls, debris flows) and all the elements at risk.

Australian Geomechanics Vol 42 No 1 March 2007



Our ref: 10885-L1

5 June 2014

Mr Rod Phipps 28 Woorarra Ave, North Narrabeen NSW

Dear Rod,

#### **Waddington Consulting Pty Ltd**

ACN 130 522 851 Structural and Civil Engineering Suite 506, Level 5 22 Central Ave, Manly P.O. Box 1044 Manly NSW 1655

> P (02) 9976 0070 F (02) 9976 0095

Subject:

Alterations & Additions at 28 Woorarra Ave, North Narrabeen Certificate for Engineering Design & Structural Adequacy

Please find attached copies of engineering drawings 10885-S0.00, S0.01, S1.00, S1.01, S2.00, S3.00, relating to the proposed alterations and additions at 28 Woorarra Ave, North Narrabeen.

I certify that the structural engineering design of the elements shown on the above-mentioned plans has been carried out in accordance with the BCA, relevant Australian Standards and normal engineering practice.

The existing double storey residence consists of mixed construction with full brick basement garage and upper floor of part timber frame and part brick veneer construction with a metal sheet roof. Overall, the structure appeared to be generally in good condition for its age and capable of withstanding the additional loading from the proposed additions if constructed in accordance with the above mentioned plans, the Building Code of Australia, generally accepted good building practice and relevant Australian Standards.

Please do not hesitate to contact me if you have any queries regarding this project or require any further structural engineering advice.

Yours sincerely,

**Simon Waddington** 

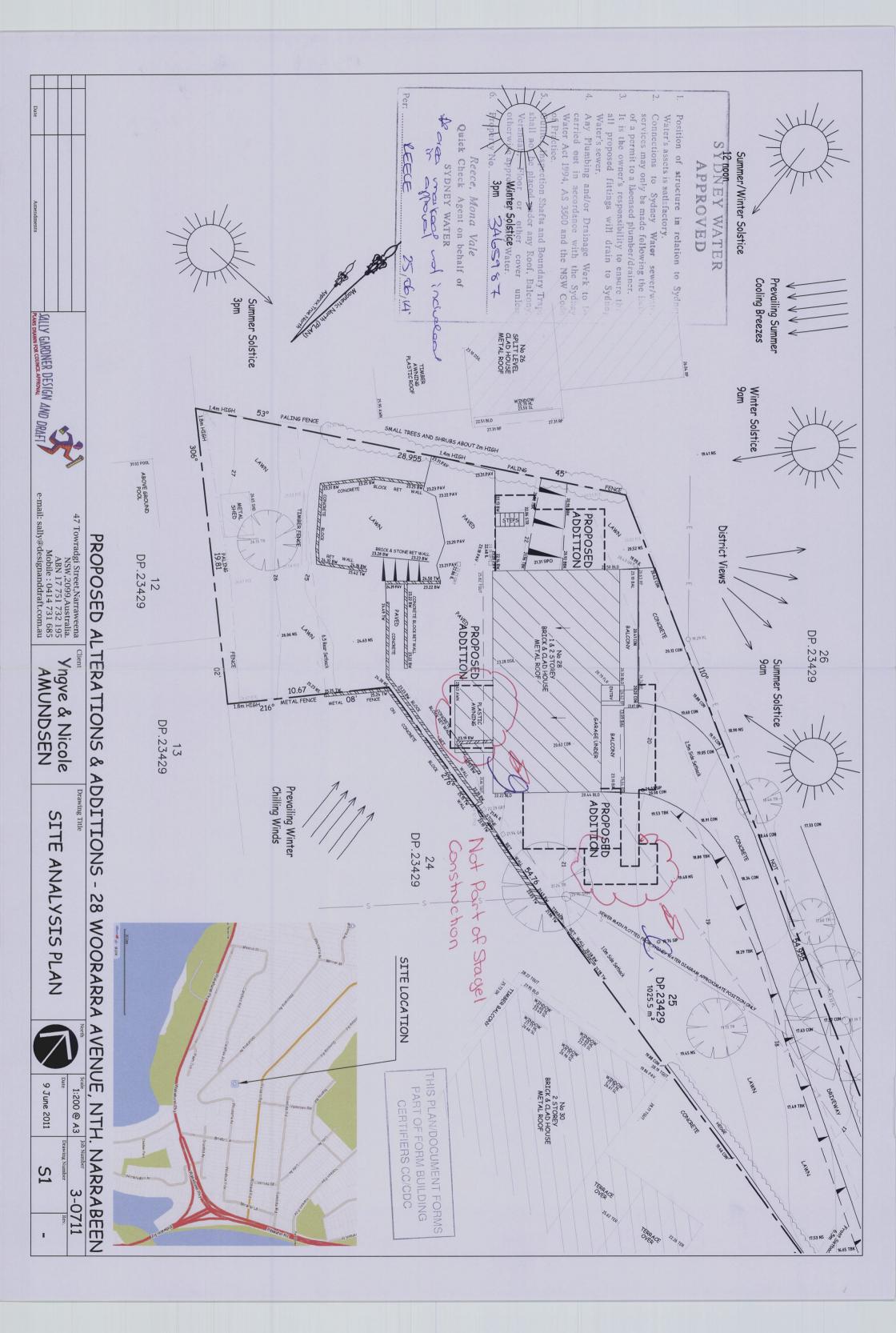
Simo libertligo

MIEAust CPEng NPER (Structural)

Director

Waddington Consulting Pty Ltd

THIS PLAN/DOCUMENT FORMS
PART OF FORM BUILDING
CERTIFIERS CC/CDC



Building Sustainability Index www basix nsw gov au

# Alterations and Additions

Certificate number A115956

This certificate confirms that the proposed development will meet the NSW government's requirements for sustainability if it is built in accordance with the governments set out below. Terms used in this certificate or in the commitments have the meaning given by the document entitled. BASIX Alterations and Additions. Definitions, dated 29/9/2006 published by Department of Planning. This document is available at www basix nsw gov au

Diractor General Date of issue: Tuesday 05: July 2011



#### Desci

Dwelling type  Separate dwelling house	Project type	Section number 0	Lot number 25	Plan type and number Deposited Plan 23429	Local Government Area Pithwater Council	Street address 28 Woorarra Avenue North Narrabeen 2101	Project name Amundsen Project V2	Project address
lse						North Narrabeen 2101		

THIS PLAN/DOCUMENT FORMS PART OF FORM BUILDING CERTIFIERS CO/CDC

Certificate Prepared by (please complete before submitting to Council or PCA)  Name / Company Name  ARN (if applicable)
---

The applicant must ensure new or altered showerheads have a flow rate no greater than 9 litres per minute or a 3 star water rating  The applicant must ensure new or altered toilets have a flow rate no greater than 4 litres per average flush or a minimum 3 star water rating  The applicant must ensure new or altered taps have a flow rate no greater than 9 litres per minute or minimum 3 star water rating
t must ensure new or altered showerheads have a flow rate no greater than 9 litres per minute or a 3 star water rating  Unust ensure new or altered toilets have a flow rate no greater than 4 litres per average flush or a minimum 3 star water rating
The applicant must ensure a minimum of 40% of new or altered light fixtures are fitted with fluorescent, compact fluorescent, or   V  V  V  V  V  V  V  V  V  V  V  V  V

				_	_		_	
raked ceiling, pitched/skillion roof framed	external wall framed (weatherboard, fibro, metal clad)	suspended floor with open subfloor framed (R0.7)	concrete slab on ground floor		is not required for parts of altered construction where insulation already exists	The applicant must construct the new or altered the table below, except that a) additional insulab	Insulation requirements	
ceiling R3 00 (up) roof foil/sarking medium (solar absorptance 0 475 - 0 70)	R1 30 (or R1 70 including construction)	R0 8 (down) (or R1 50 including construction)	ni		there insulation already exists	The applicant must construct the new or altered construction (floor(s), walls, and ceilings/roofs) in accordance with the specifications listed in the table below, except that a) additional insulation is not required where the area of new construction is less than 2m2 b) insulation specified.		
		<b>-</b>	_,	<u> </u>		< 		
						<u> </u>		
						ς.		

Windows	Windows and glazed doors	SIO							
The applic	cant must install the	e windows	glazed d must be	oors and sha	The applicant must install the windows, glazed doors and shading devices, in accordance with the specifications listed in the Relevant overshadowing specifications must be satisfied for each window and glazed door.	he specifications listed in the table below	<u> </u>	ζ.	ς,
The follow	ving requirements	must also t	oe satisfie	d in relation (	The following requirements must also be satisfied in relation to each window and glazed door		·-	<	ζ,
Each wind have a U-	dow or glazed doo value and a Solar alculated in accon	r with stand Heat Gain dance with	dard alum Coefficiet National	mum or timb nt (SHGC) no Fenestration	Each window or glazed door with standard aluminium or timber frames and single clear or toned have a U-value and a Solar Heat Gain Coefficient (SHGC) no greater than that listed in the table must be calculated in accordance with National Fenestration Rating Council (NFRC) conditions	Each window or glazed door with standard aluminium or timber frames and single clear or toned glass may either match the description, or, have a U-value and a Solar Heat Gain Coefficient (SHGC) no greater than that listed in the table below. Total system U-values and SHGCs must be calculated in accordance with National Fenestration Rating Council (NFRC) conditions.		٤.	<,
Each wind have a U- must be o only Alle	dow or glazed doo -velue and a Solar ∷alculated in accor mative systems w	r with impra Heat Gain dence with th complyi	oved fram Coefficiel National Ng U-valu	es or pyroly nt (SHGC) no Fenestration e and SHGC	Each window or glazed door with improved frames or pyrolytic low-e glass, or clear/air gap/clear glazing, or toned/air gap/nave a U-value and a Solar Heat Gain Coefficient (SHGC) no greater than that listed in the table below. Total system U-values to executated in accordance with National Fenestration Rating Council (NFRC) conditions. The description is provided only. Alternative systems with complying U-value and SHGC may be substituted.	Each window or glazed door with improved frames or pyrolytic low-e glass, or clear/air gap/clear glazing, or toned/air gap/clear glazing must have a U-value and a Solar Heat Gain Coefficient (SHGC) no greater than that listed in the table below. Total system U-values and SHGCs must be calculated in accordance with National Fenestration Rating Council (NFRC) conditions. The description is provided for information only. Alternative systems with complying U-value and SHGC may be substituted.		۲	<
For project	ctions described in head of the wind	nıllımetre: ow or glaze	s the lead	ding edge of id no more th	For projections described in millimetres, the leading edge of each eave, pergolal verandah, balcony or awning must be no above the head of the window or glazed door and no more than 2400 mm above the sill	cony or awning must be no more than 500 mm	ς,	<	۲,
Pergolas	with polycarbonat	Se roof or su	milar trans	slucent mate	Pergolas with polycarbonate roof or similar translucent material must have a shading coefficient of less than 0 35	t of less than 0 36		۲,	ς.
Pergolas shades a	with fixed battens	must have	battens p	arailei to the	Pergolas with fixed battens must have battens parallel to the window or glazed door above which they are situated unless shades a perpendicular window. The spacing between battens must not be more than 50 mm.	ch they are situated unless the pergola also		ς.	ς.
Window	Windows and glazed doors glazing requirements	doors gl	azıng re	equiremen	) ts				, ,
W1	NE	62	0	C	eave/verandah/pergola/balcony >=900 mm	Standard aluminium, single clear, (or U-value 7 63, SHGC 075)			
W2.	NE	2 85	0	0	eave/verandah/pergola/balcony >=900 mm	Standard aluminium single clear (or U-value 7 63, SHGC 0 75)			
W3	NII	3 17	6	0	eave/verandah/pergola/balcony >=900 mm	Standard aluminium single clear, (or U-value 7 63, SHGC 0 75)			

eave/verandah/pergola/balcony standard aluminium single clear (or
eave/verandat/pergola/balcony standard aluminium sir >=600 mm U-value 7.57 SHGC 0
eave/verandah/pergola/balcony standard aluminium, single toned (or >=600 mm U-value 7 57, SHGC 0 57)
eave/verandatv/pergola/balcony standard aluminium, single toned, (or >=600 mm U-value 7 57 SHGC 0 57)
eave/verandah/pergola/balcony standard alummrum single foned, (or >=750 mm U-value 7 57 SHGC 0 57)
eave/verandah/pergola/balcony standard aluminium single clear (or >=900 mm U-value 7 63 SHGC 0 75)
awning (adjustable) >=900 mm standard atuminium, single clear (or U-value 7 63 SHGC 0 75)
awning (adjustable) >=900 mm standard aluminium, single clear, (or U-value 7 63, SHGC 0 75)
eave/verandah/pergota/balcony standard aluminium, single clear, (or >=900 mm U-value 7 63 SHGC 0 75)
eave/verandah/pergola/balcony standard aluminium, single clear, (or >=900 mm U-value 7 63, SHGC 0 75)
eave/verandah/pergola/baicony standard aluminium single clear (or >=900 mm U-value 7 63, SHGC 0 75)
eave/verandah/pergola/balcony standard aluminium, single clear, (or >=900 mm U-value 7 63 SHGC 075)
eave/verandah/pergola/balcony standard aluminium, single clear (or >=900 mm U-value 7 63, SHGC 0 75)

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standard aluminium, single clear (or U-value 7 63 SHGC 0 75)	eave/verandah/pergola/balcony stanc	eave/verand	0	0	1 02	NE NE	W29
standard aluminum, single clear, (or U-value 7 63 SHGC 075)	eave/verandah/pergola/balcony stand >=900 mm U-va	eave/verand	- 0	0	068	Ž	W28
standard aluminium, single clear, (or U-value 7 63, SHGC 075)	eave/verandah/pergola/balcony stant >=900 mm U-va	eave/verand: >=900 mm	0		0 35	NE NE	W27
l alummium, s	eave/verandah/pergola/balcony standare >=900 mm U-value	eave/verand	Q	<del>-</del> <del>-</del>	1 02	NE NE	W26
standard aluminium, single clear, (or U-value 7 63, SHGC 075)	eave/verandeh/pergola/balcony stanc	eave/verand	0	0	1 02	NE NE	W25
standard aluminium single clear (or U-value 7 63, SHGC 0 75)	eave/verandah/pergola/balcony stand >=900 mm	eave/verand	0	0	89.0		W24
standard aluminium single clear, (or U-value 7 63 SHGC 0 75)	eave/verandah/pergola/balcony stand >=900 mm	eave/verando	0	0	0 68	NE NE	W23
standard aluminium, single clear (or U-value 7 63, SHGC 0 75)	eave/verandah/pergota/balcony stand >=900 mm U-val	eave/veranda >=900 mm	0		5 19	m Z	W22
standard aluminium single clear, (or U-value 7 83 SHGC 0 75)	eave/verandah/pergola/balcony stand >=900 mm	eave/veranda >=900 mm	0	- <del>-</del>	191		W21
standard aluminium, single clear (or U-value 7 63, SHGC 0 75)	eave/verandah/pergola/balcony stand >=900 mm U-val	eave/veranda >=900 mm	- <del>-</del>	_ <del>5</del>	252	E III	W20
standard alumınıum sıngle dear (or U-value 763 SHGC 075)	stand U-val	none	- 0		621	SE	61.M
standard aluminium, single clear (or U-value 7 83 SHGC 0 75)	stand U-val	none		<u>;</u>	1 89	SW	W18
standard aluminium single clear (or U-value 7 63 SHGC 0 75)	eave/verandah/pergola/balcony standard >=800 mm U-value	eave/veranda >=900 mm	0		65,	WN	2LM

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	W40	6EAA	8£M	W37	W36	W35	W34	EE/W	W32	W31	OCM	
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	,				:							
	eave/verar	eave/veran	eave/veran	eave/verar	eave/veran	enon	eave/verar >=750 mm	eave/verar	eave/veran	eave/verar	eave/veran >=750 mm	
	erandah mm	erandahi mm	erandah/ mm	erandah)	erandah/ mm		erandah/ mm	erandah/ mm	orandah/ mm	erandah/	)randah/ mm	
	eave/verandah/pergola/balcony >=900 mm	eave/verandah/pergola/balcony >=750 mm	eave/verandah/pergola/balcony >=750 mm	eave/verandah/pergola/balcony >=750 กหก	eave/verandah/pergola/balcony >=750 mm		eave/verandah/pergola/balcony >=750 mm	eave/verandah/pergola/balcony >=750 mm	eave/verandah/pergola/balcony >=750 mm	eave/verandah/pergola/balcony >=750 mm	eave/verandah/pergota/balcony >=750 mm	
	balcony	balcony	balcony	balcony	balcony	•	balcony	balcony	balcony	balcony	balcony	
	(U-va)	(U-val	(U-val	(U-val	impro	impro	(U-vat	mpro)	(U-valı	U-valı	(U-valı	
	improved aluminium, single toned, (U-value 6.39 SHGC 0.56)	Improved aluminium single toned (U-value 6.39, SHGC 0.56)	improved aluminium, single to (U-value 6 39, SHGC 0 56)	(U-value 6.39, SHGC 0.56)	Improved aluminium single to (U-value 638 SHGC 056)	improved aluminium, single to (U-value 6.39 SHGC 0.56)	Improved aluminium, single toned, (U-vatue 6.39, SHGC 0.56)	improved aluminium, single toned, (U-value 6 39, SHGC 0 58)	Improved aluminium single toned (U-value 6.39, SHGC 0.56)	improved aluminium, single toned, (U-value 6.39 SHGC 0.56)	(U-value 639, SHGC 056)	
	ninium, s	SHGC	SHGC	SHGC	SHGC	SHGC	nnium, s SHGC	SHGC	SHGC	SHGC	SHGC	
	angle tor 0 56)	ingle tor 0.56)	ingle toned. 0 56)	angle tor	ingle loned 0 56)	ingle toned, 0 56)	ingle tar 0.56)	ingle ton 0 56)	ingle ton	ingle ton 0 56)	ingle toned, 0 56)	
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Department of Planning

In these commitments "applicant" means the person carrying out the development

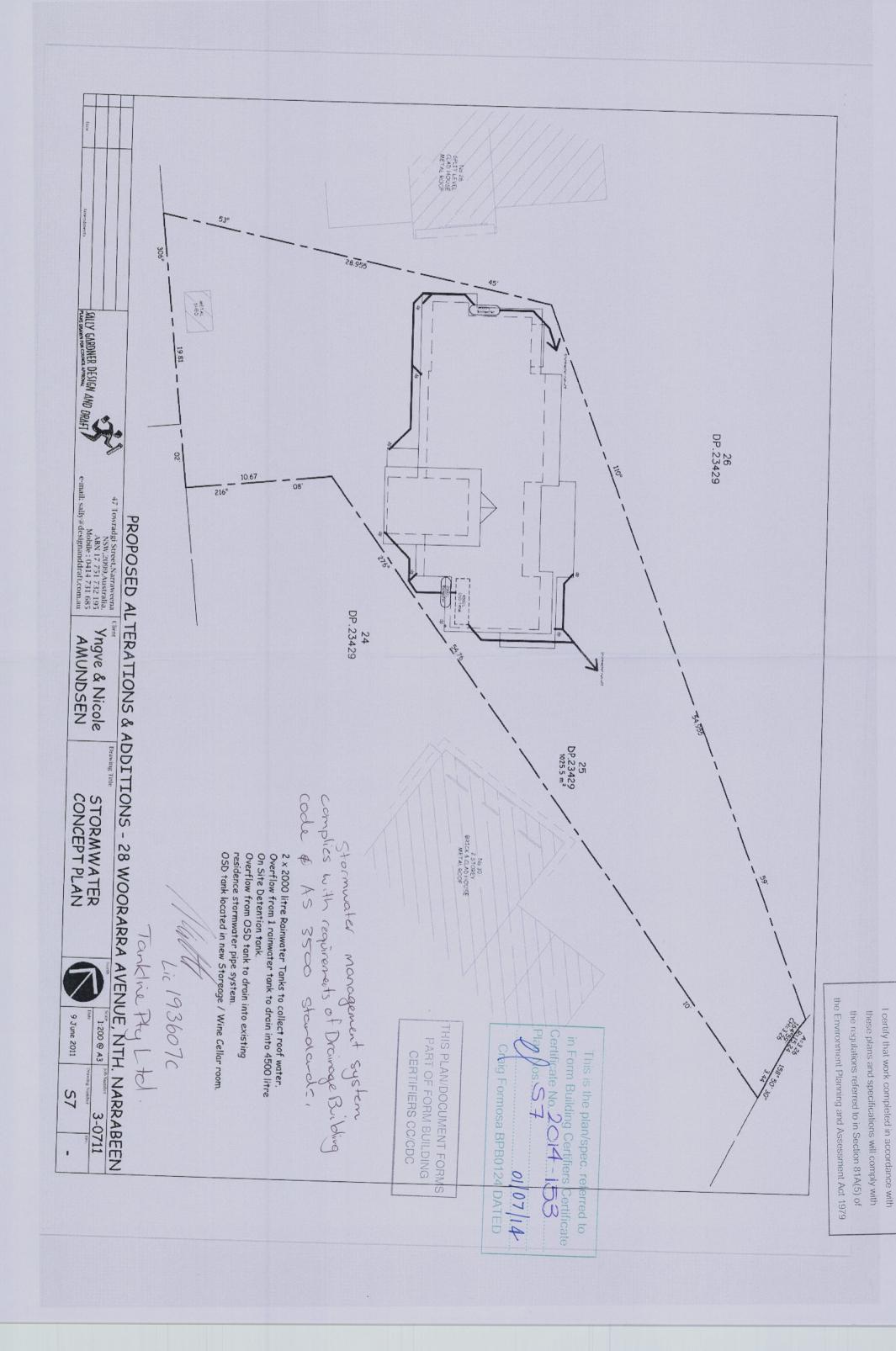
development application is to be lodged for the proposed development)

Commitments identified with a "\" in the "Show on DA plans" column must be shown on the plans accompanying the development application for the proposed development (if a

Commitments identified with a "\formula" in the "Show on CC/CDC plans & specs" column must be shown in the plans and specifications accompanying the application for a construction certificate / complying development certificate for the proposed development

Commitments identified with a "

" in the "Certifier check" column must be certified by a certifying authority as having been fulfilled before a final occupation certificate for the development may be issued



# or: ROB PHIPPS RABEEN

# STRUCTURAL NOTES GENERAL

G2 THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH ALL ARCHITECTURAL AND OTHER CONSULTANTS' DRAWINGS AND SPECIFICATIONS AND WITH SUCH OTHER WRITTEN INSTRUCTIONS AS MAY BE ISSUED DURING THE COURSE OF THE CONTRACT.

63 THE INFORMATION CONTAINED ON THESE DRAWINGS IS FOR STRUCTURAL ENGINEERING PURPOSES ONLY. IN ALL OTHER MATTERS, THE APPROVED ARCHITECTS DRAWINGS SHALL TAKE PRECEDENCE. ALL DISCREPANCIES THAT COULD RESULT IN CHANGES TO THE STRUCTURAL DETAILS SHALL BE REFERRED TO THE ENGINEER PRIOR TO PROCEEDING WITH CONSTRUCTION. ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE RELEVANT AND CURRENT AUSTRALIAN STANDARDS AND WITH THE BY-LAWS AND ORDINANCES OF THE RELEVANT BUILDING AUTHORITIES.

**G4** DURING CONSTRUCTION THE STRUCTURE SHALL BE MAINTAINED IN A STABLE CONDITION AND NO PART SHALL BE OVERSTRESSED. TEMPORARY BRACING SHALL BE PROVIDED BY THE BUILDER TO KEEP THE WORKS AND EXCAVATIONS STABLE AT ALL TIMES.

**G7** 96 THE STRUCTURAL COMPONENTS DETAILED ON THESE DRAWINGS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE RELEVANT AUSTRALIAN STANDARDS AND LOCAL GOVERNMENT ORDINANCES. THE BUILDER SHALL GIVE 48 HOURS NOTICE FOR ALL ENGINEERING INSPECTIONS.
UNLESS NOTED OTHERWISE ALL LEVELS ARE IN METRES AND ALL DIMENSIONS ARE
IN MILLIMETRES. ENGINEER'S DRAWINGS SHALL NOT BE SCALED FOR DIMENSIONS. ALL DIMENSIONS SHOWN SHALL BE VERIFIED BY THE BUILDER ON SITE.

WIND LOADS ARE DETERMINED IN ACCORDANCE WITH AS4055 FOR WIND CLASSIFICATION: 'N2' WITH A METAL SHEET ROOF.

68

10885-S0.010885-S1.010885-S1.010885-S1.010885-S2.010885-S3.00

STRUCTURAL NO STRUCTURAL NO FOOTING / BAS GROUND FLOOR ROOF FRAMING

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NT PLAN NT DETAILS MING PLAN 8

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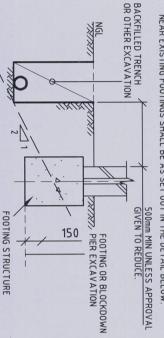
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TRUCTURAL DRAWINGS

FOUNDATIONS & EARTHWORKS FOUNDATIONS HAVE BEEN DESIGNED FOR AN ALLOWABLE BEARING INTENSITY OF 600kPa ON WEATHERED SANDSTONE BEDDROCK. FOUNDATION MATERIAL TO BE CONFIRMED ON SITE. STIFFENED RAFT SLAB FOOTING DESIGNED FOR "MIXED" CONSTRUCTION ON CLASS 'A' SITE IN ACCORDANCE WITH AS 2870.

TOPSOIL INCLUDING GRASS ROOTS IS TO BE REMOVED FROM THE AREA TO SUPPORT SLABS AND FOOTINGS. FOOTINGS TO BE CONSTRUCTED AND BACKFILLED AS SOON AS POSSIBLE FOLLOWING EXCAVATION TO AVOID SOFTENING OR DRYING OUT BY EXPOSURE. TRENCHES TO BE DEWATERED & CLEANED OUT PRIOR TO

UNLESS OTHERWISE APPROVED BY THE ENGINEER, THE LIMITS OF EXCAVATIONS NEAR EXISTING FOOTINGS SHALL BE AS SET OUT IN THE DETAIL BELOW. CONCRETE PLACEMENT



R4

CONTROLLED FILL: SAND FILL UP TO 0.8m DEEP, WELL COMPACTED IN NOT MORE THAN 300mm THICK LAYERS BY A VIBRATING PLATE OR VIBRATING ROLLER. NON-SAND FILL UP TO 0.4m DEEP, WELL COMPACTED IN LAYERS NOT MORE THAN 150mm DEEP BY A MECHANICAL ROLLER. CLAY FILL SHALL BE MOIST DURING COMPACTION. PRIOR TO ANY EXCAVATION NEAR EXISTING FOOTINGS, THE BUILDER SHALL DETERMINE THE DEPTH OF FOUNDING OF EXISTING FOOTINGS BY LOCAL INVESTIGATORY EXCAVATION. GENERAL EXCAVATION SHALL NOT PROCEED BELOW A LEVEL 150mm ABOVE THE UNDERSIDE OF EXISTING FOOTINGS UNTIL INSTRUCTION IS OBTAINED FROM THE ENGINEER ON PROCEDURES & PRECAUTIONS TO BE TAKEN.

F4

R3 R2

REINFORCEMENT SYMBOLS:

'N' - DENOTES GRADE 500 N DEFORMED BARS TO AS4671 GRADE N. 'R' - DENOTES GRADE 250 R HOT ROLLED PLAIN BARS TO AS4671.

**R13** 

N20 1000		N16 750	N12 475	BAR DIA SPLICE LEN	50
_	1000	7	274	TENSION SPLICE LENGTH	SPLICE SCHEDULE
	750	600	0 5 7	COMPRESSION SPLICE LENGTH	

R6

R7 TRUE PROJECTION.

WELDING OF REINFORCEMENT SHALL NOT BE PERMITTED UNLESS SHOWN ON THE STRUCTURAL DRAWINGS OR APPROVED BY THE ENGINEER.

MESH SHALL BE LAPPED 2 TRANSVERSE WIRES PLUS 50mm. BUNDLED BARS SHALL BE TIED TOGETHER AT 30 BAR DIAMETER CENTRES WITH 3 WRAPS OF THE

WIRE.
SLAB REINFORCEMENT SHALL EXTEND AT LEAST 65mm ONTO MASONRY SUPPORT SLAB REINFORCEMENT SHALL BE COGGED TO ACHIEVE WALLS AND 50% OF BOTTOM REINFORCEMENT SHALL BE COGGED TO ACHIEVE ANCHORAGE AT SIMPLY SUPPORTED ENDS. IF THIS CANNOT BE ACHIEVED DUE TO COVER REQUIREMENTS THEN ALL THE BARS SHALL BE COGGED. FOR MESH THE LAST WELDED CROSS ROD SHALL BE LOCATED OVER THE WALL AND 50mm MINIMUM BEYOND THE FACE OF THE WALL.

# REINFORCEMENT

R

ALL REINFORCING BARS SHALL BE GRADE D500N TO AS4671 UNLESS NOTED OTHERWISE. ALL MESH SHALL BE GRADE 500L TO AS4671 AND SHALL BE SUPPLIED IN FLAT SHEETS.

REINFORCEMENT NOTATION SHALL BE AS FOLLOWS IN THE FOLLOWING ORDER

NUMBER OF BARS IN GROUP NOMINAL BAR SIZE IN mm 17N20-250 SPACING IN BAR GRADE AND TYPE

'SL'or 'RL' - DENOTES WELDED GRADE 500 REINFORCING MESH TO AS 4671 'F' - DENOTES HARD-DRAWN WIRE REINFORCING MESH TO AS4671 'W' - DENOTES HARD-DRAWN PLAIN WIRE TO AS4672

SPLICES IN REINFORCEMENT SHALL BE MADE ONLY IN POSITIONS SHOWN OR OTHERWISE APPROVED IN WRITING BY THE ENGINEER. LAPS SHALL BE IN ACCORDANCE WITH AS 3600 AND NOT LESS THAN THE DEVELOPMENT LENGTH FOR

	SPLICE SCHEDULE	ULE
BAR DIA	TENSION SPLICE LENGTH	COMPRESSION SPLICE LENGTH
N 1 2	5.47	0.5.7
N 1 6	750	600
N 2 0	1000	750
N 2 4	1100	900
THEODOGNEN	CHICAGORIANT IS DEPOSED WITTED DIAGONATION IN AND NOT NECESSABILY IN	ALLY AND NOT NECESSABILY IN

# REINFORCEMENT Cont

WHERE TRANSVERSE TIE BARS ARE NOT SHOWN PROVIDE N12-400 SPLICED WHERE NECESSARY AND LAP WITH MAIN BARS 4.00MM UNLESS NOTED OTHERWISE. NO OPENINGS IN BEAMS OR COLUMNS SHALL BE MADE OTHER THAN THOSE SPECIFICALLY DETAILED. FOR OPENINGS IN SLABS UP TO 300mm SQUARE THE REINFORCEMENT SHALL BE DISPLACED TO THE SIDES. FOR OPENINGS BETWEEN 300mm SQUARE AND 600mm SQUARE THE REINFORCEMENT CROSSING THE PROPOSED OPENING SHALL BE CUT AND THE HOLES TRIMMED USING 2N12 BARS TOP AND BOTTOM EXTENDING 1500mm PAST EACH SIDE OF OPENING. OPENINGS LARGER THAN 600mm SQUARE SHALL BE DETAILED BY THE ENGINEER.

JOGGLES TO BARS SHALL COMPRISE A LENGTH OF 12 BAR DIAMETERS BETWEEN BEGINNING AND END OF AN OFFSET OF 1 BAR DIAMETER.

R12 ALL REINFORCEMENT SHALL BE FIRMLY SUPPORTED ON MILD STEEL PLASTIC TIPPED CHAIRS, PLASTIC CHAIRS OR CONCRETE CHAIRS AT NOT GREATER THAN 1 METRE CENTRES BOTH WAYS, AND 800 EACH WAY FOR MESH. WHEN POURED ON GROUND AS FORMWORK PROVIDE PLATES UNDER ALL BAR CHAIRS. PLASTIC TIPPED STEEL CHAIRS SHALL NOT BE USED ON EXPOSED FACES IN EXPOSURE CLASSIFICATION B1, B2 AND C ONLY PLASTIC OR CONCRETE CHAIRS.

SITE BENDING OF REINFORCEMENT SHALL BE AVOIDED IF POSSIBLE. WHERE SITE BENDING IS UNAVOIDABLE IT SHALL BE CARRIED OUT COLD, WITHOUT THE APPLICATION OF HEAT, AND IN ACCORDANCE WITH THE PRACTICE NOTE 'RPN1' OF THE STEEL REINFORCEMENT INSTITUTE OF AUSTRALIA USING MECHANICAL

the Environment Planning and Assessment Act 1979 the regulations referred to in Section 81A(5) of these plans and specifications will comply with I certify that work completed in accordance with

Craig Formosa BPB0124 DATED best of \$2.0 1707/14

# ne information contained on this drawing has been prepared for the exclusive use of the Client for this project. No ability or responsibility is accepted for use of this information by any third party or for any other project.

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a†:

PROPOSED ALTERATIONS & ADDITIONS t: 28 WOORARRA AVE, NTH NARRABEEN for: ROB PHIPPS TRUCTU

RAL NOTES-SHT 1 of 2 | 10885-S0.00 | DRAWN: J.C. | SCALE: N/A | FILENAME: 10885-S0.00 to \$3.00.DWG | SIGNED: | SIZE JUNE 201

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## CONCRETE

- 2 ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH AS 3600 CURRENT EDITION WITH AMENDMENTS. READYMIX CONCRETE SUPPLY SHALL COMPLY WITH AS 1379. ALL CEMENT TO BE TYPE "SL" PORTLAND.
- MAXIMUM DRYING SHRINKAGE SHALL BE 600 MICROSTRAIN AT 56 DAYS. PROJECT CONTROL TESTING SHALL BE CARRIED OUT IN ACCORDANCE WITH AS 1379. NO ADMIXTURES SHALL BE USED IN CONCRETE UNLESS APPROVED IN WRITING. CLEAR CONCRETE COVER TO ALL REINFORCEMENT SHALL BE AS FOLLOWS UNLESS S2
- SHOWN OTHERWISE.

SUSPENDED SLABS	SLABS ON GROUND	FOOTINGS	LOCATION
32	32	25	CONCRETE GRADE (MPa)
40	40	50	CAST AGAINST GROUND
40	40	50	CAST IN FORMS WITH EXPOSURE
25	25	30	CAST IN FORMS & NOT EXPOSED
			S

NOTE: WHERE CONCRETE IS POURED ON A VAPOURPROOF MEMBRANE 0.2 mm MINIMUM THICKNESS, THE COVER TO CONCRETE CAST AGAINST GROUND MAY BE REDUCED BY 10 mm.

45

- NO ADMIXTURES OTHER THAN LOW RANGE WRA SHALL BE USED IN CONCRETE UNLESS APPROVED IN WRITING.
- DEPTHS OF BEAMS ARE GIVEN FIRST AND INCLUDE SLAB THICKNESS.
- 9 9 CONCRETE SIZES SHOWN DO NOT INCLUDE THICKNESSES OF APPLIED FINISHES FINISHES. NO FINISH WHICH DECREASES COVER IS ALLOWED WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER.
- 7 FOR CHAMFERS, DRIP GROOVES, REGLETS, ETC REFER TO MAINTAIN COVER TO REINFORCEMENT AT THESE DETAILS. ARCHITECT'S DETAILS,
- 8 PRIOR WRITTEN APPROVAL OF THE ENGINEER. NO HOLES, CHASES OR EMBEDMENT OF PIPES OTHER THAN THOSE SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE MADE IN CONCRETE MEMBERS WITHOUT THE 55
- 9 CONSTRUCTION JOINTS AND CLOSING STRIPS SHALL BE USED TO CONTROL AND REDUCE SHRINKAGE CRACKING IN WALLS AND FLOORS, AND COLD JOINTS IN LARGE POURS. THESE JOINTS SHALL BE PLANNED IN ADVANCE, TO THE APPROVAL OF THE ENGINEER.
- 010 THE FINISHED CONCRETE SHALL BE A DENSE HOMOGENEOUS MASS, COMPLETELY FILLING THE FORMWORK THOROUGHLY EMBEDDING THE REINFORCEMENT AND FREE OF STONE POCKETS. ALL CONCRETE INCLUDING SLABS ON GROUND AND FOOTINGS SHALL BE COMPACTED WITH MECHANICAL VIBRATORS. 86
- 3 CURING OF ALL CONCRETE IS TO BE ACHIEVED BY KEEPING SURFACES
  CONTINUOUSLY WET FOR A PERIOD OF 3 DAYS, AND PREVENTION OF LOSS OF
  MOISTURE FOR A TOTAL OF 7 DAYS FOLLOWED BY A GRADUAL DRYING OUT.
  APPROVED SPRAYED ON CURING COMPOUNDS COMPLYING WITH AS 3799 MAY BE
  USED WHERE NO FLOOR FINISHES ARE PROPOSED. POLYTHENE SHEETING OR WET
  HESSIAN MAY BE USED IF PROTECTED FROM WIND AND TRAFFIC. 88 S7
- CONDUITS, PIPES, ETC, SHALL ONLY BE LOCATED IN THE MIDDLE ONE THIRD OF SLAB DEPTH AND SPACED AT NOT LESS THAN 3 DIAMETERS AND SHALL NOT BE PLACED WITHIN THE REINFORCEMENT COVER 9
- C13 REPAIRS TO CONCRETE SHALL NOT BE ATTEMPTED WITHOUT THE PERMISSION OF THE ENGINEER.

# STRUCTURAL STEEL ST ALL WORKMANSHIP AN

- ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH AS 4100 AND AS 1554 EXCEPT WHERE VARIED BY THE CONTRACT
- UNLESS NOTED OTHERWISE ALL MATERIAL SHALL BE:
   GRADE 250 HOT-ROLLED PLATES COMPLYING WITH AS 3678;
   GRADE 250 HOT-ROLLED FLATS, TFC, TFB, ANGLES 100x100EA
  OR 125x75UA AND SMALLER COMPLYING WITH AS 3679.1;
   GRADE 300PLUS UB, UC, PFC AND ANGLES 125x125EA OR 150x90UA
- GRADE C350 RHS, CHS COMPLYING WITH AS 1163; 300 WB, WC COMPLYING WITH AS 3679.2;
- THREE(3) COPIES OF WORKSHOP MESHATION DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW AT LEAST 7 DAYS PRIOR TO COMMENCEMENT OF MESHATION AND PERMISSION TO USE OBTAINED PRIOR TO MESHATION. PERMISSION TO USE DOES NOT RELIEVE THE BUILDER OF THE FULL RESPONSIBILITY FOR DIMENSIONS, FIT AND COMPLIANCE WITH ARCHITECTURAL AND ENGINEERING DRAWINGS.
- 4.6/S.....COMMERCIAL BOLTS OF GRADE 4.6 TO AS 1111, SNUG TIGHTENED 8.8/S.....HIGH STRENGTH STRUCTURAL BOLTS OF GRADE 8.8 TO AS 1252. BOLTS:-SNUG TIGHTENED.
- 8.8/TB....HIGH STRENGTH STRUCTURAL BOLTS OF GRADE 8.8 TO AS 1252 FULLY TENSIONED TO AS 4100 AS BEARING JOINT 8.8/TF....HIGH STRENGTH STRUCTURAL BOLTS OF GRADE 8.8 TO AS 1252 FULLY TENSIONED TO AS 4100 AS A FRICTION JOINT WITH FACING SURFACES LEFT UNCOATED
- WELDING SHALL BE CARRIED OUT IN ACCORDANCE WITH AS 1554.1.
  WELDING CONSUMABLES SHALL BE E48XX OR W50X U.N.O. ALL WELD
  SHALL BE 6 mm CFW SP CATEGORY U.N.O. CPBW SHALL BE SP
  CATEGORY U.N.O. INSPECTION SHALL BE CARRIED OUT TO AS 1554.1. ALL
  GP/SP WELDS SHALL BE 100% VISUALLY SCANNED. SP WELDS ALLOW
  FOR 10% VISUAL EXAMINATION U.N.O. BUTT WELDS SHALL BE COMPLETE ALL BOLTS SHALL BE M20 GRADE 8.8/S UNLESS NOTED.

  NO CONNECTION SHALL HAVE LESS THAN 2 BOLTS.
  ALL BOLTS, NUTS & WASHERS TO BE GALVANISED.

  TB AND TF BOLTS TO BE INSTALLED USING APPROVED LOAD INDICATING WASHERS, OR BY TURN OF NUT CONTROL OF TENSIONING.
- ALL DETAILS, GAUGE LINES ETC. WHERE NOT SPECIFICALLY SHOWN ARE TO BE IN ACCORDANCE WITH AISC DESIGN CAPACITY TABLES FOR STRUCTURAL STEEL AND AISC STANDARDIZED STRUCTURAL CONNECTIONS. PLATES TO BE 10mm THICK, EX-STANDARD SQUARE EDGE

PENETRATION WELDS TO AS 1554.

SECURELY TEMPORARILY BRACED AS NECESSARY TO STABILISE THE PROVIDE SEAL PLATES TO ALL HOLLOW SECTIONS. PROVIDE VENT HOLES TO HOLLOW MEMBERS & DRAIN HOLES TO ALL MEMBERS TO BE HOT DIP GALVANISED.

BL10

STRUCTURAL STEELWORK SHALL HAVE THE FOLLOWING SURFACE TREATMENT IN ACCORDANCE WITH THE SPECIFICATION. STRUCTURE DURING ERECTION.

EXTERNAL (ALT)	EXTERNAL	INTERNAL	ELEMENT
PICKLING	ABRASIVE GRIT BLASTING (CLASS 2.5) or PICKLING	POWER WIRE BRUSHING or ABRASIVE GRIT BLASTING	SURFACE CLEANING
HOT DIP GALVANISED	1 COAT INORGANIC ZINC SILICATE PRIMER OR EQUIV. + 1 TOP COAT ALL WEATHER GLOSS ACRYLIC WITH UV PROTECTOR	1 COATRUST INHIBITIVE ALKYD PRIMER OR EQUIV. + 1 TOP COAT ALL WEATHER GLOSS ACRYLIC	PROTECTIVE COATING

# THE BUILDER SHALL PROVIDE ALL CLEATS AND DRILL ALL HOLES NECESSARY FOR FIXING STEEL TO STEEL AND TIMBER TO STEEL WHETHER OR NOT DETAILED ON THE DRAWINGS. THE MESHATIOIN AND ERECTION OF THE STRUCTURAL STEELWORK SHALL SUPERVISED BY A QUALIFIED PERSON EXPERIENCED IN SUCH SUPERVISION, ENSURING ALL REQUIREMENTS OF THE DESIGN ARE MET. ALL BEAMS AND RAFTERS SHALL BE MESHATED AND ERECTED WITH NATURAL CAMBER UP. ALL MEMBERS SHALL BE SUPPLIED IN SINGLE LENGTHS. SPLICES SHALL ONLY BE PERMITTED IN LOCATIONS SHOWN ON THE STRUCTURAL DRAWINGS.

S11

S10

## BLOCKWORK

- STRENGTHS OF MASONRY UNITS AND TYPE OF MORTAR SHALL BE AS ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH AS3701
- CHARACTERISTIC UNCONFINED COMPRESSIVE STRENGTH f'uc = MORTAR (CEMENT: LIME: SAND) = 1:0.25:3 5
- ONLY LOAD BEARING MASONRY WALLS ARE SHOWN UNDER CONCRETE SLABS APPROVAL OF THE SUPERINTENDENT MORTAR ADMIXTURES SHALL NOT BE USED WITHOUT THE WRITTEN
- ALL VOIDS, TWO LAYERS OF MALTHOID SHALL BE PLACED FULL WIDTH ACROSS SUCH LOAD BEARING SURFACES EXCEPT WHERE PROPRIETARY BEARING STRIP IS NOTED OR ALTERNATIVE DETAIL IS DOCUMENTED. THE HEADS OF LOAD BEARING WALLS SHALL NOT EXTEND ABOVE THE SOFFIT OF OTHER THAN REINFORCED CONCRETE BLOCKWORK, MASONRY SUPPORTING SLABS AND BEAMS SHALL BE TROWELLED SMOOTH WITH MORTAR FILLING THE CONCRETE SLAB ABOVE.

BK4

ALL MASONRY SUPPORTING OR SUPPORTED BY CONCRETE FLOORS SHALL BIPROVIDED WITH VERTICAL JOINTS TO MATCH ANY CONTROL JOINTS IN THE CONCRETE

BL5

- NO CHASES OR RECESSES ARE PERMITTED IN LOAD BEARING MASONRY WITHOUT THE APPROVAL OF THE ENGINEER.
- PROVIDE VERTICAL CONTROL JOINTS AT 10 m MAX. CENTRES GENERALLY, AND 5 m MAX. FROM CORNERS FOR BRICKWORK AND UNREINFORCED BLOCKWORK.

Bk6

REINFORCED CONCRETE BLOCKWORK SHALL COMPLY WITH THE FOLLOWING, UNLESS NOTED: REFER TO CONCRETE NOTES FOR DE-PROPPING PRIOR TO CONSTRUCTION OF MASONRY WALLS ON SUSPENDED SLABS.

Bk7

BL9 BL8 BL7 BL6

- \* PROVIDE CLEANOUT HOLES 100 mm SQUARE MINIMUM AT BASE OF ALL WALLS AND ROD CORE HOLES TO REMOVE PROTRUDING MORTAR FINS PRIOR
- \* CORE FILLING GROUT SHALL BE:- f'c = 20 MPa MINIMUM CEMENT CONTENT = 300 kg/m, SLUMP = 230 ± 30 mm. TO GROUTING.
- \* REINFORCEMENT PROJECTING FROM FOUNDATION OR SLABS INTO CORES, SHALL BE SET ACCURATELY IN PLACE USING TEMPLATES TO ALIGN WITH THE CENTRE OF THE LENGTH OF CORES AND WITH COVER AS NOTED. WHERE HORIZONTAL BARS ARE INDICATED, THE WEBS OF THE BLOCKS BELOW THE BARS SHALL BE CUT DOWN TO ACCOMMODATE THE BARS.

  \* GROUT ALL CORES IN REINFORCED BLOCKWORK UNLESS OTHERWISE NOTED. NOTED. HEIGHT OF BLOCKWORK TO BE GROUTED ON ONE DAY SHALL BE 2400mm. GROUT SHALL BE PLACED IN LIFTS OF 1200mm MAXIMUM AND COMPACTED BY POKER VIBRATOR. A SHORT TIME SHOULD ELAPSE BETWEEN SUCCESSIVE LIFTS TO ALLOW PLASTIC SETTLEMENT TO OCCUR. \* PROVIDE 50 mm COVER FROM THE OUTSIDE OF THE BLOCKWORK UNLESS
- BACKFILL TO RETAINING WALLS SHALL BE FREE DRAINING GRANULAR MATERIAL. PROVIDE SUBSOIL DRAIN AT BASE OF WALL. DO NOT BACKFILL UNTIL 14 DAYS AFTER GROUTING, OR IF APPLICABLE, AFTER RESTRAINING SLAB OVER HAS BEEN POURED AND CURED FOR 7 DAYS. BACKFILL SHALL BE COMPACTED TO 98% STANDARD MAXIMUM DRY DENSITY AT OPTIMUM MOISTURE CONTENT ± 2 %.

# BRICKWORK

- Bk1 ALL MATERIALS AND WORKMANSHIP TO BE TO AS 3700
- SLABS. ONLY LOAD BEARING MASONRY WALLS ARE SHOWN UNDER CONCRETE
- MINIMUM CLAY BRICK COMPRESSIVE STRENGTH TO BE 20MPa. RATE OF ABSORPTION TO BE LESS THAN 1.5KG/M2/MIN AT THE TIME OF LAYING. CLAY BRICKS SHALL BE AT LEAST 30 DAYS OUT OF THE KILN AND WILL OFTEN REQUIRE PRE-WETTING UNLESS PROOF OF A MOISTURE EXPANSION LESS THAN 0.6MM/M IS PRODUCED. UNLESS NOTED OTHERWISE MORTAR FOR CLAY BRICKWORK IS TO BE CEMENT. LIME: SAND IN THE RATIO OF 1:1:6 AND THE WATER RETENTIVITY MUST BE AT LEAST 90%. NO ADDITIVES SHALL BE USED UNLESS APPROVED IN WRITING. BRICKWORK IS TO BE ADEQUATELY CURED PRIOR TO CONSTRUCTION OF SUSPENDED SLABS OVER.
- JOINTS 20MM WIDE AT MAXIMUM SPACING OF 10M (15M IN INDUSTRIAL USE) AND ARE TO CONTAIN 40MM TAR IMPREGNATED POLYURETHANE STRIP. WHERE INTERNAL SKIN IS INTERRUPTED BY STEEL FRAMES THE ABOVE UNLESS NOTED OTHERWISE CLAY BRICKWORK IS TO CONTAIN MOVEMENT JOINTING APPLIES TO EXTERNAL SKIN ONLY.
- ALL MASONRY SUPPORTING OR SUPPORTED BY CONCRETE FLOORS SHALL BE PROVIDED WITH VERTICAL JOINTS TO MATCH ANY CONTROL JOINTS IN THE CONCRETE.

BK5

- NON LOAD BEARING WALLS BUILT PRIOR TO POURING CONCRETE SHALL BE SEPARATED FROM CONCRETE ABOVE BY 16 mm THICK CLOSED CELL POLYSTYRENE STRIP. WHERE BUILT AFTER CONCRETE IS POURED LEAVE 12mm CLEAR OF CONCRETE SOFFIT.
- BRICKWORK SUPPORTING SLABS AND BEAMS SHALL BE TROWELLED SMOOTH WITH MORTAR FILLING ALL VOIDS. TWO LAYERS OF MALTHOID SHALL BE PLACED FULL WIDTH ACROSS SUCH LOAD BEARING SURFACES EXCEPT WHERE PROPRIETARY BEARING STRIP IS NOTED OR ALTERNATIVE DETAIL IS DOCUMENTED. THE HEADS OF LOAD BEARING WALLS SHALL NOT EXTEND ABOVE THE SOFFIT OF THE CONCRETE SLAB ABOVE.
- ALL DOUBLE SKIN SOLID WALLS SUCH AS 230mm THICK BRICKWORK SHALL BE BONDED BY A HEADER COURSE EVERY 4th COURSE.

#### TIMBER

BK8

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- ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH AS1684 AND AS1720.1.
- T2 ALL BOLTS, NUTS AND WASHERS FOR TIMBER CONNECTIONS TO BE HOT-DIP GALVANISED & GRADE 4.6. WHERE POSSIBLE, BOLTS SHALL BE RETIGHTENED AT THE END OF THE MAINTAINANCE PERIOD. BOLT HOLES SHALL BE DRILLED NO MORE THAN 1 mm OVERSIZE. WASHERS UNDER ALL HEADS AND NUTS SHALL BE AT LEAST 2.5 x BOLT DIA. TIMBER TO BE SEASONED & MINIMUM GRADE F7 UNLESS NOTED OTHERWISE
- 15 MINIMUM TIMBER CONNECTIONS TO BE NOMINAL FIXINGS IN ACCORDANCE WITH AS 1684 UNLESS NOTED OTHERWISE. DISTANCE FOR BOLTED CONNECTIONS TO BE 5xBOLT DIAMETER. DISTANCES FOR BOLTED CONNECTIONS TO BE 4×BOLT DIAMETER. MIN END MINIMUM BOLT SPACINGS IN TIMBER TO BE 5×BOLT DIAMETER. MIN EDGE
- 91 TIE-DOWN SHALL BE IN ACCORDANCE WITH AS1684.2 SECTION 9 UNLESS NOTED OTHERWISE.
- 17 ALL TIMBER JOINTS AND NOTCHES ARE TO BE 100mm MINIMUM AWAY FROM LOOSE KNOTS, SEVERE SLOPING GRAIN, GUM VEINS OR OTHER MINOR DEFECTS
- ALL TIMBER TO BE EITHER PLANTATION TIMBERS, TIMBER PRODUCTS TIMBERS MANUFACTURED FROM SUSTAINABLY MANAGED FORESTS OR RECYCLED

19

81

EXTERNAL TIMBER SHALL BE EITHER HARDWOOD DURABILITY CLASS I OR II TO AS 1720.2 OR IMPREGNATED PINE GRADE F7, PRESSURE TREATED TO AS 1604 AND RE-DRIED PRIOR TO USE. SUPPLEMENTARY TREATMENT SHALL BE APPLIED TO ALL CUT SURFACES. SUPPLY SUPPORTING DOCUMENTATION FOR PRESERVATIVE TREATMENT.

THIS PLAN/DOCUMENT FORMS

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DRAWN: J.C. | SCALE: FILENAME: 10885-\$0.00 to \$3.00. SIGNED: |

JUNE 20

47 Towradgi Street, NSW 2099, Australia SALLY GARDNER DESIGN & DRAFT 

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PROPOSED ALTERATIONS & ADDITIONS at: 28 WOORARRA AVE, NTH NARRABEEN for: ROB PHIPPS

RAL 10885-S0.01

NOTES-SHT 2 of 2

