



The Crest, Lot 100 Meatworks Avenue, Oxford Falls **Proposed Industrial Development (Stage 2) Assessment of Traffic and Parking Implications**

19179 Ref:

October 2019 Date:

Issue:

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1.0 Introduction

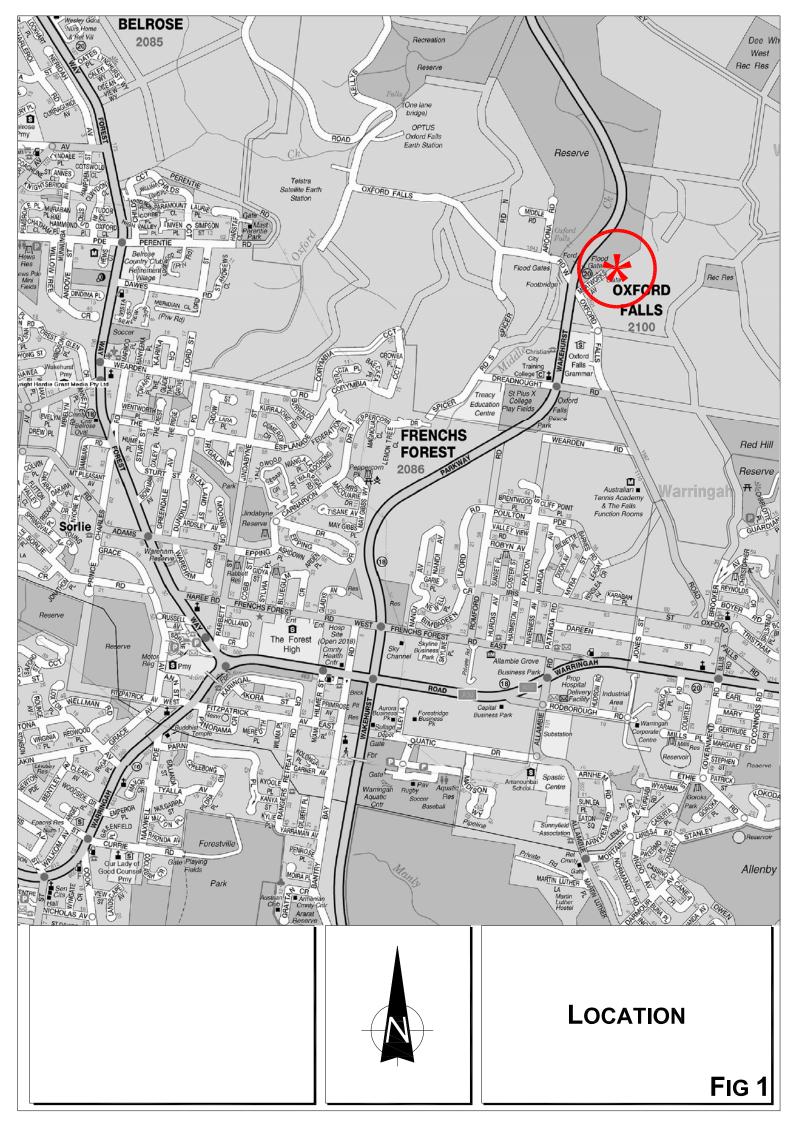
This report has been prepared to accompany a Development Application to Northern Beaches Council for the proposed industrial development (Stage 2) of the Industrial Estate on a site at Lot 100, Meatworks Avenue, Oxford Falls (Figure 1). The site is located to the south of the Stage 1 development which is currently under construction.

The proposed development scheme involves the construction of:

- 13 industrial units with a total of some 2,318m²
- a commercial/industrial building of some 1,433m²
- an ancillary building of some 398m²
- vehicle access through the Stage 1 area.

The purpose of this report is to:

- * describe the site, its context and the proposed development scheme
- * describe the road network serving the site and the prevailing traffic conditions
- * assess the potential traffic implications
- * assess the adequacy of the proposed parking provision
- * assess the suitability of the proposed vehicle access, internal circulation and servicing arrangements



2.0 Proposed Development

2.1 Site, Context and Existing Use

The development site (Figure 2) is a consolidation of Lot 100 - DP 102318, Part Lot 1046 - DP 752038, Part Lot 1047 - DP 752038 Part Lot 1053 DP 752038, which occupies an irregular shaped area of 18,525.73m².

The site is located at the north eastern end of Meatworks Avenue, which comprises a cul-de-sac of 150 metres in length and intersects with Oxford Falls Road. The site is bounded on all sides by wooded bushlands. The Oxford Falls Grammar School is located 400m to the southwest of the site on Oxford Falls Road while the C3 Church is located 700m to the southwest of the site on the western side of the Wakehurst Parkway.

The Stage 1 development on the site comprises:

- * 7 new buildings accommodating a total of 30 small industrial units with each unit will include a loading door and external standing area for service vehicles.
- ★ Vehicle access via a circulation roadway
- * parking will be located throughout the site in close proximity to the related unit.

Details of the approved Stage 1 development are provided on the plans prepared by Gelder Architects and are reproduced in part in Appendix A.

2.2 Proposed Development Scheme

It is proposed to demolish the existing structures and construct 3 new buildings comprising:

Commercial/Industrial Building 1422.86m²
Ancillary Building 364.71m²

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13 industrial units (total of 2,317.93m²) with the following breakdown:

- Unit 30: 152.38m²

- Unit 31: 151.43m²

- Unit 32: 151.43m²

- Unit 33: 151.43m²

- Unit 34: 152.13m²

- Unit 35: 120.58m²

- Unit 36: 120.13m²

- Unit 37: 120.13m²

- Unit 38: 120.13m²

- Unit 39: 120.82m²

- Unit 40 397.78m²

- Unit 41 278.69m²

- Unit 42 280.87m²

The overall floor area of the Stage 2 development equates to 4,105.50m²

Parking will be provided on 3 levels comprising:

Ground 8 on-grade spaces + 37 covered spaces

Ground (Mezzanine) 48 spaces (including 2 accessible spaces)

Level 1 (Mezzanine) 18 spaces

Total: 111 spaces (including 2 accessible spaces)

It is proposed to provide 2 combined ingress/egress driveways on the proposed access roadway to be constructed as part of Stage 1 development.

Details of the proposed Stage 2 development are provided on the plans prepared by Gelder Architects which accompany the application and are reproduced in part in Appendix B.

3.0 Existing Road Network and Traffic Conditions

3.1 Road Network

The road network serving the site (Figure 3) comprises:

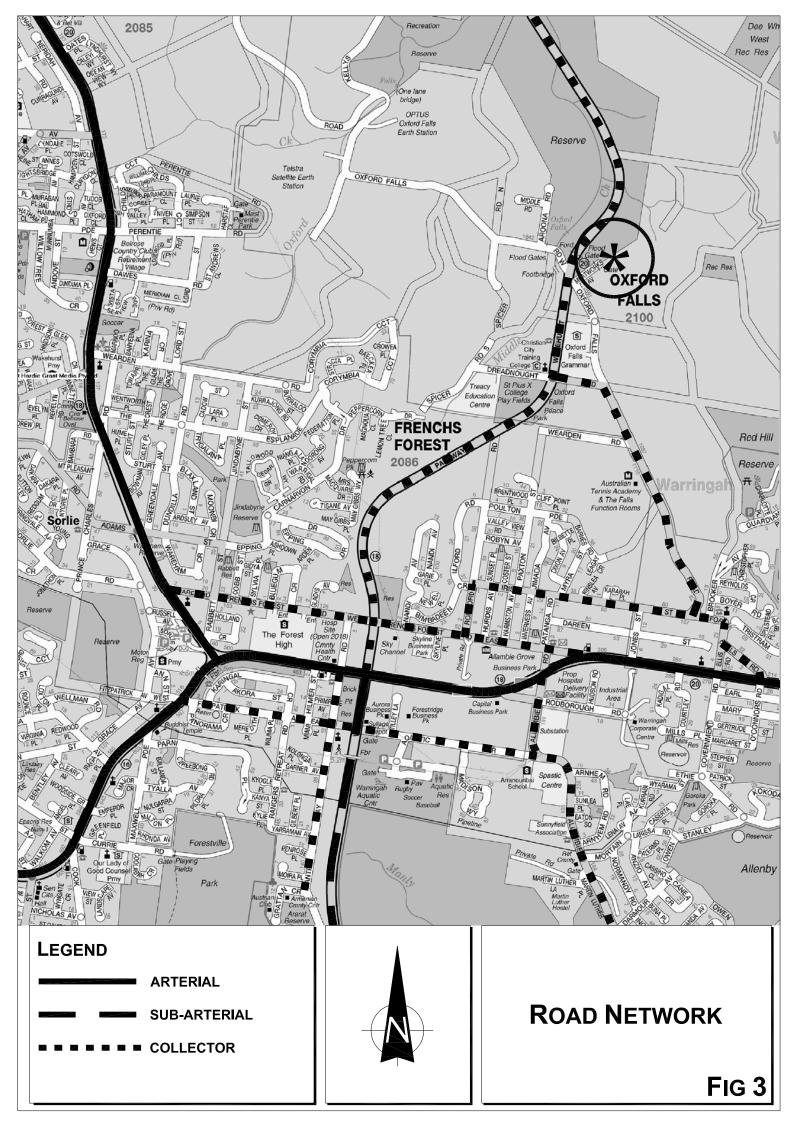
- ❖ Wakehurst Parkway a 2-lane carriageway State Road and arterial/sub-arterial route connecting between Pittwater Road at Narrabeen and Sydney Road at Balgowlah. Wakehurst Parkway provides an alternative to Pittwater Road for motorists travelling between the northwest (e.g, Chatswood) and the peninsular.
- Warringah Road a State Road and arterial route connecting between Pittwater Road at Brookvale and across Roseville Bridge to Eastern Valley Way and Pacific Highway via Boundary Road
- Pittwater Road a State Road and sub-arterial route running along the northern beaches peninsular connecting between North Sydney (as Military Road) and Palm Beach
- Oxford Falls Road a local road and provides a connection between Beacon Hill and Oxford Falls, intersecting Wakehurst Parkway via Dreadnaught Road and a traffic signal-controlled intersection.
- ❖ Meatworks Avenue a local cul-de-sac, having a length of 150 metres, which connects between the northern end of Oxford Falls Road and the site. Meatworks Avenue currently provides access to the site only.

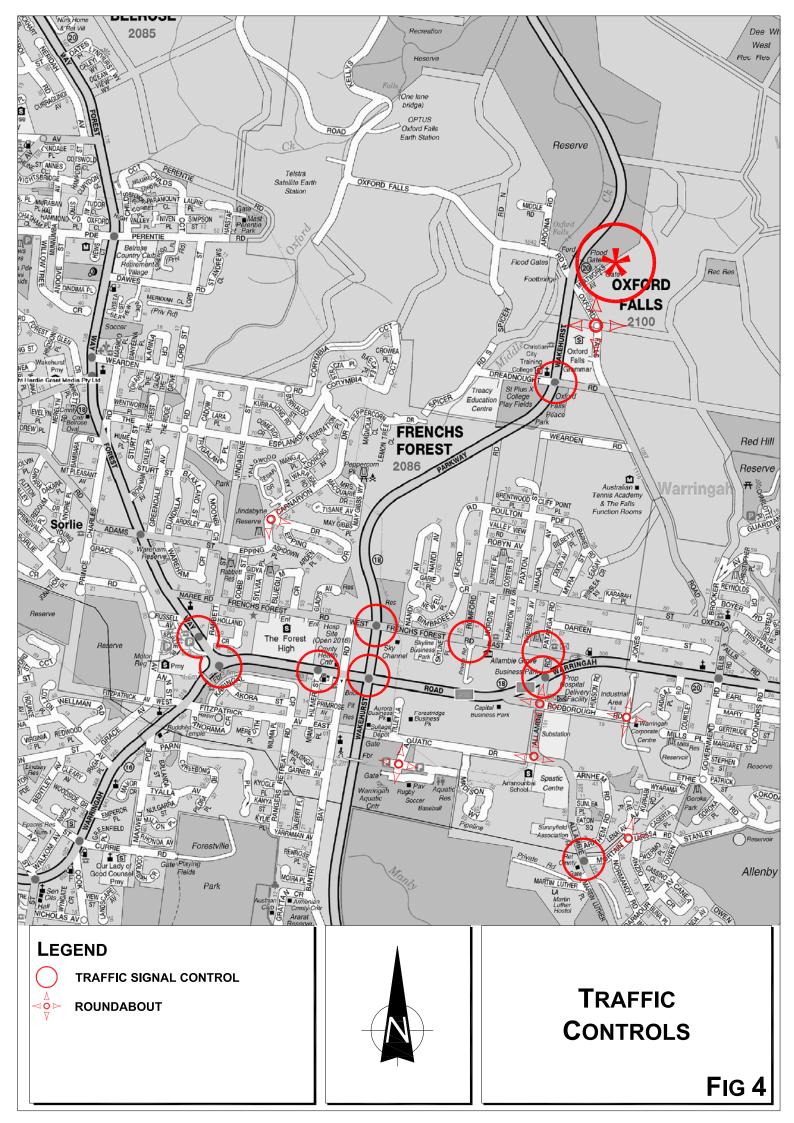
3.2 Traffic Controls

The existing traffic controls, which have been applied to the road system serving the site (Figure 4) comprise:

- the traffic signals at the intersection of Wakehurst Parkway/Dreadnought Road
- the roundabout at Oxford Falls Road/Oxford Falls Grammar School access

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- give-way intersections of:
 - Oxford Falls Road/Dreadnought Road
 - Oxford Falls Road/Iris Street
 - Spicer Road/Dreadnought Road
- the 80 kmph speed restriction on Wakehurst Parkway and 50 kmph speed restriction on the local and collector roads
- the 40 kmph school zone on Dreadnought Road and Oxford Falls Road

3.3 Traffic Conditions

An indication of traffic conditions on the road system serving the area is provided by surveys undertaken as part of the study. Traffic surveys have been undertaken at the Wakehurst Parkway/Dreadnought Road intersection during the AM and PM peak periods on Tuesday, 27 August 2019.

The operational performance of intersection of Wakehurst Parkway/Dreadnought Road has been assessed using SIDRA and the results indicating satisfactory performances are provided in Appendix C and summarised in the following, while the criteria for interpreting the results are reproduced overleaf:

AM	Peak	PM F	Peak
LOS	AVD	LOS	AVD
С	37.7s	D	54.4s

The results of the SIDRA assessments indicate that the intersection operates at acceptable LOS D or better during the AM and PM peak periods.

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Criteria for Interpreting Results of SIDRA Analysis

1. Level of Service (LOS)

LOS	Traffic Signals and Roundabouts	Give Way and Stop Signs
'A'	Good	Good
'B'	Good with acceptable delays and spare capacity	Acceptable delays and spare capacity
'C'	Satisfactory	Satisfactory but accident study required
'D'	Operating near capacity	Near capacity and Accident Study required
'E'	At capacity; at signals incidents will cause excessive delays. Roundabouts require other control mode	At capacity and requires other control mode
'F'	Unsatisfactory and requires additional capacity	Unsatisfactory and requires other control mode

2. Average Vehicle Delay (AVD)

The AVD provides a measure of the operational performance of an intersection as indicated on the table below, which relates AVD to LOS. The AVD's listed in the table should be taken as a guide only as longer delays could be tolerated in some locations (ie inner city conditions) and on some roads (ie minor side street intersecting with a major arterial route).

Level of Service	Average Delay per Vehicle (secs/veh)	Traffic Signals, Roundabouts	Give Way and Stop Signs
А	Less than 14	Good operation	Good operation
В	15 to 28	Good with acceptable delays and spare capacity	Acceptable delays and spare capacity
С	29 to 42	Satisfactory	Satisfactory but accident study required
D	43 to 56	Operating near capacity	Near capacity and accident study required
E	57 to 70	At capacity; at signals incidents will cause excessive delays. Roundabouts require other control mode	At capacity and requires other control mode

3. Degree of Saturation (DS)

The DS is another measure of the operational performance of individual intersections.

For intersections controlled by **traffic signals**¹ both queue length and delay increase rapidly as DS approaches 1, and it is usual to attempt to keep DS to less than 0.9. Values of DS in the order of 0.7 generally represent satisfactory intersection operation. When DS exceeds 0.9 queues can be anticipated.

For intersections controlled by a **roundabout or GIVE WAY or STOP signs**, satisfactory intersection operation is indicated by a DS of 0.8 or less.

the values of DS for intersections under traffic signal control are only valid for cycle length of 120 secs

4.0 Parking

Council's (Warringah) DCP specifies the following parking criteria in relation to the proposed development elements:

Warehouse/Industry 1.3 spaces per 100 m² GFA (including up to 20% of floor

area as office space component. Office space component

above 20% determined at office rate)

Office 1 space per 40 m² GFA.

While both the commercial/industrial and ancillary buildings are more likely serve as a business park, to provide a conservative assessment, it has been assumed that all the floor areas in these buildings will be used as offices.

Application of these criteria to the proposed development would indicate the following:

Industrial units 13 units (2,318m²) 31 spaces

Commercial/industrial 1,423m² 36 spaces

Ancillary building 365m² 10 spaces

Total: 77 spaces

It is proposed to provide a total of 111 parking spaces in compliance with the Council's DCP criteria. Two spaces designated for the proposed development will be suitable for disabled drivers in accordance with BCA.

It is noted that the proposed parking provision is higher than required a strict application of the DCP minimum requirements. This has been achieved partly through the availability of land, but also in response to the lack of alternative forms of transport serving the site, and the somewhat isolated location in relation to residential or commercial uses. The proposed provision will ensure that the proposed development does not result in overflow parking on to the surrounding road network.

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5.0 Traffic

An indication of the potential traffic generation of the proposed development is provided by the RMS Development Guidelines and RMS TDT 2013/04, which specify the following peak hour generation rates:

Warehouse¹

AM & PM Peaks: 0.5 vtph per 100m² GFA

Business Park and Industrial Estate²

AM Peak: 0.15 – 1.31 vtph per 100m² GFA PM Peak: 0.16 – 1.50 vtph per 100m² GFA

Given that lack of public transport services in the vicinity of the site, the highest rates of 1.31 and 1.50 vtph per 100m² GFA have been used in this assessment.

Based on the above rates, the proposed development will result in the following AM and PM peak hour traffic generations:

		Industrial units	Commercial/Industrial	Ancillary	
			Building	Building	
	GFA	2,318m ²	1,423m²	365m ²	Total
-	AM Peak	12 vtph	19 vtph	5 vtph	36 vtph
	PM Peak	12 vtph	22 vtph	6 vtph	40 vtph

Based on the traffic and parking assessment report provided by Parking & Traffic

¹ Roads and Maritime Services, Guide to Traffic Generating Developments. Version 2.2 October 2002

² RMS Technical Direction TDT 2013/04[,] Guide to Traffic Generating Developments - Updated traffic surveys

Consultants³, the Stage 1 development is anticipated to generate a peak hour traffic activity of 68 vehicle movements.

The projected vehicle generation of Stage 1 and Stage 2 developments during the peak traffic periods will therefore be:

Stage	AM		AM PM	
	IN	OUT	IN	OUT
1	28	8	32	8
2	54	14	14	54
Total	82	22	46	62

A SIDRA assessment of the intersection of Wakehurst Parkway/Dreadnought Road with the projected additional volumes reveals that acceptable operations will continue as follows:

AM	Peak	PM F	Peak
LOS	AVD	LOS	AVD
С	42.2s	D	56.4s

Overall, the intersection would continue to operate at the same levels of service with only minor increases to average delays predicted when compared with existing conditions with a minor increase of delay up to 4.5 seconds per vehicle.

The traffic generation of this order of magnitude being equivalent to some 2 vehicles every minute during the peak hours is minor in the context of the local and arterial road system and will not act to create unacceptable traffic congestion or conflict either at the vehicle access point or at the adjacent intersection.

³ Proposed Industrial Estate, Meatworks Avenue, Oxford Falls, T2-507, August 2011 by Parking & Traffic Consultants

6.0 Access, Internal Circulation and Servicing

6.1 Access

The proposed vehicle access arrangements to the Stage 2 development will comprise 2 combined ingress/egress driveways and 5 individual industrial unit accesses on the internal access road (to be constructed as part of the Stage 1 development) frontage located towards the western site boundary.

The proposed vehicle access arrangements include:

- Western driveway: A 13.5m wide, combined ingress/egress driveway to/from Level 1 Mezzanine to allow truck access (Industrial Units 35 through 42) for up to 6.4m small rigid vehicle (SRV) and carpark access for up to B99 cars. The western driveway also provides access for B99 cars to/from commercial/industrial building carpark.
- ❖ Eastern driveway: A 9.4m wide, combined ingress/egress driveway to/from Ground and Ground (Mezzanine) levels to allow carpark access for up to B99 cars. The access to/from the Ground Level will be provided off the driveway to/from Ground (Mezzanine) level.
- 5 truck accesses to/from Industrial Units 30 through 34.

These driveways comply with the design requirements of AS2890.1 & 2 and will accommodate the vehicle access needs of the relevant elements of the development. The driveway will be located on a straight on a higher reduced level section of the internal access road, allowing good sight distances and visibility for the existing vehicle to the vehicles along the access road.

The proposed ingress and egress manoeuvring arrangements at these driveways will are satisfactory as confirmed by the turning path assessment for an SRV at the western driveway and a B99 car for the eastern driveway which is provided in Appendix D.

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6.2 Internal Circulation

The carpark arrangement internal circulation and parking arrangements have been designed in accordance with the requirements of AS2890.1 of a Class 1A facility. A Class 1A facility requires parking space dimensions of 2.4 x 5.4 metres with an access aisle width of 5.8 metres. The proposed carpark has been designed to provide compliant parking space widths of 2.45 x 5.5 metres and aisle widths of 7.2 metres.

The design of the internal arrangements including truck manoeuvring, carpark aisles/bays, etc. have been designed in accordance with AS2890.1, AS2890.2 and AS2890.6. Details of the turning path assessment for the internal circulation arrangements indicating satisfactory provision for all accessing vehicles are provided in Appendix D.

6.3 Servicing

Each of the industrial units will have a dedicated loading area which can accommodate up to an SRV.

The servicing arrangement for Industrial Units 35 through 39, a commercial/industrial and ancillary buildings will be completed on-street along the internal access road southern frontage. The vehicle will stop temporarily on the access road with the waste collection personnel wheeling the bins out onto the road for collection. The internal road width can adequately provide for the standing of refuse vehicle and there will be no reversing of the waste vehicle on the access road. In summary, the on-road refuse collection activities will have a very minor impact on vehicle circulation.

All loading activities related to deliveries, courier activity, maintenance, etc. can be reliant on the available on-street and off-street parking within the site, as is normal for industrial development of this nature.

Details of the turning path assessment for service vehicles are provided in Appendix D indicating that the proposed arrangements will be satisfactory.

There will only be a relatively minor level of servicing required for the development (i.e., apart from receipt and dispatch movements). The refuse removal will be undertaken by a 6.4m private contractor's waste vehicle, which will take place outside of the development's peak periods.

It is assessed on this basis that the proposed servicing arrangements will be adequate for the needs of the envisaged operations at the site.

7.0 Conclusion

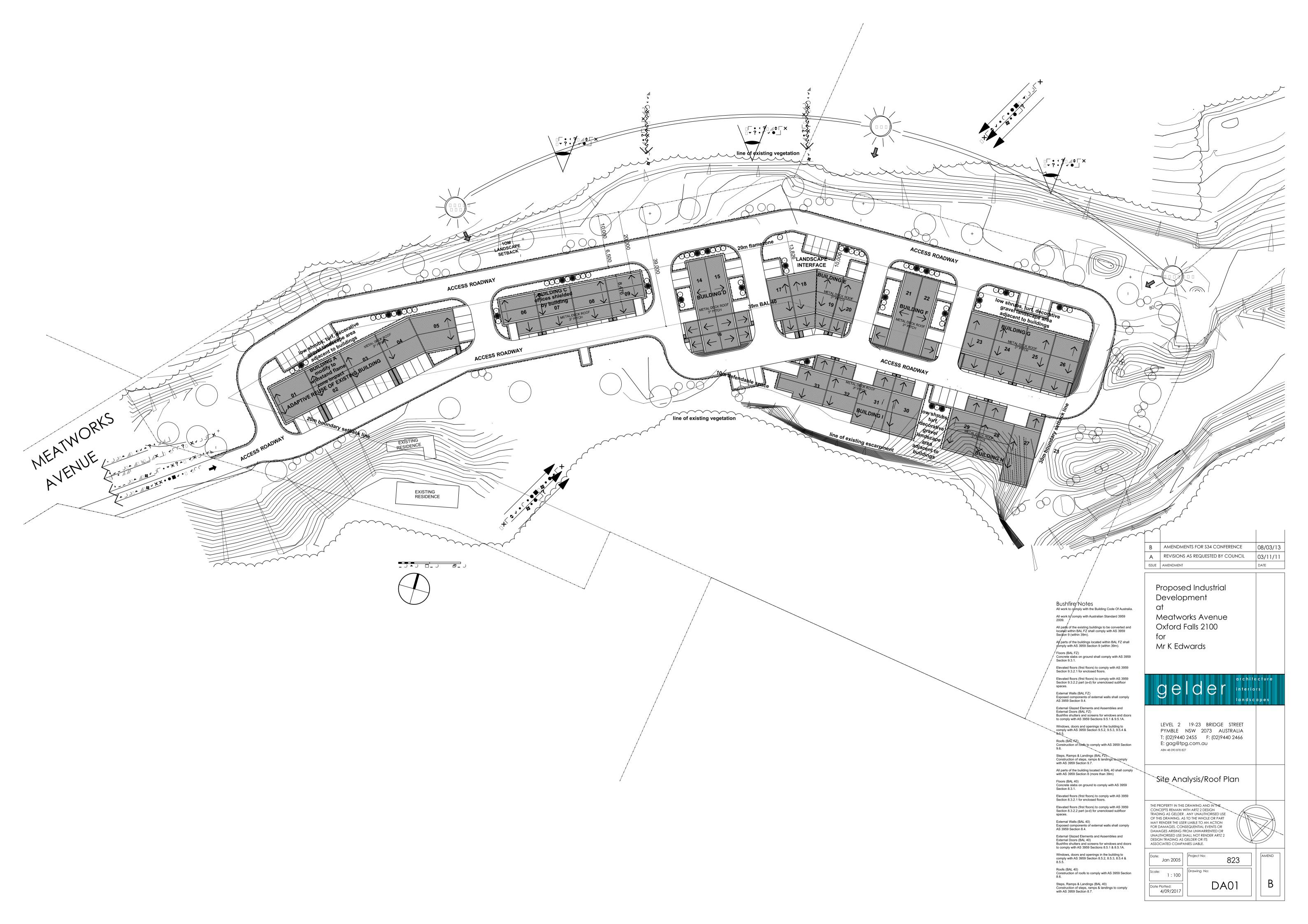
A Development Application is to be submitted to Northern Beaches Council for the proposed industrial development (Stage 2) of the Industrial Estate on a site at Lot 100, Meatworks Avenue, Oxford Falls. The traffic, transport and parking assessment provided in this report confirms that:

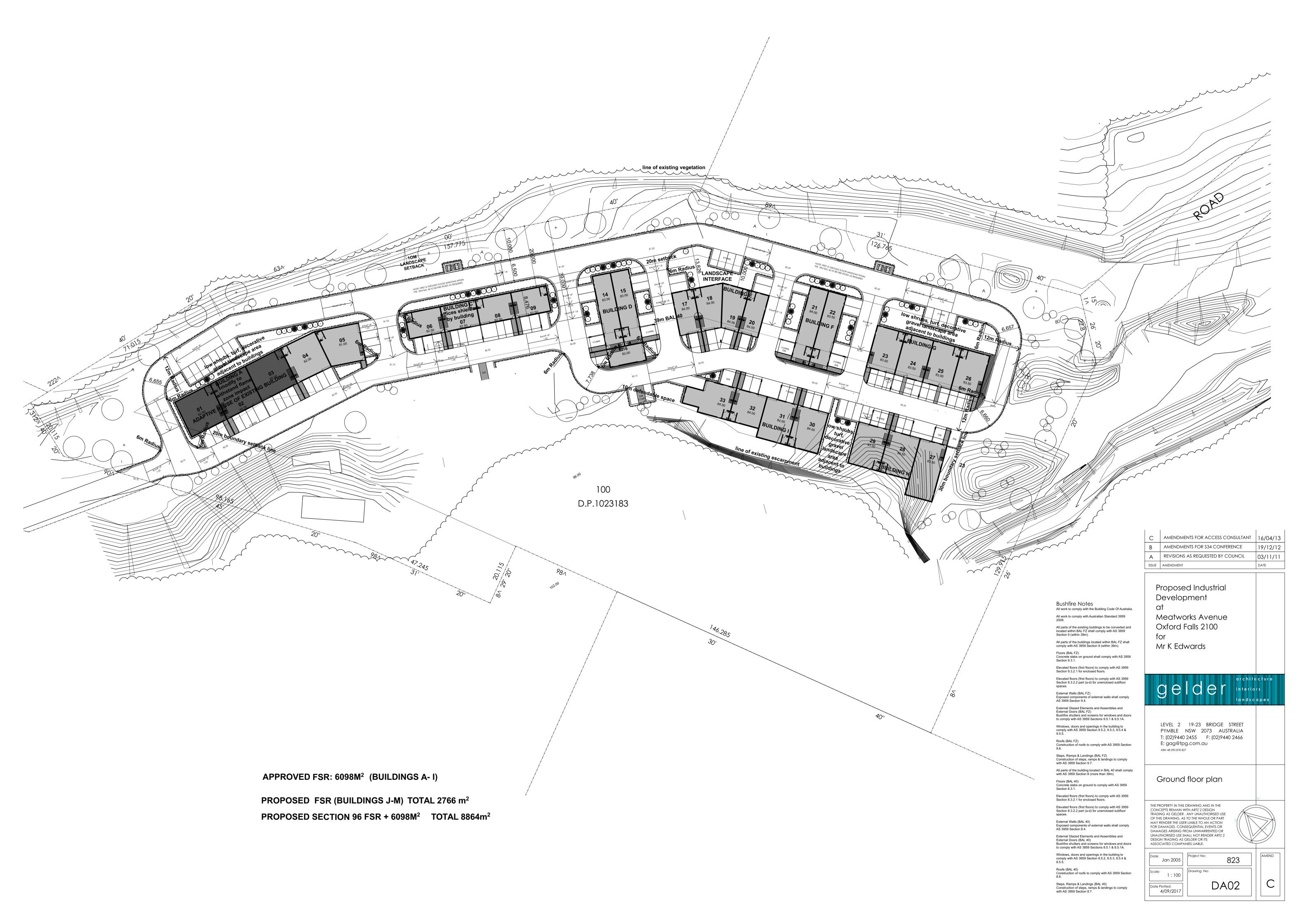
- the traffic generation of the proposed development will not present any adverse traffic implications and traffic-related environmental impacts
- the proposed parking provision will be adequate
- the vehicle accesses on the internal road access will be suitable and appropriate
- the proposed internal circulation and servicing arrangements will be in accordance with the current AS2890.1, 2 and 6 design standards
- the proposed servicing arrangements are suitable

Appendix A

Approved Stage 1 Architectural Plans





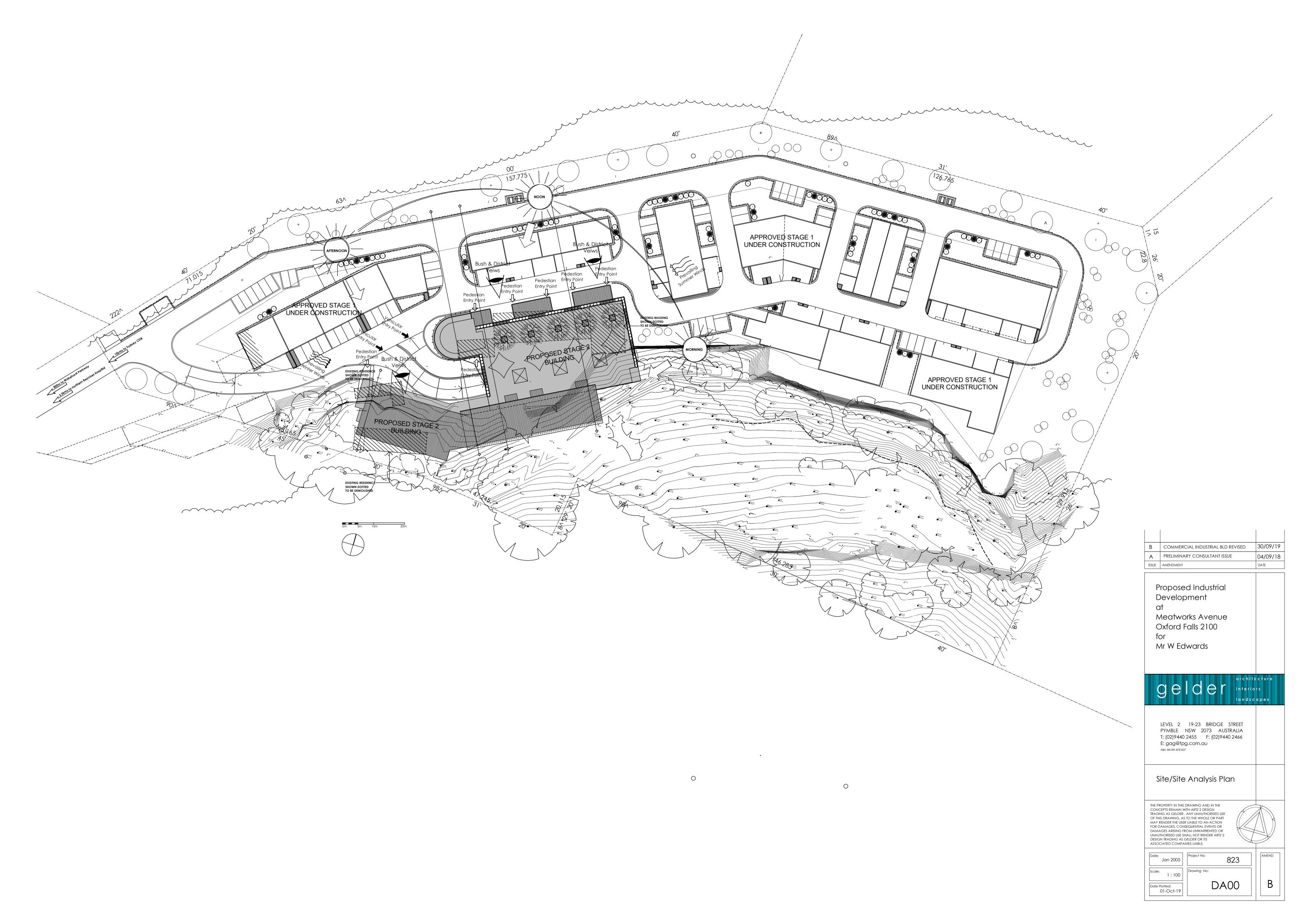


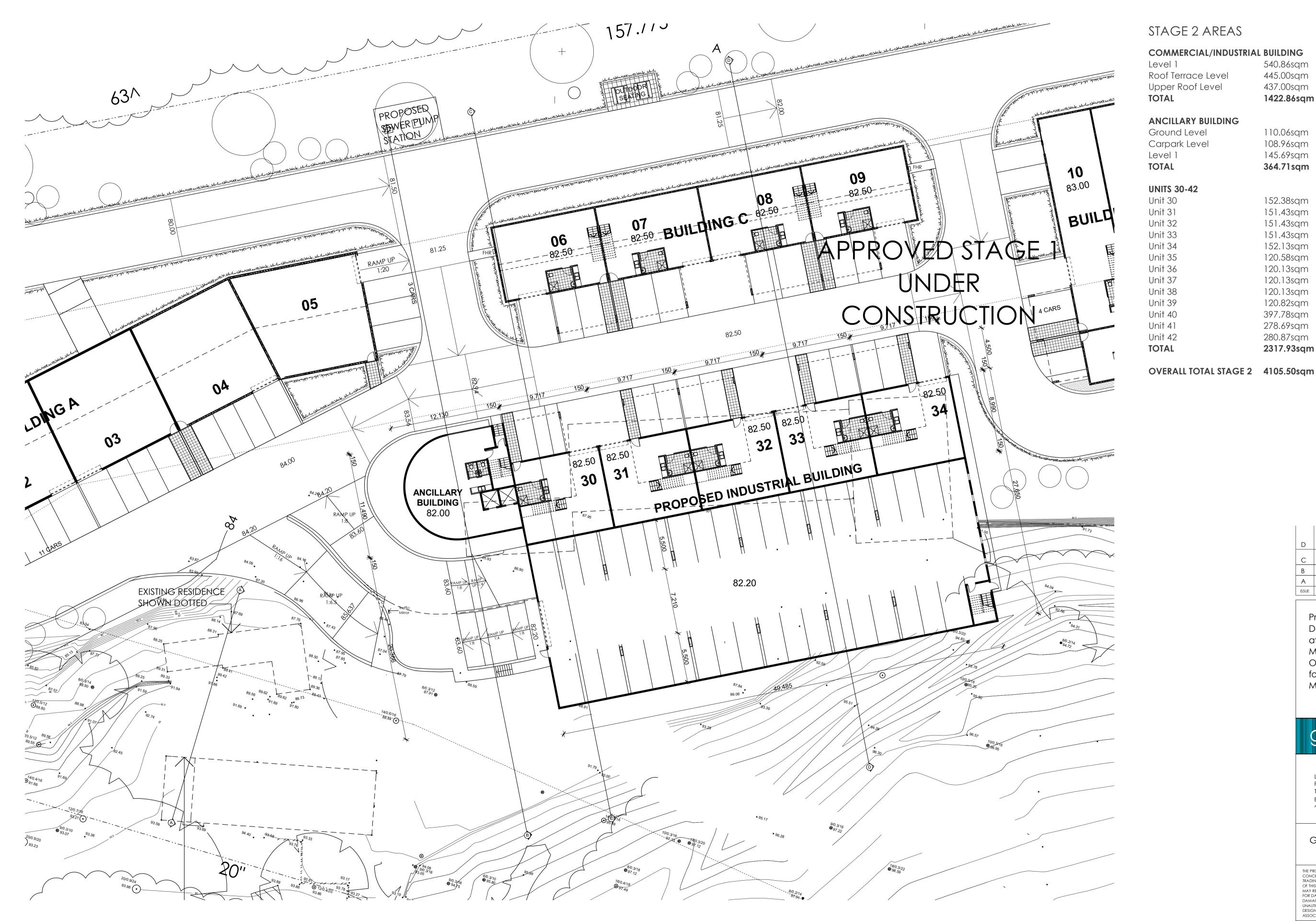


Appendix B

Stage 2 Architectural Plans





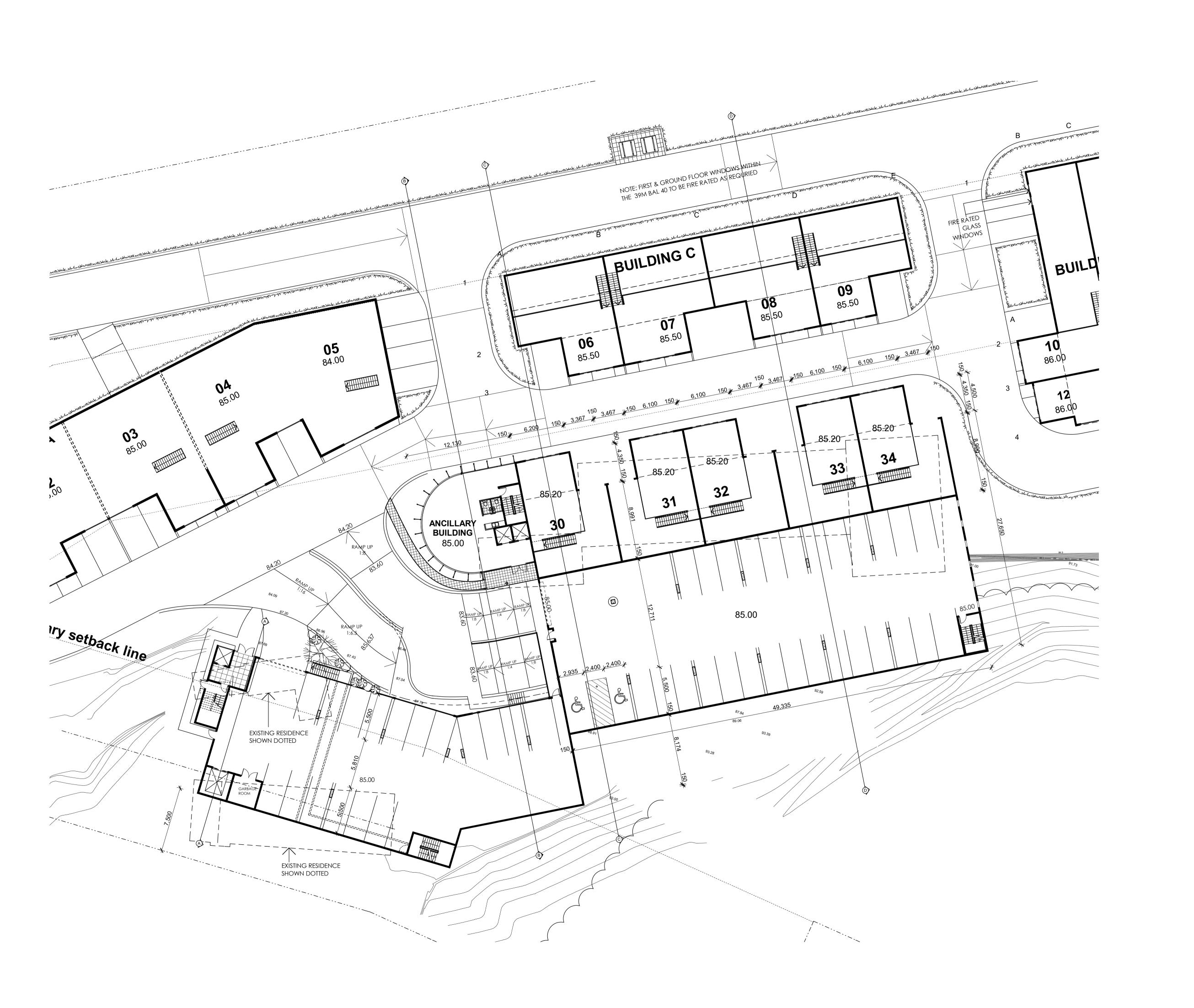


STAGE 2 AREAS

Le R U	COMMERCIAL/INDUSTRIAL evel 1 oof Terrace Level pper Roof Level OTAL	BUILDING 540.86sqm 445.00sqm 437.00sqm 1422.86sqm
C	ANCILLARY BUILDING Fround Level Carpark Level evel 1 OTAL	110.06sqm 108.96sqm 145.69sqm 364.71sqm
	NITS 30-42 Init 30 Init 31 Init 32 Init 33 Init 34 Init 35 Init 36 Init 37 Init 38 Init 39 Init 40 Init 41 Init 42 OTAL	152.38sqm 151.43sqm 151.43sqm 151.43sqm 152.13sqm 120.58sqm 120.13sqm 120.13sqm 120.13sqm 120.82sqm 397.78sqm 278.69sqm 280.87sqm 280.87sqm

D		y revision			30/09/19
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D	UPDATED CONSULTANT ISSUE	06/09/19
С	PRELIMINARY CONSULTANT ISSUE REVISED STAGE 2	02/08/19
В	UPDATED CONSULTANT ISSUE	19/09/18
Α	PRELIMINARY CONSULTANT ISSUE	04/09/18
ISSUE	AMENDMENT	DATE

Proposed Industrial
Development

Meatworks Avenue Oxford Falls 2100

for Mr W Edwards

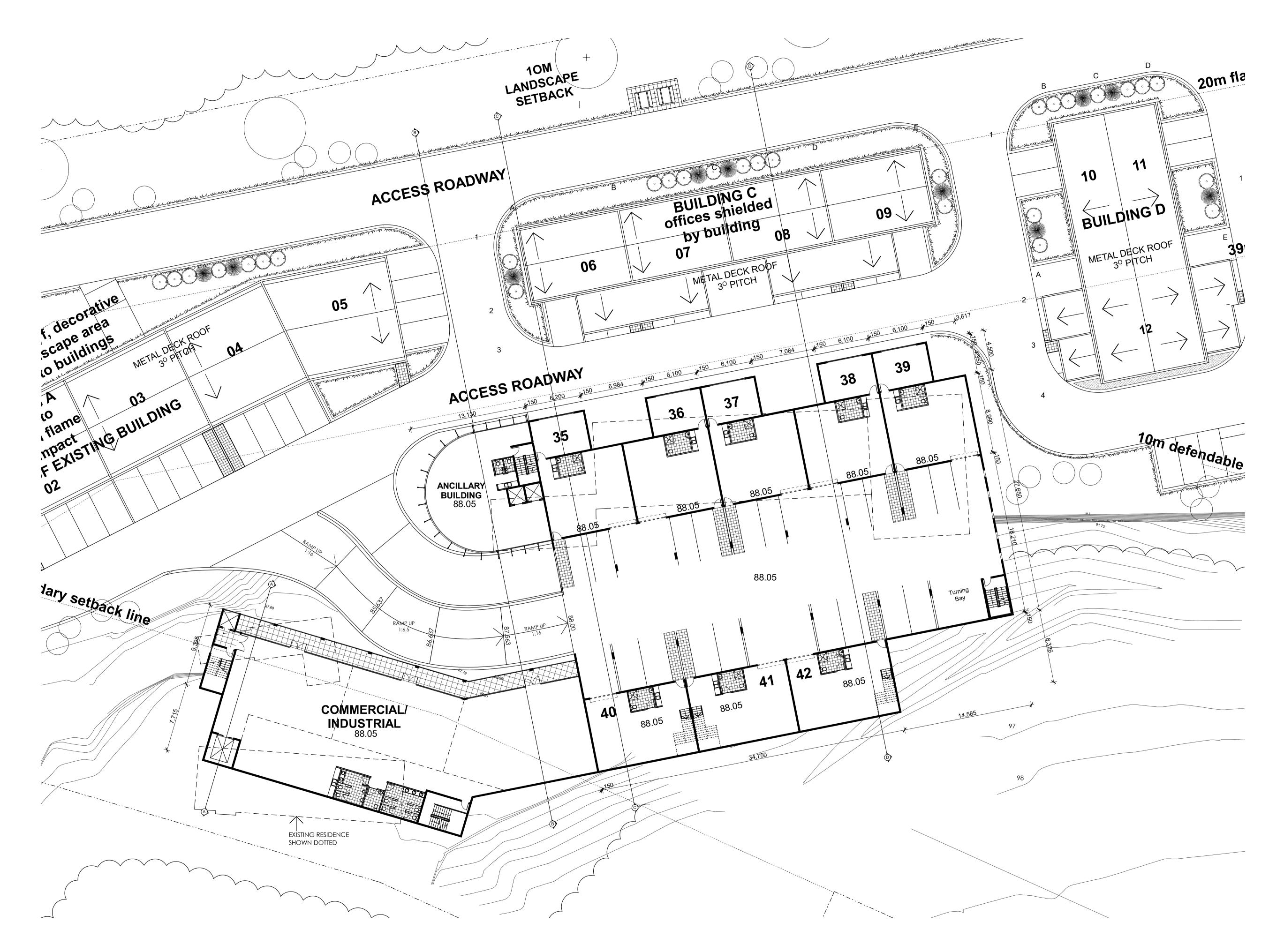
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Ground floor mezzanine Stage

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E	TURNING BAY ADDED	01/10/19
D	COMMERCIAL INDUSTRIAL BLD REVISED	30/09/19
С	PRELIMINARY CONSULTANT ISSUE REVISED STAGE 2	02/08/19
В	UPDATED CONSULTANT ISSUE	19/09/18
Α	PRELIMINARY CONSULTANT ISSUE	04/09/18
ISSUE	AMENDMENT	DATE

Proposed Industrial Development

Meatworks Avenue Oxford Falls 2100 for

Mr W Edwards

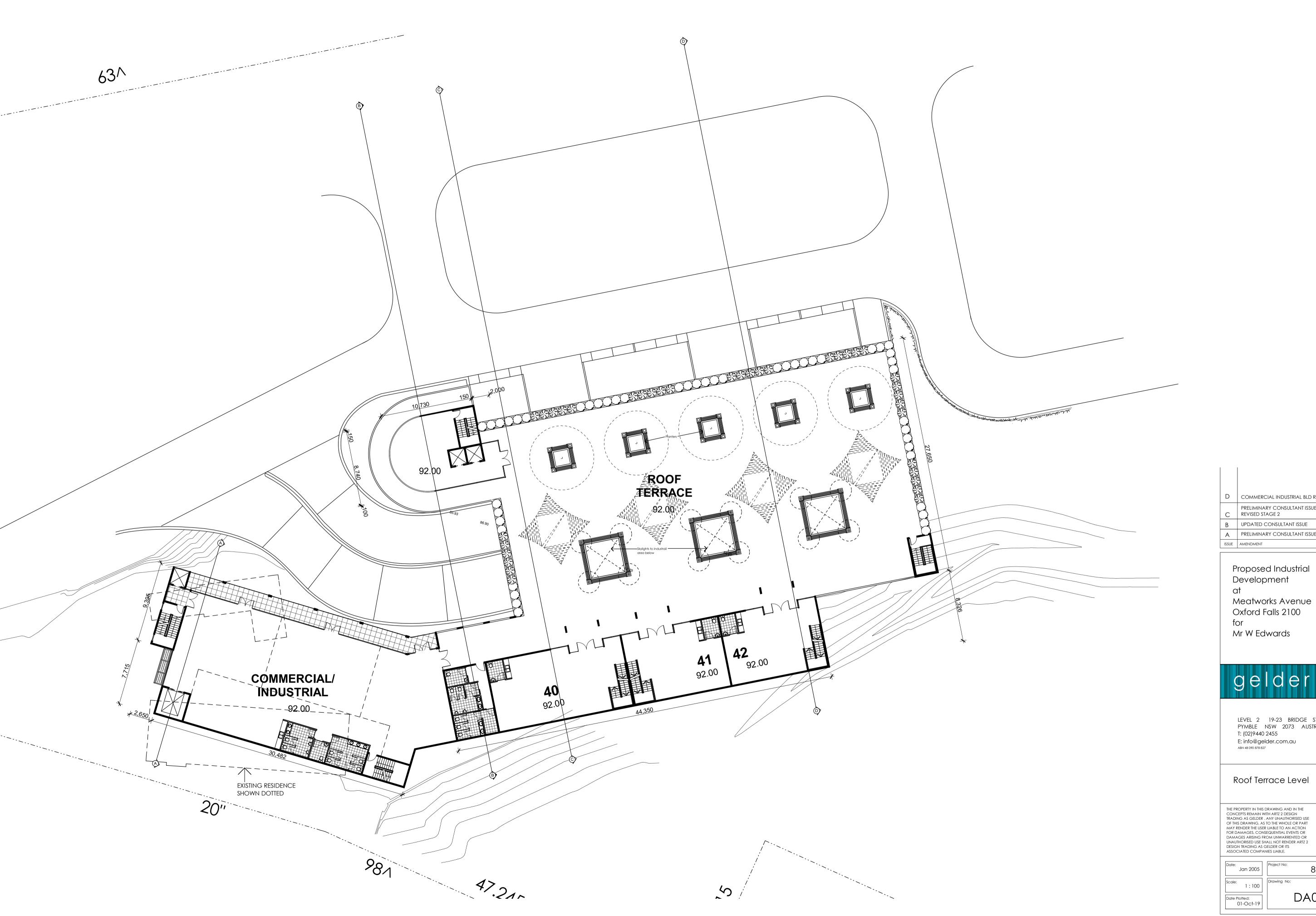
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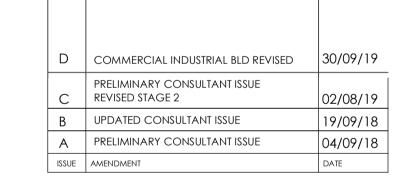
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Level 1 Mezzanine Stage 2

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Development Meatworks Avenue

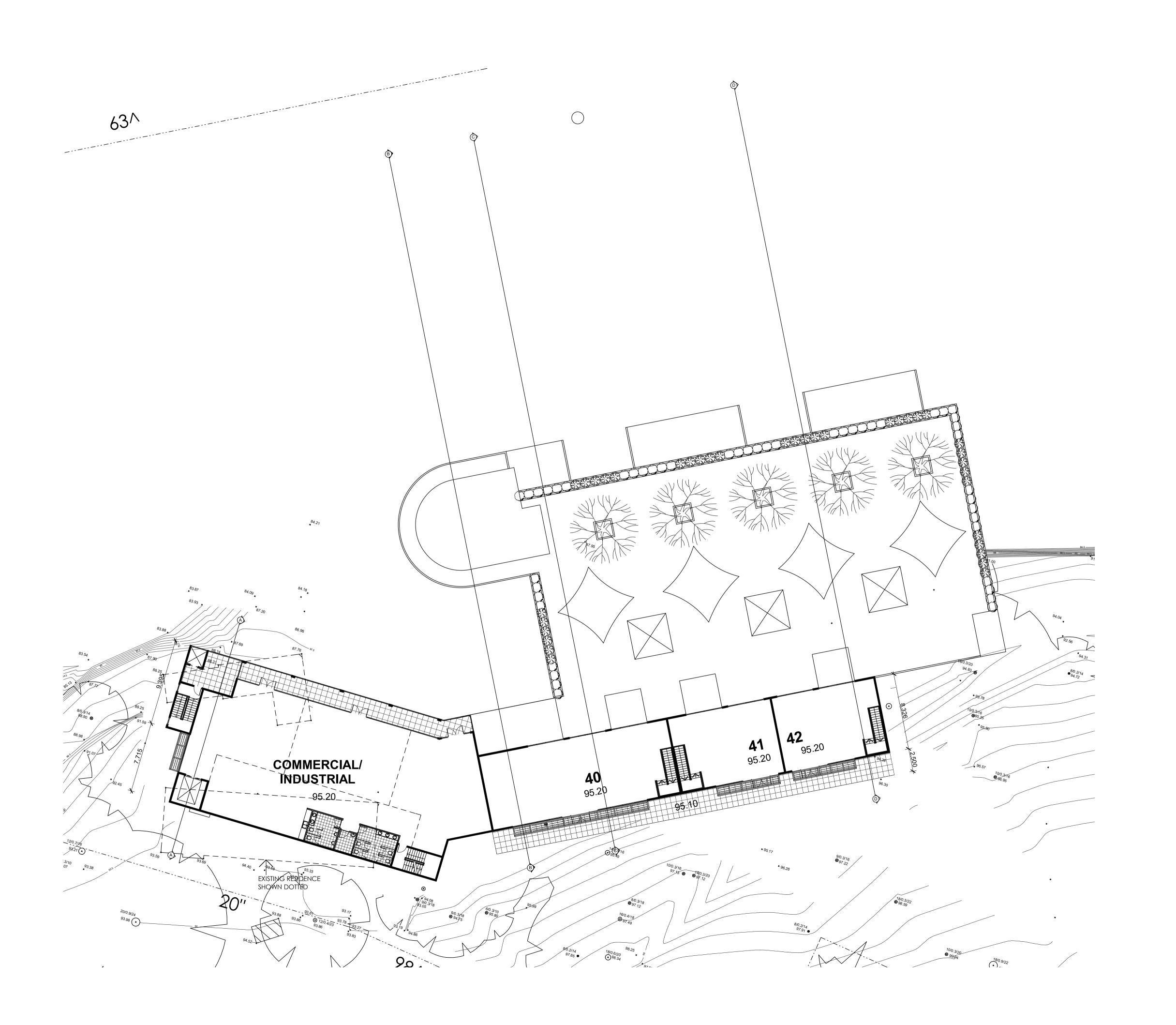
Oxford Falls 2100 for Mr W Edwards

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Roof Terrace Level

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ISSUE	AMENDMENT	DATE

Proposed Industrial
Development

Meatworks Avenue Oxford Falls 2100 for

Mr W Edwards

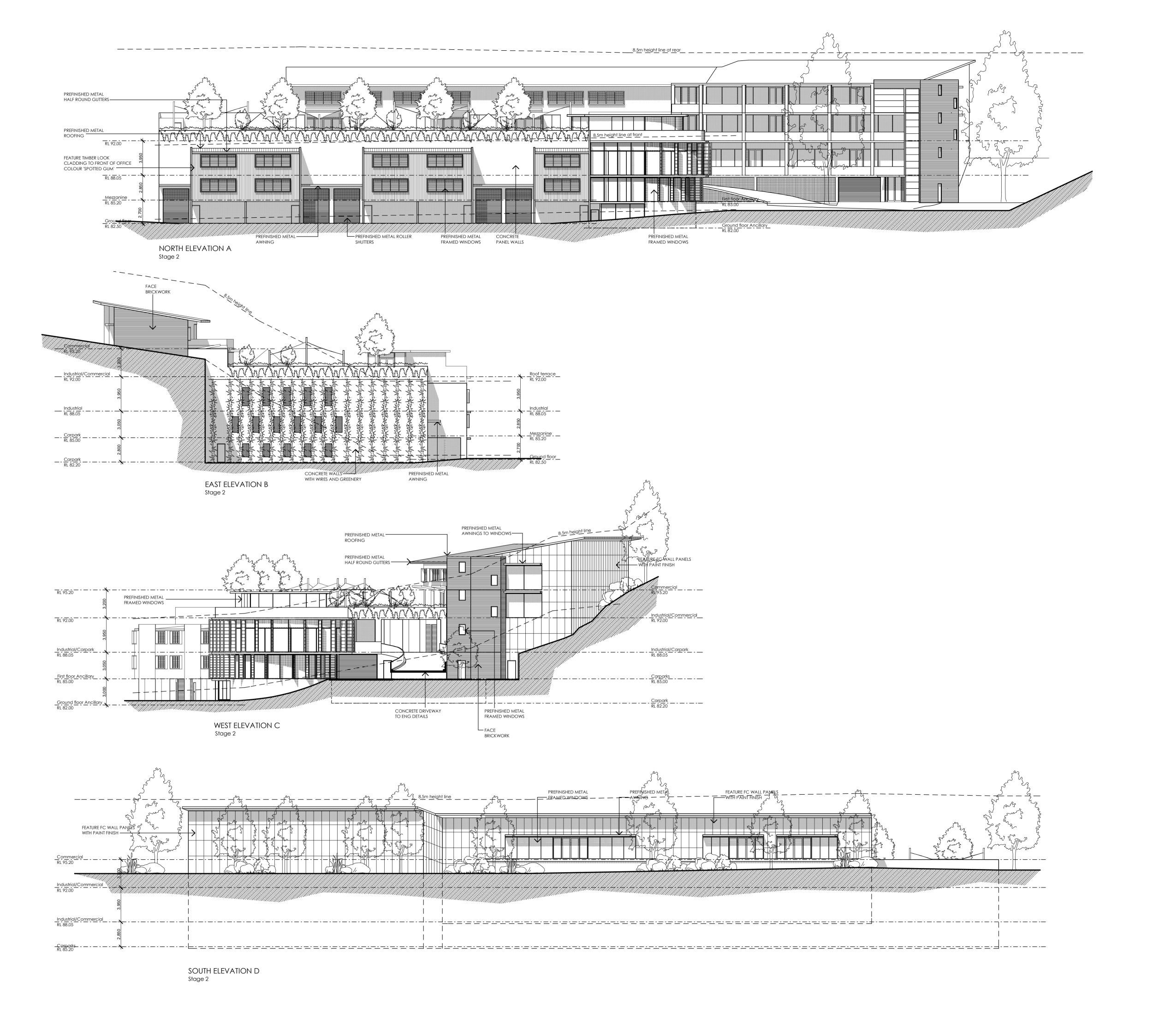
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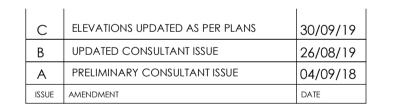
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Upper Roof Terrace

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Date Plotted: 01-Oct-19	DA05





Proposed Industrial Development

Meatworks Avenue Oxford Falls 2100 for

Mr W Edwards

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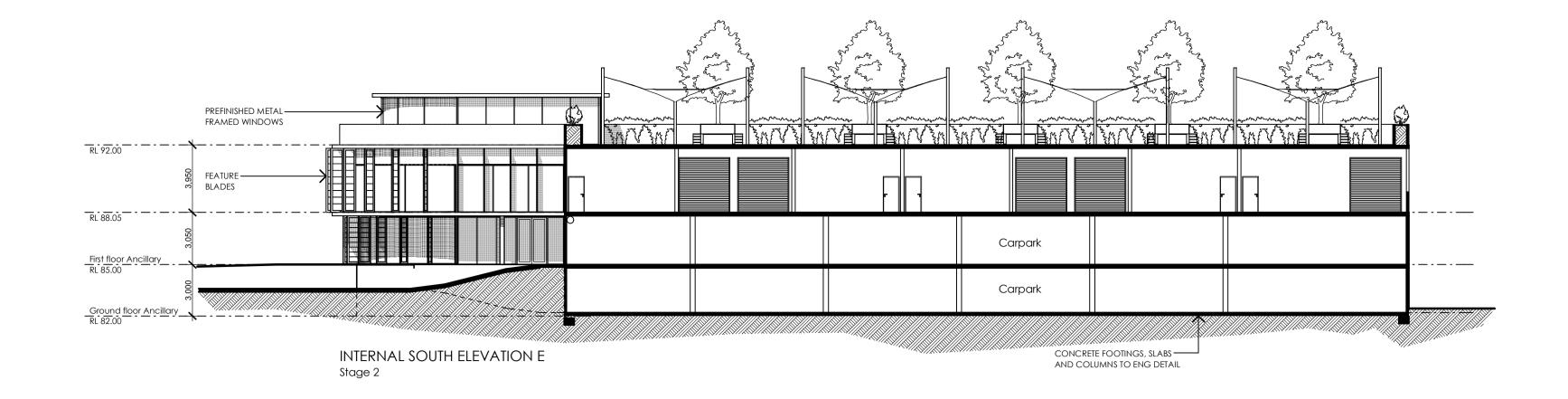
Elevations

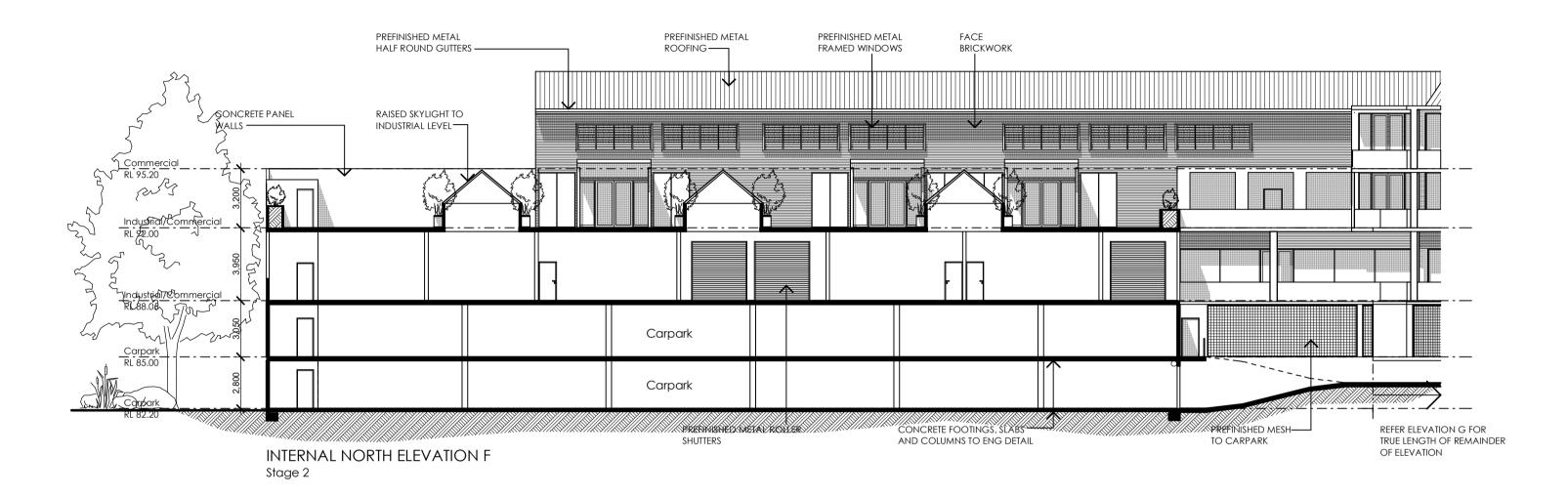
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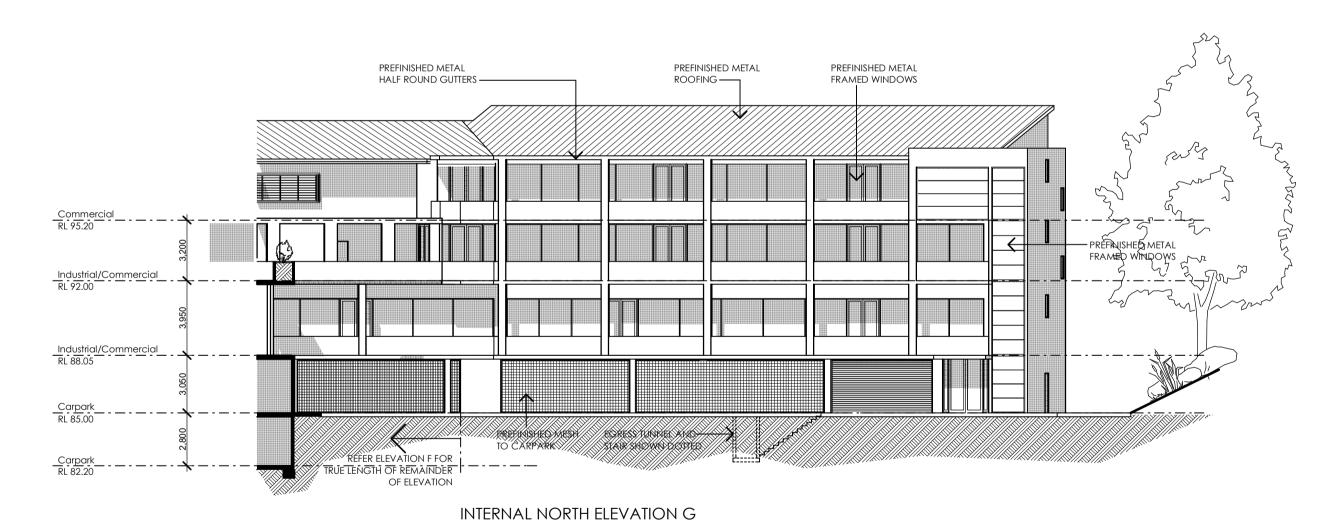
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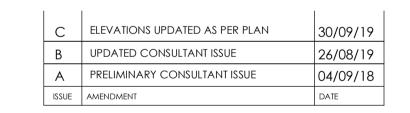
DA06







Stage 2



Proposed Industrial Development at

Meatworks Avenue Oxford Falls 2100 for

Mr W Edwards



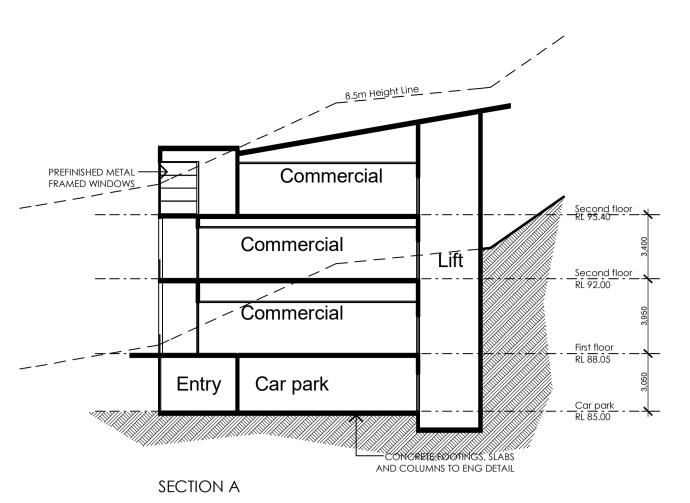
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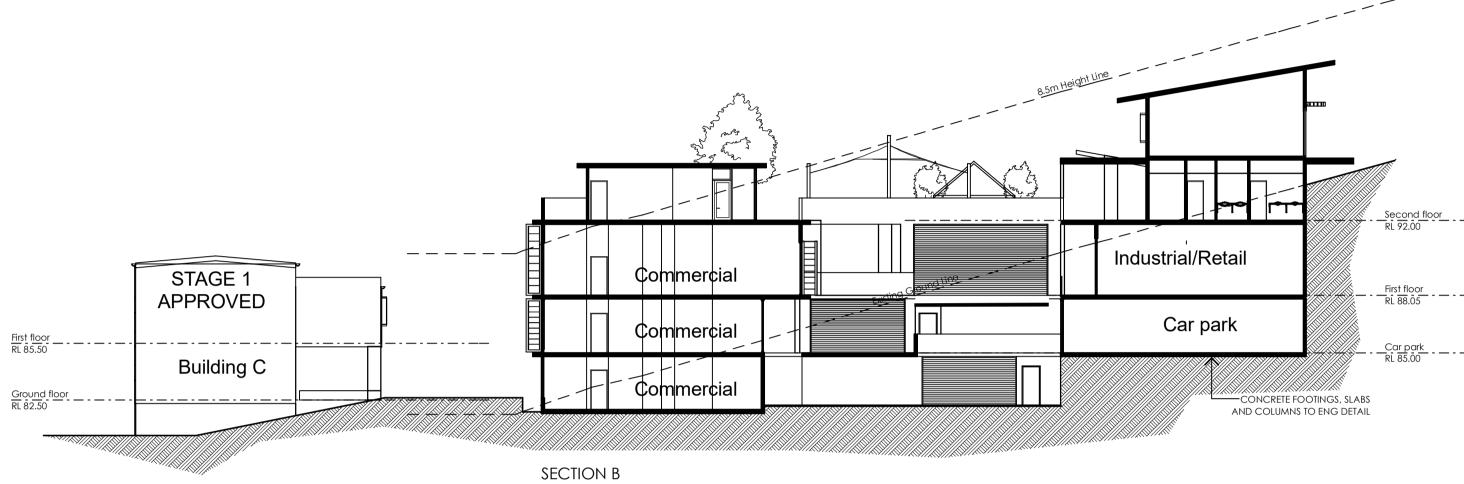
Elevations

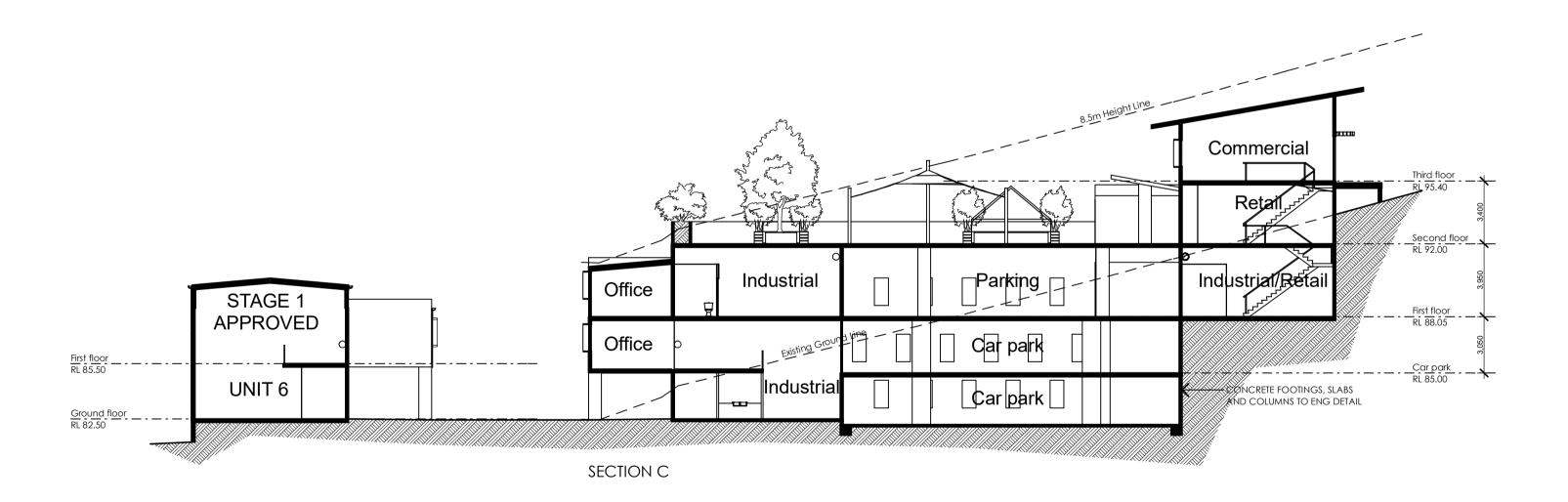
Date Plotted: 01-Oct-19

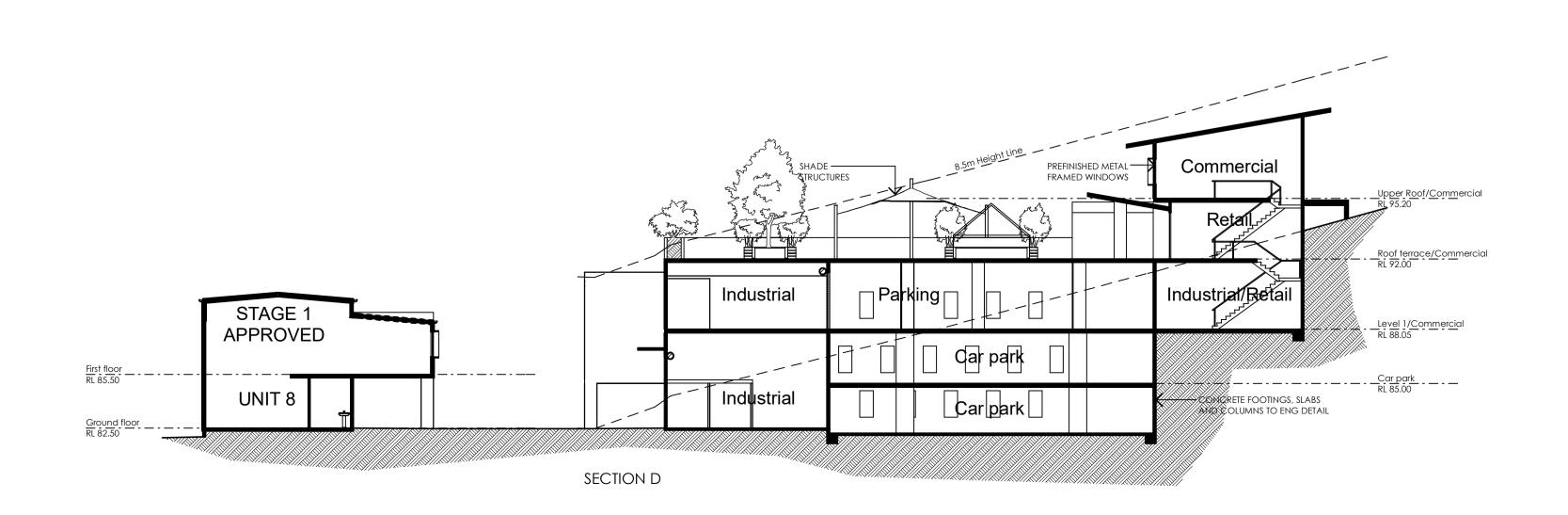
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	Date: Jan 2005	Project No:	823					
	Scale:	Drawing No:						









С	PRELIMINARY CONSULTANT ISSUE REVISED STAGE 2	02/08/19
В	UPDATED CONSULTANT ISSUE	19/09/18
Α	PRELIMINARY CONSULTANT ISSUE	04/09/18
ISSUE	AMENDMENT	DATE

Proposed Industrial
Development

Meatworks Avenue Oxford Falls 2100 for

Mr W Edwards

gelder interiors

LEVEL 2 19-23 BRIDGE STREET PYMBLE NSW 2073 AUSTRALIA T: (02)9440 2455 E: info@gelder.com.au ABN 48 090 878 827

Sections

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A33OCIATED COMFA	NNES EIABLE.	
Date: Jan 2005	Project No: 823	AMEND
Scale: 1:100	Drawing No:	
Date Plotted: 01-Oct-19	DA08	C

Appendix C

SIDRA Output



MOVEMENT SUMMARY

Site: 101 [EX AM PEAK WAKEHURST PARKWAY - DREADNOUGHT ROAD]

New Site

Site Category: (None)

Signals - Actuated Isolated Cycle Time = 140 seconds (Site User-Given Phase Times)

Movement Performance - Vehicles												
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate		Average Speed km/h
South	n: WAKE	HURST PAF	RKWAY									
1	L2	17	2.0	0.030	27.9	LOS B	0.5	3.3	0.70	0.69	0.70	43.5
2	T1	360	2.0	0.595	28.8	LOS C	15.1	92.6	0.88	0.76	0.90	49.0
3	R2	113	2.0	0.508	47.8	LOS D	5.8	35.5	0.94	0.79	0.94	37.6
Appro	oach	490	2.0	0.595	33.1	LOS C	15.1	92.6	0.88	0.76	0.91	45.6
East:	DREAD	NOUGHT R	OAD									
4	L2	150	2.0	0.194	28.1	LOS B	6.3	39.0	0.60	0.70	0.60	36.0
5	T1	13	2.0	0.194	23.5	LOS B	6.3	39.0	0.60	0.70	0.60	34.2
6	R2	328	2.0	0.822	48.5	LOS D	19.1	117.4	0.89	0.83	0.89	29.9
Appro	oach	491	2.0	0.822	41.6	LOS C	19.1	117.4	0.79	0.79	0.79	31.6
North	: WAKEI	HURST PAR	KWAY									
7	L2	335	2.0	0.876	43.1	LOS D	28.8	177.0	0.82	0.82	0.85	40.6
8	T1	722	2.0	0.876	35.9	LOS C	29.1	178.7	0.81	0.76	0.84	44.1
9	R2	14	2.0	0.031	30.9	LOS C	0.6	3.4	0.66	0.67	0.66	41.6
Appro	oach	1071	2.0	0.876	38.1	LOS C	29.1	178.7	0.81	0.78	0.84	42.9
West	DREAD	NOUGHT R	OAD									
10	L2	2	2.0	0.013	24.4	LOS B	0.3	2.0	0.65	0.48	0.65	36.8
11	T1	7	2.0	0.013	19.8	LOS B	0.3	2.0	0.65	0.48	0.65	37.2
12	R2	3	2.0	0.006	37.2	LOS C	0.1	0.8	0.66	0.61	0.66	30.7
Appro	oach	12	2.0	0.013	24.9	LOS B	0.3	2.0	0.65	0.51	0.65	35.3
All Ve	hicles	2064	2.0	0.876	37.7	LOSC	29.1	178.7	0.82	0.77	0.85	40.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians											
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate				
P3	North Full Crossing	53	64.3	LOS F	0.2	0.2	0.96	0.96				
All Pe	destrians	53	64.3	LOS F			0.96	0.96				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

Site: 101 [EX PM PEAK WAKEHURST PARKWAY - DREADNOUGHT ROAD]

New Site

Site Category: (None)

Signals - Actuated Isolated Cycle Time = 142 seconds (Site User-Given Phase Times)

Movement Performance - Vehicles												
Mov ID	Turn	Demand I Total veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South	: WAKE	HURST PAR	RKWAY									
1	L2	12	0.0	0.012	17.9	LOS B	0.3	1.8	0.49	0.67	0.49	51.2
2	T1	1004	2.0	1.001	56.9	LOS E	64.4	395.6	1.00	1.10	1.30	29.3
3	R2	59	2.0	0.086	30.9	LOS C	2.3	14.4	0.66	0.70	0.66	45.4
Appro	ach	1075	2.0	1.001	55.0	LOS D	64.4	395.6	0.98	1.07	1.26	30.0
East:	DREAD	NOUGHT R	DAC									
4	L2	82	2.0	0.111	30.4	LOS C	3.5	21.7	0.61	0.68	0.61	35.2
5	T1	6	0.0	0.111	25.8	LOS B	3.5	21.7	0.61	0.68	0.61	33.4
6	R2	298	2.0	0.911	65.0	LOS E	20.8	127.6	0.97	0.89	1.05	26.4
Appro	ach	386	2.0	0.911	57.0	LOS E	20.8	127.6	0.89	0.84	0.95	27.9
North	: WAKE	HURST PAR	KWAY									
7	L2	103	0.0	0.692	59.4	LOS E	20.3	123.8	0.93	0.81	0.93	35.2
8	T1	564	2.0	0.692	52.6	LOS D	21.0	129.2	0.93	0.81	0.93	36.8
9	R2	6	2.0	0.045	73.3	LOS F	0.4	2.4	0.94	0.64	0.94	26.2
Appro	ach	673	1.7	0.692	53.8	LOS D	21.0	129.2	0.93	0.81	0.93	36.5
West:	DREAD	NOUGHT R	OAD									
10	L2	16	0.0	0.066	27.6	LOS B	1.1	6.7	0.74	0.61	0.74	35.0
11	T1	19	0.0	0.066	23.0	LOS B	1.1	6.7	0.74	0.61	0.74	35.3
12	R2	41	0.0	0.111	48.1	LOS D	2.1	12.8	0.77	0.71	0.77	27.6
Appro	ach	76	0.0	0.111	37.5	LOSC	2.1	12.8	0.76	0.67	0.76	30.7
All Ve	hicles	2210	1.8	1.001	54.4	LOS D	64.4	395.6	0.94	0.94	1.09	31.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians								
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P3	North Full Crossing	53	65.3	LOS F	0.2	0.2	0.96	0.96
All Pedestrians		53	65.3	LOS F			0.96	0.96

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

Site: 101 [EX AM PEAK WAKEHURST PARKWAY - DREADNOUGHT ROAD + STAGES 1 + 2]

New Site

Site Category: (None)

Signals - Actuated Isolated Cycle Time = 140 seconds (Site User-Given Phase Times)

Movement Performance - Vehicles												
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	U
South	ı: WAKE	HURST PAR	RKWAY									
1	L2	17	0.0	0.029	27.8	LOS B	0.5	3.3	0.70	0.69	0.70	43.8
2	T1	403	2.0	0.666	31.9	LOS C	18.2	111.9	0.90	0.79	0.96	47.1
3	R2	113	2.0	0.517	48.6	LOS D	5.8	35.8	0.94	0.79	0.94	37.3
Appro	oach	533	1.9	0.666	35.3	LOS C	18.2	111.9	0.91	0.79	0.94	44.5
East:	DREAD	NOUGHT RO	OAD									
4	L2	150	2.0	0.193	28.1	LOS B	6.3	38.9	0.60	0.70	0.60	36.0
5	T1	13	0.0	0.193	23.5	LOS B	6.3	38.9	0.60	0.70	0.60	34.2
6	R2	367	2.0	0.917	57.1	LOS E	24.4	149.9	0.92	0.89	1.02	27.9
Appro	oach	530	2.0	0.917	48.1	LOS D	24.4	149.9	0.82	0.83	0.89	30.0
North	: WAKE	HURST PAR	KWAY									
7	L2	342	2.0	0.913	48.0	LOS D	31.8	195.5	0.82	0.85	0.91	38.5
8	T1	737	2.0	0.913	40.8	LOS C	32.1	197.3	0.82	0.80	0.90	41.7
9	R2	14	0.0	0.033	31.3	LOS C	0.6	3.3	0.68	0.67	0.68	41.5
Appro	oach	1093	2.0	0.913	43.0	LOS D	32.1	197.3	0.82	0.81	0.90	40.6
West	DREAD	NOUGHT R	OAD									
10	L2	2	0.0	0.013	24.4	LOS B	0.3	2.0	0.65	0.48	0.65	36.8
11	T1	7	0.0	0.013	19.8	LOS B	0.3	2.0	0.65	0.48	0.65	37.2
12	R2	3	0.0	0.006	37.2	LOS C	0.1	0.8	0.66	0.61	0.66	30.7
Appro	oach	12	0.0	0.013	24.9	LOS B	0.3	2.0	0.65	0.51	0.65	35.3
All Ve	hicles	2168	1.9	0.917	42.2	LOSC	32.1	197.3	0.84	0.81	0.91	38.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians											
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate			
P3	North Full Crossing	53	64.3	LOS F	0.2	0.2	0.96	0.96			
All Pe	edestrians	53	64.3	LOS F			0.96	0.96			

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

Site: 101 [FUT PM PEAK WAKEHURST PARKWAY - DREADNOUGHT ROAD + STAGES 1 + 2]

New Site

Site Category: (None)

Signals - Actuated Isolated Cycle Time = 140 seconds (Site User-Given Phase Times)

Movement Performance - Vehicles												
Mov	Turn	Demand I	Flows	Deg.	Average	Level of	95% Back	95% Back of Queue		Effective	Aver. No.	Average
ID		Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	
Canal	. \^/^!/\	veh/h HURST PAR	%	v/c	sec		veh	m				km/h
		-										
1	L2	12	0.0	0.012	17.7	LOS B	0.3	1.8	0.47	0.66	0.47	51.4
2	T1	1039	2.0	1.008	60.0	LOS E	68.8	422.6	1.00	1.12	1.33	28.6
3	R2	59	2.0	0.083	30.8	LOS C	2.3	14.3	0.64	0.70	0.64	45.5
Appro	oach	1110	2.0	1.008	57.9	LOS E	68.8	422.6	0.98	1.09	1.29	29.3
East:	DREAD	NOUGHT R	OAD									
4	L2	82	2.0	0.110	29.4	LOS C	3.4	21.1	0.60	0.68	0.60	35.5
5	T1	6	0.0	0.110	24.9	LOS B	3.4	21.1	0.60	0.68	0.60	33.7
6	R2	309	2.0	0.929	66.6	LOS E	21.9	134.6	0.98	0.91	1.09	26.1
Appro	oach	397	2.0	0.929	58.3	LOS E	21.9	134.6	0.89	0.86	0.98	27.7
North	: WAKEI	HURST PAR	KWAY									
7	L2	113	0.0	0.853	61.0	LOS E	23.0	140.2	0.96	0.84	0.97	34.7
8	T1	616	2.0	0.853	54.0	LOS D	23.1	142.0	0.96	0.83	0.97	36.3
9	R2	6	2.0	0.049	73.2	LOS F	0.4	2.4	0.95	0.64	0.95	26.2
Appro	oach	735	1.7	0.853	55.2	LOS D	23.1	142.0	0.96	0.83	0.97	36.0
West	: DREAD	NOUGHT R	OAD									
10	L2	16	0.0	0.065	26.9	LOS B	1.1	6.5	0.74	0.61	0.74	35.3
11	T1	19	0.0	0.065	22.3	LOS B	1.1	6.5	0.74	0.61	0.74	35.6
12	R2	41	0.0	0.109	47.1	LOS D	2.1	12.6	0.76	0.71	0.76	27.9
Appro	oach	76	0.0	0.109	36.6	LOS C	2.1	12.6	0.75	0.66	0.75	30.9
All Ve	ehicles	2318	1.8	1.008	56.4	LOS D	68.8	422.6	0.95	0.96	1.12	30.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate				
P3	North Full Crossing	53	64.3	LOS F	0.2	0.2	0.96	0.96				
All Pe	destrians	53	64.3	LOS F			0.96	0.96				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Transport and Traffic Planning Associates

Appendix D

Turning Path Assessment



