

## SOIL CLASSIFICATION REPORT

**Client:** Synthetic Sports Group

**Job No:** 3.25.2975.1

**Site Address:** Wakehurst Tennis Club-Upper Clontarf Road  
Seaforth NSW

**Client Job No:** .

**Date of Report:** 27-March-2025

Site Photo



## Classifications

Site

Soil

P

M

Allowable Bearing Capacity

yS Range

50kPa	0mm
100kPa	300mm
150kPa	500mm
250kPa	800mm

20 - 40mm

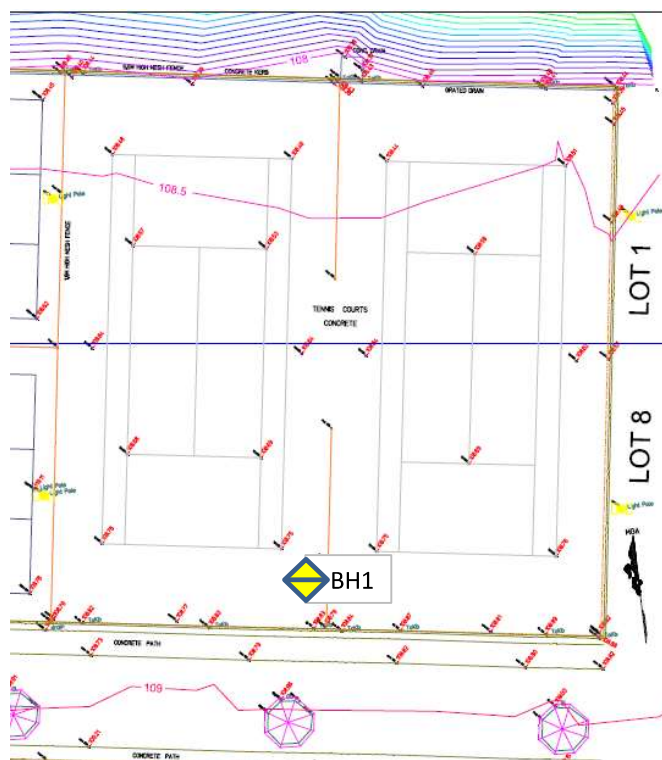
## 1. P-CLASS INDICATORS

1.1 Soft Or Collapsing Soil:	No	Notes: Trees on the adjoining sites
1.2 Trees Onsite:	Yes	
1.3 Boulders In Fill Present:	No	
1.4 Fill Containing Wood, Metal, Plastic or other Harmful Materials:	No	
1.5 Deep Fill (Over 400mm):	No	
1.6 Uncontrolled Fill	No	
1.7 Controlled Fill	No	
1.8 Knockdown / Rebuild	No	

## 2. OTHER CONDITIONS PRESENT

2.1 Rock Within 1 metre of the Surface:	No
2.2 Marine Environment:	NA
2.3 Water Table/Purged Water Table Encountered	No

## TESTING LOCATION



## SITE PHOTOGRAPHS



## BORE LOGS/BEARING

BH1

Depth (m)	DCP	Soil Profile
0.1	3	FILL. silty gravelly CLAY (Cl)
	3	Brown,Moist,Stiff
	3	
	5	Natural. gravelly CLAY trace sand (Cl)
0.5	5	Orange,Brown,Moist,Very stiff-hard
	6	
	8	
	8	
	12	
1.0	15	
	10+	End Hand Auger 1.0m UTP
1.5		
2.0		
2.5		
3.0		
3.5		
4.0		
4.5		
5.0		
5.5		
6.0		

BH2

Depth (m)	DCP	Soil Profile
0.1		
0.5		
1.0		
1.5		
2.0		
2.5		
3.0		
3.5		
4.0		
4.5		
5.0		
5.5		
6.0		

SAND			SILTS & CLAY		
Density Term	Density Index (%)	Approx. DCP Blow Count	Consistency Term	Undrained Shear Strength (kPa)	Approx. DCP Blow Count
Very Loose	<15	< 1	Very Soft	0 - 12	< 1
Loose	15 - 35	1 - 3	Soft	12 - 25	1 - 2
Medium Dense	35 - 65	3 - 9	Firm	25 - 50	2 - 3
Dense	65 - 85	9 - 15	Stiff	50 - 100	3 - 5
Very Dense	> 85	> 15	Very Stiff	100 - 200	5 - 8
			Hard	> 200	> 8

Note: DCP = Dynamic Cone Penetrometer blow counts (blows/100mm);UTP = Unable to penetrate.

## PURPOSE

This report is based on our field and laboratory test results and site construction information (if any) supplied to us by the client. Should any proposed construction vary from those advised, this office should be notified as we may need to undertake additional soil testing. It should also be noted that the test results may not be relevant if the location of a proposed structure varies from that originally advised.

This report relates to the ground conditions on the property at the time of the site investigation. If so advised by the client, this report has considered the proposed site preparation. If unadvised cutting or filling is proposed or carried out, additional testing may be required to reclassify the site as indicated in Clause 2.3.2 (B) and Clause 2.5.3 of AS2870-2011.

This site has been classified in accordance with Section 2 of AS2870-2011. The characteristic surface movement,  $s_s$ , has been determined either by shrink and swell tests as specified in AS1289.7.1.1-1992 in accordance with Clause 2.3.2 (i) of AS2870-2011, or by visual-tactile identification of the soil with assistance of Atterberg Limits in accordance with Clause 2.3.2 (iii) of AS2870-2011. Results of our site investigation are indicated in the attached Soil Test Results Page.

## CONTROLLED AND CERTIFIED FILL – AS2870-2011

If the investigation indicates that fill is present on site. This fill appears to be "controlled fill", although this must be confirmed by a suitably qualified professional. This fill would have to be "controlled" to requirements of the specification and AS2870-2011 with Level 1 supervision in accordance with AS3798-2007. The designer must be satisfied with the suitability of this fill and design footings appropriately.

## UNCONTROLLED FILL

If the investigation indicates that "uncontrolled" fill is present on this site. Any construction on such fill shall be founded in suitable material beneath. The site shall be given a P classification in accordance with Clause 2.5.3 (b) of AS2870-2011.

## SITE CLASSIFICATIONS

(in accordance with Clause 2.1.1, Clause 2.1.3, Table 2.1 and Table 2.3 of AS2870-2011)

- A** Most sand and rock sites with little or no ground movement from moisture changes.
- S** Slightly reactive clay sites, which may experience only slight ground movement from moisture changes. Estimated characteristic surface movement,  $0 < y_s < 20\text{mm}$ .
- M** Moderately reactive clay or silt sites, which may experience moderate ground movement from moisture changes,  $20 < y_s < 40\text{mm}$ .
- H1** Highly reactive clay sites, which may experience high ground movement from moisture changes,  $40 < y_s < 60\text{mm}$ .
- H2** Highly reactive clay sites, which may experience very high ground movement from moisture changes,  $60 < y_s < 75\text{mm}$ .
- E** Extremely reactive sites, which may experience extreme ground movement from moisture changes,  $y_s > 75\text{mm}$ .
- P** Problem sites, due to either: low bearing strength, potential excessive foundation settlement, fill, mine subsidence, landslide, collapse activity, coastal erosion, abnormal moisture changes or sites which cannot be classified otherwise.

For sites with deep-seated moisture changes characteristic of dry climates and corresponding to a designed depth of suction change,  $H_s$ , equal to or greater than 3.0m, classifications M, H1, H2 and E shall include the suffix 'D', as appropriate (eg H1-D).

## BEARING CAPACITY

The site shall be given a P classification in accordance with Clause 2.1.3 (a) and Clause 2.4.5 of AS2870-2011, if our investigation indicates that the founding soils have a bearing capacity of less than 50kPa, which is the minimum to support a residential slab on ground.

## SUB-SOIL SEEPAGE OR WATER TABLE

When Sub-soil seepage is encountered in the boreholes at the time of testing. Problems with footing constructions including softening of footing bases and potential collapse of piers is possible. Provisions should be made for fluctuations in the water table and sub-soil seepage entering foundations during constructions.

## INFLUENCE OF TREES

Trees onsite or on adjoining sites will be given a P classification in accordance with Clause 1.3.3, Clause 2.1.3(e), Appendix H and Appendix CH of AS2870-2011. Trees remove moisture from the soil and result in abnormal moisture conditions occurring on site. On removal of these trees, or if they remain in place, damage to a proposed building may occur without additional treatment.

The design engineer is to be informed of the proposed work in order to provide an appropriate design to address the influence of the trees. This may include one of the following:

- 1) Provisions of piers and stiffening the footing system.
- 2) Provision of a root barrier system acceptable to the design engineer.
- 3) Stiffening the entire slab and footing system to take into consideration the total  $y_t = (y_s + y_t / 0.7)$ .
- 4) Provision of piers with a suspended slab solution.



## INFLUENCE OF PREVIOUS TREES

When recent tree removal is present on this site as indicated by a research of the estate or aerial photographs. The effect of the tree removal can have an affect on the performance the foundation material due to abnormal moisture conditions will be given a P classification in accordance with Clause 1.3.3 of AS2870-2011. It is recommended the design engineer confirms the time frame and location of the tree removal to determine the effect on the footing and slab design.

## HOUSE REMOVAL

The removal of the slab and footings or any structure may result in an affect on the moisture condition of the foundation material. The designer is to take into consideration the effect of possible abnormal moisture conditions in accordance with Clause 1.3.3 of AS2870-2011.

## DISCLAIMER

*Structerre accepts no liability or responsibility what so ever for the use of this report by any third party.*

The purpose of this document is specifically to provide a site classification suitable to be used for residential structures only. Any preliminary recommendations are based upon the site investigation and site classification only. This is to enable the design engineers to determine their own professional opinion for the final design of the product.

This report is for Structerre only to use in design. Any design by anyone else for any structure must be specifically approved by Structerre. If used by anyone else for anything other than a Residential Engineering design or structure, Structerre takes no responsibility.

## LIMITS OF OUR INVESTIGATION

The recommendations made in this report are based on the assumption that the test results are representative of the overall subsurface conditions. Should excavations reveal variations from the soil conditions indicated in this report, our office should be notified before proceeding as the site classification may need revising and modifications to the design may be required. Also, should we be provided with additional information that affects the site classification, after the report has been issued, an additional fee may be charged to revise and reissue our report.

For and on behalf of

**STRUCTERRE CONSULTING ENGINEERS**



Gervase Purich  
**FIEAust. CPEng, NER, BPB, RBP, RPEQ,**

**WA | QLD | NSW | VIC**

Sydney Office: Unit 1 Second Floor 42 Birnie Avenue, Lidcombe. New South Wales 2141  
Phone (+612) 9475 3000 | Fax (+612) 9646 2311 | Email: [sydney@structerre.com.au](mailto:sydney@structerre.com.au) | Web: [www.structerre.com.au](http://www.structerre.com.au)  
ABN 87 131 748 260 Structerre Consulting Engineers (NSW) Pty Ltd ACN 131 748 260 trading as Structerre Consulting Engineers